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A.B.	ANCHOR BOLT	LVL	LAMINATED VENEER LUMBER
ACI	AMERICAN CONCRETE INSTITUTE	MAX.	MAXIMUM
ADDL.	ADDITIONAL	MBMA	METAL BUILDING MANUFACTURERS ASSOCIATION
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL	MECH.	MECHANICAL
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	MEPF	MECHANICAL, ELECTRICAL, PLUMBING AND FIRE SAFETY
ALT.	ALTERNATE	MFR.	MANUFACTURER
ALUM.	ALUMINUM	MIN.	MINIMUM
ARCH.	ARCHITECT / ARCHITECTURAL	MISC.	MISCELLANEOUS
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS	MPH	MILES PER HOUR
ASD	ALLOWABLE STRENGTH DESIGN LOAD LEVEL	MPP	MASS PLYWOOD PANELS
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	MT	MAGNETIC PARTICLE TESTING
AWS	AMERICAN WELDING SOCIETY	(N)	NEW
BLDG.	BUILDING	N.I.C.	NOT IN CONTRACT
BOT.	BOTTOM	NLT	NAIL LAMINATED TIMBER
BRBF	BUCKLING RESTRAINED BRACED FRAME	NOM.	NOMINAL
		NO.	NUMBER
		N.T.S.	NOT TO SCALE
C.G.	CENTER OF GRAVITY	o.c.	ON CENTER
C.I.P.	CAST IN PLACE	O.D.	OUTSIDE DIAMETER
C.J.	CONTROL JOINT	OPP.	OPPOSITE
C.J.P.	COMPLETE JOINT PENETRATION	OSL	ORIENTED STRAND LUMBER
CL	CENTERLINE	OWJ	OPEN WEB JOIST
CLR.	CLEAR	PAF	POWDER ACTUATED FASTENER
CLT	CROSS LAMINATED TIMBER	PART.	PARTITION
CMU	CONCRETE MASONRY UNIT	P/C	PRECAST
COL.	COLUMN	PCF	POUNDS PER CUBIC FOOT
CONC.	CONCRETE	PERIM.	PERIMETER
CONN.	CONNECTION	PL	PLATE
CONST.	CONSTRUCTION	PP	PARTIAL PENETRATION
CONT.	CONTINUOUS	PSF	POUNDS PER SQUARE FOOT
db	BAR DIAMETER	PSL	PARALLEL STRAND LUMBER
DBA	DEFORMED BAR ANCHOR	PSI	POUNDS PER SQUARE INCH
DET.	DETAIL	P/T	POST-TENSIONED
DIA., Ø	DIAMETER	P.T.	PRESSURE TREATED
DIAG.	DIAGONAL	PVC	POLYVINYL CHLORIDE
D.L.	DEAD LOAD	R. RAD.	RADIUS
DLT	DOWEL LAMINATED TIMBER	RCSC	RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS
DWG.	DRAWING	REF.	REFERENCE
ELEC.	ELECTRICAL	RET.	RETURN
EL.	ELEVATION	REINF.	REINFORCING
EQ.	EQUAL	REQ'D.	REQUIRED
EXIST., (E)	EXISTING	REQ'MTS.	REQUIREMENTS
EXP.	EXPANSION	SCHED.	SCHEDULE
EXT.	EXTERIOR	S.C.	SLIP CRITICAL
FDN.	FOUNDATION	SCL	STRUCTURAL COMPOSITE LUMBER
FIN.	FINISH	SIM.	SIMILAR
FLR.	FLOOR	SLFS	SEISMIC FORCE RESISTING SYSTEM
FRT	FIRE RETARDANT TREATED	S.O.G.	SLAB ON GRADE
FT.	FOOT	SPEC.	SPECIFICATION
FTG.	FOOTING	SQ.	SQUARE
GA.	GAUGE	SS	STAINLESS STEEL
GALV.	GALVANIZED	SSMA	STEEL STUD MANUFACTURERS ASSOCIATION
GL	GLULAM	STD.	STANDARD
HORIZ.	HORIZONTAL	STRUCT.	STRUCTURAL
HSS	HOLLOW STRUCTURAL STEEL	SYM.	SYMMETRICAL
IBC	INTERNATIONAL BUILDING CODE	THRU	THROUGH
I.D.	INSIDE DIAMETER	T&G	TONGUE AND GROOVE
IN.	INCHES	TRANS.	TRANSVERSE
INT.	INTERIOR	TS	LIGHT GAUGE TUBE STEEL
K	KIPS	TYP.	TYPICAL
KSF	KIPS PER SQUARE FOOT	U.N.O.	UNLESS NOTED OTHERWISE
KSI	KIPS PER SQUARE INCH	U.T.	ULTRASONIC TESTING
LBS.	POUNDS	ULT.	ULTIMATE STRENGTH DESIGN LOAD LEVEL
L.L.	LIVE LOAD	VERT.	VERTICAL
LLH	LONG LEG HORIZONTAL	V.I.F.	VERIFY IN FIELD
LLV	LONG LEG VERTICAL	w/	WITH
LOC.	LOCATION	WF	WIDE FLANGE
LONG.	LONGITUDINAL	w/o	WITHOUT
LSL	LAMINATED STRAND LUMBER	W.P.	WORK POINT
LVF	LOW VELOCITY FASTENER	WPS	WELDING PROCEDURE SPECIFICATION
		WWF	WELDED WIRE FABRIC

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DRAWING INDEX AND LIST OF ABBREVIATIONS

Drawn By: KEK	
Designed By: A.J.N	
Project No:	10022500801
Date:	06/12/2026
Scale:	1 : 1

S001

ISSUED FOR BID

GENERAL STRUCTURAL NOTES

GENERAL

THE CONTRACTOR SHALL PROMPTLY REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES, OR OMISSIONS IN THE CONTRACT DOCUMENTS DISCOVERED BY OR MADE KNOWN TO THE CONTRACTOR.

CODE REQUIREMENTS:

CONFORM TO THE 2025 OREGON STRUCTURAL SPECIALTY CODE (OSSC), BASED ON THE 2024 INTERNATIONAL BUILDING CODE (IBC).

TEMPORARY CONDITIONS:

THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES UNTIL COMPLETION.

CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

EXISTING CONDITIONS:

ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE SEOR OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.

ASSUMED FUTURE CONSTRUCTION:

VERTICAL: NONE
HORIZONTAL: NONE

DESIGN CRITERIA

DESIGN WAS BASED ON THE STRENGTH AND DEFLECTION CRITERIA OF THE OSSC. THE FOLLOWING LOADS AND ALLOWABLES WERE USED FOR DESIGN:

GRAVITY SYSTEM CRITERIA		
OCCUPANCY OR USE	UNIFORM LOAD	
EXISTING GLULAM FRAMED ROOF	16 PSF TOTAL SUPERIMPOSED DEAD LOAD	
SNOW CRITERIA		
DESIGN ROOF SNOW LOAD	30 PSF MINIMUM IN ACCORDANCE WITH OSSC	
WINTER WIND PARAMETER		0.35
SNOW DRIFT		NONE
GROUND SNOW LOAD		P _g = 24 PSF
FLAT ROOF SNOW LOAD		P _f = 20 PSF
SNOW EXPOSURE FACTOR		C _e = 1.0
THERMAL FACTOR		C _t = 1.2
ALL SNOW LOADS ARE ULTIMATE LEVEL U.N.O.		

LONG-SPAN GLULAM BEAM REPAIR AND STRENGTHENING

DAMAGED GLULAM BEAMS WILL BE REPAIRED AND STRENGTHENED TO SUPPORT GRAVITY LOADS PRESCRIBED BY THE REFERENCED CODE FOR NEW CONSTRUCTION. SEE PLAN AND ELEVATIONS FOR WORK THAT IS REQUIRED TO BE COMPLETED.

UNDAMAGED GLULAM BEAMS MAY ALSO BE STRENGTHENED TO SUPPORT GRAVITY LOADS PRESCRIBED BY THE REFERENCED CODE FOR NEW CONSTRUCTION. STRENGTHENING OF UNDAMAGED BEAMS MAY BE COMPLETED ON A VOLUNTARY BASIS.

THE SCOPE OF WORK FOR REPAIR AND STRENGTHENING IS BASED ON THE STRUCTURAL ASSESSMENT REPORT "MEDFORD SCHOOL DISTRICT EXISTING BUILDING ASSESSMENT - LONG SPAN TIMBER ROOFS" BY KPFF DATED MARCH 9, 2026.

OTHER EXISTING STRUCTURAL FRAMING, INCLUDING ROOF SHEATHING AND JOISTS, COLUMNS, WALLS AND PILASTERS, AND FOUNDATIONS HAVE NOT BEEN EVALUATED.

STRUCTURAL OBSERVATIONS

THE STRUCTURAL ENGINEER OF RECORD (SEOR) WILL PERFORM STRUCTURAL OBSERVATIONS BASED ON THE REQUIREMENTS OF THE OSSC AT THE STAGES OF CONSTRUCTION LISTED BELOW. CONTRACTOR SHALL PROVIDE SUFFICIENT ADVANCED NOTICE AND ACCESS FOR THE SEOR TO PERFORM THESE OBSERVATIONS.

ITEM	COMMENTS
AS NEEDED TO ADDRESS STRUCTURAL ISSUES	

STRUCTURAL OBSERVATION IS FOR THE GENERAL CONFORMANCE OF THE STRUCTURAL DRAWINGS AND DOES NOT ALLEVIATE ANY SPECIAL INSPECTION REQUIREMENTS.

SUBMITTALS

SUBMIT SHOP DRAWINGS AND OTHER SUBMITTALS TO THE ENGINEER PRIOR TO FABRICATION AND CONSTRUCTION OF STRUCTURAL ITEMS. IF THE SUBMITTALS DIFFER FROM OR ADD TO THE STRUCTURAL CONTRACT DOCUMENTS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF OREGON. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ARE SUBJECT TO REVIEW AND ACCEPTANCE BY THE SEOR.

FIELD ENGINEERED DETAILS DEVELOPED BY THE CONTRACTOR THAT DIFFER FROM OR ADD TO THE STRUCTURAL DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OREGON AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO CONSTRUCTION.

THE USE OF REPRODUCTIONS OR PHOTOCOPIES OF THE CONTRACT DRAWINGS SHALL NOT BE PERMITTED. WHEN CAD OR REVIT FILES ARE PROVIDED TO THE CONTRACTOR OR SUBCONTRACTORS, IT IS THE RESPONSIBILITY OF THE CONTRACTOR/SUBCONTRACTOR TO REMOVE ALL INFORMATION NOT DIRECTLY RELEVANT TO THE SCOPE OF THE SUBMITTAL AS WELL AS ALL REFERENCES TO OUTSIDE SOURCE FILES.

SUBMITTALS AND DELEGATED DESIGN SUBMITTALS SHALL INCLUDE THE FOLLOWING:

ITEM	SUBMITTAL	DELEGATED DESIGN SUBMITTAL	COMMENTS
STRUCTURAL STEEL	X		
STEEL WELDING PROCEDURES	X		
POST-TENSIONED REINFORCEMENT AND ANCHORAGE	X		

POST-TENSIONING SYSTEMS

POST-TENSIONING SHALL BE INSTALLED TO PROVIDE THE REQUIRED FINAL PRESTRESS FORCE (AFTER ALL LOSSES) AT THE LOCATION SHOWN ON THE PLANS OR DETAILS FOR EACH MEMBER. POST-TENSIONING REINFORCEMENT SHALL CONFORM TO ASTM A416 FOR UNCOATED SEVEN-WIRE, STRESS-RELIEVED, LOW RELAXATION, GRADE 270 STRANDS. POST-TENSIONING REINFORCEMENT SHALL BE 5/8" DIAMETER TENDONS, UNLESS NOTED OTHERWISE.

RECORDS OF TENDON ELONGATIONS SHALL BE SUBMITTED TO AND REVIEWED BY THE STRUCTURAL ENGINEER PRIOR TO CUTTING THE TENDON ENDS.

ANCHORAGES, COUPLERS, AND OTHER FITTINGS SHALL BE THE ICC APPROVED MANUFACTURER'S STANDARD DESIGN FOR THE LOADS APPLIED.

STRUCTURAL STEEL

STRUCTURAL STEEL SHALL BE OF THE MATERIAL AND TYPE LISTED BELOW, U.N.O.:

STRUCTURAL STEEL	
SHAPE	MATERIAL GRADE
CHANNELS, PLATES AND ANGLES, U.N.O.	ASTM A36
HOLLOW STRUCTURAL SECTIONS (ROUND)	ASTM A500, GRADE C (F _y =48KSI)

DESIGN, DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE AISC 360, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" WITH "COMMENTARY" AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", WITH THE FOLLOWING CLARIFICATIONS AND ADDITIONS:

- ADD THE FOLLOWING PARAGRAPH TO SECTION 7.10.3:
"THE ERECTOR SHALL HAVE THE SOLE RESPONSIBILITY FOR DETERMINING THE MEANS AND METHODS USED TO PROPERLY AND ADEQUATELY BRACE THE FRAMING DURING ERECTION."

WELDING SHALL CONFORM TO THE AWS CODES FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS D1.1 AND APPROVED BY THE STRUCTURAL ENGINEER. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER-METAL MANUFACTURER.

WELDS SHALL BE MADE USING E70XX ELECTRODES AND SHALL BE 3/16" MINIMUM, UNLESS OTHERWISE NOTED. WELDING SHALL BE BY AWS CERTIFIED WELDERS.

POST-TENSION COMPONENT
INSTALLATION AND TENSIONING PROCEDURE

- ALL BEAM BRIDGING SHALL BE INSTALLED PRIOR TO PRESTRESSING, WHERE SCREWS AND EPOXY ARE REQUIRED FOR REPAIR OF CHECKS OR DELAMINATIONS. CONSTRUCTION OF THOSE REPAIRS SHALL BE COMPLETED AND CURED PRIOR TO PRESTRESSING.
- PREPARATION: THE LOCATION OF THE CABLE ANCHORAGE, CABLE GUIDES AND OTHER DEVICES AS INDICATED ON THE PLANS SHALL BE CHECKED FOR POSSIBLE OBSTRUCTIONS TO THE PERFORMANCE OF THE WORK. LOCALIZED SHIFTING OF INDIVIDUAL ELEMENTS AND THE USE OF CABLE DIVERTERS TO SHIFT THE ALIGNMENT OF THE CABLE AROUND IMMOVABLE OBJECTS MAY BE REQUIRED TO FACILITATE THE REINFORCING WORK. SUCH OBSTRUCTIONS SHALL BE REPORTED TO THE SEOR FOR THEIR RESOLUTION PRIOR TO THE START OF THE WORK.
- BOLTED CABLE ANCHORS WITH OR WITHOUT SHEAR PLATES: THE DRILL GUIDE AND DRILLING APPARATUS SHALL BE POSITIONED SO THAT THE BORE OF THE HOLE IS PARALLEL TO THE SIDE OF THE MEMBER FOR THE FULL DEPTH OF THE BORE. TEMPLATES MATCHING THE BASE PLATE FOR THE CONNECTION SHALL BE USED FOR POSITIONING AND ALIGNMENT OF THE DRILLED HOLES. THE EDGES OF THE TEMPLATE SHALL BE TRACED ON THE WOOD SURFACE TO FACILITATE THE PLACING OF THE FINAL STEEL CONNECTION.
- CABLE GUIDES: ATTACH TO THE MEMBER AT THE LOCATION OR LOCATIONS SHOWN IN THE PLANS WITH FASTENERS AS CALLED OUT ON PLANS.
- CABLES: THE BUTTON ANCHORAGE (ANCHOR CASTING AND STRAND GRIPPER) SHALL BE INSTALLED ON ONE END OF EACH CABLE BY THE SUPPLIER PRIOR TO DELIVERY TO THE SITE. THE CABLES SHALL BE PLACED AS INDICATED ON THE PLANS WITH THE ENDS OF THE CABLES WITH THE BOTTOM ANCHORAGES ALL AT THE SAME END OF THE ZONE OR ZONES OF THE MEMBER TO BE PRE-STRESSED.
- PRE-STRESSING:
 - THE ELEVATION OF THE MEMBER TO BE PRE-STRESSED AT HALF-WAY BETWEEN THE OUTER PRE-STRESSING ANCHORAGES SHALL BE RECORDED FROM THE TOP OF THE FINISH FLOOR TO THE BOTTOM OF THE MEMBER PRIOR TO PRE-STRESSING OF THE MEMBER AT APPROXIMATELY 50% AND 75% TARGET TENSION, AND AFTER COMPLETION OF THE PRE-STRESSING OF THE MEMBER.
 - THE INITIAL POST-TENSIONING FORCE IN EACH CABLE WILL BE 2,000 POUNDS ON ALL CABLES POST-TENSIONED TO A MAXIMUM LOAD OF 8,000 POUNDS OF LESS AND WILL BE 4,000 POUNDS FOR ALL CABLES POST-TENSIONED TO A MAXIMUM LOAD OF OVER 8,000 POUNDS. AT THE INITIAL LOAD, A DISTANCE FROM THE INSIDE FACE OF THE CABLE ANCHORAGE OUT 6 OR 12 INCHES SHALL BE MARKED ON THE CABLE. WHEN A GIVEN CABLE IS POST-TENSIONED TO THE SPECIFIC LOAD INDICATED IN THE PLANS, THE DISTANCE FROM THE INSIDE FACE OF THE ANCHORAGE TO THE MARK PLACED ON THE CABLE AS DESCRIBED ABOVE, SHALL BE RECORDED TO THE NEAREST 1/16 INCH.
 - WHERE CABLES ARE USED ON EACH SIDE OF A MEMBER, THE DIFFERENCE BETWEEN THE LOAD IN THE CABLES SHALL NOT EXCEED 2,000 POUNDS DURING THE POST-TENSIONING OF THE TWO CABLES TO AVOID LATERAL BOWING OF THE MEMBER.
- PRE-STRESSING EQUIPMENT: PRIOR TO THE START OF THE PRE-STRESSING OF ANY OF THE CABLES, THE GAUGE OR GAUGES TO BE USED TO MEASURE THE ACTUAL LOAD IN EACH CABLE SHALL BE CALIBRATED BY AN APPROVED INDEPENDENT TESTING AGENCY AND THE RESULTING LOAD CURVES SHALL BE FURNISHED TO THE REGISTERED DEPUTY INSPECTOR OR OTHER QUALIFIED PERSON DESIGNATED BY THE SEOR FOR THE CONTINUOUS INSPECTION ON THE ACTUAL POST-TENSIONING OF THE CABLES.

POST-TENSIONING PARAMETERS		
BEAM	TARGET PRE-STRESS TENSION	ANTICIPATED UPWARD DEFLECTION
9 x 40.625	65,000 LBS	1 3/4"
9 x 37.375	52,000 LBS	1 1/2"
TAPERED	64,000 LBS	2 5/16"

- TENSION IS THE TOTAL ON EACH BEAM. TENSION IN EACH CABLE SHALL BE EQUAL.
- UPWARD DEFLECTION IS THE ANTICIPATED LIFTING OF THE BEAM FROM ITS INITIAL STATE DUE TO THE TARGET PRESTRESS TENSION. HALT TENSIONING AND NOTIFY SEOR IF UPWARD DEFLECTION RESULTS IN NET UPWARD CAMBER IN THE BEAM GREATER THAN 1/2".

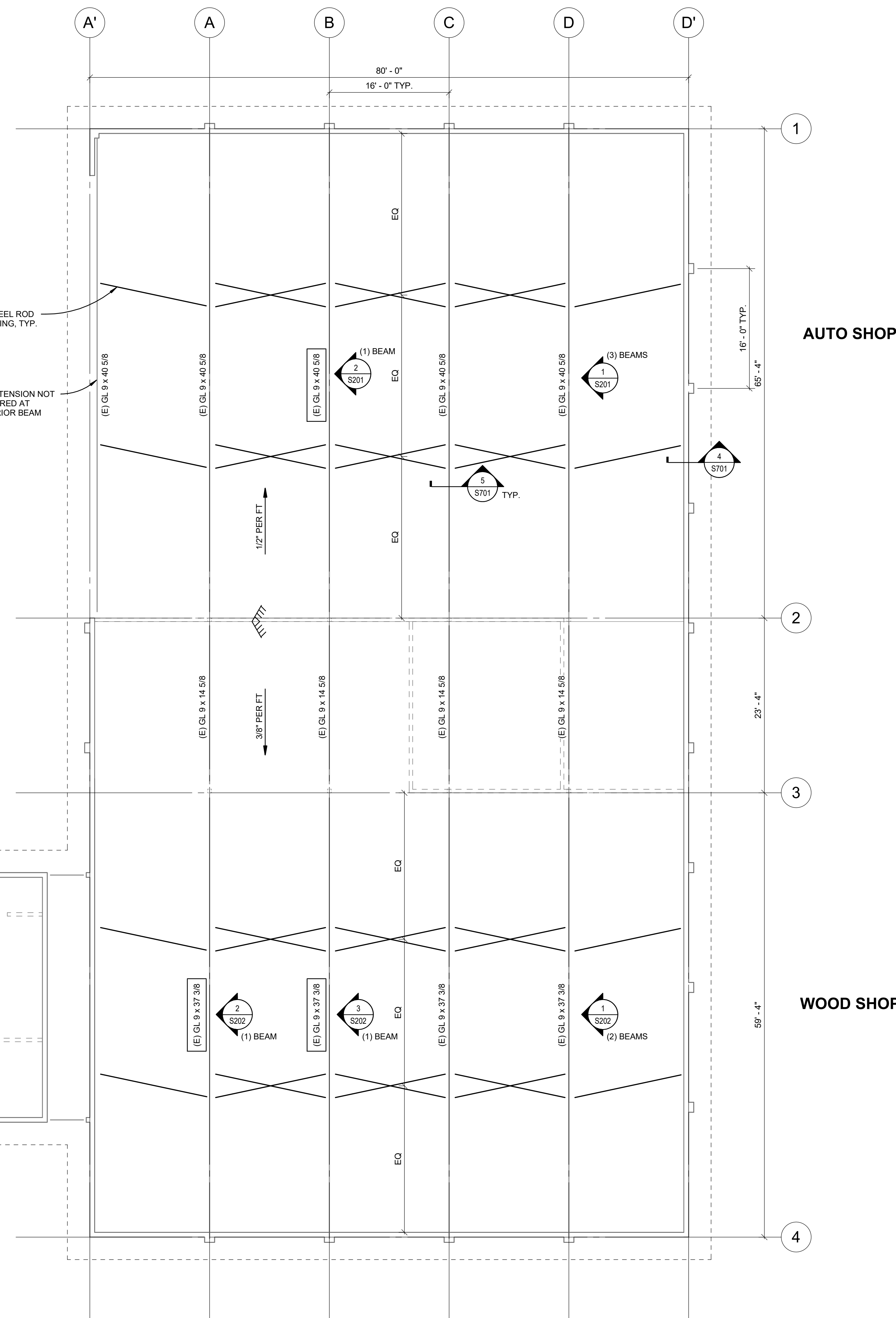
Revisions:		
Rev.	Description	Date

ROOF PLAN

Drawn By: KEK
Designed By: AJN
Project No: 10022500801
Date: 06/12/2026
Scale: As Indicated
Drawing No:

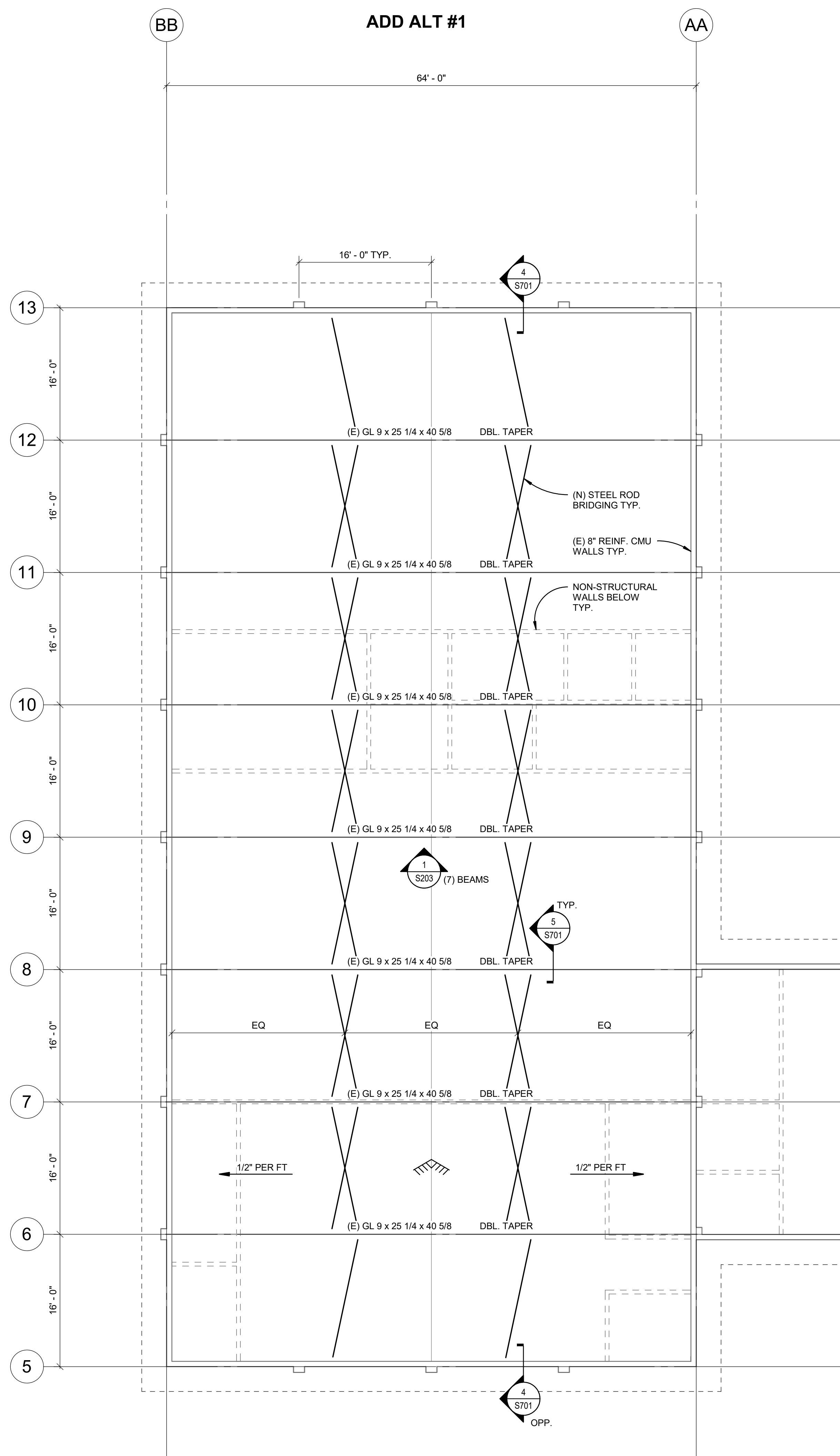
S101

ISSUED FOR BID



AUTO SHOP

WOOD SHOP



ADD ALT #1

1 ROOF FRAMING PLAN
1/8" = 1'-0"

NOTES:

1. GL XXXX INDICATES DAMAGED GLULAM BEAM. REPAIR AND STRENGTHENING OF THESE BEAMS ARE REQUIRED. REFERENCE GENERAL STRUCTURAL NOTES AND ELEVATIONS.
2. GLULAM BEAMS OTHER THAN THOSE INDICATED IN NOTE 1 MAY BE STRENGTHENED ON A VOLUNTARY BASIS. REFERENCE GENERAL STRUCTURAL NOTES AND ELEVATIONS.

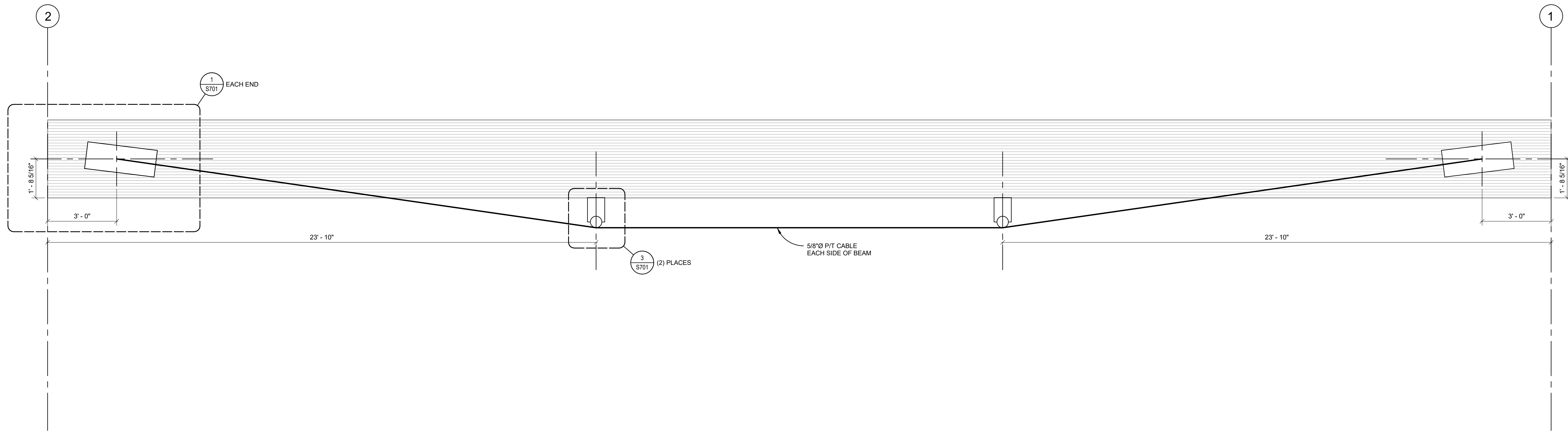
Revisions:		
Rev.	Description	Date

Drawn By:	KEK
Designed By:	AJN
Project No:	10022500801
Date:	06/12/2026
Scale:	1/2" = 1'-0"
Drawing No:	

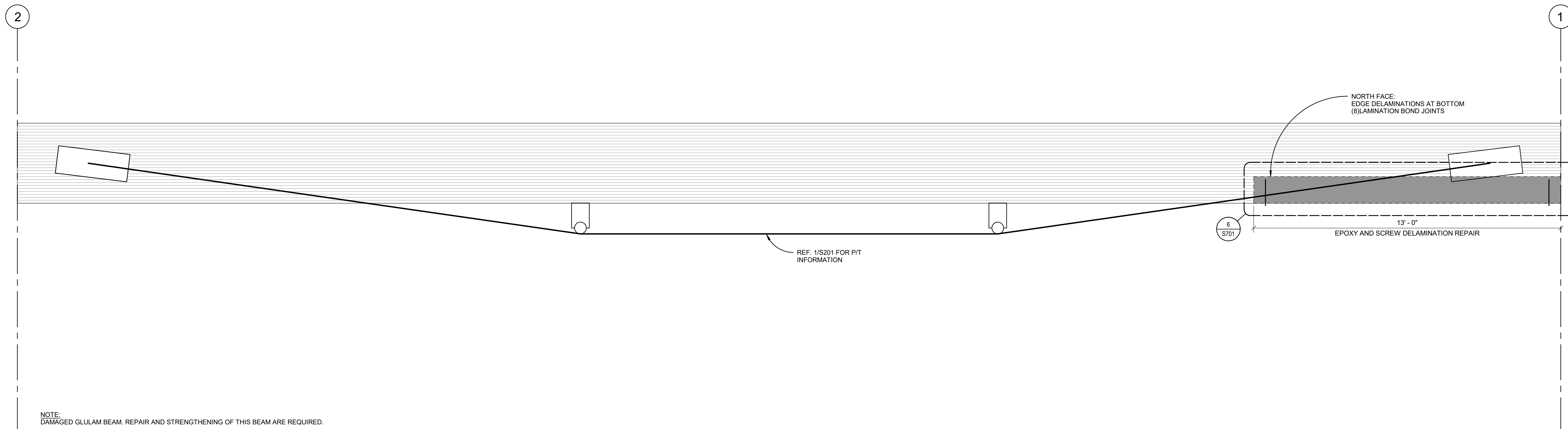
ELEVATIONS

S201

ISSUED FOR BID



1 BEAM ELEVATION - 40 5/8" DEEP - SOUTH FACE - TYP U.N.O.
1/2" = 1'-0"



2 BEAM ELEVATION - 40 5/8" DEEP - SOUTH FACE - GRID B
1/2" = 1'-0"

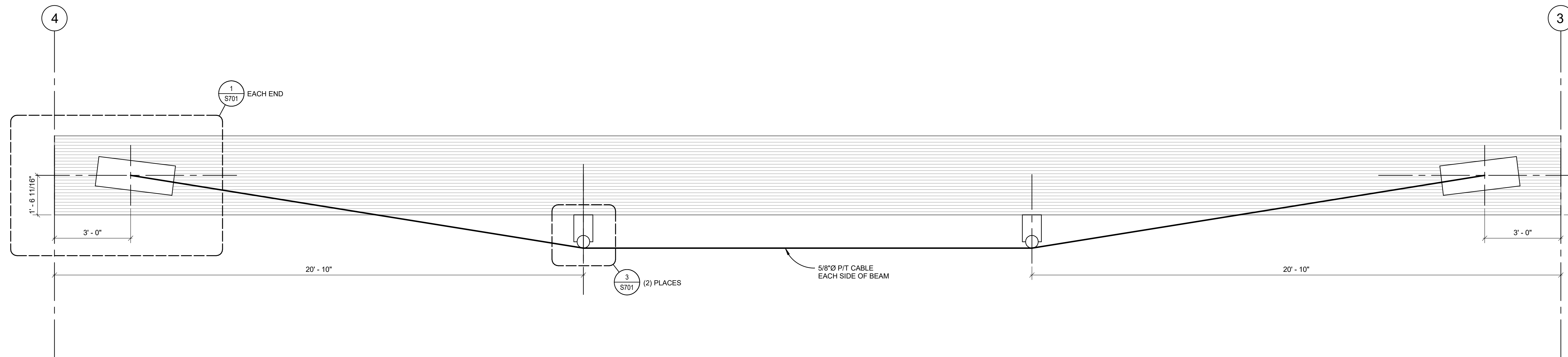
Revisions:		
Rev.	Description	Date

ELEVATIONS

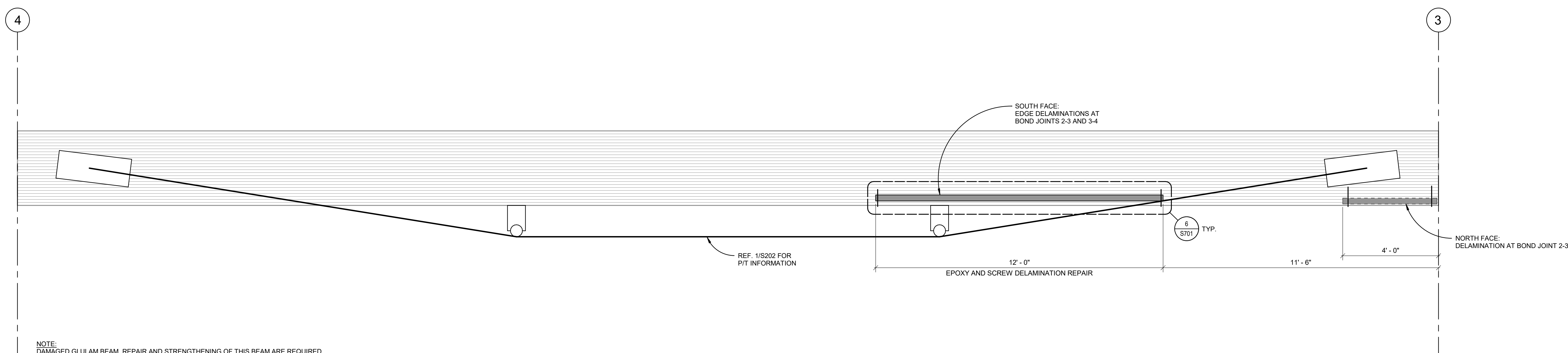
Drawn By: KEK
Designed By: AJN
Project No: 10022500801
Date: 06/12/2026
Scale: 1/2" = 1'-0"
Drawing No:

S202

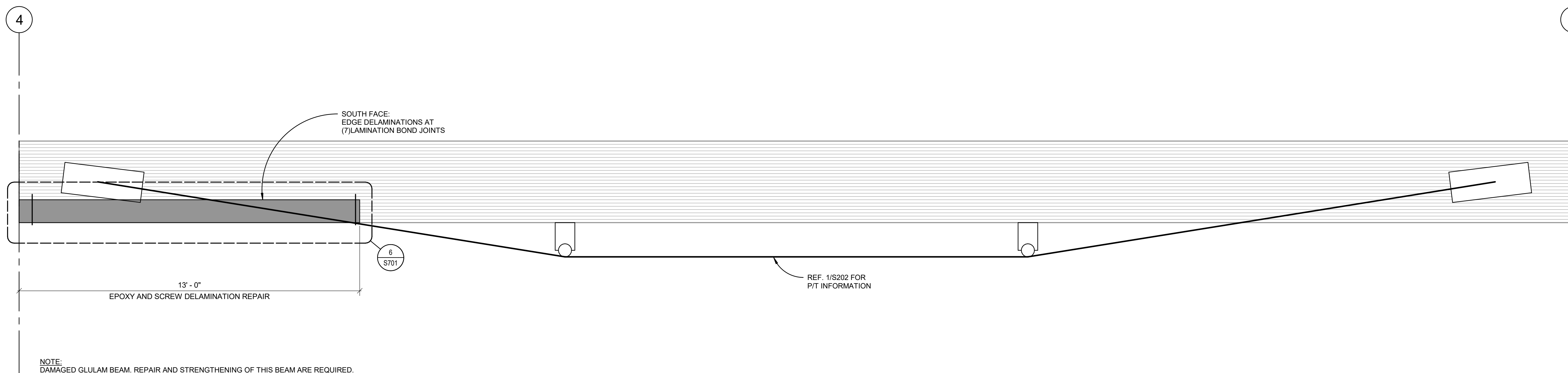
ISSUED FOR BID



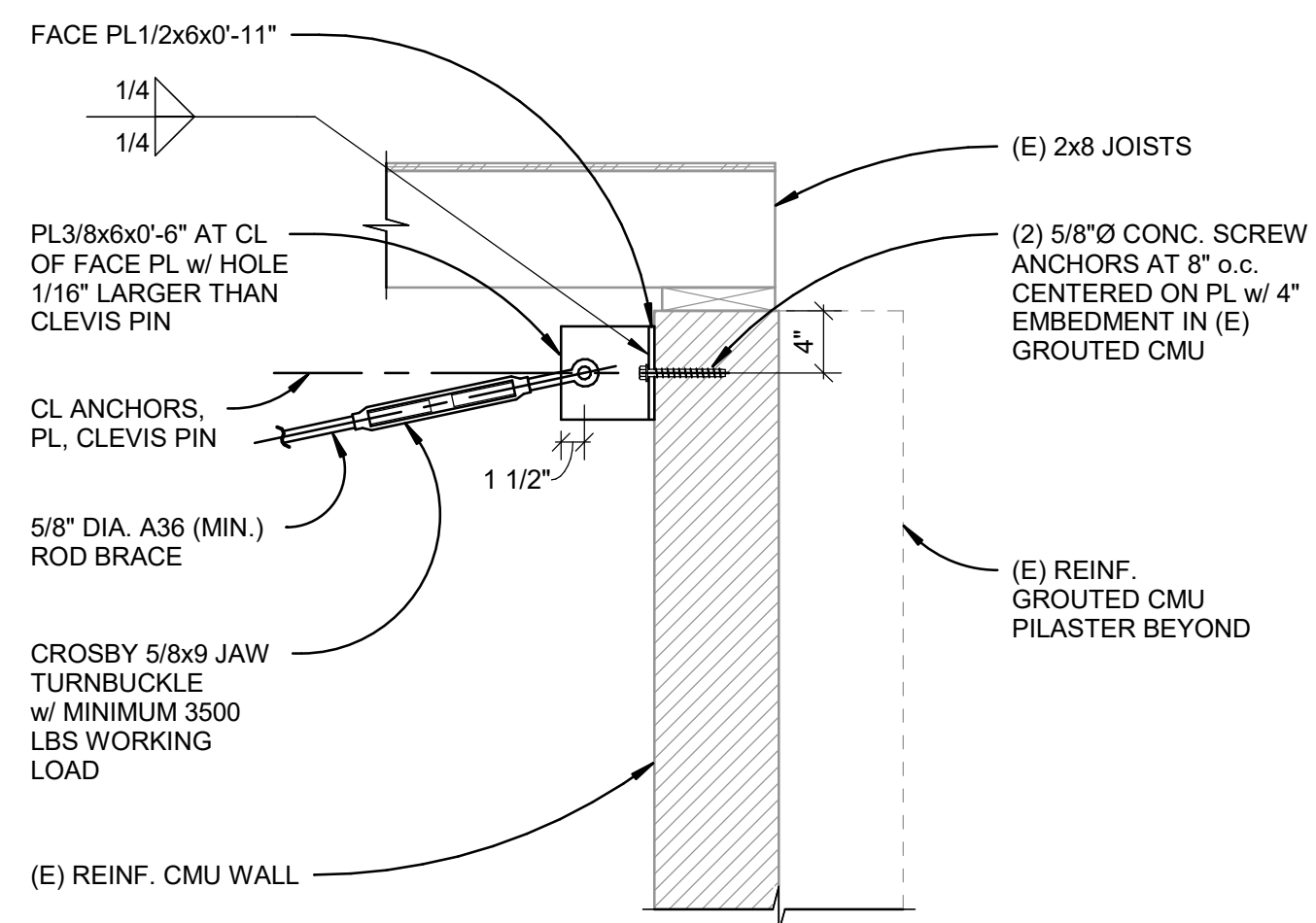
1 BEAM ELEVATION - 37 3/8" DEEP - SOUTH FACE - TYP U.N.O.
1/2" = 1'-0"



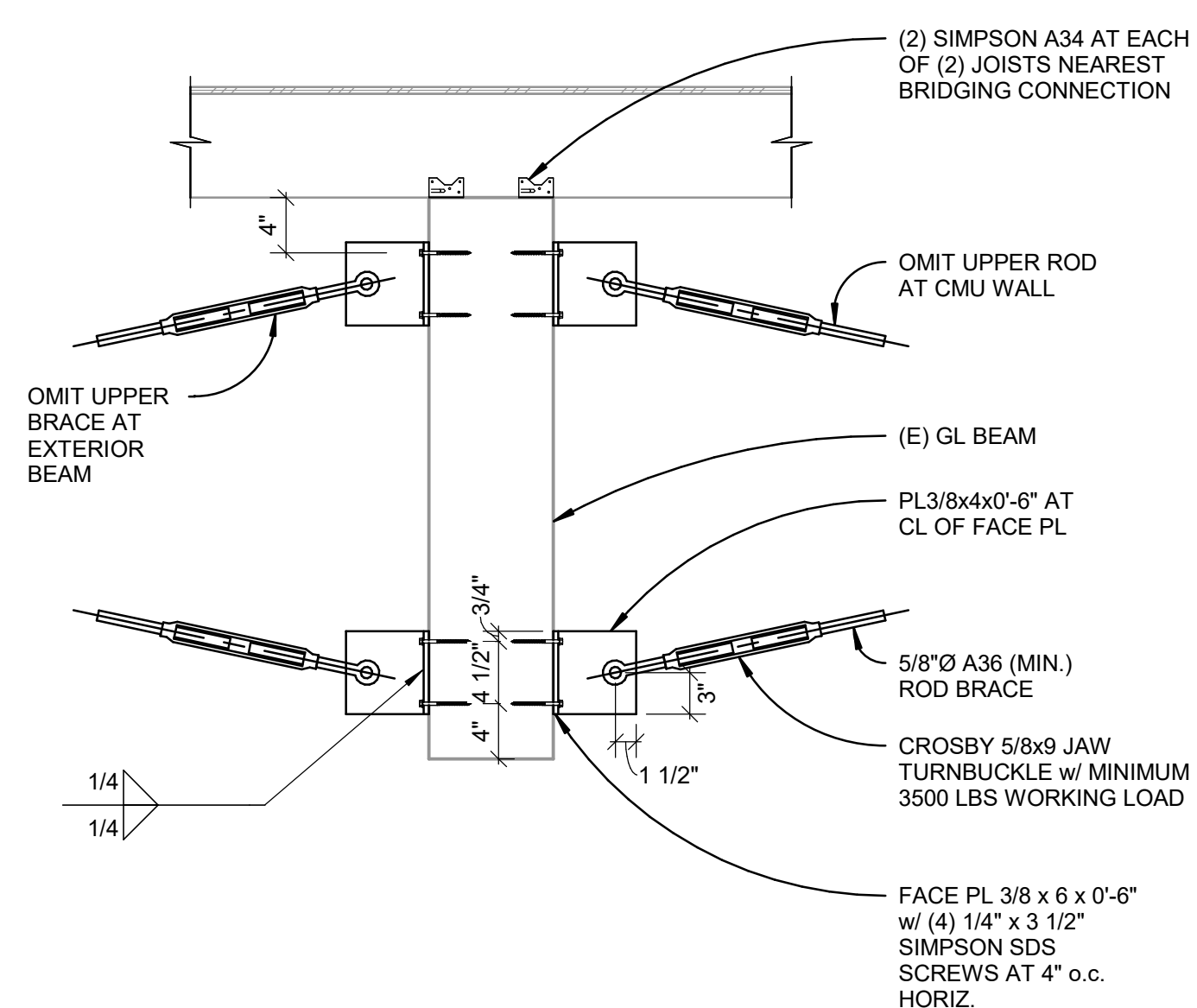
2 BEAM ELEVATION - 37 3/8" DEEP - SOUTH FACE - GRID A
1/2" = 1'-0"



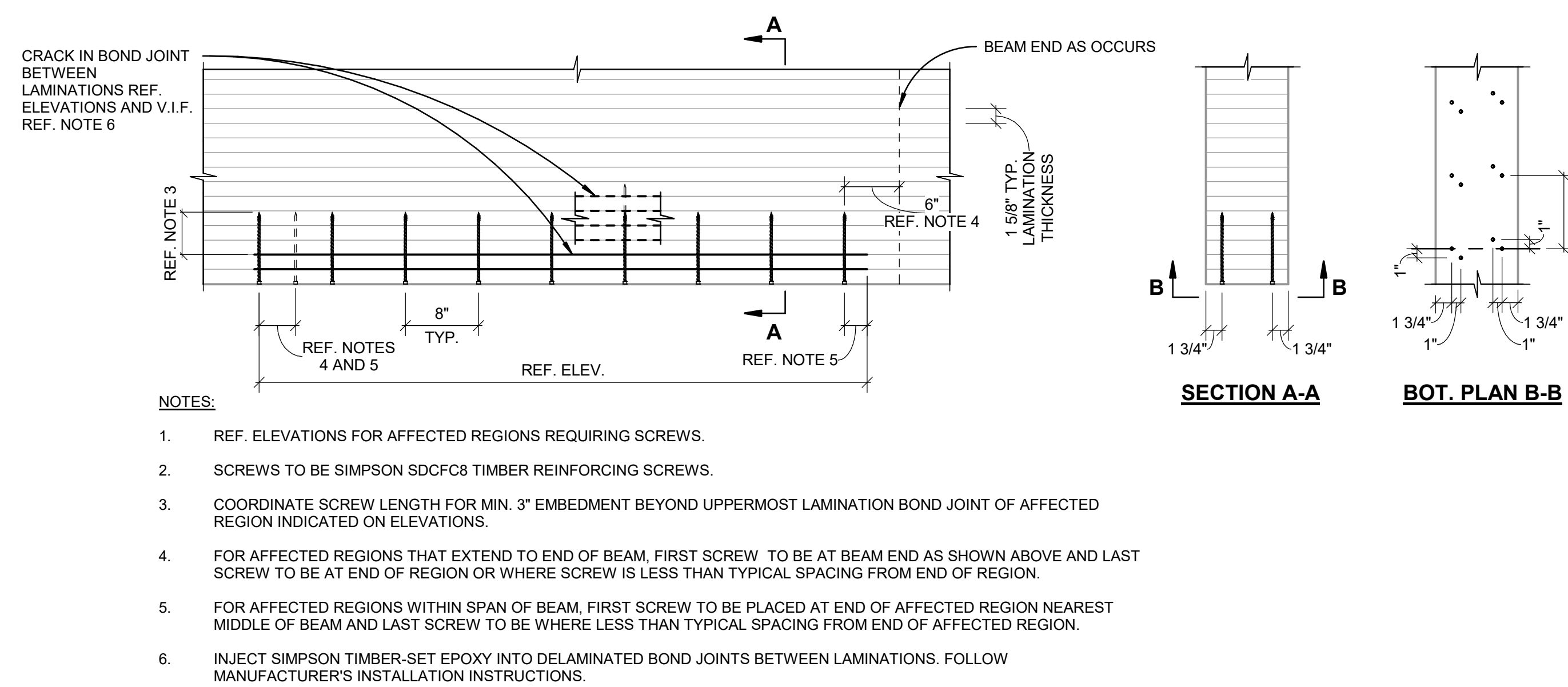
3 BEAM ELEVATION - 37 3/8" DEEP - SOUTH FACE - GRID B
1/2" = 1'-0"



4 BRIDGING AT WALL

$$1'' = 1' - 0''$$


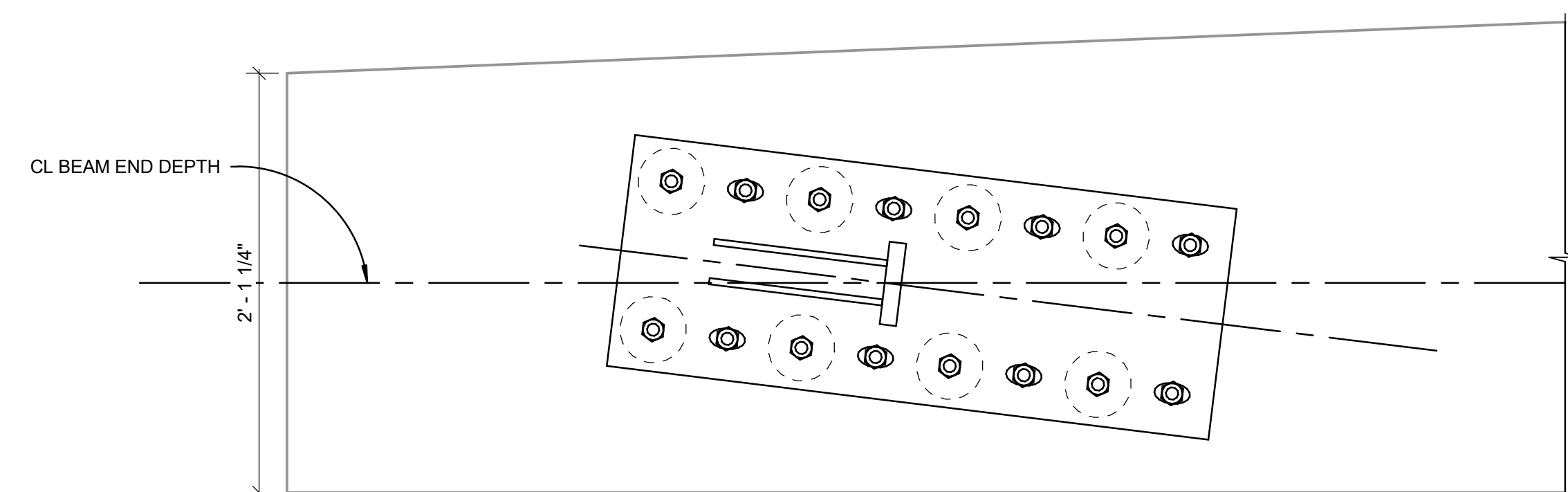
5 BRIDGING AT BEAM

$$1'' = 1' - 0''$$


6 DELAMINATION REPAIR DETAIL

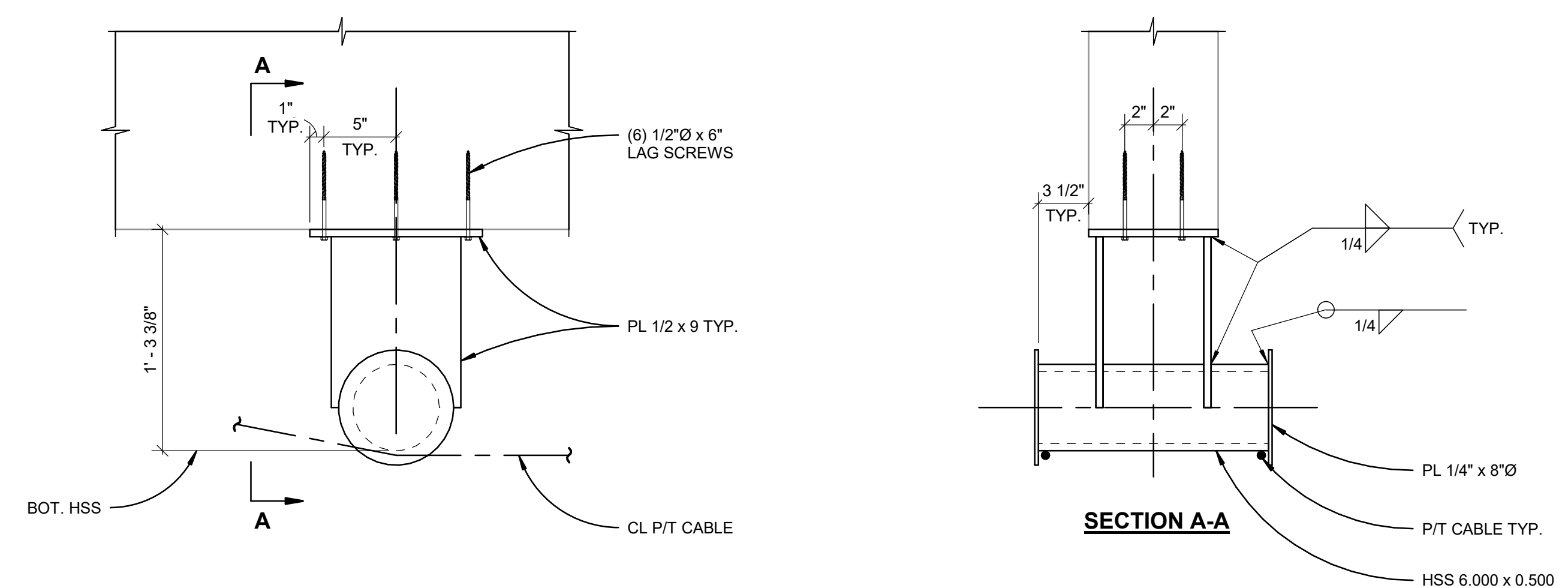
$$1'' = 1'-0''$$

1 POST-TENSION CABLE ANCHORAGE - FLAT BEAMS

$$1\frac{1}{2}'' = 1'-0''$$


NOTE:
REF. 1/S204 FOR INFORMATION NOT SHOWN.

2 POST-TENSION CABLE ANCHORAGE - TAPERED BEAMS

$$1\frac{1}{2}'' = 1'-0''$$


2 POST-TENSION CABLE PIVOT ASSEMBLY

1 1/2" = 1'-0"