

**ST. TAMMANY PARISH SCHOOL BOARD
COVINGTON, LOUISIANA**

ADDENDUM 2

**BID #1899 – RENT/LEASE OF MODULAR
BUILDINGS FOR BOYET JR. HIGH SCHOOL AND
ABITA SPRINGS ELEMENTARY**

October 27, 2026

- Item 1.** Attached is a list of questions that were received regarding BID #1899 – Rent/Lease of Modular Buildings for Boyet Jr. High School and Abita Springs Elementary with corresponding answers for clarification for all parties.
- Item 2.** Attached is the excerpt of the STPPS system IT Network Spec.
- Item 3.** Attached is the Boyet Jr. High GEOTECHNICAL REPORT (for information purposes)
- Item 4.** Attached St. Tammany Parish School Board Certificate of Insurance

Your bid should state “I have received and I am aware of the contents of Addendum 2.
Alternatively, you may sign the acknowledgement of receipt below.

ACKNOWLEDGE OF RECEIPT OF ADDENDUM 2

SIGNATURE

SIGNATORY NAME

ABITA SPRINGS MODULAR BUILDINGS

Questions and Corresponding Answers

Q1. New Building Construction

A new building will take longer than 60 days to manufacture & is based on current production schedule at time of contract. Please confirm if the Parish is willing to extend the 60-day requirement as needed for manufacturing & state approvals if the new option is chosen.

A1. The 60-day time frame is an on-site contract time. The anticipated date for this time frame is anticipated for mid-late January. The time between lease award and on-site Notice to Proceed is expected to be utilized for any requirements such as state approvals and manufacturing.

Q2. Site Visit

Will a site visit be allowed?

A2. Yes, Please contact STPPS Construction Dept. @ 985-898-3287 or 985-898-6440 to assist with coordination of site visit.

Q3. Geotechnical Report

Can a soils report be provided. If one is not available, is there an assumed psf for the proposed building location that can be provided?

A3. A geotechnical report for Abita Springs Elementary was provided by addendum and for information purposes.

Q4. Required Site Work

Please confirm that the infrastructure contractor will deliver a cleared, accessible, graded installation area suitable for foundation construction and truck/crane access. Who is responsible for unsuitable soils, buried obstructions, asphalt removal and restoration, and any required fill or drainage improvements?

A4. We have provided the drawings for the Infrastructure portion of the project. The items you list are expected to be reasonably addressed by the Infrastructure Contractor prior to modular installation.

Q5. Concrete Footers

Will concrete footings be required by the modular contractor?

A5. The modular contractor and their engineer of record are responsible for a complete design and installation of the modulares, the footings as their engineer determines and the anchors.

Q6. Plumbing

Please identify the required water, sanitary, vent, and condensate connection locations and elevations. Does "manifolded by Modular Contractor" require connections between all modules to a single point, and who performs final connections, testing, and any under-building piping?

A6. The requirement for manifolding the drain/sewer lines was addressed in a previous addendum. The infrastructure contractor performs final connections, testing and under building piping.

Q7. Electric Strikes

Does the school have a vendor they use for electric strikes, and can you share the vendor information?

A7. Benecom is the current vendor for access control for the STPPS system. The system must be compatible with Isonas Pure Access Control System. (See attached excerpt of the STPPS system IT Network Spec.)

Q8. Accessories

La. State Code requires grab bars to be installed at factory. Can modular company install all accessories on new modular?

A8. Yes

Q9. Like New Building Requirements

Confirm if alternative specifications will be accepted for the Like New option while still meeting LA state code.

A9. See Addendum 1 BID #1899 – Rent/Lease of Modular Buildings for Boyet Junior High, Item 7.

Q10. Custodial or IT Closet

Do you want an IT closet as well as the custodial closet?

A10. In the original floor plan, this is provided for. In the Option for Stock Building Floor Plan, the IT closet is eliminated and a wall rack in a classroom will be utilized.

Q11. Bid Extension Request

In order to properly evaluate the RFIs and provide a proposal accordingly, we are requesting a 2-week extension to the bid due date.

A11. The bid has been extended to October 27, 2026.

Q12. Axle, Tire, Hitch Storage

May axles, tires, and hitches remain stored beneath the building as specified, subject to required clearances?

A12. Yes

Q13. Bid Bond

Please define the “total price” used for the 5% bid security and the “total amount of the order” used for the performance bond. Do those amounts include all 30 monthly lease payments, setup, removal, and any renewal period? Is a payment bond also required?

A13. See Addendum 1 BID #1899 – Rent/Lease of Modular Buildings for Boyet Junior High and Abita Elementary School, Item 6 A. OR B. OR C. for the 5% Bid security. The “total amount of the order” for the performance bond will be based on the awarded monthly lease and ITEM 2 (delivery, installation, set-up, leveling, anchoring and skirting for all units). Also, a payment bond is not required.

Q14. Rent Start Timing

It is stated that rent begins when setup is complete such that the building may be occupied. Does rent begin upon modular completion and availability for Owner utility and infrastructure connections, or only after final occupancy approval? Who bears rent or standby costs if other work delays occupancy?

A14. This was addressed in a previous addendum. (see Q27 & A27)

Q15. 60-day Period

Does the 60-day period run from written notice to proceed, site availability, approved drawings, permit approval, or another milestone? Does the \$1,000/day assessment stop at completion of modular setup, substantial completion, or overall project occupancy??

A15. Written Notice to Proceed on site will be given with site availability and in coordination with the Infrastructure Contractor. 60-day time frame and liquidated damages stop with completion of modular setup such that the Infrastructure Contractor can start their scope of work to install other systems and make utility connections and build steps, ramps, etc. to complete the project for occupancy.

Q16. Notice to Proceed/PO

Will the PO be issued at the same time as the NTP? If not, what is the expected difference in time?

A16. Purchase Order will not be issued with On-site Notice to Proceed. Purchase Order will be issued asap subsequent to award and executed lease document. Difference in time between purchase order and on-site NTP appears to be approximately 60 - 75 days.

Q17. Return Condition

Please define “typical heavy use,” ordinary wear, and damage beyond ordinary wear. The specification says the Owner will pay no additional amount for repairs or replacements upon return; please confirm whether vandalism, casualty, misuse, and Owner-installed systems are excluded from that provision.

A17. The Owner will not pay for additional costs for use of the buildings. The Owner installed systems are identified and no additional costs related to these installations will be paid. Typical School Heavy Use is to be anticipated and will not be a reason for additional costs. Items such as vandalism that may be insurance related would be handled separately.

Q18. Lease Document

Please provide the proposed lease and construction contract terms, or confirm that the successful bidder's submitted standard lease will govern except for specifically identified negotiated revisions. Please establish an order of precedence among the bid, addenda, drawings, specifications, acknowledgment statement, and lease.

A18. The lease is to acknowledge the bid documents including addenda.

Q19. Progress Payment

When is Item 2 payable, how will monthly rent be invoiced and paid, and when is Item 3 payable? Are mobilization or progress payments available for engineering, permitting, or foundation work before occupancy?

A19. Purchase Order will be issued for delivery and set up and for monthly lease for the remainder of the STPPS fiscal year (through June 30, 2027). Lease for each subsequent year or portion of year will be issued each July 1. The final year of use will include the demobilization and removal costs.

Q20. Bonding

Please confirm that performance bonds (or P&P bonds) will only be required for the performance of work at the project site

A20. The “total amount of the order” for the performance bond will be based on the awarded monthly lease and ITEM 2 (delivery, installation, set-up, leveling, anchoring and skirting for all units.

BOYET JUNIOR HIGH AND ABITA SPRINGS MODULAR BUILDINGS

Questions and Corresponding Answers

Q1. Bid package / independent awards

Please confirm that revised Bid #1899 replaces the Abita procurement previously identified as Bid #1898, and whether any separate #1898 submission remains required. Confirm bidders may submit Boyet only, Abita only, or both and that awards may be made separately by campus.

[A1: See Addendum 1 for BID #1899 – Rent/Lease of Modular Buildings for Boyet Jr. High, Item 1, 2, 3 & 6](#)

Q2. Deadline / corrected Boyet form

Please confirm bids for either or both campuses are due October 27, 2026 at 2:30 PM in Owner local time, and issue a corrected Boyet bid form showing that date. Does the October 16 at 2:00 PM question deadline remain in effect, and will a written addendum answer these questions before bid?

[A2. See Addendum 1 for Bid #1899 – Rent/Lease of Modular Buildings for Boyet Junior High, Item 1, 2, 3 & 6](#)

Q3. Exact delivery / setup / occupancy dates

For EACH campus, please provide the planned award date, shop-drawing release date, site-ready date, on-site NTP date, first-delivery date, all-buildings-installed deadline and required occupancy date. Reconcile Boyet's February 2027 narrative with mid/end January in its bid form. Clarify whether each approximate month refers to NTP, delivery or completed setup.

[A3: For Both campuses, the anticipated time frame for units to be delivered on site and set up is to be in coordination with the Infrastructure Contractor. Our current anticipated time for modular delivery for each site is mid/end January, 2027.](#)

[Abita Springs Elementary: Infrastructure Contractor Contract is awarded and being executed. NTP for site is anticipated in November, 2026. The modular lease is expected to be awarded within 2 weeks of bid date and then executed. All work such as shopdrawings and Fire Marshal approval is expected to begin upon award in order to have units ready to be shipped and delivered to meet schedule for installation to support the overall schedule of the Infrastructure Contractor.](#)

[Boyet Junior: Infrastructure Contractor Contract has been approved and a Contract is being issued for execution. NTP for Infrastructure is anticipated for early December, 2026. The modular lease is expected to be awarded within 2 weeks of bid date and then executed. All work such as shopdrawings and Fire Marshal approval is expected to begin upon award in order to have units ready to be shipped and delivered to meet schedule for installation to support the overall schedule of the Infrastructure Contractor.](#)

Q4. Factory lead time / separate NTPs

Please confirm the 60-day period covers on-site delivery and installation only and begins after approved drawings, required industrial approvals/decals and site readiness, separate from award/design release. Will the agreed schedule accommodate the factory lead time after final drawing sign-off? Abita Q1 cites approximately 12–14 weeks, but A1 does not expressly accept that duration.

A4: For Both campuses, the 60- day period covers on-site delivery and complete installation of the modular units and all connections associated with the modular units operating as one building. The schedule is as indicated in Question 3 above.

Q5. Contract completion / liquidated damages

Please define the milestone that stops the 60-day period and \$1,000/day damages: modular Item 2 completion, tested systems or final occupancy approval. Are the days calendar days? Do damages apply once per contract, once per campus or per building? Confirm written extensions for Owner/infrastructure delays, unavailable utilities, access restrictions and documented approval delays beyond vendor control

A5. See page 2 of this Addendum, corresponding answer to question 15

Q6. Work by others / occupancy handover

Please identify who schedules and completes utilities, fire alarm, access control, ramps/walkways and final occupancy inspections at each campus, with committed completion dates. Confirm which modular tests, inspections and punch-list items must be completed before Item 2 acceptance, and whether return visits caused by incomplete infrastructure are separately compensated.

A6. For Both Campuses, the Infrastructure Contract will install and complete utilities, fire alarm, access control, ramps/walkways, and final occupancy inspections. Modular Contractor is responsible for anything related to the modular units themselves and their installation that may be inspected such as anchoring, foundations, connections of buildings and internal electrical systems, etc. Modular Contractor will be required to provide return visits as necessary for associated final inspections and completion of any punch list items attributable to the modulares themselves. There will be no additional compensation for return visits for these purposes.

Q7. Rent commencement

Does Abita A27 also apply to Boyet? Please define the rent-start date for each building/campus, including the 30-day utility-delay and whether the minimum lease term begins on that date. If modular scope and system testing are complete but occupancy is delayed by other trades or approvals unrelated to modular work, when will rent start?

A7. Yes, Abita A27 applies similarly to Boyet Jr. modular.

Q8. Installation payment / utility delay

Please provide item 2 invoicing and payment milestones for each campus. Under may payment be made for completed modular scope while utility-dependent testing remains outstanding. If modular scope and system testing are complete but occupancy is delayed by other trades or approvals unrelated to modular work, when will rent start.

A8. Utility site work and tie in is dependent upon Modular installation. As such, there is no delay for Owner/Infrastructure work relative modular scope. The ability to test all systems of the modular building completely cannot be accomplished until utility connections are complete. At that time, the modular company should anticipate a return visit to test all systems.

Q9. Document precedence / Q&A applicability

Please issue the document order of precedence and confirm exactly which Abita Bid #1898 Q&A answers incorporated by Item 14 also amend Boyet. In particular, address A3–A9, A14–A25 and A27–A32 covering boards, damage, roof warranty, engineers, MEP, interiors, rent, insurance and forms. If an answer applies only to Abita, please expressly identify that limit.

[A9. All apply to Boyet Jr.](#)

Q10. Bid / performance bond calculation

Please provide the exact 5% bid-security calculation for Boyet only, Abita only and both campuses, including whether Items 2 and 3 are included and how mixed new/like-new selections are evaluated. Confirm the performance-bond amount, whether rental payments are included, and the duration of required bond coverage.

[A10. See Addendum 1 for BID #1899 – Rent/Lease of Modular Buildings for Boyet Jr. High, Item 6; see Item 7; The performance bond amount Shall include the monthly lease rental along with Item 2.](#)

Q11. Required price options / evaluation

Please confirm a bid offering new units only is responsive and how unavailable like-new options should be marked. For Abita, must both the program Item 1 and stock Item 1.1 be priced? Please state the evaluation formula, including lease, setup, removal and discounts, and how Items 2 and 3 should be differentiated if the two Abita layouts have different costs.

[A11. A bid offering new units only is responsive. If not providing a bid for an option mark N/A.](#)

Q12. Lease terms / precedence

Please provide the proposed Owner lease rider or required contract terms for review before bid. Confirm how conflicts between the bid requirements and the vendor's standard lease will be resolved and whether material commercial terms can be agreed before award, since Item 12 moves submission of the standard lease to after award.

[A12. The lease is to acknowledge the bid documents including addenda. The lease agreement will be sent to our Attorney for review.](#)

Q13. Property insurance / loss payee

Please confirm the School Board will provide replacement-cost property insurance for each leased building, including applicable windstorm, flood, theft and vandalism risks, naming MH Leasing LLC as loss payee and providing the insured protections required by the agreed lease. Please supply proposed coverage, deductibles and endorsements. A29 states certificate-holder status only, and Nadler's damage waiver is unavailable in Louisiana.

[A13. See attached St. Tammany Parish School Board Certificate of Insurance](#)

Q14. Excess damage / restoration

Please confirm the treatment of casualty, vandalism, broken components, unreported water damage and other damage beyond normal classroom wear. Do Abita A4 and A24–A25 prohibit all additional return charges, including restoration after Owner wall attachments and canopy flashing, and do those answers apply to Boyet? Please identify any costs the Owner will reimburse and any restoration the modular vendor must include.

A14. These answers apply to Boyet Jr. also. The Owner will not reimburse modular company for ANY damage due to our use of these modulares for their intended use. We are clear that items will be added into these buildings. There will be no reimbursement relative to these items being added.

Q15. Roof warranty / lease service

Please confirm the A5 deletion of the 10-year roof warranty also applies to Boyet. Identify the vendor's required HVAC, roof/wall leak and other maintenance obligations, response times and exclusions for occupant damage or work by others. Confirm whether Boyet must also provide a certified roofer located within St. Tammany Parish.

A15. Yes, the A5 to Abita Springs Elementary applies to Boyet Jr.

Q16. Professional liability evidence

Please confirm Abita A30's certificate-holder requirement replaces the additional-insured wording for Boyet as well. Will proof of \$1 million professional E&O carried by the responsible engineer satisfy the requirement, and is a separate professional policy required of the modular leasing vendor?

A16. The engineer of record will provide E & O insurance of \$1 million with STPSB as additional insured. The awarded modular vendor will need to provide all other insurance as required.

Q17. Markerboards / tackboards conflict

Boyet's narrative excludes added markerboards/tackboards, but A101-P0 directs the modular vendor to provide them in each classroom for the full scope. Which governs? If modular scope, provide quantities, sizes, mounting locations and whether band/choral rooms are included. Does Abita A3 assigning boards to Owner/others also apply to Boyet?

A17. For Boyet Junior, the modular contractor is to provide for 2 (two) 6'x4' markerboards and 1 (one) 4'x4' tack board per classroom including the Band and the Choral Buildings. Locations to be determined with the awarded modular vendor. The marker and tack boards for the Abita Springs Elementary project were included in the Infrastructure Contractor scope of work.

Q18. Fire assembly / design criteria

Please confirm whether the classroom-wing dividing assembly is a 2-hour fire barrier or a structurally independent fire wall and provide required continuity, opening and mate-line details. Confirm the 1-hour corridor and rated-door requirements, final sprinkler determination, and which wind criteria govern: 149 mph on A005-P0 versus 154 mph on A001-P0, including Risk Category, exposure and design loads.

A18. In the Architect's opinion, the separation wall needs to be 2 hour fire barrier as defined by the IBC that breaks the building into 2 Fire Areas which brings occupant load for each fire area down below the requirement for sprinkler protection. **Final design of Fire Barrier, continuity, 1 hour corridor, rated doors, wind criteria etc. are the responsibility of the modular building Professional of Record.**

Q19. Building designations / quantities

Please confirm A = classroom wing, B = choral and C = band for pricing and utility coordination, correcting the specification's different labels. Confirm required module counts and net/gross areas for each building, including the apparent two-module band / four-module choral inconsistency, and acceptance criteria for an equivalent factory layout.

A19. As indicated in the documents including A100-P0 for Boyet Jr., Building A is the Classroom Wing Building, Building B is the Choral Building and Building C is the Band Building. The number of modular units required should together provide to satisfy the approximately dimensions shown on the same drawing.

Q20. Foundation / flood elevation

Please provide Boyet-specific geotechnical data, allowable bearing, prepared site grades and the controlling benchmark/datum. Confirm FFE for all three buildings, reconciling the 16-foot-or-greater leasing note and infrastructure plan elevations, and identify who procures/pays for required elevation certificates. Confirm differing conditions and unsuitable soil remediation remain Owner/infrastructure scope.

A20. See Attached geotechnical report for Boyet Junior High. Modular company needs to provide and pay for elevation certificates and are responsible for meeting code required elevation. Infrastructure contractor is responsible for site prep, so any issues discovered that require correction or additional work will also be their responsibility.

Q21. Engineers / permits / approval handoff

Please issue a campus-specific approval and permit DOR, including industrialized-building review/decals, site-install review, local permits, fees, foundation approval and final occupancy. Confirm whether Boyet may use a separate coordinated Louisiana foundation engineer under A7. Identify the modular architect/POR requirement at each campus and the remaining actions on Abita AR-26-022183.

A21. Modular Architect / Engineer PORS are the modular company's responsibility. Modular company may use a separate Louisiana licensed engineer. This project has been submitted to the Louisiana Fire Marshall and City of Slidell; both entities require the modular submittal information to complete their review.

Q22. Site readiness / delivery access

Please provide each campus's approved delivery route, staging area, lifting locations and allowable work windows, including weekends/holidays. Identify responsibility for traffic control, fence removal/replacement, overhead utility protection and utility marking. Confirm a written site-

ready notice before mobilization and the process for storage, standby and repeat mobilization caused by delayed site preparation.

A22. The Abita Elementary Site plan indicates the location for a crane if utilized. The narrative indicates the route for site access and the considerations of the site for other vehicular access requirements. The Boyet Jr. Site is an open field with access from a parking lot area through gates/fencing or utilizing limestone road in the area of the site as long as no impact to school car and bus lines are impacted. Schedule needs to align with timeframes outside of car and bus traffic for Boyet Jr. and Little Oak Middle Schools on school days or be achieved after hours on weekends/holidays. Modular Contractor and Infrastructure Contractor will need to coordinate relative to site preparation and modular unit delivery.

Q23. Electrical panels / revised baseline

For Boyet, please reconcile the single-phase panel specification with the infrastructure electrical design and confirm required ratings, quantities, fault-current criteria and spare circuits/capacity shown on the leasing plan. For Abita, confirm the revised nine-panel stock-layout E1.01/E4.00 baseline versus earlier ten-panel plans and state the panel baseline for full-program Item 1 with offices/data closet.

A23. Modular Contractor and Infrastructure Contractor will need to coordinate and review with Architect and Engineer. Stock panel may be different from full panel option.

Q24. Plumbing / special-system interfaces

Please confirm the precise modular-versus-infrastructure boundary at each campus for in-wall and underfloor plumbing, manifolds, freeze protection, final feeders, testing and fire/low-voltage pathways. Do Abita A19–A20 and A25 apply to Boyet? Provide modular rough-in locations/quantities, including entry conduits and spares, and confirm who supplies and installs extinguishers/cabinets by campus.

A24. Abita Springs Elementary 19-20 and A25 apply to Boyet Junior High.

Q25. Stock layout / site and design basis

Please confirm Item 1.1 retains the ADA staff restroom, custodial closet and internal hallway, with only offices/data closet omitted from the full program. Identify controlling footprint, classroom area, fixture counts, occupancy/design loads and required rated assemblies for each layout. Confirm infrastructure ramps, utilities and approvals will be revised by others to the selected layout and final factory connection points.

A25. See Addendum 1 Bid #1899 – Rent/Lease of Modular Buildings for Boyet Junior High.

Q26. Accessory quantities / blocking

Please provide quantities and performance criteria for A23 mirrors, grab bars, Student hand dryers and vendor-installed Owner dispensers including staff-restroom dryer/dispenser selection and wall blocking locations. Confirm rated IT-closet work is omitted under Item 1.1 where the data closet is deleted, and identify any replacement electrical/fire-rated room required.

A26. One (1) mirror per hand lavatory, grab bars at ADA toilets; location for owner dispensers will be coordinated with awarded modular vendor.

Q27. Minimum term / extension / removal

Please confirm 36 months for Boyet and 30 months for Abita begin at each agreed rent-start date, with no uncompensated early termination or quantity reduction after commitment. State extension rate, notice, utility-disconnect and access requirements at return. Does Abita A29's fixed Item 3 price without escalation apply after extensions and to Boyet, or may later removal be adjusted to prevailing rates?

A27. Lease is based upon number of months anticipated; quantity reduction is not anticipated. Should early termination be possible the cost of the remaining months would be negotiated. Should modular units remain beyond the lease period, lease rate would continue at same monthly rate.

Q28. Forms / post-opening deadlines

Please provide corrected campus-specific forms and confirm: stamped applicable layouts and signatory authority evidence with bid or before October 29 at 2:30 PM; original electronic bid bond within 48 hours of October 27 opening; and whether A32's ten-days-after-opening affidavit/acknowledgment deadline applies to Boyet as well as Abita. Confirm project numbers and replacement of the named competitor with the awarded vendor.

A28. See Addendum 1 for #1899 – Rent/Lease of Modular Buildings for Boyet Junior High, Item 2, 6 and 13, also see Bid instructions for #1899 – Rent/Lease of Modular Buildings for Boyet Junior High item 7 and 8 for electronic submittals.

3. Conduits running under walkways or roadways shall be encased in concrete **sleeves** for additional protection.

8.6 Safety and Compliance

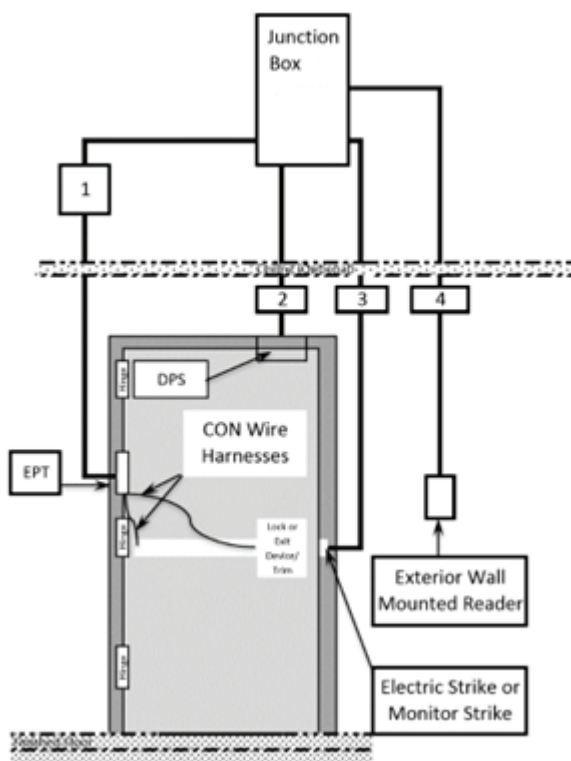
1. All underground work shall adhere to OSHA safety standards and local excavation requirements.
2. Contractors must verify utility locations before boring or trenching to prevent accidental damage.
3. Fire-rated penetrations through rated walls or floors must use **approved firestop assemblies** matching the wall or floor rating.

This section ensures **long-term reliability**, **ease of maintenance**, and **compliance with safety and building codes** for all underground and bored conduit installations.

9. Access Control Doors and Perimeter Cabling

9.1 General Overview

This section specifies the requirements for **access control door cabling and hardware installation** across St. Tammany Parish School Board facilities. All work shall comply with applicable **local, state, and national codes**, including NEC, TIA-569-E, and manufacturer guidelines for access control hardware. The objective is to ensure secure, reliable, and maintainable installations for all perimeter doors and points of access.



- | | |
|---|---|
| 1 | Conduit run from EPT to junction box. Vendor to provide pull string. |
| 2 | Conduit run from DPS to junction box. Vendor to provide pull string. |
| 3 | Conduit run from Elec Strike/Monitor to junction box. Vendor to provide pull string. |
| 4 | Conduit run from Wall Mounted Reader to junction box. Vendor to provide pull string. |
| 5 | Conduit from desk push button location to junction box. 18/6 stranded cable is needed from the push button to either the bridge or reader controller. |

See Section 9.5 for power requirements

9.2 Cabling Requirements

1. All access control doors shall be supplied with **black Category 6 UTP (or higher) cabling**.
2. Cable runs shall originate from the **nearest IDF** and terminate at a **junction box** located within 10 feet of the door opening, unless otherwise specified.
3. **Service Loop:**
 - Standard perimeter doors: 15-foot service loop.
 - Glass storefront or special doors: 20-foot service loop.
4. All cables must be **labeled at both ends** to indicate source IDF and door location.

9.3 Junction Box Requirements

1. Junction boxes shall be **weatherproof** and a minimum of **12" x 12" x 4"**.
2. Boxes shall be mounted above the door opening unless otherwise specified.
3. All conduit connections to junction boxes shall maintain a **minimum bend radius of 1 inch**, with no "LB" type fittings used.

9.4 Conduit and Pathway Requirements

1. All access control cabling shall be routed in **½" EMT conduit** from the junction box to the door reader/controller location.
2. Conduit bends shall maintain the **minimum 1-inch radius**.
3. All conduits shall be securely fastened and continuous to ensure mechanical protection of cables.
4. Removable mullions are preferred for double doors. Where doors are delivery points, mullions may be removed if **120 VAC in / 24 VDC out power supplies** are installed near the junction box.

9.5 Power Requirements

1. Electrified door hardware requiring **24 VDC** must be supplied via a **120 VAC in / 24 VDC out power supply** installed adjacent to the junction box.
2. Access control devices cannot directly provide 24 VDC; power must be supplied externally.
3. Panic bar electronic latch retractors must receive **24 VDC** power for operation.
4. All AC outlets serving access control hardware must be **non-switched** and dedicated.

9.6 Installation Practices

1. Conduit runs shall **avoid sharp bends, mechanical stress, and exposure to potential damage**.
2. All cabling shall be supported in accordance with TIA-569-E guidelines.
3. Wall penetrations shall use **fire-rated sleeves or approved firestop assemblies** where penetrating fire-rated walls.
4. Cable pathways shall be planned to **minimize exposure and maintain separation** from power and other services, preventing interference and ensuring safety.

9.7 Hardware

1. Exterior doors will be secured using access control hardware that is compatible with Isonas Pure Access Control System.

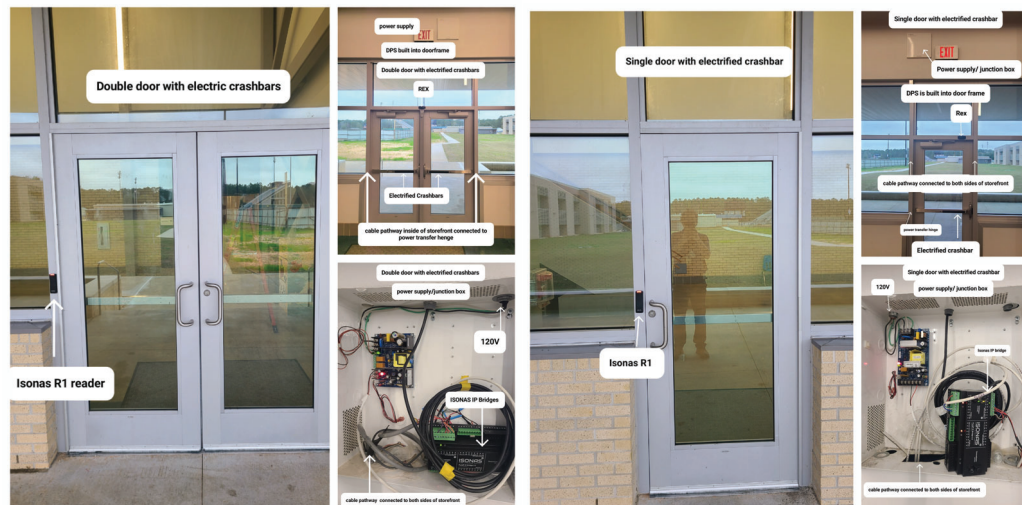
- **Example of access control, retractable crash bar (24V):**

Junction box: Isonas IP Bridge, Cat 6 (ran to IDF), 18\6 cable to the following locations:
Wall mounted reader, DPS, Rex, and power supply.

Power Supply: 12-v, Appropriate cable to retractable crash bars (including power transfer hinge at door) 18\6 cable to junction box

Single door: RC-04-PRX-M Mullion Reader Controller, ACC-EDK-3A Exterior Door Kit (EDK), Cable-RC04-25 Cable 8-Pin Snap-In Connector 25', DC-HD-16 Heavy Duty Door Cord with Aluminum Boxes, 16" (or power transfer hinge equivalent), Door position switch (DPS), Power supply

Double door: IPB2-0-2 Door Pure IP Bridge 2.0 (2 Door), R-1-MCT-M Mullion Reader, EMS-100 Request to Exit PIR Sensor (REX), 2X DC-HD-16 Heavy Duty Door Cord with Aluminum Boxes, 16" (or power transfer hinge equivalent), 2x Door position switch (DPS), Power supply.

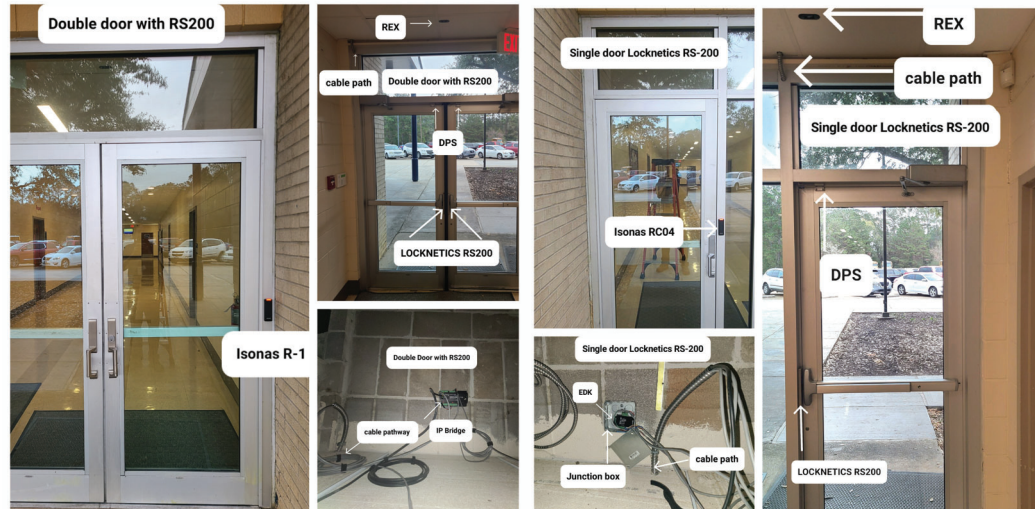


- **Example of working access control with 12V locks**

Junction box: Isonas IP Bridge, Cat 6 (ran to IDF), 18\6 cable to the following locations:
Wall mounted reader, DPS, Rex, latch with Locknetics RS210 (or compatible) lock.

Single door: RC-04-PRX-M Mullion Reader Controller, ACC-EDK-3A Exterior Door Kit (EDK), Cable-RC04-25 Cable 8-Pin Snap-In Connector 25', EMS-100 Request to Exit PIR Sensor (REX) Door position switch (DPS), Quick connect (if lock is installed on removeable mullion)

Double door: IPB2-0-2 Door Pure IP Bridge 2.0 (2 Door), R-1-MCT-M Mullion Reader, EMS-100 Request to Exit PIR Sensor (REX), 2x Door position switch (DPS), 2X Quick connect (if lock is installed on removeable mullion)



9.8 Compliance and Testing

1. All access control cabling shall be tested for **continuity and correct termination** prior to final acceptance.
2. All junction boxes, power supplies, and conduit installations shall be verified for **code compliance, mechanical integrity, and operational readiness**.

This section ensures **reliable operation, ease of maintenance, and code-compliant installations** for all access-controlled doors across the district.

10. Classroom Cameras, Vape Sensors, Voice, Data, and Additional Systems

10.1 General Overview

This section defines the requirements for the installation of **classroom cameras, vape sensors, Voice, Data, and other network-connected devices** including interactive panels, intercom systems, and bells. All work shall comply with **local, state, and national codes**, including NEC, TIA-569-E, and manufacturer-specific installation requirements.

The goal is to ensure **consistent, reliable, and maintainable installations** while maintaining safety and minimizing interference with existing network infrastructure.

10.2 Classroom Cameras

1. Classroom camera cabling shall be installed in the middle of the classroom with a service loop with enough slack to reach every corner to maximize field of view and minimize blind spots.
 - If a classroom is divided or segmented into multiple areas, classroom camera cabling shall be installed in all segments/areas.
2. All cameras shall be connected using **green Category 6 UTP (or higher) cabling**.



ECS Southeast, LLC

Geotechnical Engineering Report

Boyett Junior High School Renovations & Additions

9295 Rebel Drive
Slidell, St. Tammany Parish, Louisiana

ECS Project No. 65:2073

May 6, 2026





ECS SOUTHEAST, LLC

Geotechnical • Construction Materials • Environmental • Facilities

May 6, 2026

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ECS Project No. 65:2073

Reference: Geotechnical Engineering Report
Boyet Junior High School Renovations & Additions
9295 Rebel Drive
Slidell, St. Tammany Parish, Louisiana

Ms. Catoe:

ECS Southeast, LLC (ECS) has completed the subsurface exploration, laboratory testing, and geotechnical engineering analyses for the Boyet Junior High School Renovations & Additions project in Slidell, Louisiana. Our services were performed in accordance with our agreed to scope of work. This report presents our geotechnical analysis of the project along with the results of the field exploration and laboratory testing conducted, and our design and construction recommendations.

It has been our pleasure to be of service to St. Tammany Parish Public School System during the design phase of this project. We would appreciate the opportunity to remain involved during the continuation of the design phase, and we would like to provide our services during construction phase operations as well to evaluate subsurface conditions assumed for this report. Should you have any questions concerning the information contained in this report, or if we can be of further assistance to you, please feel free to contact us.

Respectfully submitted,

ECS Southeast, LLC

Leigh Brister, E.I.

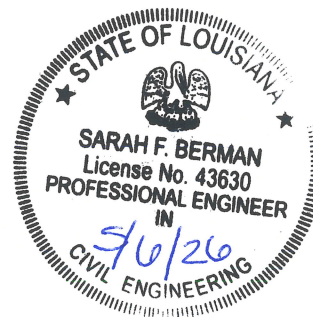
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APPENDICES

Appendix A – Drawings & Reports

- Site Location Diagram
- Boring Location Diagram

Appendix B – Field Operations

- Reference Notes for Boring Logs
- Boring Logs B-1

Appendix C – Laboratory Testing

- Laboratory Testing Results Summary

EXECUTIVE SUMMARY

The following summarizes the main findings of the exploration, particularly those that may have a cost impact on the planned development. Further, our principal foundation recommendations are summarized. Information gleaned from the Executive Summary should not be utilized in lieu of reading the entire geotechnical report.

PROJECT UNDERSTANDING:

- Structure Information: 5,900 square foot school building addition to Building 300
- Considerations: Near surface moisture sensitive soils

SUBSURFACE CONDITIONS:

- Surface Material: Approximately 3 inches of topsoil with grass cover
- Probable Fill: Not encountered
- Expected Undercut: Approximately 4-inches to remove topsoil in undisturbed areas, and 24 inches for any existing foundation and/or pavement elements and for existing surficial silt layer
- Natural Material: Dark Brown Clayey Silt (ML), Orange, Gray, Tan, and Brown Lean Clay (CL)
- Swell Potential (PVR): Moderate
- Groundwater: Encountered at a depth of approximately 17 feet below existing site grade at the time of drilling

DESIGN & CONSTRUCTION RECOMMENDATIONS:

- Shallow Foundations: Max. Net Allowable Bearing Pressure: 2,000 psf for column and wall footings with column load up to 50 kips
- Slab-on-Grade: Modulus of Subgrade Reaction of 115 pci
- Seismic Site Class: IBC Site Class "D"

ECS should be retained to review all project documents to confirm conformance with our recommendations, and to perform CMT testing for earthwork and foundation construction activities to document that our recommendations are strictly followed. This also allows us to quickly provide recommendations for remedial activities, where necessary.

1.0 INTRODUCTION

The purpose of this study was to provide geotechnical information for the design of a new addition to Building 300 at the existing Boyett Junior High School in Slidell, LA. The project will include a single-story education building addition with associated bus and vehicle parking and drive areas. We anticipate the plan area of the building to encompass approximately 5,900 square feet. The recommendations developed for this report are based on project information supplied by St. Tammany Parish Public School System and MSH Architects through email, or estimations made by ECS.

Our services were provided in accordance with our Proposal No. 3668P, dated March 25, 2026, as authorized by Mr. Cameron Tipton with St. Tammany Parish Public School System on March 30, 2026, which includes our Terms and Conditions of Service.

This report contains the procedures and results of our subsurface exploration and laboratory testing programs, review of existing site conditions, engineering analyses, and recommendations for the design and construction of the project.

2.0 PROJECT INFORMATION

2.1 PROJECT LOCATION/CURRENT SITE USE/PAST SITE USE

The project is located at 9295 Rebel Drive in Slidell, LA. The site is an existing junior high school campus with multiple classroom buildings and asphalt pavement. The topography of the site is relatively flat with surface elevations ranging from +13 feet MSL to +15 feet MSL. The elevations and topographic variations were estimated from Google Earth, Figure 2.1.1.



FIGURE 2.1.1: General Site Location Outlined in Red

2.2 PROPOSED CONSTRUCTION

The following table explains our understanding of the planned development including proposed buildings and related infrastructure. If ECS's understanding of the project is not correct, especially if the structural loads or elevations are different, please contact ECS so that we may review these changes and revise our recommendations, as appropriate.

Subject	Design Information/Understandings
Building Footprint	Approximately 5,900 Square Feet
Number of Stories	One
Usage	Education
Assumed Column Loads	50 kips Maximum
Assumed Wall Loads	2 Kips Per Linear Foot (klf) Maximum
Anticipated Finish Floor Elevation	Within 2 feet of existing grade at EL +14 feet

3.0 FIELD EXPLORATION AND LABORATORY TESTING

3.1 FIELD EXPLORATION PROGRAM

The field exploration scope was to characterize the project site in general geotechnical and geological terms and to evaluate subsequent field and laboratory data to assist in the determination of geotechnical recommendations.

3.1.1 Soil Borings

Our scope of work included drilling a total of one (1) soil test boring. The soil test boring was performed within the footprint of the proposed building addition and was advanced to a depth of approximately 40 feet below existing site grade. Our boring was located with a handheld GPS unit, and its approximate location is shown on the Boring Location Diagram in Appendix A. The approximate ground surface elevations noted in this report were estimated from Google Earth.

Representative soil samples were obtained by means of Shelby tube sampling procedures in accordance with ASTM Specifications D-1587 in cohesive soils. In the Shelby tube sampling procedure, a thin walled, steel, seamless tube with sharp cutting edges is pushed hydraulically into the soil, and a relatively undisturbed sample is obtained.

Field logs of the soils encountered in the borings were maintained by the drill crew. After recovery, each geotechnical soil sample was removed for the sampler and visually classified. Representative portions of each soil sample were then wrapped in plastic and transported to our laboratory for further visual examination and laboratory testing. After completion of the drilling operations, the boreholes were backfilled with cuttings to the existing ground surface.

3.2 SUBSURFACE CHARACTERIZATION

The following text provides generalized characterizations of the soil strata encountered during our subsurface exploration. For specific subsurface information, please refer to the Boring Logs in Appendix B:

Approximate Depth (ft)	Stratum No.	Soil Description
0 – 0.25 ft	-	GRASS COVER
0.25 – 2.0 ft	I	CLAYEY SILT (ML) , Firm, Dark Brown
2.0 – 33.0 ft	II	LEAN CLAY (CL) , Firm to Stiff, Orange, Gray, Tan, and Brown
33.0 – 38.0 ft	III	LEAN CLAY (CL) , Soft, Brown and Gray
38.0 – 40.0 ft	IV	LEAN CLAY (CL) , Stiff, Orange and Gray

Notes:

- (1) Soil descriptions show generalized strata to 40'. Strata in the borings vary with depth, please see attached Boring Log in Appendix B.

Please refer to the attached boring logs and laboratory data summary for this field exploration for a more detailed description of the subsurface conditions encountered in the borings as the stratification descriptions above are generalized for presentation purposes.

3.3 GROUNDWATER OBSERVATIONS

Groundwater levels, if observed, were made in the borings during and shortly after drilling operations. In auger drilling operations, water is not introduced into the borehole and the groundwater position can often be evaluated by observing water flowing into and out of the excavation. Furthermore, visual observation of soil samples retrieved can often be used in evaluating the groundwater conditions.

Groundwater was encountered at an approximate depth of 17 feet below existing site grade at the time of drilling.

The highest groundwater observations are normally encountered in the late winter or early spring or following seasonal heavy rainfall events. Fluctuations in the location of the long-term water table may occur due to changes in precipitation, evaporation, surface water runoff and other factors not immediately apparent at the time of this exploration. Therefore, the groundwater conditions at this site are expected to be significantly influenced by surface water runoff and seasonal rainfall.

3.4 LABORATORY TESTING

The laboratory testing consisted of selected tests performed on samples obtained during our field exploration operations. Classification and index property tests were performed on representative soil samples. The soil samples were tested for Moisture Content (ASTM D2216), Atterberg Limits (ASTM D4318), and Unconfined Compression (ASTM D2166).

Each sample was visually classified based on texture and plasticity in accordance with ASTM D2488 Standard Practice for Description and Identification of Soils (Visual-Manual Procedures) and including USCS classification symbols. After classification, the samples were grouped in the major zones noted on the boring logs in Appendix B. The group symbols for each soil type are indicated in parentheses along

with the soil descriptions. The stratification lines between strata on the logs are approximate; in situ, the transitions may be gradual.

The soil samples will be retained in our laboratory for a period of 60 days, after which, they will be discarded unless other instructions are received as to their disposition.

4.0 DESIGN RECOMMENDATIONS

The following recommendations have been developed based on Sections 2 and 3. If there are any changes to the project characteristics or if different subsurface conditions are encountered during construction, ECS should be consulted so that the recommendations of this report can be reviewed. Site grading information was not provided during this report; however, we have assumed that the finished floor elevation will be less than 2 feet above existing site elevations. If the finished floor elevation deviates from this assumed site grades, the recommendations provided below should be evaluated by our office.

4.1 GEOTECHNICAL CONSIDERATIONS

Based on the subsurface conditions encountered in the borings, the anticipated loading conditions and the lowest level bearing elevation, the site appears suited for the proposed development provided the recommendations herein are strictly adhered to.

4.1.1 Presence of Expansive Soils

Based on the laboratory test results, the subsurface soils encountered within the building pad and pavement areas generally have a moderate swelling potential. Soils with swelling potential located above the water table and within depths that are subject to changes in moisture are expected to experience volume change and were considered in our potential vertical rise (PVR) estimation. **The potential vertical rise (PVR) is estimated to be on the order of about 1 inch or less using an applied load of 1.0 psi.**

Generally, one (1) inch of PVR is acceptable as the maximum allowable value used for design and construction. However, the structural engineer must confirm if these PVR values are within acceptable limits for the specific project. These PVR estimations assume that the soils are allowed to increase/decrease in moisture content from a relatively dry condition to a relatively wet condition over a depth of approximately 10 feet from the existing ground surface at the time of field exploration.

4.1.2 Moisture Sensitive Soils

Based on the laboratory test results, fine grained soils were disclosed directly beneath the surface layer across the site. These soils are moisture sensitive, subject to volume changes and will become inadequate when wet of their optimum moisture content as evaluated by ASTM D698 and may be prone to desiccation cracking that can cause degradation over seasonal wetting and drying periods. Effective site drainage should be implemented into the final designs, the onset of construction, and maintained throughout the construction process. Care should be taken to keep construction traffic to a minimum across the site during wet periods. Water should not be allowed to pond on construction areas (building pads or pavement subgrade) or near foundational supports.

4.1.3 Perimeter Conditions

Positive drainage away from the structure should be provided during construction and maintained throughout the life of the proposed project. Water should not be allowed to infiltrate into the excavations during construction. Foundation soils should not be allowed to become wet. Grades must be sloped to provide effective drainage away from the building during and after construction. Adjacent concrete sidewalks and pavements should be sloped to provide drainage away from the building, and joints should be sealed; close attention should be paid to those directly abutting the building.

Roof runoff and surface drainage should be collected and discharged away from the structure to prevent wetting of the foundation soils. Roof gutters should be installed and connected to downspouts and pipes directing roof runoff into stormwater collection systems or discharged onto positively sloped pavements.

4.2 SHALLOW FOUNDATIONS

Based on the results of our subsurface investigation, for structures with maximum column loads of 50 kips or less and provided subgrades and engineered fills are prepared as recommended in this report, the proposed structure can be supported by shallow foundations. For column loads greater than 50 kips, ECS should be consulted to provide additional recommendations. We recommend that the foundation design use the following parameters:

Design Parameter	Column Footing	Wall Footing
Net Allowable Bearing Pressure ⁽¹⁾	2,000 psf	
Acceptable Bearing Soil Material	Compacted Engineered Fill or Stiff Native Soil	
Minimum Width	24 inches	24 inches
Minimum Footing Embedment Depth (below slab or finished grade) ⁽²⁾	24 inches	24 inches
Estimated Total Settlement ⁽³⁾	Less than 1-inch	Less than 1- inch
Estimated Differential Settlement ⁽⁴⁾	Less than 1/2 inches	Less than 1/2 inches

Notes:

- (1) Net allowable bearing pressure is the applied pressure in excess of the surrounding overburden soils above the base of the foundation.
- (2) For bearing considerations and expansive soil concerns.
- (3) The estimated total settlement is for up to a 5 x 5 ft square footing at a maximum imposed bearing pressure of 2,000 psf. If final column loads are greater than 50 kips, ECS must be contacted to approve foundation recommendations and update settlement calculations, if needed.
- (4) Based on maximum column/wall loads and variability in borings. Differential settlement can be re-evaluated once the foundation plans are more complete.

Potential Undercuts: Many of the soils at the foundation bearing elevation are anticipated to be adequate for support of the proposed structure without remediation. If soft or loose soils are observed during footing observations, the footings should be extended to adequate bearing soils. Undercut areas should be backfilled with compacted engineered fill or lean concrete ($f'c \geq 1,000$ psi at 28 days) to the original design bottom of footing elevation; the original footing should be constructed on top of the hardened lean concrete or engineered fill, seen in Figure 4.2.2. If engineered fill is used to backfill the undercut footing, the over-excavated footings should be widened accordingly on all sides for each one (1) foot of over excavation in Figure 4.2.2. If lean concrete is used for backfill, the over-excavation does not require widening.

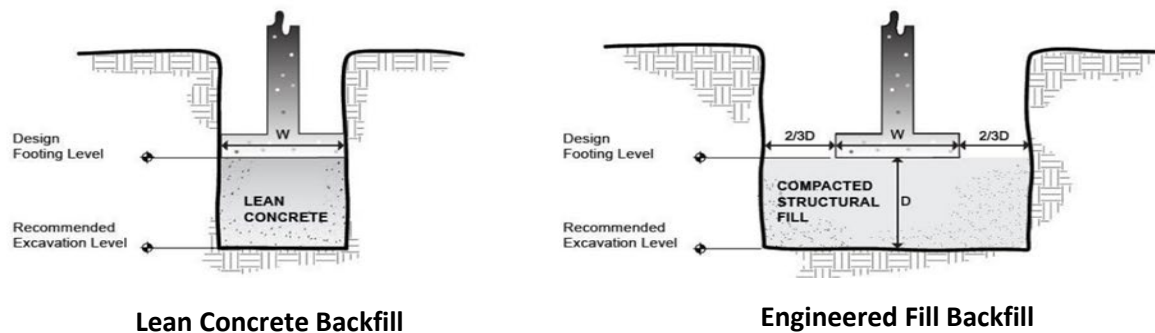


Figure 4.2.2: Undercut backfill schematic.

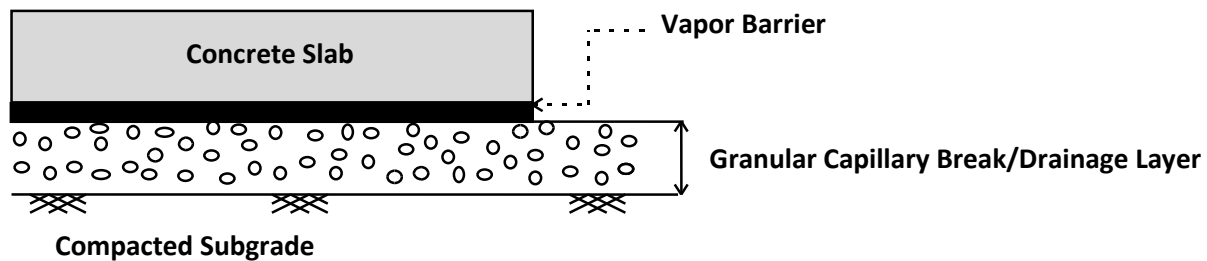
The net allowable soil bearing pressure refers to that pressure which may be transmitted to the foundation bearing soils in excess of the final minimum surrounding overburden pressure. The final footing elevation should be evaluated by ECS's geotechnical engineering personnel to evaluate that the bearing soils can support the recommended net allowable bearing pressure and adequate for foundation construction. These evaluations should include visual observations using a T-probe or static cone penetrometer, or with the use of a Dynamic Cone Penetrometer (DCP), if necessary. Evaluations should be performed within each column footing excavation (minimum of 2 tests per column footing) and at intervals not greater than 25 feet in continuous footings. The DCP testing should extend at least 2 feet below the final foundation subgrade. A minimum DCP value of 10 blows should be used for the evaluation of the foundations.

The settlement of a structure is a function of the compressibility of the bearing materials, bearing pressure, actual structural loads, fill depths, and the bearing elevation of footings with respect to the final ground surface elevation. Estimates of settlement for foundations bearing on engineered or non-engineered fills are strongly dependent on the quality of fill placed. Factors that may affect the quality of fill include loose lift thickness of the fills placed and the amount of compactive effort placed on each lift. If the recommendations outlined in this report are followed, we expect total settlements for the proposed construction to be in the range of 1 inch or less, while the differential settlement will be approximately half of the anticipated total settlement. This evaluation is based on our engineering experience and the anticipated loadings for this type of structure and is intended to aid the structural engineer with the design.

Exposure to the environment may weaken the soils at the foundation bearing level if the foundation excavations remain exposed during periods of inclement weather. Therefore, foundation concrete should be placed the same day that final excavation is achieved, and the design bearing pressure verified. If the bearing soils are softened by surface water absorption or exposure to the environment, the softened soils must be removed from the foundation excavation bottom immediately prior to placement of concrete. If the foundation excavation must remain open overnight, or if rainfall is apparent while the bearing soils are exposed, we recommend that a 1 to 3-inch thick "mud mat" of "lean" concrete be placed over the exposed bearing soils before the placement of reinforcing steel.

4.3 SLABS ON GRADE

Provided subgrades and engineered fills are prepared as discussed herein, the proposed floor slabs can be constructed as Ground Supported Slabs (or Slab-On-Grade) on lean natural soils, newly placed engineered fill, or lime treated soils. Based on the assumed finished floor elevation, it appears that the slabs will bear on native clay material or newly placed engineered fill. Figure 4.2.1 depicts our soil-supported slab recommendations with positive drainage away from the concrete slab:



1. Drainage Layer Thickness: 4 inches
2. Drainage Layer Material: GRAVEL (GP, GW), SAND (SP, SW)

FIGURE 4.2.1: Slab on Grade Detail

Soft or yielding soils may be encountered in some areas. Those soils should be removed and replaced with compacted Engineered Fill in accordance with the recommendations included in this report.

Subgrade Modulus: Provided the engineered fill and granular drainage layer are constructed in accordance with our recommendations, the slab may be designed assuming a modulus of subgrade reaction, k_1 of **115 pci (lbs./cu. inch)**. The modulus of subgrade reaction value is based on a 1 ft by 1 ft plate load test basis.

Vapor Barrier: Before the placement of concrete, a vapor barrier may be placed on top of the granular drainage layer to provide additional protection against moisture penetration through the floor slab. When a vapor barrier is used, special attention should be given to surface curing of the slab to reduce the potential for uneven drying, curling and/or cracking of the slab. Depending on proposed flooring material types, the structural engineer and/or the architect may choose to eliminate the vapor barrier.

Slab Isolation: Soil-supported slabs should be isolated from the foundations and foundation-supported elements of the structure so that differential movement between the foundations and slab will not induce excessive shear and bending stresses in the floor slab. Where the structural configuration prevents the use of a free-floating slab such as in a drop-down footing/monolithic slab configuration, the slab should be designed with adequate reinforcement and load transfer devices to reduce risk of overstressing of the slab.

4.4 SEISMIC DESIGN CONSIDERATIONS

Seismic Site Classification: The International Building Code (IBC) 2021 requires site classification for seismic design based on the upper 100 feet of a soil profile. The methods are utilized in classifying sites, namely the shear wave velocity (v_s) method; the unconfined compressive strength (s_u) method; and the Standard Penetration Resistance (N-value) method. The unconfined compressive strength (s_u) method was used in classifying this site.

Site Class	Soil Profile Name	Shear Wave Velocity, V_s , (ft./s)	Soil Undrained Shear Strength (psf)	Standard Penetration Resistance (N-value)
A	Hard Rock	$V_s > 5,000$ fps	N/A	N/A
B	Rock	$2,500 < V_s \leq 5,000$ fps	N/A	N/A
C	Very dense soil and soft rock	$1,200 < V_s \leq 2,500$ fps	≥ 2000	$N > 50$
D	Stiff Soil Profile	$600 \leq V_s \leq 1,200$ fps	$1000 \leq S_u \leq 2000$	$15 \leq N \leq 50$
E	Soft Soil Profile	$V_s < 600$ fps	< 1000	$N < 15$

Based upon our interpretation of the subsurface conditions, the appropriate **Seismic Site Classification** is “**D**” as shown in the table above.

The Site Class definition should not be confused with the Seismic Design Category designation which the Structural Engineer typically assesses. If a higher site classification is beneficial to the project, we can provide additional testing methods that may yield more favorable results.

4.5 PAVEMENTS

Subgrade Characteristics: Based on the results of our borings, it appears that the pavement subgrade will consist mainly of lean clay soils. The soils across the site are moisture sensitive and will become difficult to work with when wet. Care should be taken if construction is performed during wet weather periods.

California Bearing Ratio [CBR] testing was not performed as part of this study. Therefore, we have assumed a CBR value of the onsite subsoil to be 3 for preliminary design purposes.

We were not provided traffic loading information, so we have assumed loadings typical of this type of project. We assumed a maximum daily traffic volume of 500 automobiles and 12 buses/delivery trucks for medium duty pavement areas, and a maximum daily traffic volume of 250 automobiles, and three buses/delivery trucks for light duty pavement areas. Our pavement section recommendations for medium duty (drives) pavements should accommodate occasional heavier loadings due to trash trucks, delivery vehicles and light truck traffic and may be considered for main drives. Typical pavement sections are presented in the table below. Actual pavements sections and joint spacing, if applicable, should be designed based on specific traffic loads.

MATERIAL	FLEXIBLE PAVEMENT		RIGID PAVEMENT		
	Medium Duty	Light Duty	Heavy ^(3,4) Duty	Medium Duty	Light Duty
Portland Cement Concrete ($f'_c = 4000$ psi)	-	-	8 in.	6 in.	5 in.
Asphaltic Concrete Surface Course	2 inches	1 ½ inches	-	-	-
Asphaltic Concrete Binder Course	2 inches	1 ½ inches	-	-	-
Graded Aggregate Base Course ^(1,2)	6 inches	6 inches	4 inches	4 inches	4 inches
Proofrolled In-Situ Lean Clay or Compacted Engineered Fill (Min. Thickness)	12 inches	12 inches	12 inches	12 inches	12 inches

Notes:

- (1) Should be a No. 610 limestone or similarly graded alternative, compacted to 95% of its max dry density. See Section 5.2.2 for more details.
- (2) 12-inches of lime treated fill for rigid pavement and 12-inches of cement treated fill for flexible pavement may be used as an alternative to aggregate base course. Review Section 5.2.1 for additional information.
- (3) Large, front loading garbage trucks frequently impose concentrated front wheel loads on pavements during loading. This type of loading typically results in rutting of asphalt pavement and ultimately pavement failures. For preliminary design purposes, we recommend that the pavement in trash pickup areas consist of an 8-inch thick, 4,000 psi, reinforced concrete slab over at least 12 inches of properly compacted engineered fill material.
- (4) We recommend that dumpster pads and entrances/exits be considered heavy-duty pavement areas.

Pavement Considerations: In regions of improper surface and/or subsurface drainage, a softening of the subgrade and other problems related to the deterioration of the pavement can be expected. Positive drainage will reduce the possibility of the subgrade materials becoming saturated during the normal service period of the pavement.

The reinforced pavement in the trash pick-up area should extend to a minimum of 5 feet past the location of the expected wheel loads. When traffic loading becomes available, ECS or the Civil Engineer can design the pavements. Appropriate jointing should also be incorporated into the design of the PCC pavement which should be specified, constructed, and tested to meet the following requirements:

1. Proper pavement joint spacing and saw-cutting will be required to prevent excessive slab curling and shrinkage cracking. Joints should be sealed to prevent entry of foreign material and dowelled where necessary for load transfer and saw cutting should be performed while the concrete is in its “green” state. The design engineer should refer to ACI330R-08 for more detailed for the design of rigid pavement.
2. Portland Cement Concrete: Minimum compressive strength of 4,000 psi at 28 days.
3. Hot Mix asphaltic concrete should conform to the latest edition of the LSSRB Section 502. Engineered fill should meet the criteria for material properties and compaction recommended in Section 5.1 of this report.
4. Crushed aggregate base should be compacted to maximum lift height of eight inches to a minimum of 95 percent of the Standard Proctor (ASTM D698) maximum dry density. Aggregate should conform to the latest edition of the Louisiana Standard Specifications for Roads and Bridges (LSSRB) Section 1003.03.

Representative soil samples should be collected from the upper 2 feet of the final pavement subgrade to assess the suitability of the in-situ CBR values, prior to implementation of the pavement sections provided herein. Often during construction and preparation of the roadway subgrade, the soil materials may be improved and can sometimes yield reduced pavement sections based on the actual CBR values and traffic loads.

5.0 SITE CONSTRUCTION RECOMMENDATIONS

5.1 SUBGRADE PREPARATION

The existing soils are moisture sensitive and will become inadequate when above their optimum moisture content as evaluated by ASTM D698. Effective site drainage should be implemented at the beginning of and maintained throughout construction activities. Care should be taken to keep construction traffic to a minimum during and immediately after times of inclement weather.

ECS should be on-site full-time during earthwork and foundation construction activities to document that our recommendations are strictly followed and to provide recommendations for remedial activities, if necessary.

5.1.1 Stripping and Grubbing

The subgrade preparation should consist of stripping approximately 4 to 6 inches of topsoil and organic-laden material, as well as the complete removal of any existing fill, construction debris, existing foundation and pavement elements, and utilities and soft or yielding materials from the 10-foot expanded building limits, and 5 feet beyond the toe of engineered fills. Furthermore, trees within the proposed footprint will need to be removed for construction, and associated roots or other debris may be present in the subsurface.

The in-situ silts encountered within the upper 2 feet across the project site will undergo a significant loss of stability when construction activities are performed. The soils in the project area can become easily disturbed if subjected to conventional rubber tire or narrow track-type equipment and excessive moisture. Therefore, strong consideration should be given during the design phase to undercut and replace this material with an engineered fill material meeting the soil property requirements as outlined in Section 5.2.3 of this report.

Deeper topsoil or organic laden soils may be present in wet, low-lying, and poorly drained areas. In wooded areas, the root balls may extend as deep as about 2 feet and will require additional localized stripping depth to completely remove the organics. ECS should be retained to evaluate that topsoil and poor surficial materials have been removed prior to the placement of Engineered Fill or construction of structures.

Note: Following stripping and grubbing, the entire construction area should be proofrolled as outlined in Section 5.1.2 of this report. Soils observed to rut or deflect greater than an inch in depth should be undercut and replaced or otherwise mitigated.

5.1.2 Proofrolling

Following clearing activities and prior to fill placement or other construction on subgrades, the subgrades should be evaluated by an ECS field technician. The exposed subgrade should be thoroughly proofrolled with a half loaded tandem-axle dump truck or similar construction equipment weighing a minimum of 15 tons. Proofrolling should be traversed in two perpendicular directions with overlapping passes of the vehicle under the observation of an ECS technician. This procedure is intended to assist in identifying localized yielding materials.

Where proofrolling identifies areas of yielding or “pumping” subgrade those areas should be repaired prior to the placement of subsequent engineered fill or other construction materials. Observations of yielding or “pumping” should be addressed with ECS to establish the appropriate remediation as outlined in Section 5.2.

5.2 EARTHWORK OPERATIONS

5.2.1 Subgrade Stabilization

Methods of stabilization include undercutting, moisture conditioning, or chemical stabilization. Test pits may be excavated to explore the shallow subsurface materials to help in determining the cause of the observed inadequate materials, and to assist in the evaluation of appropriate remedial actions to stabilize the subgrade. Anticipated methods of subgrade stabilization of the near surface soils are provided below:

Moisture Conditioning: If it is established that high moisture content is the cause of the inadequate subgrade, the geotechnical engineer may require the earthwork contractor process the upper 12 to 18 inches of in-situ subgrade by windrowing with a dozer or plowing with a set of heavy-duty disk harrows until soil moisture is observed to be within 2 percent of its optimum moisture content as evaluated by ASTM D698 to improve subgrade conditions before consideration other mitigation approaches. The drying effort should begin after the exposed subgrade is free of standing water and the windrowing/disking should be continuous during a period of dry weather. ECS should be onsite to periodically perform soil moisture testing. The processed areas should be sealed with compaction equipment and a flat drum roller or dozer blade at the end of the day in case of overnight rain. If weather conditions do not allow appropriate time to dry the native subgrade, the geotechnical engineer may recommend chemical treatment with lime or cement in order to provide an adequate working surface for fill placement.

Lime Stabilization: Lime stabilization may be used to modify onsite clay soils to achieve an adequate working surface and achieve PIs between 10 and 25 for reuse as engineered fill. The amount of lime necessary to achieve lime stabilization will vary depending on the clay mineral, plasticity and type of lime used for stabilization. For estimating purposes 4 to 6% percent of lime by volume should be used; however, a laboratory lime series should be performed at the time of construction to establish the optimum lime content. Surficial samples should be collected from across the site and testing should be conducted on the composite sample. The subgrade soils should meet the requirements of Section 305.4, and lime treatment of the subbase should meet the requirements of Section 304 - Type B, of the latest edition of the LSSRB. An ECS Field Engineer or Senior Technician should be present during lime treatment activities to observe lime quantities and document that treated areas are in conformance with the project

requirements. Please note that caution should be used when powdered lime is used in closely populated areas. To control dust, a lime slurry or pelletized lime may be used where dust must be controlled. In addition, pelletized lime will generally require 2 to 3 times the effort to properly pulverize and mix into the clay soils than a powder or slurry.

Cement Stabilization: When soils have PI values of 15 or below, cement stabilization should be used in lieu of lime treatment. Additionally, 12 inches of cement stabilized soil can be used as an alternative to aggregate base course for light and medium duty flexible pavement. A minimum of 10% by volume of cement is recommended to use for a cement stabilized base course and should be prepared in general accordance with LSSRB, Section 303-04. Note that the cement treatment of the roadways should be conducted in general accordance with LSSRB, Section 303. Cement stabilized base course should yield a compressive strength of at least 250 psi at 7 days as evaluated by a mix design in accordance with DOTD TR 432 Standard Procedure. The treated soil should be compacted at least 95% of maximum dry density +/-3% the optimum moisture content in accordance with the Sub-section 303.11 of LSSRB.

Undercut and Replace: If other means of soil stabilization are not practical, the undercutting or removal of the inadequate subsurface material may be required. The undercutting of such material will be conducted, inspected, and tested in accordance with Section 5.1.

5.2.2 Probable Fill

Fill material was not encountered in the soil borings at the time of exploration; however, the site is developed and there may be fill or other deleterious materials beneath existing structures. Fill material or debris may be encountered between or beyond our borings. If encountered, ECS recommends removing the existing fill and debris in its entirety and replacing it with well compacted engineered fill meeting the parameters outlined in this report.

5.2.3 Engineered Fill

Prior to placement of engineered fill, representative bulk samples (approximately 50 pounds) of on-site and/or off-site borrow should be submitted to ECS for laboratory testing, which will typically include Atterberg limits, natural moisture content, grain-size distribution, and moisture-density relationships (i.e., Proctors) for compaction. Import materials should be tested prior to being hauled to the site to evaluate if they meet project specifications. Alternatively, Proctor data from other accredited laboratories can be submitted if the test results are within the last 90 days.

Satisfactory Engineered Fill Materials: Materials satisfactory for use as Engineered Fill should consist of inorganic soils and conform to the criteria below.

ENGINEERED FILL INDEX PROPERTIES		
Soil Type	USCS Classification	Property
Imported Clay Fill	CL, SC	LL < 45, 10<PI<25
Imported Sand Fill	SP, SP-SM	Less than 10% passing #200 sieve
Aggregate Base	GP	LADOTD 610 crushed limestone or similarly graded recycled aggregate

ENGINEERED FILL INDEX PROPERTIES		
Soil Type	USCS Classification	Property
On-Site Soils	CL/CH	Most of the lean clay soils in the appear to meet the requirements for reuse as engineered fill. The fat clay soils do not meet the requirements without lime treatment due to avg. LL's greater than 45.

ENGINEERED FILL COMPACTION REQUIREMENTS	
Subject	Requirement
Compaction Standard	Standard Proctor, ASTM D698
Required Compaction	95% of Max. Dry Density
Moisture Content	Optimum to +3 % Points of the Soil's Optimum Value
Loose Thickness	8 Inches Prior to Compaction

Fill Placement: Excessively wet fill soils or aggregates should be scarified, aerated, and moisture conditioned prior to compaction.

On-Site Borrow Suitability: Natural deposits of soils that meet the definition above may be used as engineered fill on the site.

5.3 FOUNDATION AND SLAB OBSERVATIONS

Protection of Foundation Excavations: Exposure to the environment may weaken the soils at the footing bearing level if the foundation excavations remain open; therefore, foundation concrete should be placed the same day that excavations are made. Bearing soils that are weakened by surface water intrusion or exposure must be removed from the foundation excavation bottom immediately prior to placement of concrete. If the excavation must remain open overnight, or if rainfall becomes imminent while the bearing soils are exposed, a 1 to 3-inch thick "mud mat" of "lean" concrete should be placed on the bearing soils before the placement of reinforcing steel.

Footing Subgrade Observations: Most of the soils at the foundation bearing elevation are anticipated to be adequate for support of the proposed structure. ECS should observe the foundation subgrade prior to placing foundation concrete, to confirm the bearing soils are as recommended.

Slab Subgrade Verification: Prior to placement of a drainage layer, the subgrade should be prepared in accordance with the recommendations found in Section 5.1.

5.4 UTILITY INSTALLATIONS

Utility Subgrades: The soils encountered in our exploration are expected to be generally adequate for support of utility pipes. The pipe subgrades should be observed and probed for stability by ECS. Loose or inadequate materials encountered should be removed and replaced with adequate compacted Engineered Fill, or pipe stone bedding material.

Utility Backfilling: The granular bedding material (often AASHTO #57 stone) should be at least 4 inches thick, but not less than that specified by the civil engineer's project drawings and specifications. We recommend that the bedding materials be placed up to the springline of the pipe. Fill placed for support of the utilities, as well as backfill over the utilities, should conform to Section 5.2.

Excavation Safety: Excavations and slopes should be constructed and maintained in accordance with OSHA excavation safety standards. The contractor is solely responsible for designing, constructing, and maintaining adequate excavations and slopes. The contractor's responsible person, as defined in 29 CFR Part 1926, should evaluate the soil exposed in the excavations as part of the contractor's safety procedures. In no case should slope height, slope inclination, or excavation depth, including utility trench excavation depth, exceed those specified in local, state, and federal safety regulations. ECS is providing this information solely as a service to our client. ECS is not assuming responsibility for construction site safety or the contractor's activities; such responsibility is not being implied and should not be inferred.

6.0 CLOSING

ECS has prepared this report to guide the geotechnical-related design and construction aspects of the project. We performed these services in accordance with the standard of care expected of professionals in the industry performing similar services on projects of like size and complexity at this time in the region. No other representation expressed or implied, and no warranty or guarantee is included or intended in this report.

The description of the proposed project is based on information provided to ECS by St. Tammany Parish Public School System and MSH Architects. If any of this information is inaccurate or changes, either because of our interpretation of the documents provided or site or design changes that may occur later, ECS should be contacted so we can review our recommendations and provide additional or alternate recommendations that reflect the proposed construction.

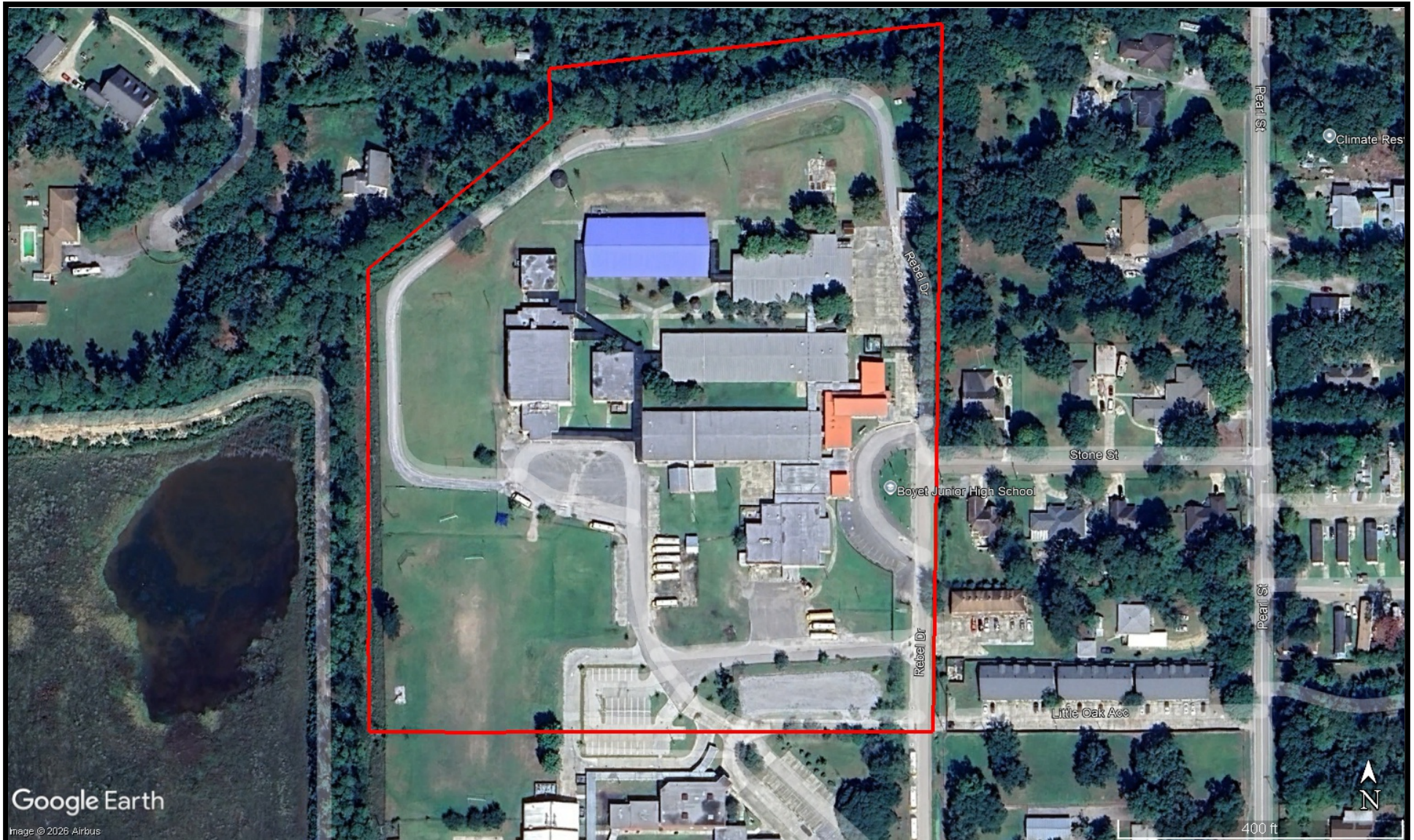
We recommend that ECS review the project plans and specifications so we can confirm that those plans/specifications are in accordance with the recommendations of this geotechnical report.

Field observations, and quality assurance testing during earthwork and foundation installation are an extension of, and integral to, the geotechnical design. We recommend that ECS be retained to apply our expertise throughout the geotechnical phases of construction, and to provide consultation and recommendation should issues arise.

ECS is not responsible for the conclusions, opinions, or recommendations of others based on the data in this report.

APPENDIX A – Diagrams & Reports

Site Location Diagram
Boring Location Diagram



Google Earth

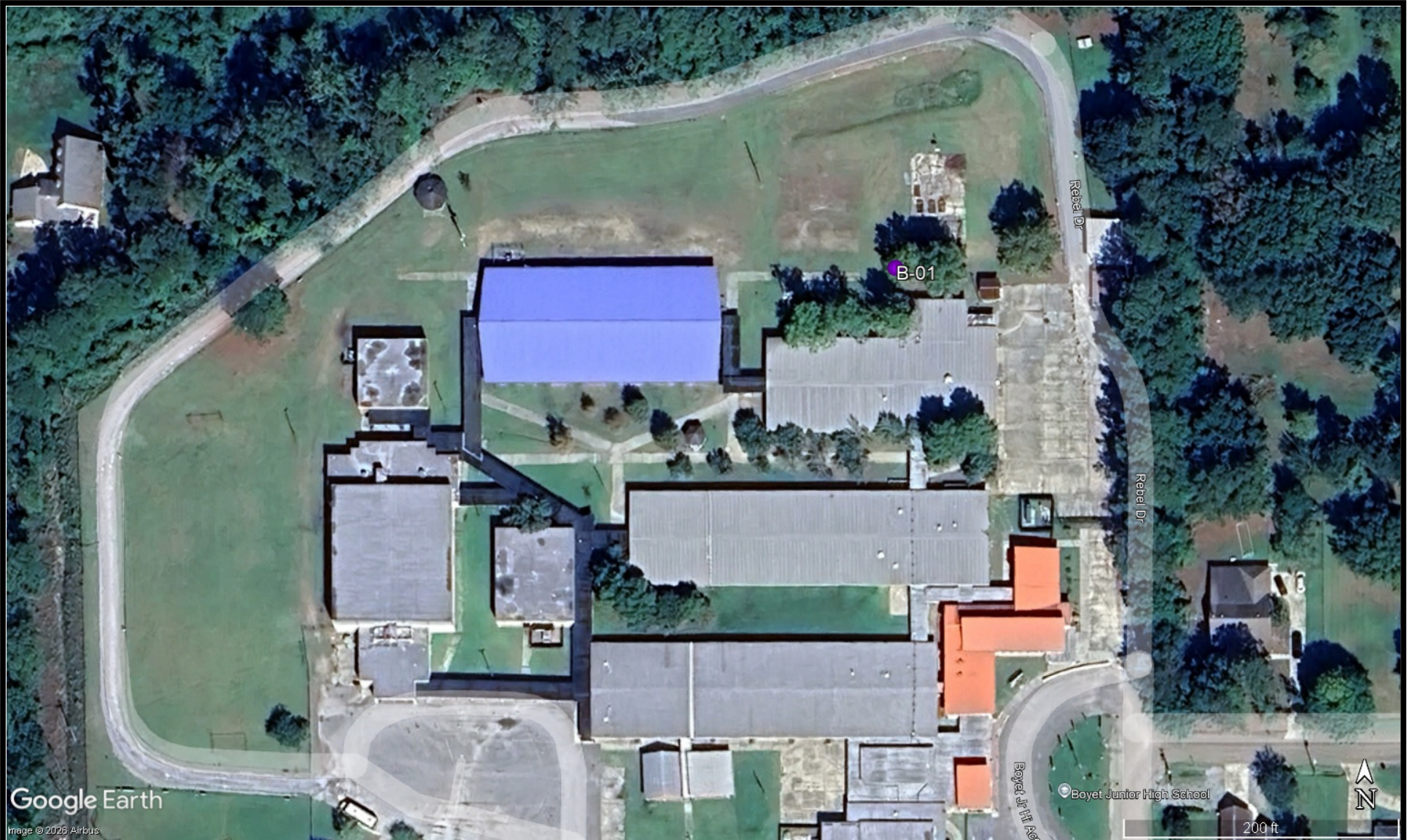
Image © 2026 Airbus



SITE LOCATION DIAGRAM BOYET JUNIOR HIGH SCHOOL RENOVATIONS & ADDITIONS

9295 REBEL DRIVE, SLIDELL, LOUISIANA

ENGINEER LB8
SCALE AS NOTED
PROJECT NO. 65:2073
FIGURE 1 OF 1
DATE 5/1/2026



BORING LOCATION DIAGRAM BOYET JUNIOR HIGH SCHOOL RENOVATIONS & ADDITIONS

9295 REBEL DRIVE, SLIDELL, LOUISIANA

ENGINEER LB8
SCALE AS NOTED
PROJECT NO. 65:2073
FIGURE 1 OF 1
DATE 5/1/2026

APPENDIX B – Field Operations

Reference Notes for Boring Logs

Boring Log B-01



REFERENCE NOTES FOR BORING LOGS

MATERIAL^{1,2}

	ASPHALT
	CONCRETE
	GRAVEL
	TOPSOIL
	VOID
	BRICK
	AGGREGATE BASE COURSE
	GW WELL-GRADED GRAVEL gravel-sand mixtures, little or no fines
	GP POORLY-GRADED GRAVEL gravel-sand mixtures, little or no fines
	GM SILTY GRAVEL gravel-sand-silt mixtures
	GC CLAYEY GRAVEL gravel-sand-clay mixtures
	SW WELL-GRADED SAND gravelly sand, little or no fines
	SP POORLY-GRADED SAND gravelly sand, little or no fines
	SM SILTY SAND sand-silt mixtures
	SC CLAYEY SAND sand-clay mixtures
	ML SILT non-plastic to medium plasticity
	MH ELASTIC SILT high plasticity
	CL LEAN CLAY low to medium plasticity
	CH FAT CLAY high plasticity
	OL ORGANIC SILT or CLAY non-plastic to low plasticity
	OH ORGANIC SILT or CLAY high plasticity
	PT PEAT highly organic soils

DRILLING SAMPLING SYMBOLS & ABBREVIATIONS

SS	Split Spoon Sampler	PM	Pressuremeter Test
ST	Shelby Tube Sampler	RD	Rock Bit Drilling
WS	Wash Sample	RC	Rock Core, NX, BX, AX
BS	Bulk Sample of Cuttings	REC	Rock Sample Recovery %
PA	Power Auger (no sample)	RQD	Rock Quality Designation %
HSA	Hollow Stem Auger		

PARTICLE SIZE IDENTIFICATION

DESIGNATION	PARTICLE SIZES
Boulders	12 inches (300 mm) or larger
Cobbles	3 inches to 12 inches (75 mm to 300 mm)
Gravel: Coarse	¾ inch to 3 inches (19 mm to 75 mm)
Fine	4.75 mm to 19 mm (No. 4 sieve to ¾ inch)
Sand: Coarse	2.00 mm to 4.75 mm (No. 10 to No. 4 sieve)
Medium	0.425 mm to 2.00 mm (No. 40 to No. 10 sieve)
Fine	0.074 mm to 0.425 mm (No. 200 to No. 40 sieve)
Silt & Clay ("Fines")	<0.074 mm (smaller than a No. 200 sieve)

COHESIVE SILTS & CLAYS

UNCONFINED COMPRESSIVE STRENGTH, QP ⁴	SPT ⁵ (BPF)	CONSISTENCY ⁷ (COHESIVE)
<0.25	<2	Very Soft
0.25 - <0.50	3 - 4	Soft
0.50 - <1.00	5 - 8	Firm
1.00 - <2.00	9 - 15	Stiff
2.00 - <4.00	16 - 30	Very Stiff
4.00 - 8.00	31 - 50	Hard
>8.00	>50	Very Hard

RELATIVE AMOUNT ⁷	COARSE GRAINED (%) ⁸	FINE GRAINED (%) ⁸
Trace	≤5	≤5
With	10 - 20	10 - 25
Adjective (ex: "Silty")	25 - 45	30 - 45

GRAVELS, SANDS & NON-COHESIVE SILTS

SPT ⁵	DENSITY
<5	Very Loose
5 - 10	Loose
11 - 30	Medium Dense
31 - 50	Dense
>50	Very Dense

WATER LEVELS⁶

	WL (First Encountered)
	WL (Completion)
	WL (Seasonal High Water)
	WL (Stabilized)

FILL AND ROCK

FILL	POSSIBLE FILL	PROBABLE FILL	ROCK

¹Classifications and symbols per ASTM D 2488-17 (Visual-Manual Procedure) unless noted otherwise.

²To be consistent with general practice, "POORLY GRADED" has been removed from GP, GP-GM, GP-GC, SP, SP-SM, SP-SC soil types on the boring logs.

³Non-ASTM designations are included in soil descriptions and symbols along with ASTM symbol [Ex: (SM-FILL)].

⁴Typically estimated via pocket penetrometer or Torvane shear test and expressed in tons per square foot (tsf).

⁵Standard Penetration Test (SPT) refers to the number of hammer blows (blow count) of a 140 lb. hammer falling 30 inches on a 2 inch OD split spoon sampler required to drive the sampler 12 inches (ASTM D 1586). "N-value" is another term for "blow count" and is expressed in blows per foot (bpf). SPT correlations per 7.4.2 Method B and need to be corrected if using an auto hammer.

⁶The water levels are those levels actually measured in the borehole at the times indicated by the symbol. The measurements are relatively reliable when augering, without adding fluids, in granular soils. In clay and cohesive silts, the determination of water levels may require several days for the water level to stabilize. In such cases, additional methods of measurement are generally employed.

⁷Minor deviation from ASTM D 2488-17 Note 14.

⁸Percentages are estimated to the nearest 5% per ASTM D 2488-17.

CLIENT: St. Tammany Parish Public School System				PROJECT NO.: 65:2073		BORING NO.: B-01		SHEET: 1 OF 2																
PROJECT NAME: Boyet Junior High School Renovations & Additions				DRILLER/CONTRACTOR: Precision Sampling, Inc																				
SITE LOCATION: 9295 Rebel Drive, Slidell, Louisiana, 70461								LOSS OF CIRCULATION																
LATITUDE: 30.292886		LONGITUDE: -89.738282		STRUCTURE:		SURFACE ELEVATION: 14		BOTTOM OF CASING																
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DISTANCE	SAMPLE RECOVERY (IN)	DESCRIPTION OF MATERIAL	STRATIGRAPHY	ELEVATION (FT)	WATER LEVELS	BLOWS/6" (TCP/MC/SPT N-VALUE)	Rec %  RQD % 	LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)	QP (TSF)	FINES CONTENT	DRY UNIT WEIGHT (PCF)	COMPRESSIVE STR. (TSF)							
5	S-1	ST	24	4	Topsoil [Thickness=3"]. (ML) CLAYEY SILT - dark brown, firm.		10					22	4	18.1	3.0									
	S-2	ST	24	8	(CL) LEAN CLAY - orange and gray, stiff.															2.0				
	S-3	ST	24	8										45	28				18.7	1.5				
	S-4	ST	24	7															18.7	3.75				
10	S-5	ST	24	8	(CL) LEAN CLAY - tan and gray, very stiff.															21.6	2.5	106.54	2.81	
15	S-6	ST	24	6	(CL) LEAN CLAY - tan and gray, firm.																29.2	1.5		
20	S-7	ST	24	8	(CL) LEAN CLAY - brown and gray, firm to stiff.																45.5	2.5	75.60	1.07
25	S-8	ST	24	8										45.0	2.5									
	S-9	ST	24	8	(CL) LEAN CLAY - dark gray, firm.									52.1	1.5	68.10	0.51							
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL																								
 WL (First Encountered):				17 FT		BORING STARTED:				04/01/2026		CAVE IN DEPTH:				Not Observed								
 WL (Completion):						BORING COMPLETED:				04/01/2026		HAMMER TYPE:												
 WL (Seasonal High Water):						EQUIPMENT:		LOGGED BY:		DRILLING METHOD:														
 WL (Stabilized):						Track		Mason W																
GEOTECHNICAL BOREHOLE LOG																								

CLIENT: St. Tammany Parish Public School System				PROJECT NO.: 65:2073				BORING NO.: B-01				SHEET: 2 OF 2															
PROJECT NAME: Boyet Junior High School Renovations & Additions				DRILLER/CONTRACTOR: Precision Sampling, Inc																							
SITE LOCATION: 9295 Rebel Drive, Slidell, Louisiana, 70461												LOSS OF CIRCULATION															
LATITUDE: 30.292886				LONGITUDE: -89.738282				STRUCTURE:				SURFACE ELEVATION: 14				BOTTOM OF CASING											
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DISTANCE	SAMPLE RECOVERY (IN)	DESCRIPTION OF MATERIAL	STRATIGRAPHY	ELEVATION (FT)	WATER LEVELS	BLOWS/6" (TOP/MC/SPT N-VALUE)	Rec %  RQD % 					LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)	QP (TSF)	FINES CONTENT	DRY UNIT WEIGHT (PCF)	COMPRESSIVE STR. (TSF)						
										0 20 40 60 80 100																	
35	S-10	ST	24	7	(CL) LEAN CLAY - dark gray, firm. (continued)		-20										35.3	1.25									
					(CL) LEAN CLAY - brown and gray, soft.																						
	S-11	ST	24	7	(CL) LEAN CLAY - orange and gray, stiff.												-25										
					END OF BORING at 40.0 FT																						
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL																											
 WL (First Encountered):					17 FT					BORING STARTED:					04/01/2026					CAVE IN DEPTH:					Not Observed		
 WL (Completion):										BORING COMPLETED:					04/01/2026					HAMMER TYPE:							
 WL (Seasonal High Water):										EQUIPMENT:					LOGGED BY:					DRILLING METHOD:							
 WL (Stabilized):										Track					Mason W												
GEOTECHNICAL BOREHOLE LOG																											

APPENDIX C – Laboratory Testing

Laboratory Test Results Summary
Compressive Strength Test Results

Laboratory Testing Summary

Sample Location	Sample Number	Depth (ft)	^MC (%)	Soil Type	Atterberg Limits			**Percent Passing No. 200 Sieve	Moisture - Density		CBR (%)		#Organic Content (%)
					LL	PL	PI		<Maximum Density (pcf)	<Optimum Moisture (%)	0.1 in.	0.2 in.	
B-01	S-1	0.0-2.0	18.1		22	18	4						
B-01	S-2	2.0-4.0	19.7										
B-01	S-3	4.0-6.0	18.7		45	17	28						
B-01	S-4	6.0-8.0	18.7										
B-01	S-5	8.0-10.0	21.6										
B-01	S-6	13.0-15.0	29.2										
B-01	S-7	18.0-20.0	45.5										
B-01	S-8	23.0-25.0	45.0										
B-01	S-9	28.0-30.0	52.1										
B-01	S-10	33.0-35.0	35.3										

Notes: See test reports for test method, ^ASTM D2216-19, *ASTM D2488, **ASTM D1140-17, #ASTM D2974-20e1 < See test report for D4718 corrected values

Definitions: MC: Moisture Content, Soil Type: USCS (Unified Soil Classification System), LL: Liquid Limit, PL: Plastic Limit, PI: Plasticity Index, CBR: California Bearing Ratio, OC: Organic Content

Project: Boyet Junior High School Renovations & Additions
Client: St. Tammany Parish Public School System

Project No.: 65:2073
Date Reported: 4/27/2026



Office / Lab
ECS Southeast LLC - Baton Rouge

Address
11211 Industriplex Blvd
Suite 300
Baton Rouge, LA 70809

Office Number / Fax
(225)224-2583
(225)612-7062

Tested by	Checked by	Approved by	Date Received
JMercante	JMercante	JMercante	

Laboratory Testing Summary

Sample Location	Sample Number	Depth (ft)	^MC (%)	Soil Type	Atterberg Limits			**Percent Passing No. 200 Sieve	Moisture - Density		CBR (%)		#Organic Content (%)
					LL	PL	PI		<Maximum Density (pcf)	<Optimum Moisture (%)	0.1 in.	0.2 in.	
B-01	S-11	38.0-40.0	38.7										

Notes: See test reports for test method, ^ASTM D2216-19, *ASTM D2488, **ASTM D1140-17, #ASTM D2974-20e1 < See test report for D4718 corrected values

Definitions: MC: Moisture Content, Soil Type: USCS (Unified Soil Classification System), LL: Liquid Limit, PL: Plastic Limit, PI: Plasticity Index, CBR: California Bearing Ratio, OC: Organic Content

Project: Boyet Junior High School Renovations & Additions
Client: St. Tammany Parish Public School System

Project No.: 65:2073
Date Reported: 4/27/2026



Office / Lab	Address	Office Number / Fax
ECS Southeast LLC - Baton Rouge	11211 Industriplex Blvd Suite 300 Baton Rouge, LA 70809	(225)224-2583 (225)612-7062

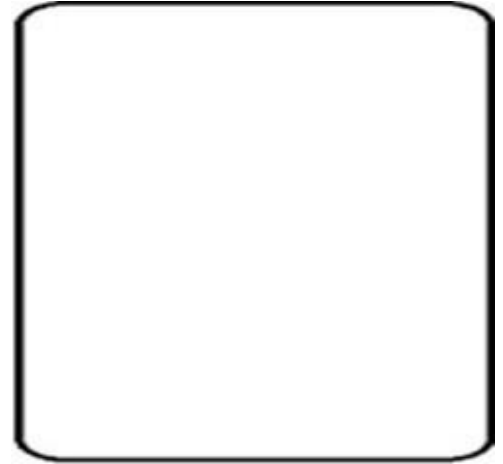
Tested by	Checked by	Approved by	Date Received
JMercante	JMercante	JMercante	

Unconfined Compressive Strength Test Report

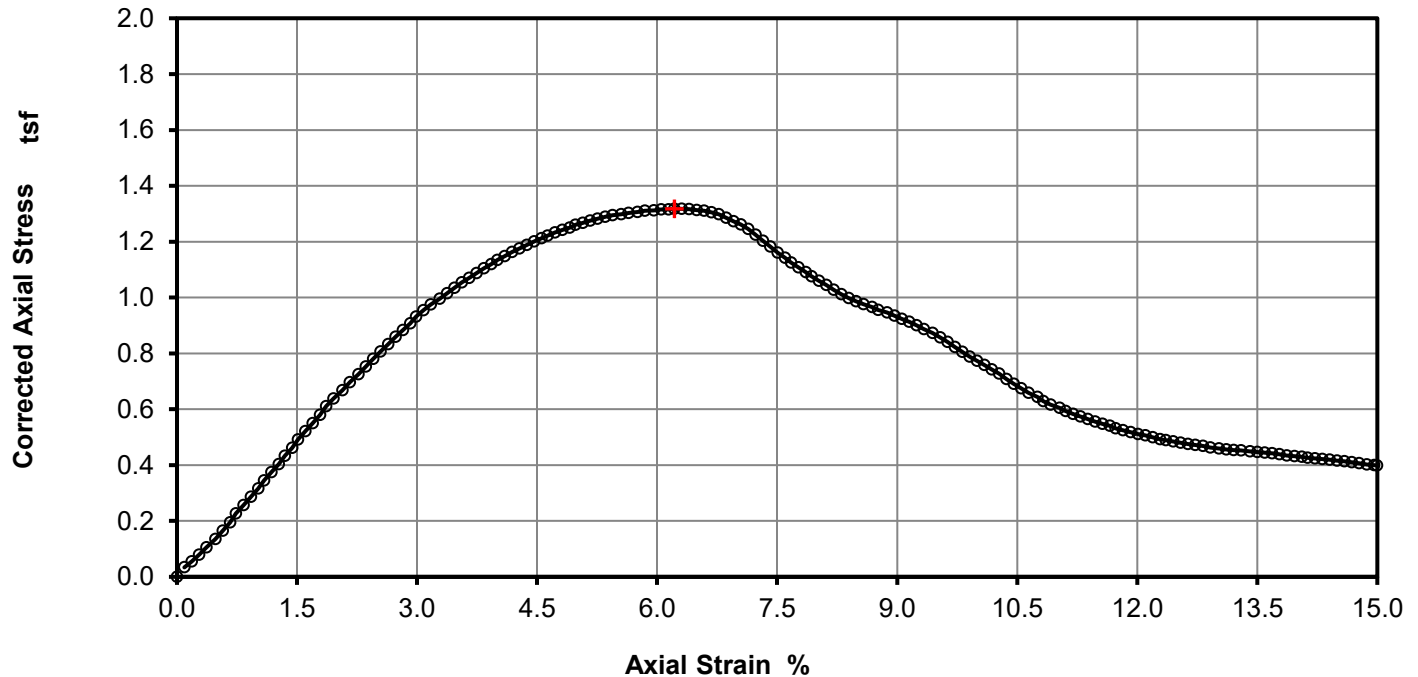
ASTM D2166

Initial Conditions

Height	in	5.42
Diameter	in	2.85
Bulk Density	pcf	129.79
Water Content	%	18.7
Dry Density	pcf	109.36
Voids Ratio		0.512
(Specific gravity assumed 2.65)		
Degree of Saturation	%	97
Rate of Strain applied	%/min	1.11
At failure	%	6.2
	tsf	1.32
	tsf	0.7



Stress vs Axial Strain



Project: Boyet Junior High School Renovations & Additions

Client: St. Tammany Parish Public School System

Sample Source: B-01

Sample Description:

Project No.: 65:2073

Depth (ft): 4 - 6

Sample No.: S-3

Date Reported: 4/27/2026



Office / Lab

Address

Office Number / Fax

ECS Southeast LLC - Baton
Rouge

11211 Industriplex Blvd
Suite 300
Baton Rouge, LA 70809

(225)224-2583

(225)612-7062

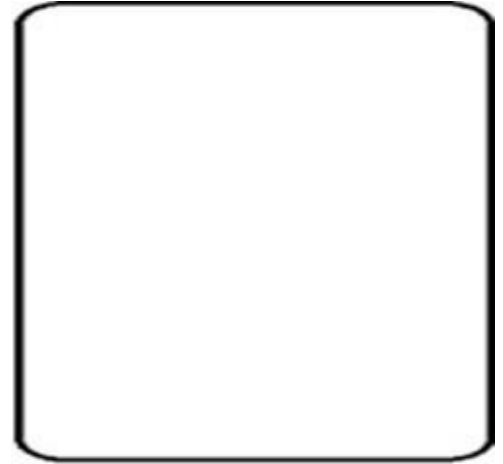
Tested by	Checked by	Approved by	Date Received	Remarks
JMercante	NBurke	NBurke		

Unconfined Compressive Strength Test Report

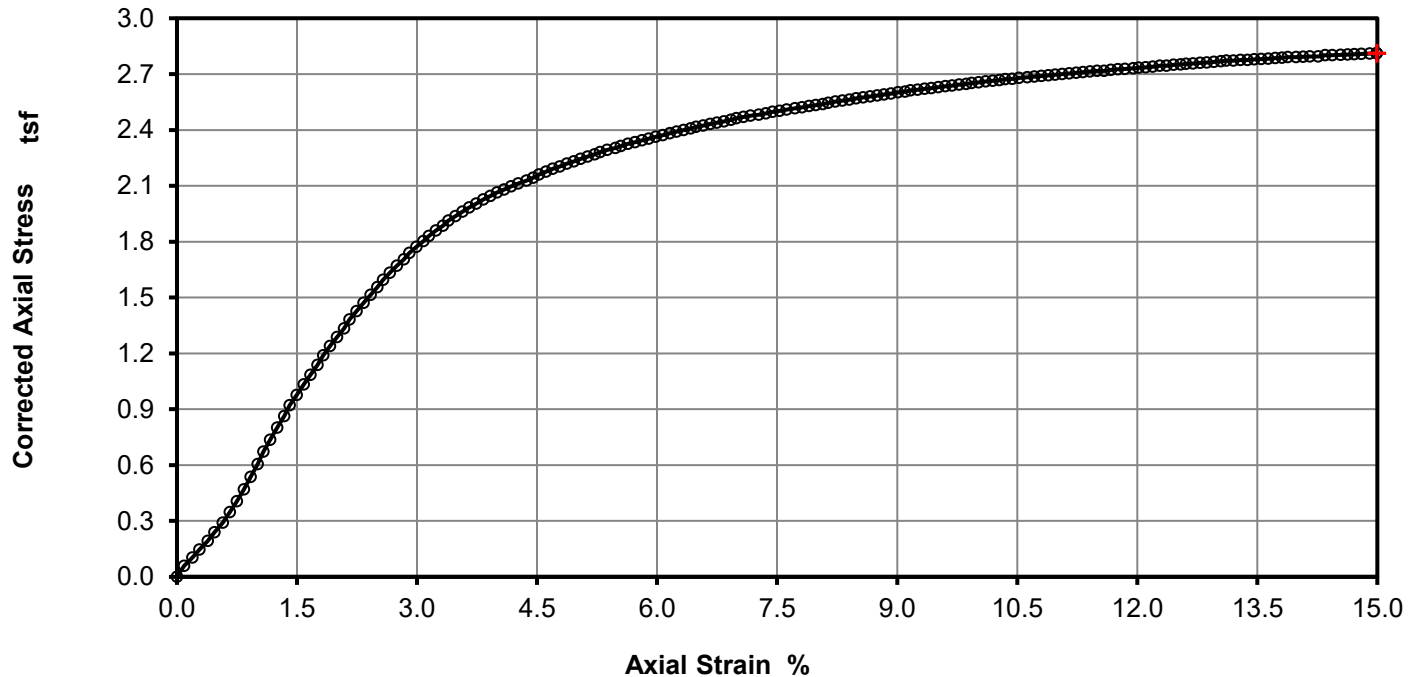
ASTM D2166

Initial Conditions

Height	in	5.75
Diameter	in	2.85
Bulk Density	pcf	129.51
Water Content	%	21.6
Dry Density	pcf	106.54
Voids Ratio		0.552
(Specific gravity assumed 2.65)		
Degree of Saturation	%	103
Rate of Strain applied	%/min	1.04
At failure	%	15.0
	tsf	2.81
	tsf	1.4



Stress vs Axial Strain



Project: Boyet Junior High School Renovations & Additions
Client: St. Tammany Parish Public School System
Sample Source: B-01
Sample Description:

Project No.: 65:2073
Depth (ft): 8 - 10
Sample No.: S-5
Date Reported: 4/27/2026



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Rouge

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(225)224-2583
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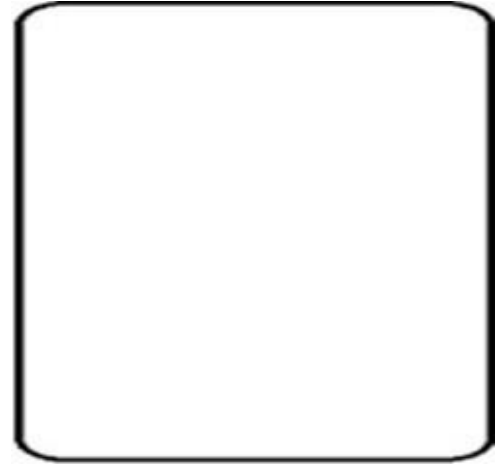
Tested by	Checked by	Approved by	Date Received	Remarks
JMercante	NBurke	NBurke		

Unconfined Compressive Strength Test Report

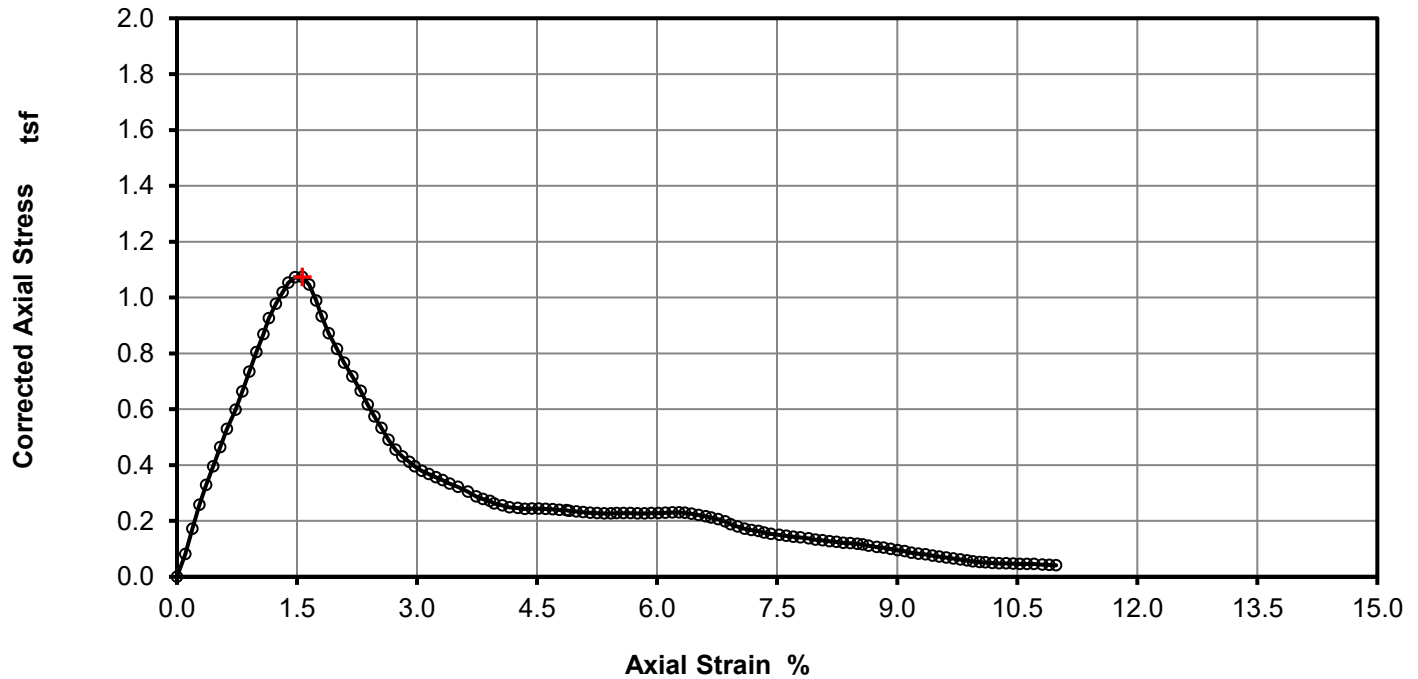
ASTM D2166

Initial Conditions

Height	in	5.75
Diameter	in	2.87
Bulk Density	pcf	110.04
Water Content	%	45.5
Dry Density	pcf	75.60
Voids Ratio		1.187
(Specific gravity assumed 2.65)		
Degree of Saturation	%	102
Rate of Strain applied	%/min	1.04
At failure	%	1.6
	tsf	1.07
	tsf	0.5



Stress vs Axial Strain



Project: Boyet Junior High School Renovations & Additions
Client: St. Tammany Parish Public School System
Sample Source: B-01
Sample Description:

Project No.: 65:2073
Depth (ft): 18 - 20
Sample No.: S-7
Date Reported: 4/27/2026



Office / Lab	Address	Office Number / Fax
ECS Southeast LLC - Baton Rouge	11211 Industriplex Blvd Suite 300 Baton Rouge, LA 70809	(225)224-2583 (225)612-7062

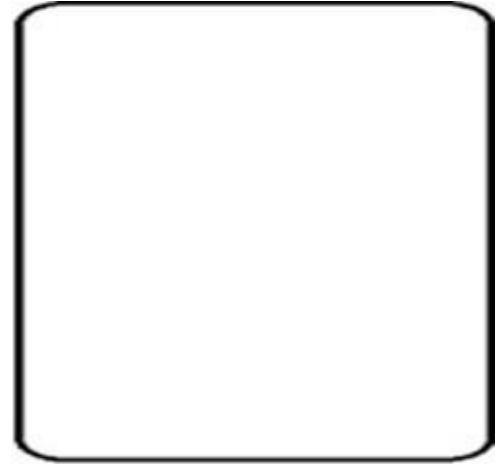
Tested by	Checked by	Approved by	Date Received	Remarks
JMercante	NBurke	NBurke		

Unconfined Compressive Strength Test Report

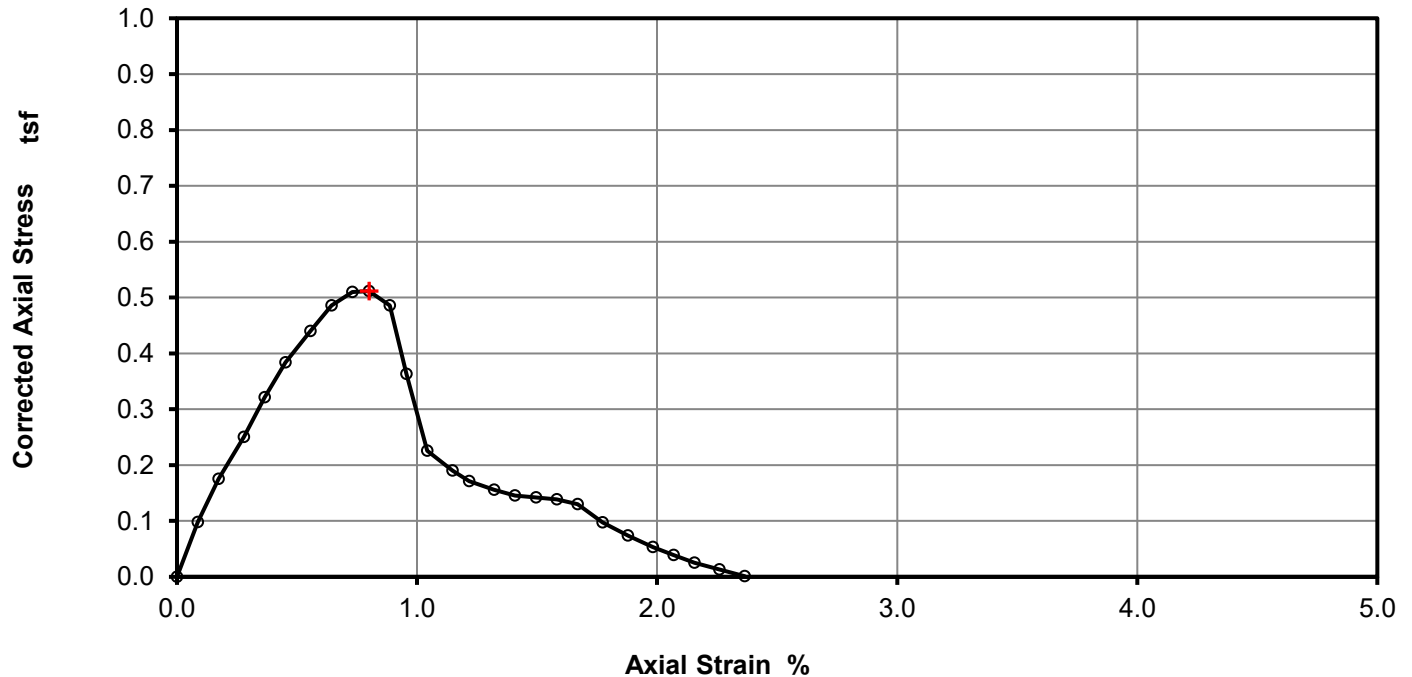
ASTM D2166

Initial Conditions

Height	in	5.75
Diameter	in	2.87
Bulk Density	pcf	103.60
Water Content	%	52.1
Dry Density	pcf	68.10
Voids Ratio		1.428
(Specific gravity assumed 2.65)		
Degree of Saturation	%	97
Rate of Strain applied	%/min	1.04
At failure	%	0.8
	tsf	0.51
	tsf	0.3



Stress vs Axial Strain



Project: Boyet Junior High School Renovations & Additions

Client: St. Tammany Parish Public School System

Sample Source: B-01

Sample Description:

Project No.: 65:2073

Depth (ft): 28 - 30

Sample No.: S-9

Date Reported: 4/27/2026



Office / Lab

Address

Office Number / Fax

ECS Southeast LLC - Baton
Rouge

11211 Industriplex Blvd
Suite 300
Baton Rouge, LA 70809

(225)224-2583

(225)612-7062

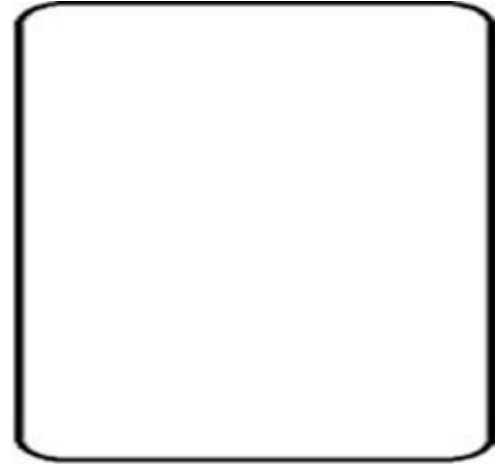
Tested by	Checked by	Approved by	Date Received	Remarks
JMercante	NBurke	NBurke		

Unconfined Compressive Strength Test Report

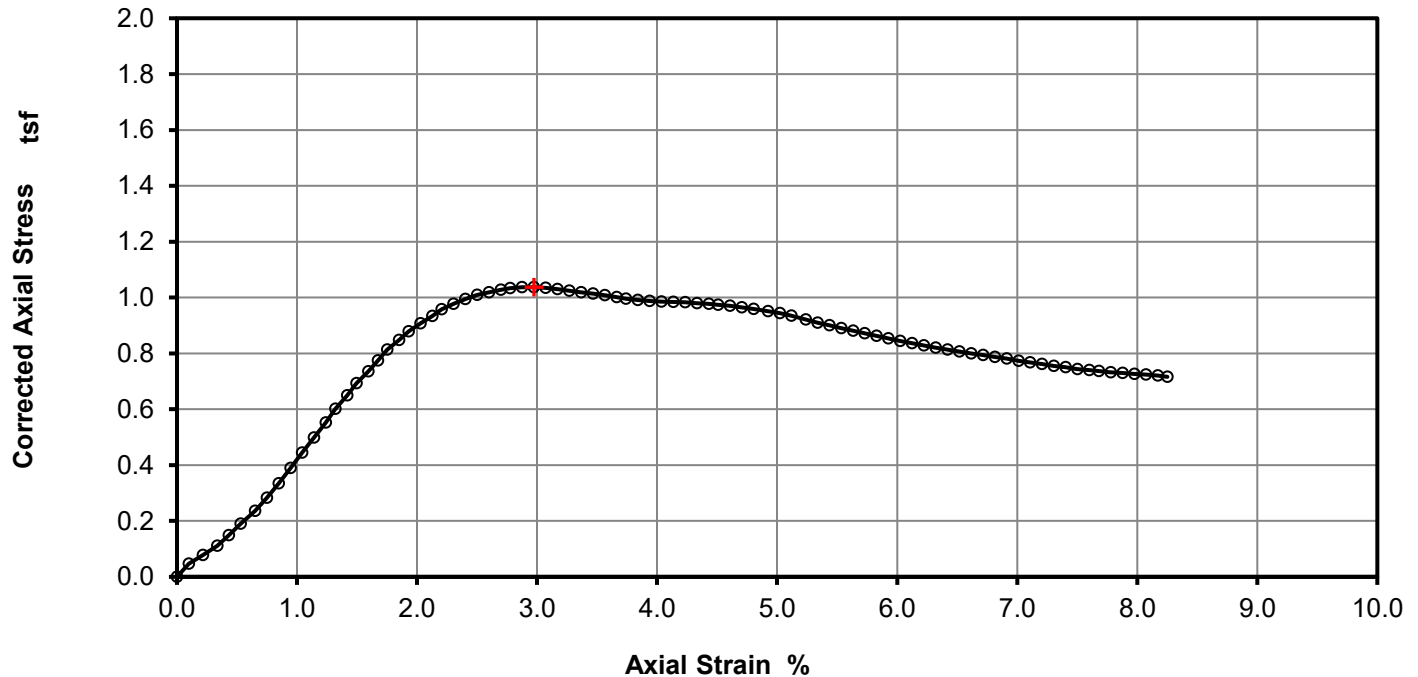
ASTM D2166

Initial Conditions

Height	in	5.08
Diameter	in	2.87
Bulk Density	pcf	111.84
Water Content	%	38.7
Dry Density	pcf	80.64
Voids Ratio		1.051
(Specific gravity assumed 2.65)		
Degree of Saturation	%	98
Rate of Strain applied	%/min	1.18
At failure	%	3.0
	tsf	1.04
	tsf	0.5



Stress vs Axial Strain



Project: Boyet Junior High School Renovations & Additions
Client: St. Tammany Parish Public School System
Sample Source: B-01
Sample Description:

Project No.: 65:2073
Depth (ft): 38 - 40
Sample No.: S-11
Date Reported: 4/27/2026



Office / Lab	Address	Office Number / Fax
ECS Southeast LLC - Baton Rouge	11211 Industriplex Blvd Suite 300 Baton Rouge, LA 70809	(225)224-2583 (225)612-7062

Tested by	Checked by	Approved by	Date Received	Remarks
JMercante	NBurke	NBurke		

Important Information about This Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative – interpret and apply this geotechnical-engineering report as effectively as possible. In that way, clients can benefit from a lowered exposure to the subsurface problems that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed below, contact your GBA-member geotechnical engineer. Active involvement in the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Geotechnical-Engineering Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a given civil engineer will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. *Those who rely on a geotechnical-engineering report prepared for a different client can be seriously misled.* No one except authorized client representatives should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one – not even you – should apply this report for any purpose or project except the one originally contemplated.*

Read this Report in Full

Costly problems have occurred because those relying on a geotechnical-engineering report did not read it *in its entirety*. Do not rely on an executive summary. Do not read selected elements only. *Read this report in full.*

You Need to Inform Your Geotechnical Engineer about Change

Your geotechnical engineer considered unique, project-specific factors when designing the study behind this report and developing the confirmation-dependent recommendations the report conveys. A few typical factors include:

- the client's goals, objectives, budget, schedule, and risk-management preferences;
- the general nature of the structure involved, its size, configuration, and performance criteria;
- the structure's location and orientation on the site; and
- other planned or existing site improvements, such as retaining walls, access roads, parking lots, and underground utilities.

Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.*

This Report May Not Be Reliable

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, that it could be unwise to rely on a geotechnical-engineering report whose reliability may have been affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If your geotechnical engineer has not indicated an "apply-by" date on the report, ask what it should be, and, in general, if you are the least bit uncertain about the continued reliability of this report, contact your geotechnical engineer before applying it.* A minor amount of additional testing or analysis – if any is required at all – could prevent major problems.

Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface through various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing were performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgment to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team from project start to project finish, so the individual can provide informed guidance quickly, whenever needed.

This Report's Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, *they are not final*, because the geotechnical engineer who developed them relied heavily on judgment and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* revealed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.*

This Report Could Be Misinterpreted

Other design professionals' misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a full-time member of the design team, to:

- confer with other design-team members,
- help develop specifications,
- review pertinent elements of other design professionals' plans and specifications, and
- be on hand quickly whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction observation.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note conspicuously that you've included the material for informational purposes only*. To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report, but they may rely on the factual data relative to the specific times, locations, and depths/elevations referenced. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may

perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely*. Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures*. If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. As a general rule, *do not rely on an environmental report prepared for a different client, site, or project, or that is more than six months old*.

Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, none of the engineer's services were designed, conducted, or intended to prevent uncontrolled migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, *proper implementation of the geotechnical engineer's recommendations will not of itself be sufficient to prevent moisture infiltration*. Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. *Geotechnical engineers are not building-envelope or mold specialists*.



Telephone: 301/565-2733

e-mail: info@geoprofessional.org www.geoprofessional.org



CERTIFICATE OF LIABILITY INSURANCE

DATE (MM/DD/YYYY)

10/21/2026

THIS CERTIFICATE IS ISSUED AS A MATTER OF INFORMATION ONLY AND CONFERS NO RIGHTS UPON THE CERTIFICATE HOLDER. THIS CERTIFICATE DOES NOT AFFIRMATIVELY OR NEGATIVELY AMEND, EXTEND OR ALTER THE COVERAGE AFFORDED BY THE POLICIES BELOW. THIS CERTIFICATE OF INSURANCE DOES NOT CONSTITUTE A CONTRACT BETWEEN THE ISSUING INSURER(S), AUTHORIZED REPRESENTATIVE OR PRODUCER, AND THE CERTIFICATE HOLDER.

IMPORTANT: If the certificate holder is an ADDITIONAL INSURED, the policy(ies) must have ADDITIONAL INSURED provisions or be endorsed. If SUBROGATION IS WAIVED, subject to the terms and conditions of the policy, certain policies may require an endorsement. A statement on this certificate does not confer rights to the certificate holder in lieu of such endorsement(s).

PRODUCER Arthur J. Gallagher Risk Management Services, LLC 235 Highlandia Dr Ste 200 Baton Rouge LA 70810		CONTACT NAME: PHONE (A/C, No, Ext): 225-292-3515 FAX (A/C, No): 225-292-3893 E-MAIL ADDRESS:	
		INSURER(S) AFFORDING COVERAGE	NAIC #
		INSURER A : Underwriters at Lloyd's London	15792
		INSURER B : Safety National Casualty Corporation	15105
		INSURER C :	
		INSURER D :	
		INSURER E :	
		INSURER F :	

COVERAGES**CERTIFICATE NUMBER:** 948533349**REVISION NUMBER:**

THIS IS TO CERTIFY THAT THE POLICIES OF INSURANCE LISTED BELOW HAVE BEEN ISSUED TO THE INSURED NAMED ABOVE FOR THE POLICY PERIOD INDICATED. NOTWITHSTANDING ANY REQUIREMENT, TERM OR CONDITION OF ANY CONTRACT OR OTHER DOCUMENT WITH RESPECT TO WHICH THIS CERTIFICATE MAY BE ISSUED OR MAY PERTAIN, THE INSURANCE AFFORDED BY THE POLICIES DESCRIBED HEREIN IS SUBJECT TO ALL THE TERMS, EXCLUSIONS AND CONDITIONS OF SUCH POLICIES. LIMITS SHOWN MAY HAVE BEEN REDUCED BY PAID CLAIMS.

INSR LTR	TYPE OF INSURANCE	ADDL INSD	SUBR WVD	POLICY NUMBER	POLICY EFF (MM/DD/YYYY)	POLICY EXP (MM/DD/YYYY)	LIMITS
A	☒ COMMERCIAL GENERAL LIABILITY ☐ CLAIMS-MADE ☒ OCCUR ☒ Claims Made GEN'L AGGREGATE LIMIT APPLIES PER: ☒ POLICY ☐ PRO-JECT ☐ LOC OTHER:			PK1043426	7/15/2026	7/15/2027	EACH OCCURRENCE \$ 2,000,000 DAMAGE TO RENTED PREMISES (Ea occurrence) \$ 1,000,000 MED EXP (Any one person) \$ PERSONAL & ADV INJURY \$ GENERAL AGGREGATE \$ 3,000,000 PRODUCTS - COMP/OP AGG \$ SIR \$ 500,000
A	☒ AUTOMOBILE LIABILITY ☒ ANY AUTO ☐ OWNED AUTOS ONLY ☐ SCHEDULED AUTOS ☐ HIRED AUTOS ONLY ☐ NON-OWNED AUTOS ONLY			PK1043426	7/15/2026	7/15/2027	COMBINED SINGLE LIMIT (Ea accident) \$ 1,000,000 BODILY INJURY (Per person) \$ BODILY INJURY (Per accident) \$ PROPERTY DAMAGE (Per accident) \$ SIR \$ 500,000
	☐ UMBRELLA LIAB ☐ OCCUR ☐ EXCESS LIAB ☐ CLAIMS-MADE DED ☐ RETENTION \$						EACH OCCURRENCE \$ AGGREGATE \$ \$
B	WORKERS COMPENSATION AND EMPLOYERS' LIABILITY ANY PROPRIETOR/PARTNER/EXECUTIVE OFFICER/MEMBER EXCLUDED? (Mandatory in NH) If yes, describe under DESCRIPTION OF OPERATIONS below	Y / N ☐	N / A	SP 4069431	7/15/2026	7/15/2027	PER STATUTE ☐ OTH-ER ☐ E.L. EACH ACCIDENT \$ 1,000,000 E.L. DISEASE - EA EMPLOYEE \$ 1,000,000 E.L. DISEASE - POLICY LIMIT \$ 1,000,000

DESCRIPTION OF OPERATIONS / LOCATIONS / VEHICLES (ACORD 101, Additional Remarks Schedule, may be attached if more space is required)

CERTIFICATE HOLDER**CANCELLATION**

Master	SHOULD ANY OF THE ABOVE DESCRIBED POLICIES BE CANCELLED BEFORE THE EXPIRATION DATE THEREOF, NOTICE WILL BE DELIVERED IN ACCORDANCE WITH THE POLICY PROVISIONS.
	AUTHORIZED REPRESENTATIVE Arthur J. Gallagher Risk Management Services, LLC

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NEW YORK CONSTRUCTION CERTIFICATE OF LIABILITY INSURANCE ADDENDUM

DATE (MM/DD/YYYY)

10/21/2026

THIS ADDENDUM SUMMARIZES SOME OF THE POLICY PROVISIONS IN THE REFERENCED INSURANCE POLICIES AND IS ISSUED AS A MATTER OF INFORMATION ONLY; IT CONFERS NO RIGHTS UPON THE CERTIFICATE HOLDER. ALL TERMS, EXCLUSIONS AND CONDITIONS IN THE ACTUAL POLICY SHOULD BE CONSULTED FOR A MORE DETAILED ANALYSIS OF COVERAGE, AS THIS ADDENDUM DOES NOT AFFIRMATIVELY OR NEGATIVELY AMEND, EXTEND OR ALTER THE COVERAGE AFFORDED BY THE POLICIES.

AGENCY Arthur J. Gallagher Risk Management Services, LLC		NAMED INSURED(S) St. Tammany Parish School Board	
POLICY NUMBER PK1043426	EFFECTIVE DATE 7/15/2026	CARRIER Underwriters at Lloyd's London	NAIC CODE 15792

ADDENDUM INFORMATION**CERTIFICATE NUMBER:**948533349**REVISION NUMBER:****A. Insurer**

- ☐ Admitted / authorized
- ☐ Excess line or free trade zone

B. General Liability (GL) policy form

- ☐ ISO / ISO modified
- ☐ Other

C. Specific operations excluded or restricted (GL policy)

- ☐ Location: _____
- ☐ Type of construction: _____
- ☐ Building height: _____
- ☐ Classifications [see attached declarations / endorsement]
- ☐ Designated work [see attached endorsement]

D. Additional insured endorsement (GL policy)

- ☐ CG 20 10 ☐ CG 20 26 ☐ CG 20 32 ☐ CG 20 33 ☐ CG 20 37 ☐ CG 20 38
- ☐ Other: #: _____ Title: _____

E. According to the terms of this GL policy, the additional insured has primary and noncontributory coverage

- ☐ Yes ☐ No and ☐ no other option is available with this insurer

F. Additional insured will receive advance notice if insurer cancels (GL policy)

- ☐ Yes ☐ No and ☐ no other option is available with this insurer

G. Blanket contractual liability located in the "insured contract" definition (Section V, Number 9, Item f. in the ISO CGL policy) is removed or restricted

- ☐ Yes and ☐ no other option is available with this insurer ☐ No changes made

H. "Insured contract" exception to the employers liability exclusion is removed or modified (GL policy)

- ☐ Yes and ☐ no other option is available with this insurer ☐ No changes made

I. GL policy (including endorsements) does not cover the additional insured for claims involving injury to employees of the named insured or subcontractors (not workers' compensation)

- ☐ Yes and ☐ no other option is available with this insurer ☐ No changes made

J. Earth movement, excavation or explosion / collapse / underground property damage is excluded or restricted (GL policy)

☐ Yes and ☐ no other option is available with this insurer ☐ No changes made

K. Insured vs. insured suits (cross liability in the ISO CGL policy) are excluded or restricted (other than named insured vs. named insured)

☐ Yes and ☐ no other option is available with this insurer ☐ No changes made

L. Property damage to work performed by subcontractors (exception to the "damage to your work" exclusion in the ISO CGL policy) is excluded or restricted

☐ Yes and ☐ no other option is available with this insurer ☐ No changes made

M. Excess / umbrella policy is primary and non-contributory for additional insureds

☐ Yes, by specific policy provision ☐ Yes, by endorsement ☐ No and ☐ no other option is available with this insurer

Arthur J. Gallagher Risk Management Services, LLC

AUTHORIZED REPRESENTATIVE SIGNATURE

10/21/2026

DATE (MM/DD/YYYY)