

BID ADDENDUM NO. 3

OWNER: EDGEMONT SCHOOL DISTRICT

PROJECT NAME: HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

This Addendum is hereby included in and made a part of the Contract Documents, dated March 24, 2026 whether or not attached thereto.

All requirements of the original project specifications and drawings shall remain in force except as amended by this addendum.

DATE: April 10, 2026

This addendum consists of three (3) pages plus revised drawings E-100 and M100 and Geotechnical Report dated 7/28/2025.

THE FOLLOWING ARE MODIFICATIONS, CLARIFICATIONS, DELETIONS OR ADDITIONS TO THE SPECIFICATIONS:

01 1000 – SUMMARY OF CONTRACTS

01 1000-2, 2.1.C.1 **REVISE** “Date 10/7/25” to read “Date 3/24/26”.
01 1000-2, 2.1.C.2 **REVISE** “Date 10/21/25” to read “Date 4/7/2026”.
01 1000-2, 2.1.C.3 **REVISE** “Date 10/27/25” to read “Date 4/14/2026”.
01 1000-2, 2.1.C.4 **REVISE** “Date 11/4/25” to read “Date 4/21/2026”.
01 1000-2, 2.1.C.5 **REVISE** “Date 11/6/25” to read “Date 4/24/2026”.
01 1000-2, 2.1.C.6 **REVISE** “Date 11/10/25” to read “Date 5/5/2026”.

01 1000-13, 1.16 B. **ADD** “13. As part of the High School Resource & Building E renovation project, it is the District’s intent to install RTU-E1, RTU-E2, and RTU-E3, including all associated components and related scope of work, by August 24, 2026.

If the equipment for RTU-E1, RTU-E2, and RTU-E3 cannot be procured in time to meet this date due to supply chain constraints or other causes beyond the Prime Contractors’ control, each affected Prime Contractor (GC-1, PC-2, HC-3, EC-4) shall maintain their original bid pricing to complete this work during Summer 2027, with final completion no later than August 25, 2027. This shall constitute a permissible schedule adjustment without assessment of delay damages or penalties and shall include all related mechanical, plumbing, electrical, and roofing work.

01 1000-15, 1.17 B. **ADD** “10. As part of the High School Resource & Building E renovation project, it is the District’s intent to install RTU-E1, RTU-E2, and RTU-E3, including all associated components and related scope of work, by August 24, 2026.

If the equipment for RTU-E1, RTU-E2, and RTU-E3 cannot be procured in time to meet this date due to supply chain constraints or other causes beyond the Prime Contractors’ control, each affected Prime Contractor (GC-1, PC-2, HC-3, EC-4) shall maintain their original bid pricing to complete this work during Summer 2027, with final completion no later than August 25, 2027. This shall constitute a permissible

schedule adjustment without assessment of delay damages or penalties and shall include all related mechanical, plumbing, electrical, and roofing work.

01 1000-16, 1.18 E. **ADD** “13. As part of the High School Resource & Building E renovation project, it is the District’s intent to install RTU-E1, RTU-E2, and RTU-E3, including all associated components and related scope of work, by August 24, 2026.

If the equipment for RTU-E1, RTU-E2, and RTU-E3 cannot be procured in time to meet this date due to supply chain constraints or other causes beyond the Prime Contractors’ control, each affected Prime Contractor (GC-1, PC-2, HC-3, EC-4) shall maintain their original bid pricing to complete this work during Summer 2027, with final completion no later than August 25, 2027. This shall constitute a permissible schedule adjustment without assessment of delay damages or penalties and shall include all related mechanical, plumbing, electrical, and roofing work.

01 1000-19, 1.19 E. **ADD** “14. As part of the High School Resource & Building E renovation project, it is the District’s intent to install RTU-E1, RTU-E2, and RTU-E3, including all associated components and related scope of work, by August 24, 2026.

If the equipment for RTU-E1, RTU-E2, and RTU-E3 cannot be procured in time to meet this date due to supply chain constraints or other causes beyond the Prime Contractors’ control, each affected Prime Contractor (GC-1, PC-2, HC-3, EC-4) shall maintain their original bid pricing to complete this work during Summer 2027, with final completion no later than August 25, 2027. This shall constitute a permissible schedule adjustment without assessment of delay damages or penalties and shall include all related mechanical, plumbing, electrical, and roofing work.

01 1000-19, 1.19 E **ADD** “14. Electrical Contractor shall provide precast light pole bases to be installed by Site Contract.

15. Electrical Contractor is responsible for all excavation and backfill in the High School courtyard for electrical service.”

01 1000-21, 1.20 **ADD** “H. The work of the Site Construction Contract includes but not limited to the following:

1. Site improvements including roadways, walkways, site development, site furnishings, equipment, landscaping and synthetic athletic fields.
2. Site preparation including clearing.
3. Scoping of site.
4. Removals.
5. Excavation.
6. Excavation for electrical contract (at field).
7. Setting light pole bases supplied by electrical contract.
8. Backfilling.
9. Site water supply, distribution and bottle fillers.
10. Temporary construction access roads.
11. Site storm sewer.
12. Site concrete.
13. Site amenities.
14. Precast structures.
15. Scoreboards, dugouts and bleachers.
16. Fencing and gates.
17. EHS Logo.

01 6190 – MATRIX OF BUILDING SYSTEM RESPONSIBILITY

01 6190 Door Access (Interior Doors), Electronic Door Locksets (including batteries) **REVISE** “Furnished by GC” to read “Furnished by ESD”

01 6190 Door Access (Interior Doors), Panel Interface Modules **REVISE** “Furnished by GC” to read “Furnished by ESD”

08 1116 – ALUMINUM AND FRP DOORS AND ALUMINUM FRAMES

08 1116-7, 2.4 5. **DELETE** in its entirety.

08 7100 – DOOR HARDWARE

08 7100-13, 1.2 F.1.a **ADD** “b) Provided by Owner, installed by GC”.

08 7100-16, 1.2 F.2.h **ADD** “b) Provided by Owner, installed by GC”.

08 7100 Hardware Group No.03, No.8 and No.11 **ADD** “1 EA Deadbolt **B660-626AM-G SCE**”.

10 1100 – VISUAL DISPLAY UNITS

10 1100-3, 2.2 **REVISE** “C. Tackboards: Fabric laminated to cork.” to read “C. Tackboards: Cork.”.

10 1100-3, 2.2 C.1 **REVISE** “7/32 inch” to read “1/4”.”.

10 1100-3, 2.2 C.2 **DELETE** in its entirety.

APPENDIX

ADD Geotechnical Report

THE FOLLOWING ARE MODIFICATIONS, CLARIFICATIONS, DELETIONS OR ADDITIONS TO THE DRAWINGS:

DRAWING AR-103 – ROOF REMOVAL PLAN

REVISE note “Remove portion of existing roof assembly for installation of deck infill. See detail 6/A625” to read “Remove portion of existing roof assembly for installation of mechanical roof curbs. See detail 6/A625 for typical splice detail.”.

DRAWING A725 – WINDOW SCHEDULE, TYPES, AND DETAILS

Window Types SF1 and SF1a **DELETE** 135 degree angle and **REVISE** tag 16 to 14.

DRAWING A726 – WINDOW DETAILS

DELETE detail 16.

END OF SED ADDENDUM NO. 3

The following attachments are being provided for informational purposes:

Bidder RFI Responses (In the event of a discrepancy between these responses and the contract documents, the contract documents shall prevail unless modified by addendum.)

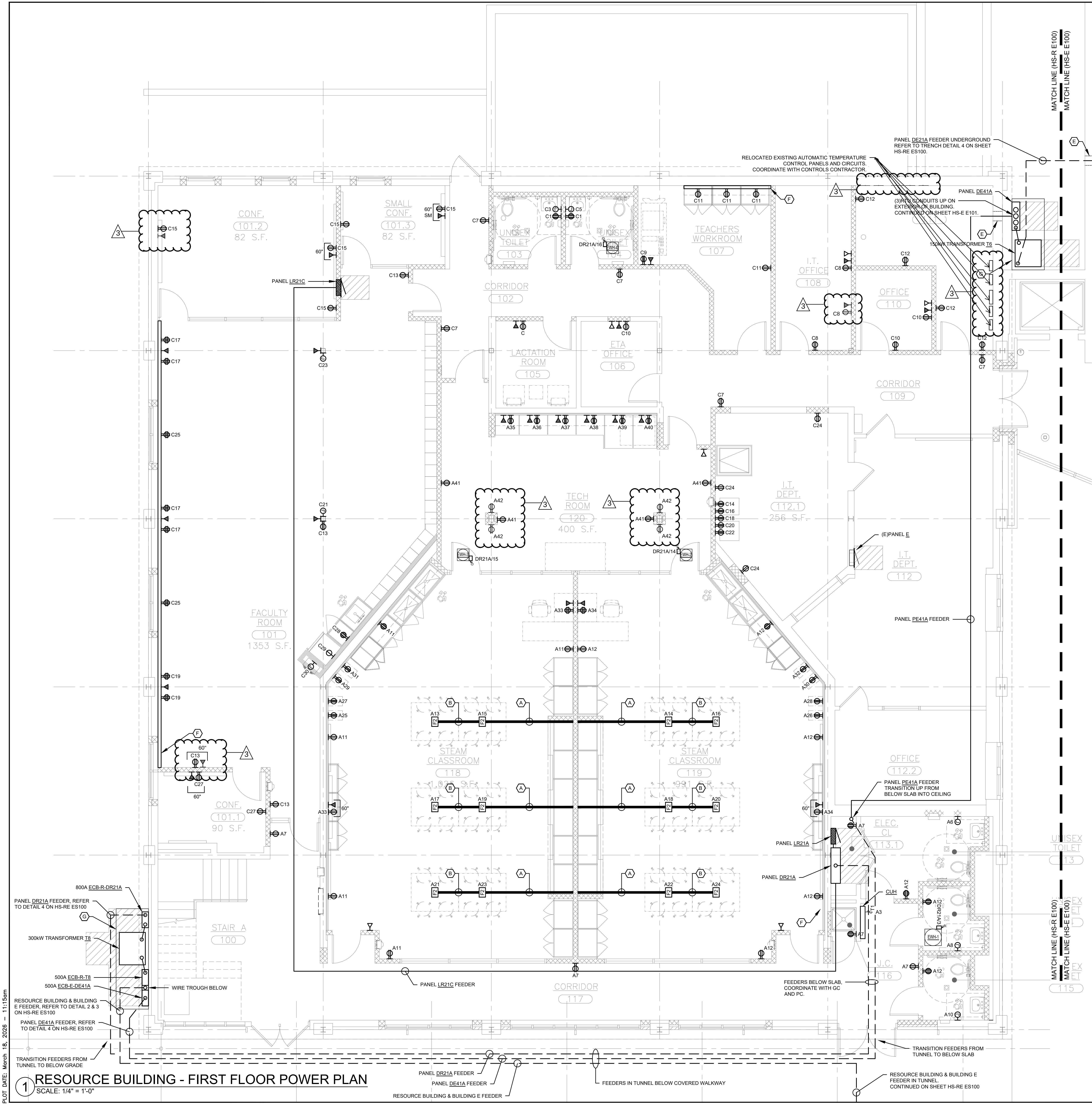
Bid RFI-#004 dated 3/26/2026 (2 pages).
Bid RFI-#005 dated 4/2/2026 (3 pages).
Bid RFI-#006 dated 4/8/2026 (2 pages).
Bid RFI-#007 dated 4/8/2026 (2 pages).
Bid RFI-#008 dated 4/9/2026 (4 pages).
Bid RFI-#009 dated 4/8/2026 (2 pages).
Bid RFI-#010 dated 4/8/2026 (2 pages).
Bid RFI-#011 dated 4/8/2026 (6 pages).
Bid RFI-#012 dated 4/10/2026 (3 pages).

PLOT DATE: March 18, 2026 - 11:15am

1 RESOURCE BUILDING - FIRST FLOOR POWER PLAN

SCALE: 1/4" = 1'-0"

FILE: N:\Edgemont School District\Edgemont Jr-Sr High\2025 AC Project\Resource\CAD\Electrical\E-Power Plan.dwg



CIRCUITING NOTES:

- CIRCUITING DESIGNATIONS ARE AS FOLLOWS:
- *## (EX. L24) WHERE * IS THE PANEL DESIGNATION AND ## IS THE CIRCUIT DESIGNATION.
- PANELS ARE AS FOLLOWS UNLESS NOTED OTHERWISE:
- A = PANEL LR21A
 - C = PANEL LR21C
 - E = PANEL LE21E
- ## KEYNOTES:
- INSTALL CONDUIT UNDER NEW FLOOR LEVEL. PROVIDE (2) 3/4"C.
 - INSTALL CONDUIT UNDER NEW FLOOR LEVEL. PROVIDE (1) 3/4"C.
 - TRENCH EXISTING FLOOR. INSTALL CONDUIT UNDER NEW FLOOR LEVEL. PROVIDE (2) 3/4"C. REPAIR FLOOR PER DETAIL 3 ON E400.
 - PROVIDE EQUIPMENT PAD PER DETAIL 1 ON E400.
 - PROVIDE BELOW GRADE WALL PENETRATION FOR FEEDER. PROVIDE SLEEVE AND SEAL PER DETAIL 2 ON E400.
 - PROVIDE SURFACE MOUNTED RACEWAY FOR WIRING DEVICES ON EXISTING WALL. PROVIDE DUAL CHANNEL ONE FOR POWER ONE FOR DATA. WIREMOLD 4000 OR EQUAL.
 - NEW PAD FOR ELECTRICAL EQUIPMENT, PAD PROVIDED BY GC.

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LFG

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**FULLER
D'ANGELO
P.C.**

**ARCHITECTS
PLANNERS**

LICENSE EXP. DATE: 03-31-2026

S.E.D. CONTROL NUMBER:
RESOURCE BUILDING
66-04-06-03-010-009

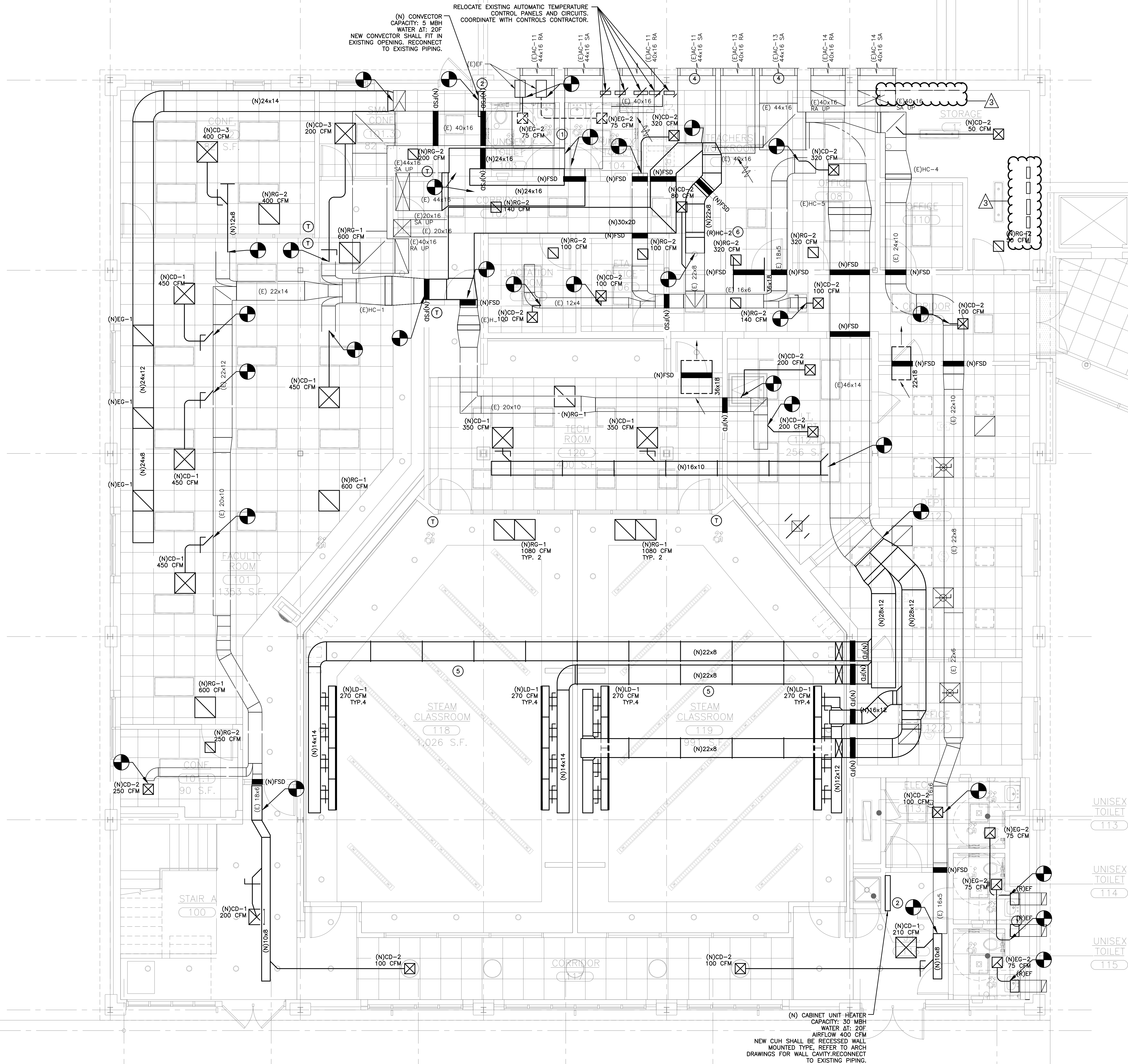
BUILDING E
66-04-06-03-010-005

PROJECT:
EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E
RECONSTRUCTION
500 WHITE OAK LANE, SCARSDALE, NY 11583

DRAWING TITLE:
FIRST FLOOR ELECTRICAL POWER PLAN

04-10-2026	BID ADDENDUM #3
03-24-2026	BID
1-30-2026	ADDENDUM 1
11-24-2025	SED SUBMISSION
DATE	ISSUED TO

SHEET SIZE 30"x42"	DRAWING NO. HS-R E100
SCALE AS NOTED	FILE NO. 24529.06
DRAWN BY LFG	

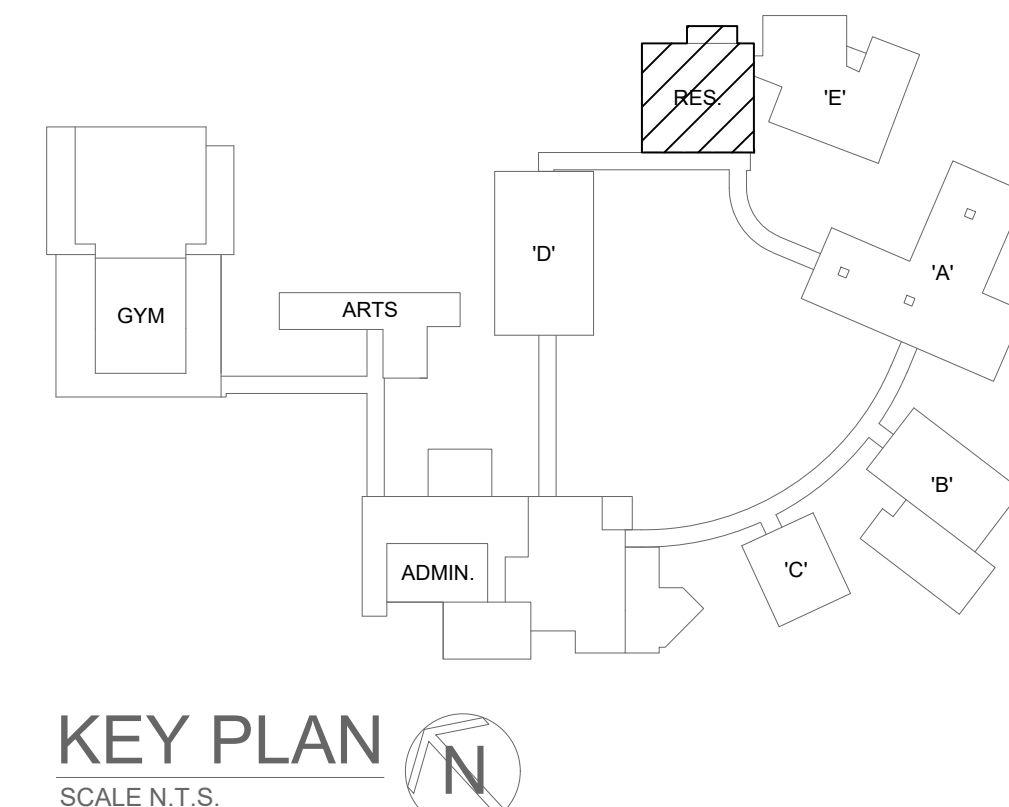


NEW WORK KEYNOTES

1. RECONNECT BATHROOM TO EXISTING EXHAUST SYSTEM.
2. COORDINATE FINAL LOCATION WITH ARCHITECT PRIOR TO INSTALLATION. CONNECT TO EXISTING HW SUPPLY AND RETURN. REFER TO HOT WATER COIL CONNECTION DETAIL ON M-200 FOR ADDITIONAL INFORMATION.
3. ROUTE CONDENSATE TO NEAREST SANITARY LINE. MAKE INDIRECT CONNECTION.
4. EXISTING OUTDOOR UNITS AC-12 AND AC-13 SHALL HAVE THE OA DAMPER ADJUSTED TO MEET VENTILATION AIR REQUIREMENTS. SEE VENTILATION AIR REQUIREMENTS TABLE ON M300 FOR ADDITIONAL INFORMATION.
5. EXPOSED DUCTWORK SHALL BE INTERNALLY LINED.
6. EXTEND EXISTING HW BRANCH PIPING TO RELOCATED HWC. FURNISH NEW 2-WAY CONTROL VALVE. REFER TO DETAIL FOR ADDITIONAL INFORMATION.
7. PROVIDE CABLE OPERATED DAMPERS AT ALL BRANCH ABOVE HARD CEILING.

RESOURCE BUILDING NEW WORK NOTES

1. THE EXISTING HVAC SYSTEM UTILIZES A PLENUM RETURN. CONTRACTOR TO FIELD VERIFY THAT THE NEW AIR DISTRIBUTION HAS AN UNBROKEN PATH TO EXISTING OUTDOOR UNIT RA CONNECTION.
2. COORDINATE NEW THERMOSTAT LOCATION WITH ATC CONTRACTOR. CONFIRM LOCATION WITH ARCHITECT PRIOR TO ROUGH IN.
3. CONTRACTOR SHALL CARRY A ALLOWANCE IN HIS BID OF \$10,000 FOR SHEET METAL AND PIPING CONFLICTS ARISING FROM CONSTRUCTION OF NEW FULL HEIGHT WALLS.
4. PROVIDE WIRE MESH SCREEN AT ALL AIR TRANSFER DUCT OPENINGS ABOVE CEILING.



1 FIRST FLOOR MECHANICAL PLAN

SCALE: 1/4" = 1'-0"

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 ARCHITECTS
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LICENSE EXP. DATE: 03-31-2026

S.E.D. CONTROL NUMBER:
 RESOURCE BUILDING
 66-04-06-03-010-009
 BUILDING E
 66-04-06-03-016-005

EDGEMONT SCHOOL DISTRICT
 HIGH SCHOOL RESOURCE & BUILDING E
 RECONSTRUCTION
 500 WHITE OAK LANE, SCARSDALE, NY 11583
 DRAWING TITLE:
 RESOURCE BUILDING FIRST FLOOR
 MECHANICAL PLAN

04-10-2026	BID ADDENDUM #3
03-24-2026	BID
1-30-2026	ADDENDUM 1
11-24-2025	SED SUBMISSION
DATE	ISSUED TO
SHEET SIZE: 30"x42"	DRAWING NO.: HS-RS
SCALE: AS NOTED	M100
DRAWN BY: LFG	FILE NO.: 24529.06

Phone
(203) 262-9328

Telefax
(203) 264-3414

WHITE PLAINS, N.Y.
(914) 946-4850

SOILTESTING, INC.

90 DONOVAN ROAD - OXFORD, CONN. 06478-1028

GEOTECHNICAL / ENVIRONMENTAL SUBSURFACE INVESTIGATIONS - Test Borings - Core Drilling
Monitoring Wells - Recovery Wells - Direct Push/Probe Sampling
UNDERPINNING - HELICAL PILES - SOIL NAILS

August 6, 2025

Fuller & D'Angelo P.C.
45 Knollwood Road
Elmsford NY 10523
Attn: William Means

Re: Edgemont HS Field Improvements
300 White Oak Lane, Scarsdale NY

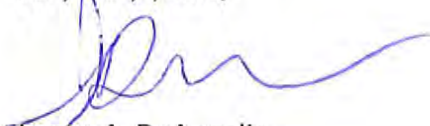
G142-3218-25

Dear Mr. Means,

Attached please find the Geotechnical Report, Test Boring Logs and location plan for work in Scarsdale NY along with our invoice.

If you have any questions, please do not hesitate to contact us.

Very truly yours,



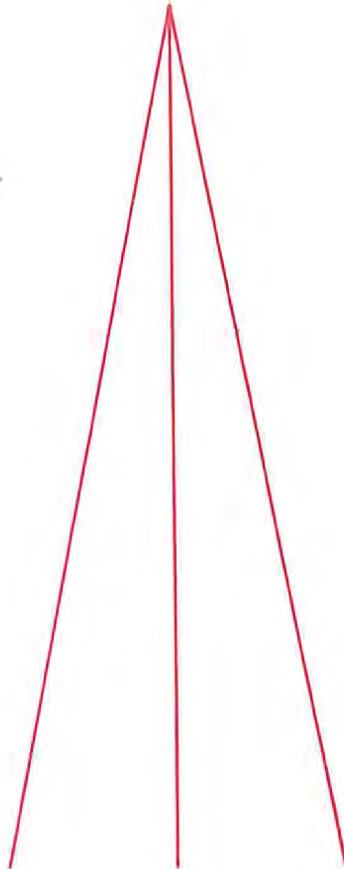
James A. DeAngelis

President

JAD:mv

SOILTESTING, INC.

TO	Fuller & D'Angelo P.C.	DATE	6-Aug-25
ADDRESS	45 Knollwood Rd., Elmsford NY 10523		
SITE LOCATION	300 White Oak Ln., Scarsdale NY		
REPORT SENT TO	William Means		
SAMPLES SENT TO	Storage (Max 60 days)		



90 Donovan Road
Oxford, Connecticut 06478-1028
203-262-9328

Branch Office:
White Plains, New York 10607
914-946-4850

JOB NO.
G142-3218-25

GEOTECHNICAL ENGINEERING REPORT

Edgemont School District HS Field Improvements

300 White Oak Lane
Scarsdale, New York 10583

Prepared For:

Fuller And D'Angelo P.C.
45 Knollwood Road
Elmsford, New York 10523

Prepared By:

Siamak Koochak, P.E.

Report Date:

July 28, 2025

Project No.

25-026



Source: Google Earth – Imagery date 05/2022

July 28, 2025
William J. Means
Fuller And D'Angelo P.C.
45 Knollwood Road
Elmsford, New York 10523

Subject: Geotechnical Engineering Report
Edgemont School District HS Field Improvements
300 White Oak Lane
Scarsdale, New York 10583
Project Number: 25-026

Dear William Means:

We are pleased to present this Geotechnical Engineering Report (the "Report"), which summarizes the findings of our subsurface investigation and provides geotechnical recommendations for the proposed Edgemont School District HS field improvements at the location referenced above.

This Report has been prepared exclusively for the use of Northeast Fuller and D'Angelo P.C. (the "Client") in support of the proposed development as outlined herein. It is intended to be used in its entirety and may not be reproduced or used for other purposes without the prior written consent of the Client and/or our firm. The primary objective of this geotechnical study is to evaluate the subsurface conditions at the project site through exploratory borings, assess the engineering properties of encountered soils, and provide design and construction recommendations based on our findings.

Thank you for the opportunity to assist you with this phase of the project. Should you have any questions or require additional information, please feel free to contact us at (908) 456-0026 or via email at siamak@koochakPE.com.

We appreciate the opportunity to be of service to you during this phase of the work.

Sincerely,



Siamak Koochak, P.E.
Geotechnical Engineer

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1.0 EXECUTIVE SUMMARY

Engineering analyses were performed to evaluate subsurface conditions for the proposed HS field improvement recommendations for construction and utility support. The following summarizes the main findings of the exploration, particularly those that may have a cost impact on the planned development. This summary in no way replaces or overrides the detailed sections of the report. The principal conclusions are described below:

PROJECT INFORMATION:

- Proposed Construction:
 - o Reconstruction and improvement of the existing athletic field and site improvements.

FIELD EXPLORATION SCOPE AND SUBSURFACE CONDITIONS:

- Field Exploration and Scope of Work:
 - o Eight (8) SPT borings within the proposed improvement to depths ranging from 10 to 12 feet below existing site grades.
 - o Three (3) groundwater monitoring wells.
 - o Eight (8) borehole infiltration tests.
- Subsurface Conditions:
 - o Boring B-2 Only: possible fill soil consisting of silty SAND (SM) with gravel underlain by organic SILT (OH), followed by native silty SAND (SM) with Gravel.
 - o Remaining of the borings: Native silty or Clayey SAND (SM/SC) with Gravel.
- Auger Refusal:
 - o Encountered at depths ranging from about 10 to 10.5 feet below existing site grades in Borings B-5 and B-8.
- Split Spoon Refusal:
 - o Encountered at a depth of about 10 feet below existing site grades in Borings B-5 and B-8.
- Ground Water/Perched water:
 - o Groundwater was not encountered across the site; however, perched/trapped water was encountered at a depth of about 5 feet in Boring B-5.

GEOTECHNICAL CONCERNS:

- Insufficient data regarding the slope along the western side of the field where the proposed bleachers are planned to be extended on top of the existing slope. Additional exploratory borings and slope stability analysis are recommended.

EARTHWORK/DEWATERING CONSIDERATIONS:

- Removal of the topsoil layer within the proposed site improvement areas.
- Regrading the site, to direct runoff away from the newly installed synthetic grass (turf).
- Soil Reuse: Some site soils may be suitable for reuse if the soil is screened and meets the criteria provided in this report.

DESIGN & CONSTRUCTION RECOMMENDATIONS:

- Foundations and Slab Type:
 - o Concrete slab on-grade supported on a helical pile system or sonotubes.
 - o Anticipated Allowable Bearing Pressure: 2,000 to 3,000 psf (Additional borings in vicinity of the improvement area are recommended).
 - o Min. foundation embedment (Frost): 48 inches below adjacent properties.

2.0 PROJECT INFORMATION

2.1 Geologic Setting and Hazards

The subject site is in the Town of Scarsdale, in Westchester County, New York. This area is situated within the New England Province Physiographic Region of the State of New York. According to the United States Geological Survey (USGS), the Lower-Hudson Map, the surficial geology, it appears that the site is located within the Till Formation. Till Formation generally consists of a poorly sorted mixture of rock particles ranging in size from boulders to clay.

Based on information obtained from the United States Department of Agriculture (USDA) Natural Resources Conservation Service Web Soil Survey online database, the subject property is mapped as Udorthents. Udorthents series generally is characterized by soils that have been significantly disturbed by human activities such as cutting, filling, or construction, resulting in a lack of normal soil horizons.

2.2 Geologic Hazards

Geologic hazards with the potential to affect development in the State of New York are earthquakes, landslides, oil and gas wells, and minefields. New York is generally considered to have a low to moderate seismic risk per the USGS 2014 Hazard Risk Map. Additionally, this site is in an area of New York that is generally comprised of relatively flat terrain that is not at a high risk for landslide activity. This region of New York is not within an area of abandoned oil and gas wells or minefields. According to the Federal Emergency Management Agency (FEMA), the site is located within Flood Zone X (unshaded); defined as minimal risk areas outside the 1-percent and 0.2-percent-annual-chance floodplains.

2.3 Project Location and Site Conditions

The project site is situated within the Edgemont School property on an approximately 2.5-acre parcel of land within a residential and commercial area of the Town of Scarsdale, New York. The project site is currently a baseball field. Based on our visual observations and a review of the elevations provided in Google Earth, the site exhibits a gradual change in grade. Elevations increase from approximately 315 feet in the southwestern portion of the site to an elevation of about 319 feet at the northwestern corner. It should be noted that these elevations are obtained from Google Earth. Therefore, the elevation data provided should be considered approximate and verified through a current, site-specific topographic survey prior to final design and construction. The project site is immediately bound by:

Adjoining Properties			
North:	Commercial buildings	South:	Blandford Field
East:	Commercial buildings and a cemetery	West:	Trees and recreational fields

2.4 Historical Land Use

According to publicly available historical aerial photographs and topographic maps, the subject site was characterized as undeveloped, wooded land from at least 1953 to 1954. Between 1954 and 1960, the site was cleared of vegetation and trees, graded, and subsequently developed into its current configuration as a baseball field. Since that time, the site's overall layout has remained unchanged. Historical aerial imagery and topographic maps prior to 1953 are not publicly available.

Historical Use Information		
Period/Date	Source	Description/Use
Prior to 1954	Aerial Photographs, Topographic Maps	Wooded/Undeveloped land
1960 – Present	Aerial Photographs, Topographic Maps	Current state

GEOTECHNICAL ENGINEERING REPORT

Project No. 25-026

July 28, 2025

2.5 Proposed Construction Development

A site layout plan (Sheet No. C102) – Titled “Edgemont School District High School Field, Site Improvements & Related Work” - Prepared by Barton and Loguidice – Plan dated 06/19/2025 (provided in Appendix E) was provided to us. According to the site layout plan and information provided to us by William Means, from Fuller and D’Angelo P.C., the proposed improvement for the ES Fields includes upgrades to the athletic facilities, circulation paths, and supporting infrastructure. A new baseball field with a 300-foot outfield and a new softball field with a 200-foot outfield are planned, along with a synthetic turf multi-use field measuring approximately 170 by 300 feet.

Bleacher seating is proposed to be built into the hillside, with provisions for separate home and away sections. Existing scoreboards will be removed and replaced, and space will be allocated for a batting cage and storage shed adjacent to the turf field. Additionally, new speakers will be installed in multiple locations within the project site.

Two (2) approximately 130-linear-foot-long retaining walls are proposed for construction along the north and northwest sides of the existing baseball field. At the time this report was prepared, information regarding the type and height of the proposed retaining walls had not been provided. Lateral earth pressure parameters for the design of the retaining walls are presented in Section 6 of this report. It should be noted that the evaluation of slope stability and the stability of the existing slope and proposed retaining walls is beyond the scope of this study.

3.0 GEOTECHNICAL EXPLORATION AND TESTING

3.1 Field Exploration

As part of the geotechnical investigation for the proposed development, a total of eight (8) borings were planned to be drilled within the development areas. Between June 20 and June 26, 2025, the exploration borings, designated B-1 and B-8, were advanced to depths ranging from about 10 to 12 feet below the existing site grades. The approximate locations of the exploration borings are shown in Appendix A, on Figure 3.

Drilling was conducted by Soiltesting, Inc. of Oxford, Connecticut, using a track-mounted Dietrich D-50 drill rig equipped with a 140-pound automatic hammer. The borings were advanced using hollow-stem auger (HSA) drilling techniques with a 4¼-inch inside diameter auger. Soil sampling was conducted in accordance with ASTM D1586, the Standard Penetration Test (SPT) method. This procedure involves driving a 2-inch outside diameter split-spoon sampler a total of 24 inches into the soil using a 140-pound hammer that dropped from a height of 30 inches. The standard penetration resistance (N-value) is defined as the number of hammer blows required to advance the sampler to the final 12 inches, following an initial seating drive of 6 inches, and is reported in blows per foot (bpf).

Additionally, three (3) temporary groundwater monitoring wells were installed in Borings B-3 through B-5 to depths ranging from approximately 10 to 12 feet below the existing site grades. The purpose of these wells was to monitor groundwater levels after drilling, providing a more accurate assessment of groundwater elevation and potential fluctuations over time.

All recovered soil samples were visually classified in general accordance with the Unified Soil Classification System (USCS) and ASTM D2488. Field logs documenting the subsurface conditions encountered during drilling were prepared on site. Final boring logs were developed based on these field logs and are presented in Appendix B of this report.

3.2 Laboratory Testing

Selected samples obtained from drilling were transported to a local soil laboratory, Skyland Testing LLC, of Sparta, NJ, for basic laboratory testing. Grain size analysis was conducted on six (6) samples based on ASTM D6913 - Standard Test Methods for Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis. The test results of the laboratory testing are provided in Appendix C

3.3 Infiltration Testing

Between June 24 and June 26, 2025, eight (8) infiltration tests were performed at designated locations INF-1 through INF-8 to evaluate the rate at which water infiltrates into and flows through the underlying soils. The tests were conducted using a 4-inch outside diameter (O.D.) casing installed to depths ranging from approximately 3 to 6.5 feet below the existing site grades, in accordance with the New York State Stormwater Management Design Manual – Appendix D guidelines. Each test location was presoaked for 24 hours prior to testing. Measured infiltration rates ranged from 0.25 to 4.5 inches per hour. The detailed infiltration test results are provided in Appendix D.

4.0 GENERALIZED SUBSURFACE CONDITIONS

The following section provides a generalized description of the subsurface conditions encountered during the site investigation.

4.1 Surface Cover

An approximate 3- to 4-inch-thick topsoil layer was encountered across the site.

4.2 Possible Fill: Silty SAND With Gravel (USCS: SM)

A layer of possible fill soil consisting of Silty SAND (SM) with gravel was encountered in Boring B-2, beneath the surface layer. The fill soil layer extends to a depth of about 5 feet below the site grades at the time of drilling. Uncorrected SPT N-values in the possible fill soil ranged from 12 to 17 blows per foot (bpf). The possible fill soils are characterized as medium dense in terms of relative density.

4.3 Organic SILT (USCS: OH)

An approximate 12-inch-thick layer of organic silt (OH) was encountered in Boring B-2 at a depth of approximately 5 feet, extending to about 6 feet below existing site grades at the time of drilling.

4.4 Native: Silty Sand (USCS: SM)

Native soils consisting of Silty SAND (SM) and Clayey SAND (SC) with varying amounts of gravel were encountered below the possible fill layer in Boring B-2 and the surface cover layer in the remaining boring locations. The native soil extended to terminal depths of the borings or standard split-spoon refusal, at depths ranging from approximately 10 to 12 feet below the existing site grades at the time of drilling. Uncorrected SPT N-values in the native sandy soil ranged from 6 to 40 blows per foot (bpf). The native sandy soils are characterized as loose to dense in terms of relative density.

4.5 Native: Bedrock/Decomposed Rock

Decomposed rock or bedrock was encountered in Borings B-5, B-7, and B-8 at depths ranging from approximately 10 to 11.5 feet below the existing site grades. Additionally, split spoon refusal was observed in Borings B-5 and B-8 at depths of about 10 feet. Split spoon refusal refers to conditions where a 2-inch outside diameter (O.D.) split spoon sampler cannot be advanced further, typically indicated by an N-value exceeding 50 blows per 6 inches of penetration. Refusal may be caused by large boulders, rock ledges, lenses, seams, weathered or decomposed rock, or the top of competent bedrock. Based on subsurface conditions and drilling resistance, we interpret the refusal material in these borings to be weathered rock. The summary of subsurface conditions is shown in the table below.

Summary of Subsurface Conditions			
Stratum	Approximate Depth to <u>Bottom</u> of Stratum ¹	Material Description Based on SPT Borings	Relative Density / Quality
Surface Cover	Approx. 3 to 4 in.	Topsoil	-
Possible Fill	Approx. 5 feet (Boring B-2 – ONLY)	Silty Sand (SM) with Gravel	Medium dense
Native	Approx. 6 feet (Boring B-2, Only)	Organic SILT (OH)	-

Summary of Subsurface Conditions			
Stratum	Approximate Depth to <u>Bottom</u> of Stratum ¹	Material Description Based on SPT Borings	Relative Density / Quality
Native	Approx. 10 to 12 feet	Silty Sand (SM) or Clayey Sand (SC) with Gravel	Loose to dense
Bedrock/Decomposed Rock	Approx. 10 to 11.5 feet± (Borings B-5, B-7 and B-8 – ONLY)	-	-

NOTE:¹ Below the existing site grades**4.6 Groundwater**

Groundwater was not encountered across the site during or after drilling, with the exception of perched or trapped water observed in Boring B-5 at a depth of approximately 5 feet below existing site grades at the time of drilling. It should be noted that groundwater levels are subject to seasonal and climatic fluctuations and may vary during construction and throughout the life of the project.

Temporary groundwater monitoring wells were inspected during drilling and again 24 hours after installation, between June 20 and June 26, 2025. Based on these inspections, groundwater was not observed in any of the monitoring wells.

5.0 CONCLUSION AND RECOMMENDATIONS

Based on the results of this study, it is our opinion that construction of the proposed improvement is feasible, provided the geotechnical recommendations are followed and that the standard level of care is maintained during construction. Geotechnical issues that may impact site development include the presence of undocumented fill and organic silt, and loose native sandy soils. Further discussions of our geotechnical recommendations for site development are presented in the following sections of this report.

5.1 Site Preparation Considerations

All vegetation, roots, topsoil, and other deleterious materials should be removed from the proposed improvement areas extending 2 feet beyond the proposed development limits. Approximately up to 4 inches of topsoil were observed in the borings. However, the actual stripping/removing thickness will depend on local development, construction traffic disturbance, and contractor care during clearing and stripping. A geotechnical engineer should observe stripping operations to evaluate that all unsuitable materials are removed from locations for proposed improvement.

Materials disturbed during clearing operations should be stabilized in place or, if necessary, undercut to undisturbed materials and backfilled with properly compacted, approved structural fill. During site preparation activities, the contractor should identify the borrowed source materials that will be used as structural fill, if needed, and provide samples to the testing laboratory so that conformance to the structural fill requirements outlined below and appropriate moisture-density relationship curves can be determined.

Remnants of deleterious material and structural elements should be completely removed from the subgrade level. Any particle size larger than 4 inches in greatest dimension should not be used directly below the proposed new synthetic turf area. Therefore, it should be removed from within the building pad subgrade area. Any possible voids resulting from removing cobbles/boulders and possible construction debris should be backfilled with approved compacted structural fill.

Following this work, the exposed surface should be proofrolled and evaluated by the engineer with a loaded water truck (4,000 gallons) or equivalent rubber-tired equipment. Where excessive pumping and rutting, loose soils, or otherwise unsuitable materials are encountered during the evaluation, it should be over-excavated and replaced with approved compacted structural fill. In locations where proofrolling is not feasible, probing, dynamic cone penetration testing, or other methods may be employed. Any isolated loose areas that pump, deflect, or rut excessively should be over-excavated and replaced with compacted structural fill.

We recommend that the earthwork phase of the project be performed during the warmer, drier months of the year. Consideration should be given to weather conditions at the time of grading. Earthwork should be scheduled during seasonally dry periods, and proper preparations should be made to deal with rain events, winter storms, frozen soil, etc.

5.2 Baseball Field Subgrade Preparation Considerations

Following initial site preparation and the anticipated grading (less than 1 foot of grade change) within the areas proposed for synthetic turf, sandy soils with varying amounts of silt and clay to be exposed after initial site preparation.

Prior to the start of fill placement or turf installation, the exposed subgrade should be scarified to a depth of 12 inches, moisture conditioned, and recompacted to at least 95 percent of the Standard Proctor maximum dry density.

The project geotechnical engineer or a qualified representative should evaluate the subgrade once moisture-conditioned and recompacted. Some unstable areas may be present in parts of the site. All areas should be carefully proofrolled with a heavily loaded tandem-axle dump truck at the following times:

- After an area has been prepared as recommended, prior to fill placement.
- After areas have been exposed to any precipitation, and/or have been exposed for more than 48 hours prior to synthetic turf installation.

It should be emphasized that, depending on weather conditions at the time of construction, the near-surface soils may be unstable. If scarification, moisture conditioning, and re-compaction are not effective to establish a stable platform, the loose/unstable soils should be undercut full depth to expose underlying stable material. The area should then be replaced with structural fill in accordance with the recommendation noted within the Soil Reuse and Structural Fill Considerations section of this report. Evaluation of the exposed subgrade should be performed by the geotechnical engineer or their qualified representative.

The subgrade soils often become disturbed during the period between initial site grading and construction of surface improvements. The amount and depth of disturbance will vary with soil type, weather conditions, construction traffic, and drainage. The engineer should evaluate the subgrade soil during final grading to verify that the subgrade is suitable for the installation of a synthetic turf system. The final evaluation may include proofrolling or in-place moisture-density tests.

All structural fill placed within the baseball field area should consist of controlled compacted fill that has been approved by and installed under the observation of a representative of the geotechnical engineer. Structural fill should be placed in layers on the order of 8 to 10 inches in loose thickness when heavy compaction equipment is used. If hand-guided equipment, such as a jumping jack or plate compactor, is planned to be used, the structural fill should be placed in no more than 4 to 6 inches in loose thickness. Each lift should be compacted to a minimum of 95% of the maximum dry density in accordance with Modified Proctor testing per ASTM D1557 specifications.

These operations should be performed under the direction of a full-time geotechnical inspection and testing agency utilizing either the Sand Cone Method (ASTM D1556), Nuclear Density Gauge (ASTM D2922 or D6938), or other industry-approved moisture/density test methods at sufficient frequencies to ensure proper compaction.

5.3 Dewatering Considerations

Groundwater was not encountered at the time of drilling. However, perched or trapped water was encountered in Boring B-5 at a depth of about 5 feet below existing site grades. Groundwater levels are subject to seasonal and climatic fluctuations and may vary at the time of construction and over the lifespan of the structure. Based on current observations, groundwater is not anticipated to pose significant concerns or adversely affect planned excavations.

Nevertheless, positive surface and subsurface drainage must be maintained during and after construction to prevent surface runoff from infiltrating and saturating subgrade soils. Accumulation of water adjacent to the building can result in soil softening, erosion, or movement, which may lead to differential settlement of slabs and foundations. Such conditions can contribute to structural distress, including cracked slabs and walls, and may also lead to roof leaks due to distortion of the building envelope.

5.4 Soil Reuse and Structural Fill Considerations

With the exception of the Organic Silt (OH) layer, the existing on-site soils are generally considered suitable for reuse as structural fill, provided they are free of organic matter, roots, debris, and other deleterious materials. If

undocumented fill is encountered during excavation activities associated with the residential building and is proposed for reuse as structural fill, it must be carefully screened.

All deleterious inclusions should be removed from the soil matrix prior to reuse to ensure long-term performance and stability. The suitability of the soil can be determined by lab testing on the representative bulk samples from excavated soil during the construction activities and inspection. Organic soils should not be mixed with structural fill used to support the foundations and slab on-grade.

If imported structural fill is required, we recommend that the material consist predominantly of well-graded sand and/or gravel with no more than 15% fines (i.e., material passing the No. 200 sieve), a plasticity index (PI) less than 20, and free of organic content or other deleterious substances. The maximum particle size should not exceed 1 inch in any dimension. Acceptable imported materials should conform to one of the following Unified Soil Classification System (USCS) designations: SM, SP, SW, SC, GP, GM, GC, or GW. All proposed off-site borrowed materials must be reviewed and approved by the geotechnical engineer prior to use.

As an alternative, crushed aggregate material such as AASHTO No. 57 stone may be utilized beneath foundations in over-excavated areas to provide stable support and enhance drainage.

5.5 Subsurface Utilities

The site soils, as well as imported approved structural fill, are considered suitable for the support of subsurface utilities. We recommend a 6-inch-thick granular bedding layer consisting of AASHTO No. 57 stone aggregate be placed if water seepage occurs at or above the planned invert elevations.

Contractors should provide adequate earth support and dewatering systems in utility trench excavations, if encountered. Depending on the location and depths of the utility lines, the groundwater table should not impact efforts. However, dewatering through the use of “sump and pump” techniques may be required in some areas to remove water seepage, especially if utility installation is performed during the wet season or after prolonged periods of inclement weather.

Utilities installed below any possible structural areas should be backfilled using controlled fill, compacted in accordance with the recommendations presented in the Excavation / Earthwork section of this report.

6.0 DESIGN RECOMMENDATIONS

The following discussion of findings for the site is based on the assumed construction, geologic review, and results of the field exploration.

6.1 Proposed Athletic Field – New Synthetic Turf Considerations

Following preparation of the site and removal of the surface cover, before placement of the new synthetic turf, the exposed surface should once again be evaluated and proofrolled using a loaded tri-axle dump truck, having an axle weight of at least 10 tons, to aid in identifying any possible localized loose or unusable/unsuitable materials. Any loose areas or otherwise unsuitable materials encountered during the proof rolling effort should be over-excavated until firm and unyielding layers are encountered. Prior to placement of the synthetic turf layer, the area should be compacted in place using a heavy vibratory compactor with a minimum static weight of at least 20 tons, performing several passes in each direction. Once again, upon completion of the compaction, the surface should be evaluated and repaired as needed.

We recommend a layer of filter fabric be placed over the compacted subgrade to prevent the migration of the soil particles. An impervious line layer, a cushion layer, and AirDrain Geocell should be placed on top of the geofabric. In addition, a drainage line should be installed around the field perimeter to collect water that could potentially infiltrate beneath the field and should be discharged to an approved storm drain. All grades must provide effective drainage away from the athletic field during and after construction and should be maintained throughout the life of the field. The synthetic turf layer should be installed per the manufacturer's installation guidelines.

In general, a minimum 6- to 8-inch-thick layer of drainage aggregate in conjunction with drainage lines (AdvanEdge or similar installed in a herringbone arrangement with a spacing of approximately 10 to 15 feet center-to-center and ½ percent sloping subgrade) is required to convey water to perimeter drains. However, if the subgrade is not stabilized, 6 to 8 inches of drainage aggregate will not be sufficient to protect the subgrade from damage. If the subgrade is stabilized, a minimum 6- to 8-inch-thick layer of drainage aggregate with the above-referenced drainage considerations will be adequate. If a drainage shock pad is used in conjunction with a stabilized subgrade, 4 inches of imported granular fill with a maximum size of ¾ inch can be used.

6.1 Proposed Bleacher Structures – Concrete Sonotubes Considerations

According to the project plans, the proposed bleacher structure is to be extended along the higher side of the existing slope. The bleachers may be supported by concrete sonotube foundations extending to at least below the toe of the slope, bearing directly on bedrock or on competent native soils that are free of deleterious materials, roots, and organic content. Extending the foundations below the slope toe is recommended to reduce the potential for slope instability or failure. Alternatively, re-grading the area may be necessary to stabilize the existing slope. A detailed slope stability analysis is outside the current scope of work and strongly recommended. It can be performed upon client request.

No exploration borings have been performed in the vicinity of the proposed bleacher location to determine the depth to bedrock. Therefore, we recommend that an additional subsurface exploration be conducted to assess soil conditions and establish the depth to competent bearing material.

According to the available resources online, the frost depth in this region is typically at a depth of about 42 inches. As such, we recommend all the foundations be embedded to a minimum depth of about 42 inches below the final grades for frost protection and heave conditions.

If, upon completion of the excavation, it appeared that the subgrade level consists of soils, prior to placement of the footings, the base of the excavations should be evaluated under the direction of the geotechnical engineer to verify

that competent native site soils are present. Once approved, the base of the excavation should then be scarified to a minimum depth of 12 inches, moisture conditioned and compacted in place using appropriate 10-ton vibratory compaction equipment or similar to create a uniform firm bearing stratum prior to placement of the foundations.

If loose soils encountered at the planned foundation subgrade cannot be adequately densified in place or are within the zone of foundation stress influence, the foundation excavations should extend to stable materials. Foundation subgrades requiring over excavation may be backfilled to the design bearing grade with controlled compacted fill, open-graded crushed stone meeting the gradational requirements of AASHTO Size No. 57 aggregate, or concrete. Open-graded stones may be placed in approximately 12-inch-thick loose lifts and be compacted by tamping with the equipment bucket. A layer of filter fabric should be placed below and above the open-graded crushed stone layers that are used as backfill material to prevent the mitigation of fine content.

Following subgrade preparation as provided above, settlements on the order of 1-inch total and 1/2-inch differential can be anticipated for footings bearing on soil, based on the assumed loads. An allowable bearing pressure of 2,000 to 3,000 psf can be used during the design of the foundations, depending on the subgrade level material.

6.2 Retaining Wall Considerations

Upon completion of site preparation and excavation of the proposed bottom of the wall foundation elevations, the base of the excavation should be evaluated by the geotechnical engineer to ensure competent soils are present, with careful attention paid to the removal of any loose or unsuitable materials, or particles larger than 4-in. in diameter being removed from within the proposed retaining wall foundation subgrade and replaced with approved compacted structural fill.

The wall foundation should be founded with an embedment depth of at least 48 inches below adjacent grades for adequate frost protection and support. However, the wall designer may increase the foundation embedment depths during the design based on the potential of wall failures such as sliding, overturning, internal stability, and bearing capacity failure.

Based on the soils encountered in our borings, an allowable bearing pressure of 2,000 psf, assuming the foundations bear on competent compacted soils, may be used for the design of the wall foundation. This value may be increased by 1/3 for wind and seismic loading. However, exploration borings have been performed in the vicinity of the proposed bleacher location to determine the depth to bedrock. Therefore, we recommend that an additional subsurface exploration be conducted to assess soil conditions and establish the depth to competent bearing material.

Any structural fill utilized to support the wall foundation shall extend at least 12 inches beyond the proposed limits of the foundation. The structural fill should be placed 12 inches horizontally for each 12 inches of over-excavation and replacement with structural fill placed below the base of the foundation. Alternatively, a thin layer of concrete (mud mats) at the foundation subgrade elevation to protect the subgrade may be considered.

Proper back drainage behind and at the toe of the proposed retaining wall should be considered to collect water that could potentially increase the hydrostatic pressure and should be discharged to an approved storm drain. The retaining wall foundation may be supported on a 12-inch-thick base layer of consolidated, open-graded gravel, such as AASHTO #57 stone or similar material for free drainage. Additionally, surficial grading around the top of the walls should be considered in the design to prevent water from flowing over the wall and causing soil erosion. Weep holes should also be provided along the retaining wall. We recommend a minimum of 18 inches of open-graded gravel, such as ASTM C-33 #57 stone or similar material, be placed directly behind the walls to provide a free drainage layer. We recommend that a layer of non-woven geotextile fabric be placed between the existing soil and the newly placed structural fill. A minimum 2-ft overlap is recommended between adjoining geotextile sheets. The geofabric textile should be determined by the wall designer and manufacturer.

For new fill zones along the existing slope and a free-draining stone layer behind the foundation walls, we recommend that the backfill soils consist of free-draining, non-expansive soils. Structural fill should be placed in maximum 8 in. loose lifts and compacted to 95% of its maximum dry density and within 3% of the optimum moisture content as determined by the Modified Proctor Density Test (ASTM D 1557). Only smaller hand-held compacting equipment, such as a vibratory plate tamper or dual drum walk behind Rammex roller, should be used within 3 feet of the walls to avoid additional temporary or long-term lateral pressure and movement. Larger ride on heavy compaction equipment may be used outside of this zone. Finished site slopes should be protected from erosion as described above. The excavation and retaining wall construction should be continuously monitored in the field by a representative of the geotechnical engineer or a geologist.

6.3 Lateral Earth Pressure Parameters

Depending on the types of structures and soil types encountered, different lateral stress distributions may be needed. For this project, we anticipate that the information below will be used to design the retaining wall. We recommend that a specialty contractor with in-house engineering capabilities perform the design. Values provided in this table are unfactored. The designer should select appropriate safety factors for their design.

Lateral Earth Pressures			
Soil Type	On Site Retained Soils (Sandy Soils)	Import Fill (Granular Soils)	Aggregate Backfill (Critical Zone)
Soil Parameters			
Total Unit Weight (γ)	120	125	130
Friction Angle ($^{\circ}$)	30	32	32
Cohesion (psf)	0	0	0
Coefficient of Friction (μ)	0.35	0.40	0.40
At-Rest Equivalent Fluid Pressure (psf)	60H	55H	60H
Active Equivalent Fluid Pressure (pcf)	40H	35H	40H
Passive Fluid Pressure (pcf)	350	400	400

NOTE:

1. Groundwater was not encountered drilling.
2. These loads should be modified by surcharge loads, if any.
3. The values provided in this table are UNFACTORED. The wall designer should select appropriate safety factors for their design.
4. Fluid pressures are the unit weight times lateral earth coefficients, rounded conservatively. Equations used are $K_o = (1 - \sin \phi')$, $K_a = \tan^2(45^{\circ} - \phi/2)$; $K_p = \tan^2(45^{\circ} + \phi/2)$.

7.0 REFERENCES

The following references were used to generate this report:

- Federal Emergency Management Agency, FEMA Flood Map Service Center – Accessed on 07/24/2025.
- Google Earth Pro (Online) – Accessed on 07/24/2025.
- Historic Aerials by NETR Online - Accessed on 07/24/2025.
- United States Geological Survey (USGS) – Cadwell, D.H., Connally, G.G., Dineen, R.J., Fleisher, P.J., Franzi, D.A., Fuller, M.L., Gurrieri, J.T., Haselton, G.M., Kelley, G.C., LaFleur, R.G., Muller, E.H., Pair, D.L., Rich, J.L., Sirkin, Les, Street, J.S., Young, R.A., and Wiles, G.C. (in alphabetical order), 1991, Surficial geologic map of New York: New York State Museum, Map and Chart Series 40, scale 1:250,000 – Accessed on 07/24/2025.
- United States Geological Survey, Lower 48 States 2014 Seismic Hazard Map – Accessed on 07/24/2025.
- United States Geological Survey (USGS) US Topo 7.5-minute map for White Plains, NY - 2023: USGS - National Geospatial Technical Operations Center (NGTOC) – Accessed on 07/24/2025.
- United States Geologic Survey, Earthquake Hazards Program (Online) – Accessed on 07/24/2025.

Additionally, the following documents were used to prepare this report:

- A site layout plan (Sheet No. C102) – Titled “Edgemont School District High School Field, Site Improvements & Related Work” - Prepared by Barton and Loguidice – Plan dated 06/19/2025.

8.0 LIMITATIONS

The conclusions, recommendations, and opinions in this report are based upon soil samples and data obtained in widely spaced locations accessible at the time of the field exploration and based on project information available at that time. Our findings assume that the samples collected are representative of the general subsurface conditions at the site. If actual site conditions differ from those encountered in our borings, our recommendations should be reviewed and may require revision based on the new data and, if necessary, additional subsurface exploration. It should be noted that geotechnical subsurface evaluations are not capable of identifying and predicting all subsurface conditions with certainty. This evaluation was performed in accordance with generally accepted geotechnical engineering practices in effect at the time of the study. No other warranty or guarantee is expressed or implied.

Likewise, our review of available documents and geological information was conducted in good faith, based on materials accessible and known to us at the time, as listed in this report. However, we cannot guarantee that all relevant site documents or conditions were discovered, reviewed, or considered.

Any changes in the design, location, or configuration of the proposed structure may invalidate the findings and recommendations contained herein. In such cases, we should be notified so that we may review the changes and update our conclusions and recommendations accordingly. If conditions encountered during construction differ from those described in this report, or if unanticipated subsurface features are observed, we should be contacted immediately to evaluate the findings and revise the recommendations where necessary.

If there is a substantial lapse in time between the submission of this report and the start of construction, or if site conditions or the project layout have significantly changed (due to further development of grading plans, natural causes, or construction operations at or adjacent to the site), we recommend that this report be reviewed to determine the applicability of our previous conclusions and recommendations.

This report is intended for the use of the client (Fuller And D'Angelo P.C.) in its entirety and applies solely to the proposed project described in this report. Information from this report is not to be used for other projects or other sites. Partial use or misapplication of this report may result in invalid conclusions. All of the report must be reviewed and applied to the project or else the report recommendations may no longer apply.

If pertinent changes are made in the project plans or conditions encountered during construction that appear to be different than indicated by this report, please contact this office for review. Significant variations may necessitate a re-evaluation of the recommendations presented in this report.

The recommendations and findings in this report are valid for a period of one (1) year from the report date, unless significant site changes occur. This report has been prepared subject to our standard terms and conditions, including limitations of liability, indemnification provisions, and dispute resolution clauses, which are applicable to all parties relying on the information contained herein.



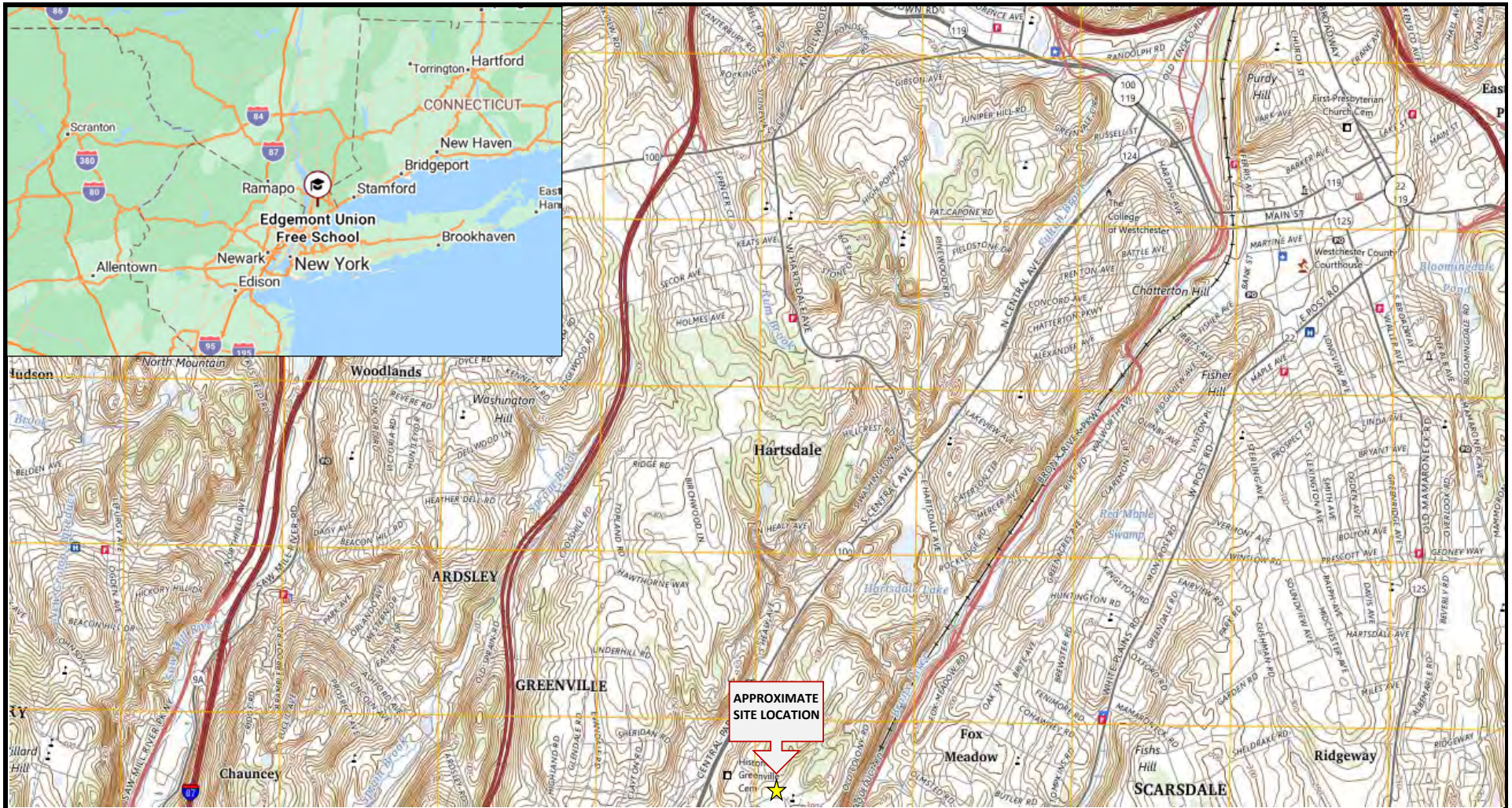
APPENDIX A

Figure 1: Site Vicinity Plan

Figure 2: Approximate Site Location

Figure 3: Approximate Boring Location Plan

Figure 4: Geologic Map



SOURCE: U.S. Geological Survey, USGS US Topo 7.5-minute quadrant map for White Plains, NY - 2023: USGS - National Geospatial Technical Operations Center (NGTOC)®

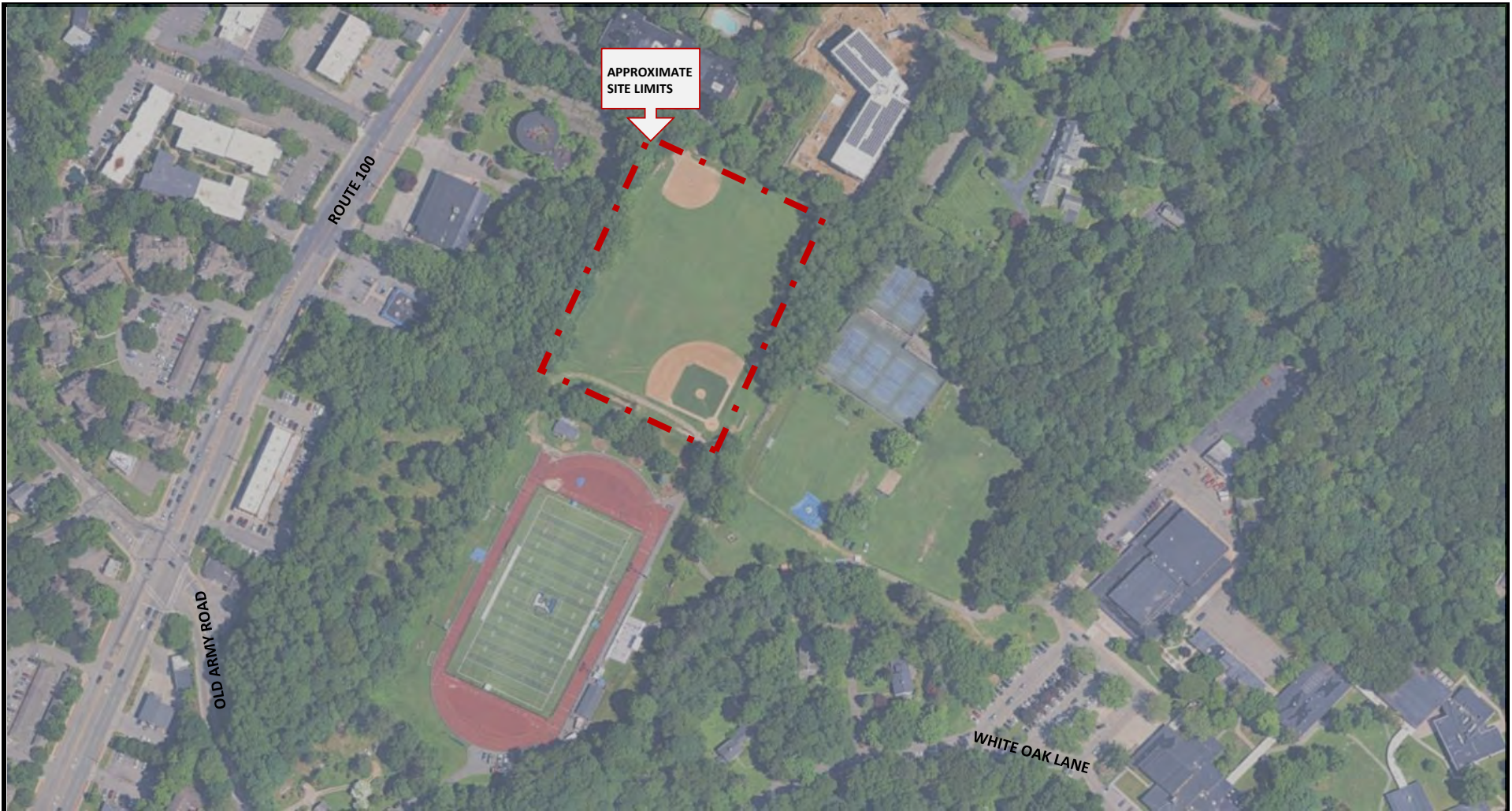
LEGEND:

★ Approximate Site Location

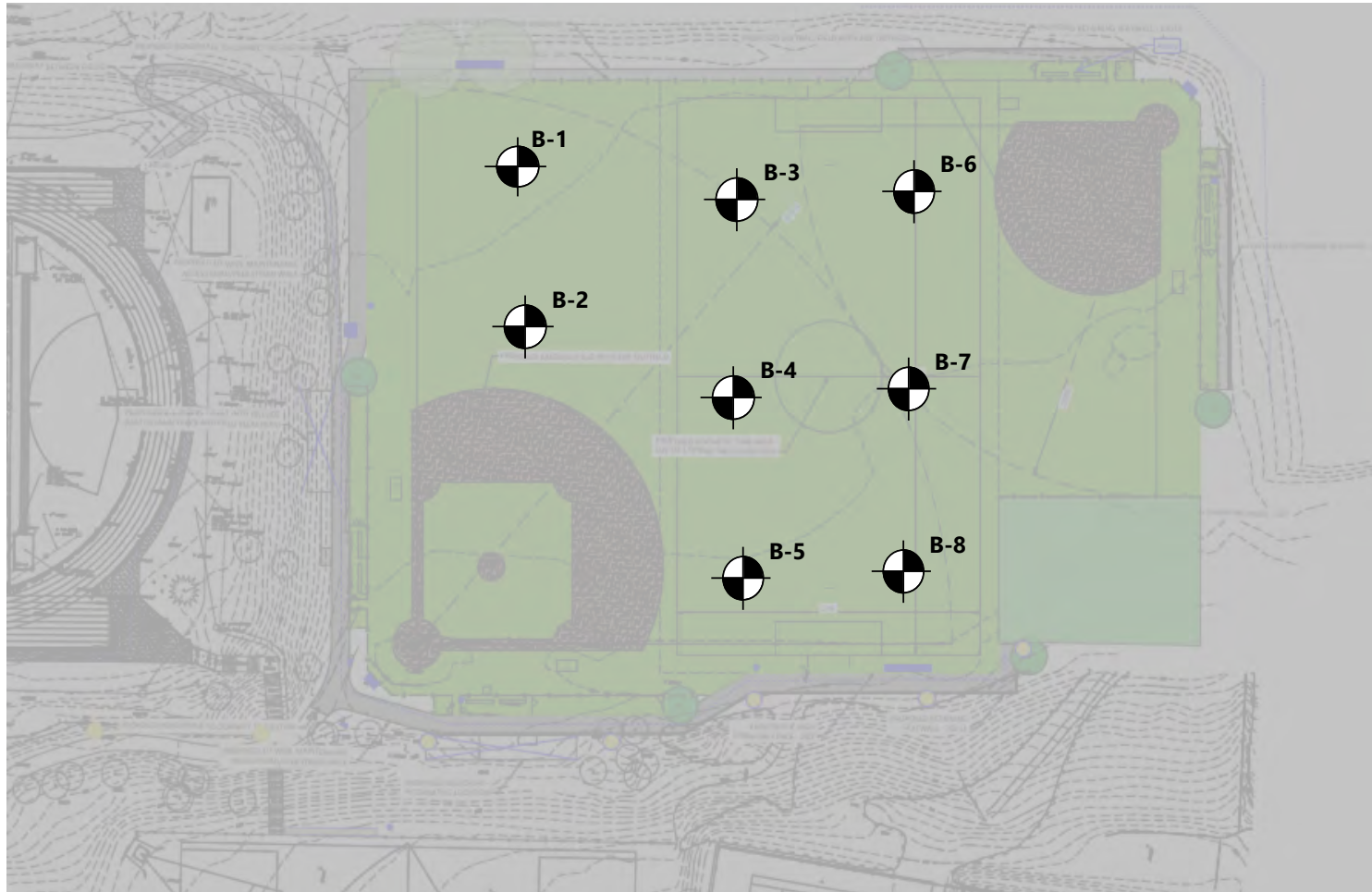
FIGURE 1: Approximate Site Vicinity

PROJECT:	Edgmont School District HS Field Improvement
PROJECT NO.:	25-026
PREPARED FOR:	Fuller And D'Angelo P.C.
SCALE:	N.T.S





SOURCE: Google Map Pro									
LEGEND:  Approximate Site Boundaries									
FIGURE 2: Approximate Site Limits <table> <tr> <td>PROJECT:</td><td>Edmont School District HS Field Improvement</td></tr> <tr> <td>PROJECT NO.:</td><td>25-026</td></tr> <tr> <td>PREPARED FOR:</td><td>Fuller And D'Angelo P.C.</td></tr> <tr> <td>SCALE:</td><td>N.T.S</td></tr> </table>		PROJECT:	Edmont School District HS Field Improvement	PROJECT NO.:	25-026	PREPARED FOR:	Fuller And D'Angelo P.C.	SCALE:	N.T.S
PROJECT:	Edmont School District HS Field Improvement								
PROJECT NO.:	25-026								
PREPARED FOR:	Fuller And D'Angelo P.C.								
SCALE:	N.T.S								
									



SOURCE: A site layout plan (Sheet No. C102) – Titled “Edgemont School District High School Field, Site Improvements & Related Work” - Prepared by Barton and Loguidice – Plan dated 06/19/2025

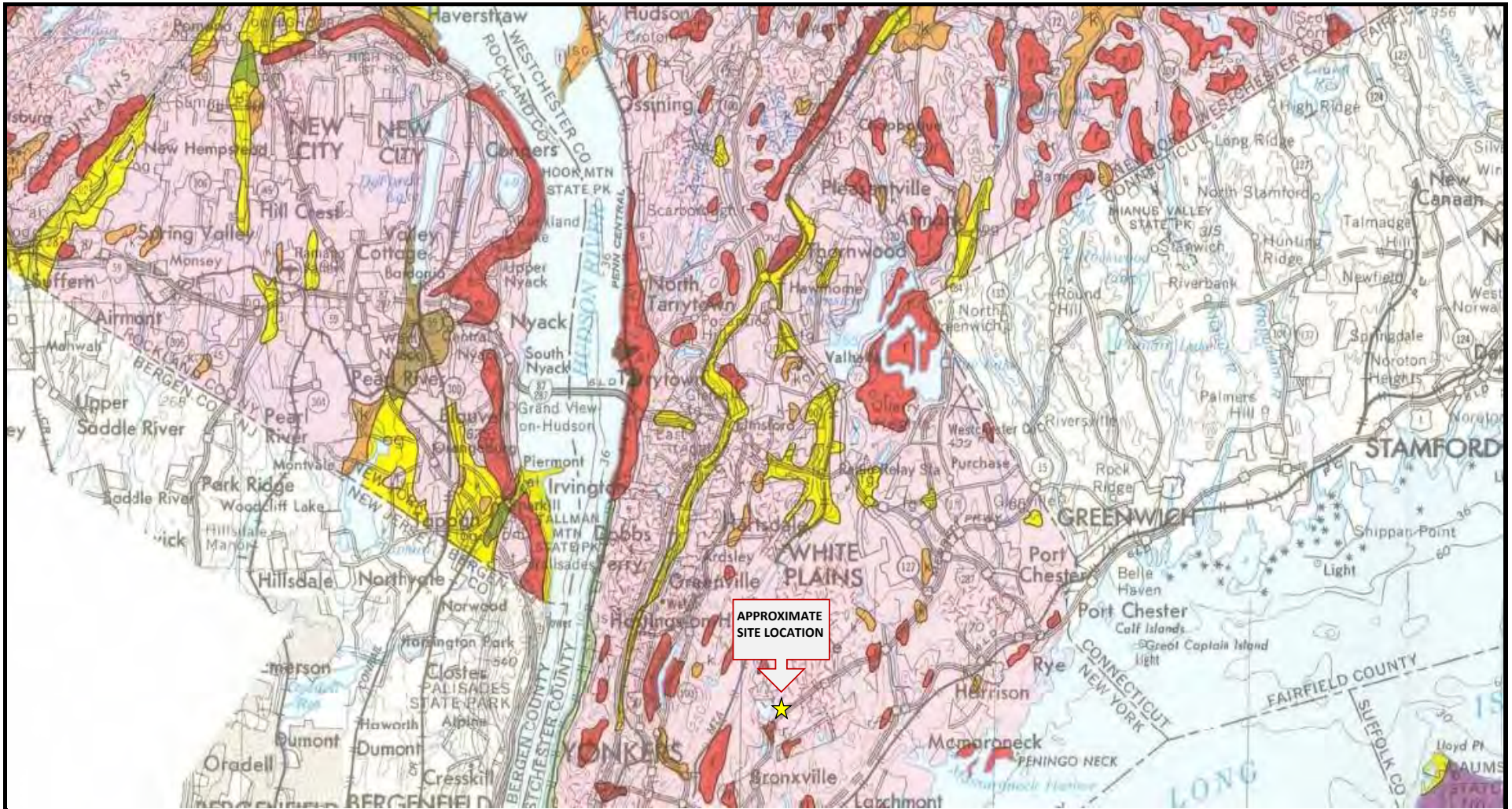
LEGEND:

⊕ Approximate Boring Location

FIGURE 3: Approximate Boring Location Plan

PROJECT:	Edgemont School District HS Field Improvement
PROJECT NO.:	25-026
PREPARED FOR:	Fuller And D'Angelo P.C.
SCALE:	N.T.S





SOURCE:	USGS – United States Geological Survey (USGS) – Cadwell, D.H., Connally, G.G., Dineen, R.J., Fleisher, P.J., Franz, D.A., Fuller, M.L., Gurrieri, J.T., Haselton, G.M., Kelley, G.C., LaFleur, R.G., Muller, E.H., Pair, D.L., Rich, J.L., Sirkin, Les, Street, J.S., Young, R.A., and Wiles, G.C. (in alphabetical order), 1991, Surficial geologic map of New York: New York State Museum, Map and Chart Series 40, scale 1:250,000										
LEGEND:											
 Approximate Site Boundaries	<table border="1"> <tr> <td data-bbox="1123 1250 1291 1282">FIGURE 4:</td><td data-bbox="1291 1250 2005 1282">Geologic Map</td></tr> <tr> <td data-bbox="1123 1282 1291 1307">PROJECT:</td><td data-bbox="1291 1282 2005 1307">Edgmont School District HS Field Improvement</td></tr> <tr> <td data-bbox="1123 1307 1291 1331">PROJECT NO.:</td><td data-bbox="1291 1307 2005 1331">25-026</td></tr> <tr> <td data-bbox="1123 1331 1291 1356">PREPARED FOR:</td><td data-bbox="1291 1331 2005 1356">Fuller And D'Angelo P.C.</td></tr> <tr> <td data-bbox="1123 1356 1291 1388">SCALE:</td><td data-bbox="1291 1356 2005 1388">N.T.S</td></tr> </table> 	FIGURE 4:	Geologic Map	PROJECT:	Edgmont School District HS Field Improvement	PROJECT NO.:	25-026	PREPARED FOR:	Fuller And D'Angelo P.C.	SCALE:	N.T.S
FIGURE 4:	Geologic Map										
PROJECT:	Edgmont School District HS Field Improvement										
PROJECT NO.:	25-026										
PREPARED FOR:	Fuller And D'Angelo P.C.										
SCALE:	N.T.S										



APPENDIX B

Boring Logs (Prepared by Soiltesting, Inc.)

SOILTESTING, INC. 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850		CLIENT: <u>Fuller & D'Angelo Architects</u>		SHEET <u>1</u> OF <u>1</u>								
		PROJECT NO. <u>G142-3218-25</u>		HOLE NO. <u>B-1</u>								
FOREMAN - DRILLER AK/pvd		PROJECT NAME Edgemont School District HS Field Improvements		BORING LOCATIONS Per Plan								
INSPECTOR		LOCATION 300 White Oak Lane Scarsdale NY										
GROUND WATER OBSERVATIONS AT <u>none</u> FT AFTER <u>0</u> HOURS AT <u> </u> FT AFTER <u> </u> HOURS		TYPE SIZE I.D. HAMMER WT. HAMMER FALL		CASING SAMPLER CORE BAR OFFSET DATE START DATE FINISH SURFACE ELEV. GROUND WATER ELEV.								
DEPTH	CASING BLOWS PER FOOT	SAMPLE					BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE)		CORE TIME PER FT (MIN)	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		NO	Type	PEN	REC	DEPTH @ BOT	0 - 6	6 - 12				
5		1	ss	24"	12"	2'0"	3	3		loose dry compact dry dry	4"	Topsoil Brn F-C SAND & SILT, trace gravel Brn F-M SAND, trace silt Same
		2	ss	24"	18"	4'0"	5	6			2'	
						6	6					
		3	ss	24"	18"	6'0"	9	4				
10							4	4		dry	12'	Same EOB 12'
		4	ss	24"	19"	12'0"	4	3				
15							3	3				
20												
25												
30												
35												
40												

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO _____ FT. USED _____ CASING THEN _____ CASING TO _____ FT.		HOLE NO. B-1
A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND = 35 - 50% F = FINE		

SOILTESTING, INC. 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850	CLIENT: Fuller & D'Angelo Architects			SHEET 1 OF 1	
	PROJECT NO. G142-3218-25			HOLE NO. B-2	
FOREMAN - DRILLER AK/pvd	PROJECT NAME Edgemont School District HS Field Improvements			BORING LOCATIONS Per Plan	
	LOCATION 300 White Oak Lane Scarsdale NY				
INSPECTOR	CASING	SAMPLER	CORE BAR	OFFSET	
	HSA	SS			
GROUND WATER OBSERVATIONS AT <u>none</u> FT AFTER <u>0</u> HOURS AT <u> </u> FT AFTER <u> </u> HOURS	TYPE	SIZE I.D.		DATE START 6/20/25	
	HAMMER WT.			DATE FINISH 6/20/25	
	HAMMER FALL			SURFACE ELEV.	
				GROUND WATER ELEV.	

DEPTH	CASING BLOWS PER FOOT	SAMPLE					BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0 - 6 6 - 12 12- 18			CORE TIME PER FT (MIN)	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		NO	Type	PEN	REC.	DEPTH @ BOT							
							MOIST	ELEV					
5		1	ss	24"	14"	2'0"	2	3		compact	4"	Topsoil	
							9	9		dry		Brn F-M SAND, some silt	
		2	ss	24"	20"	4'0"	6	7		compact			
							7	6		dry		Brn F-M SAND, some silt, trace gravel (poss Fill)	
		3	ss	24"	18"	6'0"	8	8		compact	5'		
10							9	7		dry	6'	Blk organic SILT	
												Brn F-M SAND, trace silt	
		4	ss	24"	20"	12'0"	9	14		dense			
							19	22		dry	12'	Lt brn F-M SAND, trace gravel	
15												EOB 12'	
20													
25													
30													
35													
40													

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO _____ FT. USED _____ CASING THEN _____ CASING TO _____ FT. HOLE NO. **B-2**

A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST

WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS

SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER

PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND = 35 - 50%

C = COARSE
M = MEDIUM
F = FINE

SOILTESTING, INC. 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850						CLIENT: Fuller & D'Angelo Architects						SHEET 1 OF 1 HOLE NO. B-3							
						PROJECT NO. G142-3218-25													
						PROJECT NAME Edgemont School District HS Field Improvements						BORING LOCATIONS Per Plan							
FOREMAN - DRILLER AK/pvd						LOCATION 300 White Oak Lane Scarsdale NY													
INSPECTOR						TYPE CASING HSA SAMPLER SS CORE BAR						OFFSET							
						SIZE I.D. 4 1/4" 1 3/8"						DATE START 6/20/25							
						HAMMER WT. 140# BIT						DATE FINISH 6/20/25							
						HAMMER FALL 30"						SURFACE ELEV.							
GROUND WATER OBSERVATIONS												GROUND WATER ELEV.							
AT none FT AFTER 0 HOURS																			
AT none FT AFTER 24 HOURS																			
SAMPLE						BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0 - 6 6 - 12 12 - 18						CORE TIME PER FT (MIN)		DENSITY OR CONSIST		STRATA CHANGE DEPTH		FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.	
DEPTH		CASING BLOWS PER FOOT	NO	Type	PEN	REC	DEPTH @ BOT					MOIST	ELEV						
5			1	ss	24"	20"	2'0"	2	3			loose	4"	Topsoil					
								4	6			dry		Brn F-M SAND, trace F gravel					
			2	ss	24"	24"	4'0"	8	11			compact		Same					
								11	9			dry		Same					
10			3	ss	24"	20"	6'0"	5	3			loose		Same					
								3	3			dry							
15																			
20																			
25																			
30																			
35																			
40																			
NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.																			
GROUND SURFACE TO _____ FT. USED _____ CASING THEN _____ CASING TO _____ FT. [HOLE NO. B-3]																			
A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST C = COARSE																			
WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS M = MEDIUM																			
SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER F = FINE																			
PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND = 35 - 50%																			

SOILTESTING, INC. 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850	CLIENT: Fuller & D'Angelo Architects			SHEET 1 OF 1	
	PROJECT NO. G142-3218-25			HOLE NO. B-4	
	PROJECT NAME Edgemont School District HS Field Improvements			BORING LOCATIONS Per Plan	
FOREMAN - DRILLER AK/pvd	LOCATION 300 White Oak Lane Scarsdale NY				
INSPECTOR	CASING	SAMPLER	CORE BAR	OFFSET	
	TYPE HSA	SS		DATE START 6/23/25	
GROUND WATER OBSERVATIONS AT <u>none</u> FT AFTER <u>0</u> HOURS AT <u>none</u> FT AFTER <u>24</u> HOURS	SIZE I.D.	4 1/4"	1 3/8"	DATE FINISH 6/23/25	
	HAMMER WT.		140#	SURFACE ELEV.	
	HAMMER FALL		30"	GROUND WATER ELEV.	

DEPTH	CASING BLOWS PER FOOT	SAMPLE					BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0 - 6 6 - 12 12 - 18			CORE TIME PER FT (MIN)	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		NO	Type	PEN	REC	DEPTH @ BOT							
											MOIST	ELEV	
5		1	ss	24"	20"	2'0"	2	2			loose	3"	Topsoil
							4	4			dry		Lt brn F-C SAND, some silt, lit F-C gravel
		2	ss	24"	20"	4'0"	4	4			loose		Lt brn F-C SAND, lit silt, lit gravel
							5	6			dry		Lt brn F-M SAND & SILT, lit C sand, F gravel
		3	ss	24"	18"	6'0"	2	3			loose		
10							3	2			moist		
15													
20													
25													
30													
35													
40													

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO _____ FT.	USED _____ CASING	THEN _____ CASING TO _____ FT.	HOLE NO. B-4
A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST			
WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS			
SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER			
PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND = 35 - 50%			
C = COARSE M = MEDIUM F = FINE			

SOILTESTING, INC. 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850		CLIENT: <u>Fuller & D'Angelo Architects</u>		SHEET <u>1</u> OF <u>1</u>	
		PROJECT NO. <u>G142-3218-25</u>		HOLE NO. <u>B-5</u>	
PROJECT NAME <u>Edgemont School District HS Field Improvements</u>		BORING LOCATIONS Per Plan			
FOREMAN - DRILLER <u>AK/pvd</u>		LOCATION <u>300 White Oak Lane Scarsdale NY</u>			
INSPECTOR		CASING <u>HSA</u> SAMPLER <u>SS</u> CORE BAR		OFFSET	
GROUND WATER OBSERVATIONS AT <u>5</u> FT AFTER <u>0</u> HOURS		TYPE <u>HSA</u> SIZE I.D. <u>4 1/4"</u>		DATE START <u>6/24/25</u>	
AT <u>none</u> FT AFTER <u>24</u> HOURS		HAMMER WT. <u>140#</u> HAMMER FALL <u>30"</u>		DATE FINISH <u>6/24/25</u>	
				SURFACE ELEV.	
				GROUND WATER ELEV.	

DEPTH	CASING BLOWS PER FOOT	SAMPLE				DEPTH @ BOT	BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0 - 6 6 - 12 12 - 18				CORE TIME PER FT (MIN)	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		NO	Type	PEN	REC		MOIST	ELEV						
5		1	ss	24"	13	2'0"	2	6			compact	3"	Topsoil Brn F-M SAND & SILT, C sand, F gravel Same Lt brn F-M SAND, some C sand, F gravel, silt	
						12	5			dry				
		2	ss	24"	1"	4'0"	2	2			loose			
		3	ss	24"	20"	6'0"	2	3			dry			
10							6	12			compact		Bedrock EOB 10'3"	
						12	17			dry				
		4	ss	3"	3"	10'3"	50/3"				dense/dry	10'		
15													Installed 1" SCH 40 PVC Observation Well with 5' screen to 10' depth	
20														
25														
30														
35														
40														

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO _____ FT. USED _____ CASING THEN _____ CASING TO _____ FT.		HOLE NO. <u>B-5</u>
A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND = 35 - 50%		
		C = COARSE M = MEDIUM F = FINE

SOILTESTING, INC. 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850	CLIENT: Fuller & D'Angelo Architects			SHEET <u>1</u> OF <u>1</u>	
	PROJECT NO. G142-3218-25			HOLE NO. B-6	
	PROJECT NAME Edgemont School District HS Field Improvements			BORING LOCATIONS Per Plan	
FOREMAN - DRILLER AK/pvd	LOCATION 300 White Oak Lane Scarsdale NY				
INSPECTOR	TYPE SIZE I.D. HAMMER WT. HAMMER FALL	CASING HSA	SAMPLER SS	CORE BAR	OFFSET
GROUND WATER OBSERVATIONS					DATE START 6/23/25
AT <u>none</u> FT AFTER <u>0</u> HOURS					DATE FINISH 6/23/25
AT <u> </u> FT AFTER <u> </u> HOURS				BIT	SURFACE ELEV.
					GROUND WATER ELEV.

DEPTH	CASING BLOWS PER FOOT	SAMPLE					BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0 - 6 6 - 12 12 - 18			CORE TIME PER FT (MIN)	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		NO	Type	PEN	REC.	DEPTH @ BOT					MOIST	ELEV	
5		1	ss	24"	13"	2'0"	3	6			compact dry	3"	Topsoil
							6	6					Brn F-M SAND, some silt, lit C sand
		2	ss	24"	2"	4'0"	6	9			compact dry		Same
		3	ss	24"	24"	6'0"	3	4			compact moist		Brn F-M SAND, some silt, C sand
10							5	4					
							5	6			compact moist		
		4	ss	24"	22"	12'0"	7	8				12'	Lt brn F-M SAND
													EOB 12'
15													
20													
25													
30													
35													
40													

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO _____ FT.	USED _____ CASING	THEN _____ CASING TO _____ FT.	HOLE NO. B-6
A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND = 35 - 50%			
			C = COARSE M = MEDIUM F = FINE

SOILTESTING, INC. 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850	CLIENT: Fuller & D'Angelo Architects		SHEET <u>1</u> OF <u>1</u>	
	PROJECT NO. G142-3218-25		HOLE NO. B-7	
	PROJECT NAME Edgemont School District HS Field Improvements		BORING LOCATIONS Per Plan	
FOREMAN - DRILLER AK/pvd	LOCATION 300 White Oak Lane Scarsdale NY			
INSPECTOR	CASING	SAMPLER	CORE BAR	OFFSET
	TYPE HSA	SS		DATE START 6/24/25
GROUND WATER OBSERVATIONS AT <u>none</u> FT AFTER <u>0</u> HOURS	SIZE I.D. 4 1/4"	1 3/8"		DATE FINISH 6/24/25
AT <u> </u> FT AFTER <u> </u> HOURS	HAMMER WT. 140#	BIT		SURFACE ELEV.
	HAMMER FALL 30"			GROUND WATER ELEV.

DEPTH	CASING BLOWS PER FOOT	SAMPLE					BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0 - 6 6 - 12 12 - 18			CORE TIME PER FT (MIN)	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		NO	Type	PEN	REC	DEPTH @ BOT							
											MOIST	ELEV	
5		1	ss	24"	18"	2'0"	2	3			loose	3"	Topsoil
							5	7			dry		Brn/lt brn F-M SAND, trace silt, C sand
		2	ss	24"	16"	4'0"	5	7			compact		
							6	4			dry		Lt brn F-M SAND, C sand, F gravel, trace silt
		3	ss	24"	24"	6'0"	2	2			compact		
10							9	9			dry		Lt brn F-M SAND, C sand, lit F-C gravel, trace silt
15													
20													
25													
30													
35													
40													

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO _____ FT.	USED _____ CASING	THEN _____ CASING TO _____ FT.	HOLE NO. B-7
A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST			
WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS			
SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER			
PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND = 35 - 50%			
C = COARSE M = MEDIUM F = FINE			

SOILTESTING, INC. 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850	CLIENT: Fuller & D'Angelo Architects		SHEET <u>1</u> OF <u>1</u>	
	PROJECT NO. G142-3218-25		HOLE NO. B-8	
	PROJECT NAME Edgemont School District HS Field Improvements		BORING LOCATIONS Per Plan	
FOREMAN - DRILLER AK/pvd	LOCATION 300 White Oak Lane Scarsdale NY			
INSPECTOR	TYPE SIZE I.D. HAMMER WT. HAMMER FALL	CASING HSA	SAMPLER SS	CORE BAR
GROUND WATER OBSERVATIONS AT <u>none</u> FT AFTER <u>0</u> HOURS AT <u> </u> FT AFTER <u> </u> HOURS				OFFSET
				DATE START 6/24/25
				DATE FINISH 6/24/25
				SURFACE ELEV.
				GROUND WATER ELEV.

DEPTH	CASING BLOWS PER FOOT	SAMPLE					BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0 - 6 6 - 12 12- 18			CORE TIME PER FT (MIN)	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		NO	Type	PEN	REC.	DEPTH @ BOT							
											MOIST	ELEV	
5		1	ss	24"	24"	2'0"	2	2			loose	3"	Topsoil
							3	3			dry		Brn F-M SAND & SILT, lit gravel
		2	ss	24"	24"	4'0"	4	8			compact		Lt brn F-C SAND, some silt, lit gravel
		3	ss	24"	18"	6'0"	7	10			compact		Lt brn F-M SAND, some C sand, F gravel, lit silt
							12	11			dry		
10													
		4	ss	1"	1"	10'1"	50	1"			dense/dry	10'1"	Bedrock fragments
15													EOB 10'1"
20													
25													
30													
35													
40													

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO _____ FT.	USED _____ CASING	THEN _____ CASING TO _____ FT.	HOLE NO. B-8
A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND = 35 - 50%			
		C = COARSE	
		M = MEDIUM	
		F = FINE	



APPENDIX C

Infiltration Test Results

[illegible]

[illegible]

[illegible]

[illegible]

[illegible]



APPENDIX D

Laboratory Test Results

USCS based on dilatancy & plasticity per ASTM D2488 if no limits data and % fines > 5%, and estimated D10 if necessary.

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0.0	4.3	4.8	3.0	17.3	33.4	37.2

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1	100.0		
.75	95.7		
.375	92.9		
#4	90.9		
#10	87.9		
#20	81.8		
#40	70.6		
#60	56.5		
#100	45.5		
#200	37.2		

* (no specification provided)

Soil Description		
Dark orange-brown clayey sand		
Atterberg Limits		
PL=	LL=	PI=
Coefficients		
D ₉₀ = 3.4753	D ₈₅ = 1.2081	D ₆₀ = 0.2860
D ₅₀ = 0.1891	D ₃₀ =	D ₁₅ =
D ₁₀ =	C _u =	C _c =
Classification		
USCS= SC	AASHTO=	
Remarks		

Source of Sample: B-1 Depth: 0-2 ft.
Sample Number: S-1

Date:

SKYLANDS TESTING, LLC

Sparta, NJ

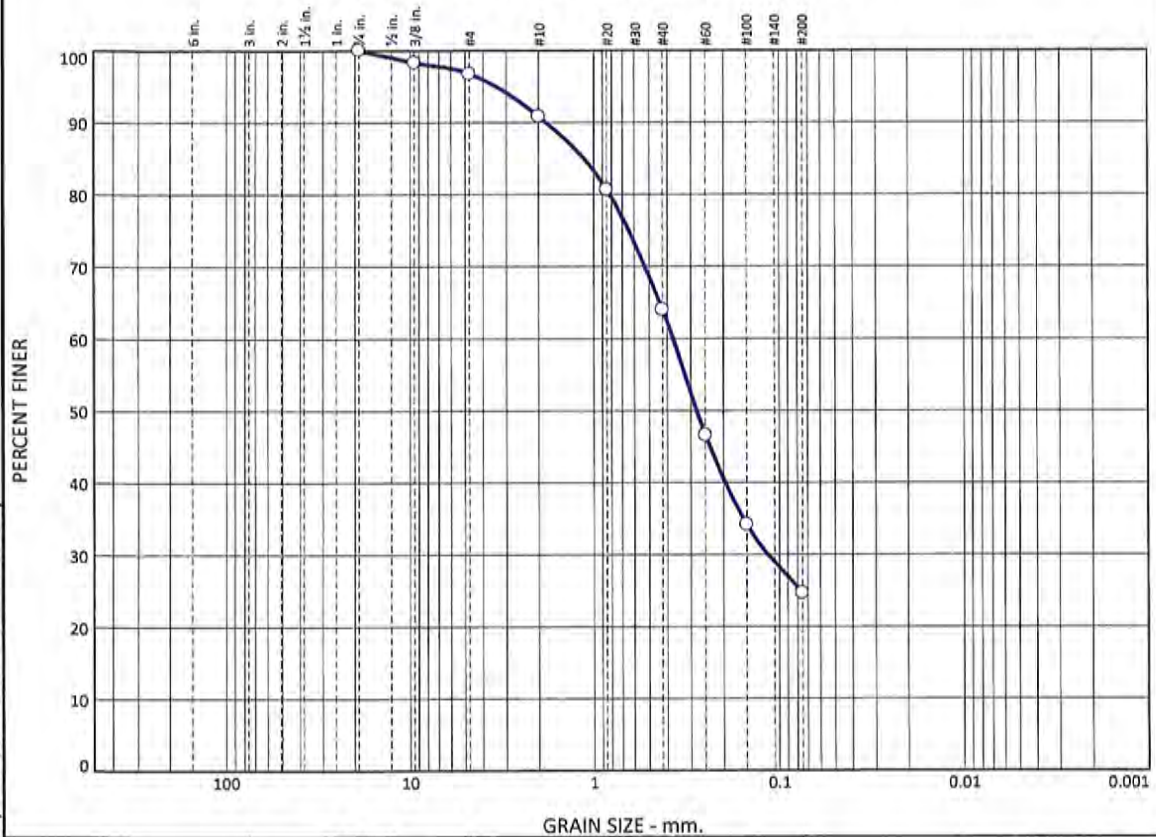
Client: Soiltesting, Inc.
Project: Edgemont School District
Scarsdale, NY

Project No: 3218

Report Date: 7-14-2025

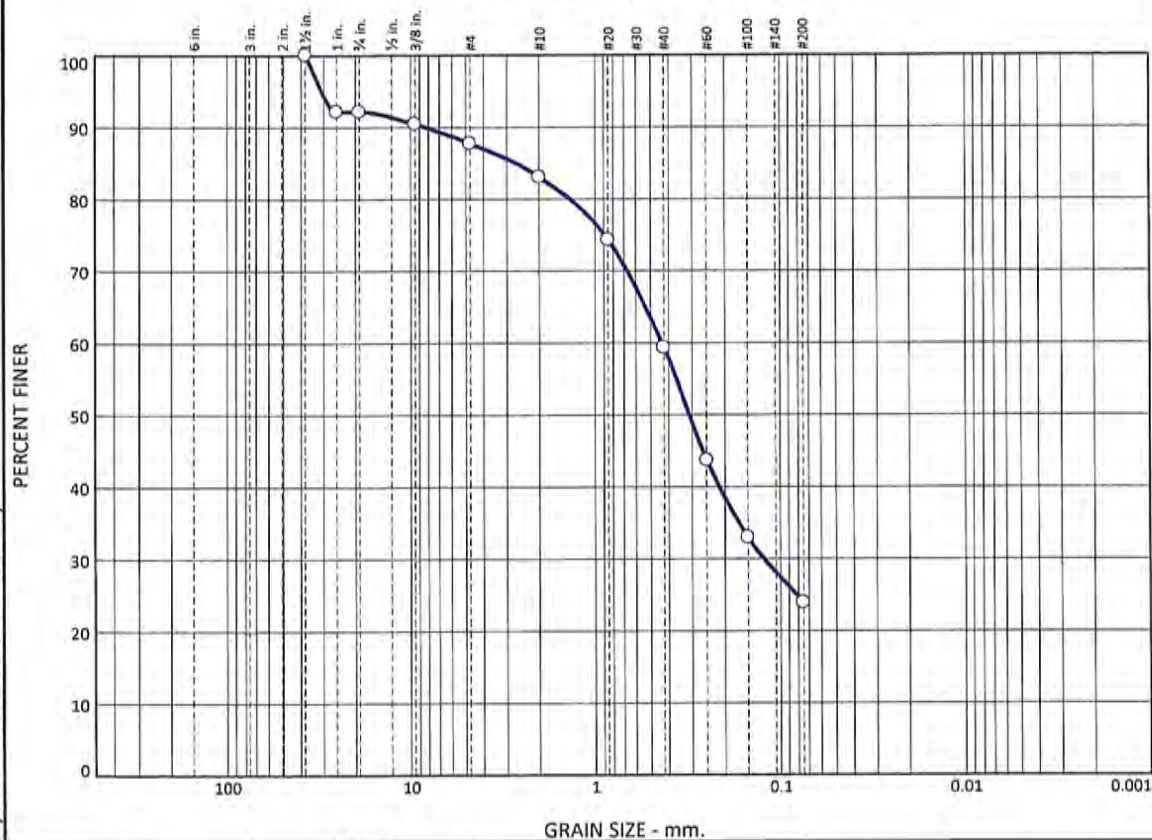
USCS based on dilatancy & plasticity per ASTM D2488 if no limits data and % fines > 5%, and estimated D10 if necessary.

Particle Size Distribution Report



USCS based on dilatancy & plasticity per ASTM D2488 if no limits data and % fines > 5%, and estimated D10 if necessary.

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0.0	7.9	4.4	4.7	23.7	35.5	23.8

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1.5	100.0		
1	92.1		
.75	92.1		
.375	90.4		
#4	87.7		
#10	83.0		
#20	74.2		
#40	59.3		
#60	43.6		
#100	32.9		
#200	23.8		

* (no specification provided)

Soil Description

Dark orange-brown clayey sand

PL= Atterberg Limits PI=

LL=

Coefficients

D₉₀= 8.4420 D₈₅= 2.6999 D₆₀= 0.4371

D₅₀= 0.3116 D₃₀= 0.1237 D₁₅=

D₁₀= C_u= C_c=

Classification

USCS= SC AASHTO=

Remarks

Source of Sample: B-4 Depth: 0-2 ft.
Sample Number: S-1

Date:

SKYLANDS TESTING, LLC

Sparta, NJ

Client: Soiltesting, Inc.
Project: Edgemont School District
Scarsdale, NY
Project No: 3218

Report Date: 7-14-2025

USCS based on dilatancy & plasticity per ASTM D2488 if no limits data and % fines > 5%, and estimated D10 if necessary.

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0.0	0.0	13.6	6.7	24.5	35.1	20.1

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
.75	100.0		
.375	92.5		
#4	86.4		
#10	79.7		
#20	69.7		
#40	55.2		
#60	39.7		
#100	28.3		
#200	20.1		

* (no specification provided)

Soil Description
 Brown silty sand

Atterberg Limits
 PL= LL= PI=

Coefficients
 D₉₀= 7.2633 D₈₅= 3.9443 D₆₀= 0.5199
 D₅₀= 0.3539 D₃₀= 0.1646 D₁₅=
 D₁₀= C_u= C_c=

Classification
 USCS= SM AASHTO=

Remarks

Source of Sample: B-4 Depth: 2-4 ft.
 Sample Number: S-2

Date:

SKYLANDS TESTING, LLC

Sparta, NJ

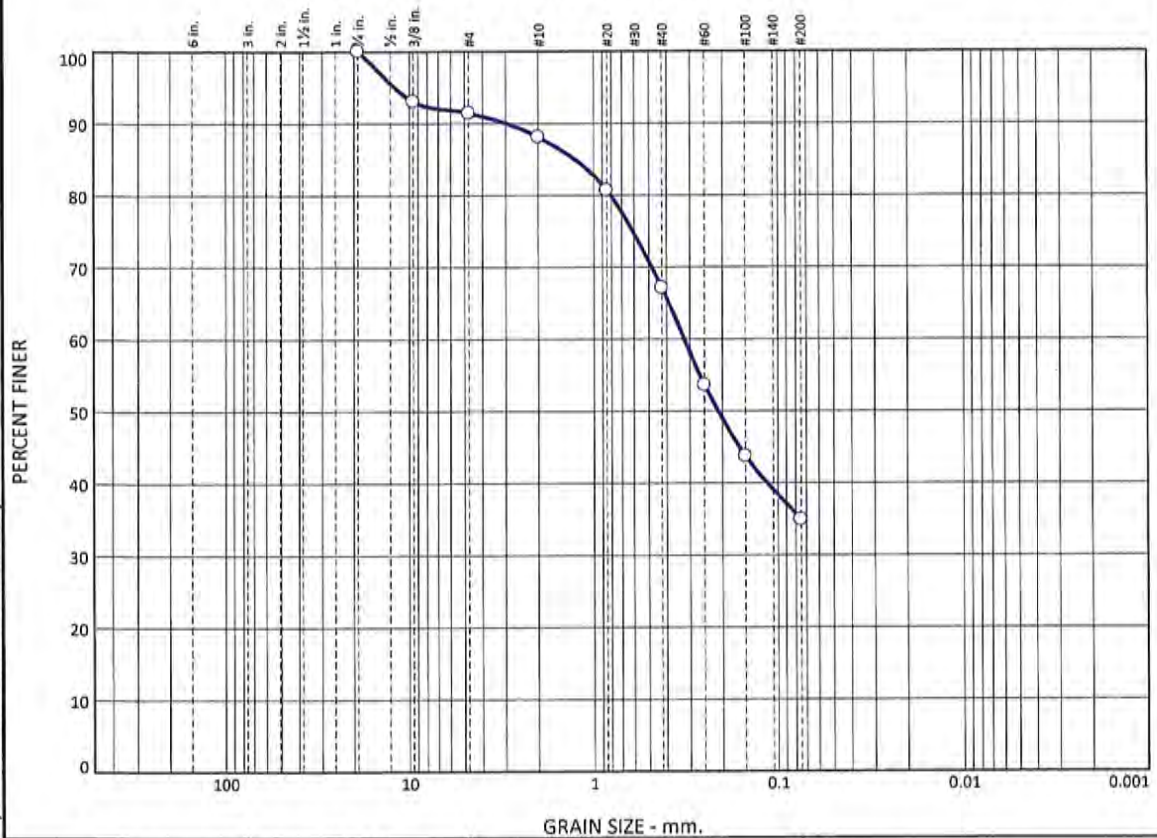
Client: Soiltesting, Inc.
 Project: Edgemont School District
 Scarsdale, NY

Project No: 3218

Report Date: 7-14-2025

USCS based on dilatancy & plasticity per ASTM D2488 if no limits data and % fines > 5%, and estimated D10 if necessary.

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0.0	0.0	8.6	3.4	20.9	32.3	34.8

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
.75	100.0		
.375	93.0		
#4	91.4		
#10	88.0		
#20	80.7		
#40	67.1		
#60	53.6		
#100	43.7		
#200	34.8		

* (no specification provided)

Soil Description		
Brown silty sand		
Atterberg Limits		
PL=	LL=	PI=
Coefficients		
D ₉₀ = 3.0228	D ₈₅ = 1.2725	D ₆₀ = 0.3222
D ₅₀ = 0.2118	D ₃₀ =	D ₁₅ =
D ₁₀ =	C _u =	C _c =
Classification		
USCS= SM	AASHTO=	
Remarks		

Source of Sample: B-8
Sample Number: S-1

Depth: 0-2 ft.

Date:

SKYLANDS TESTING, LLC

Sparta, NJ

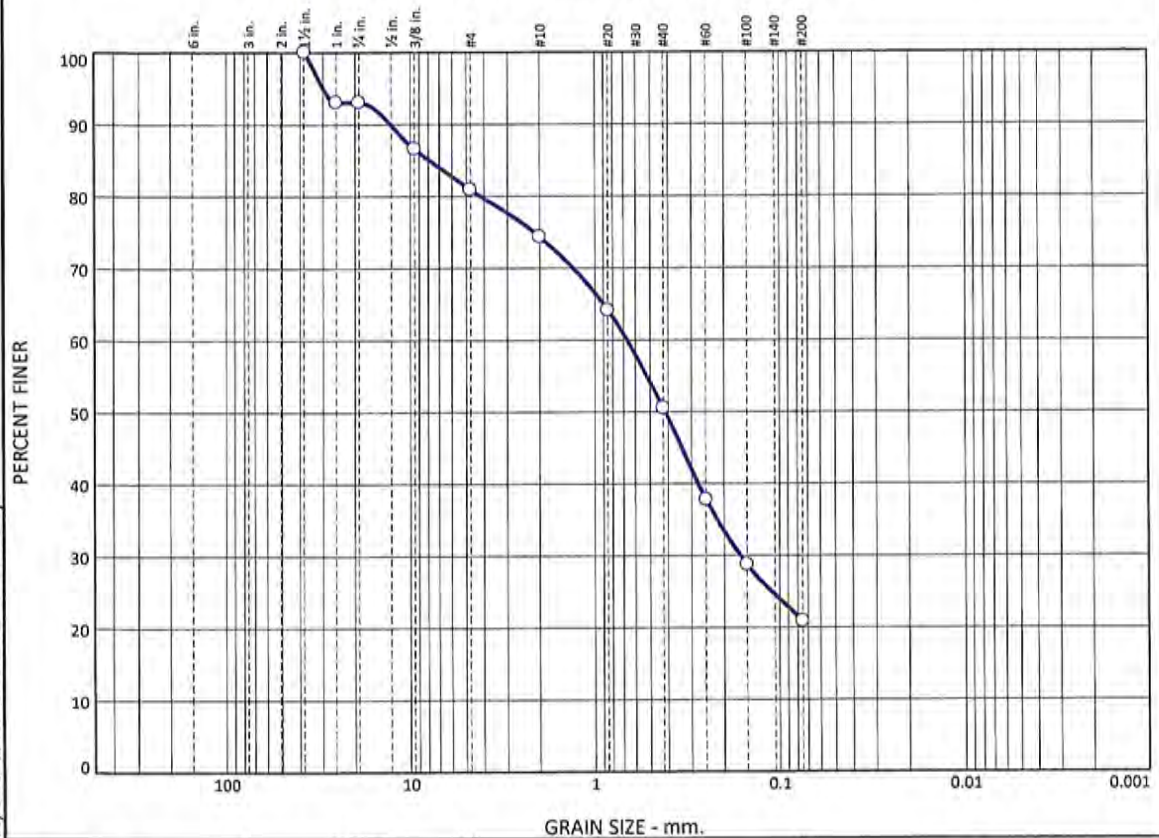
Client: Soiltesting, Inc.
Project: Edgemont School District
Scarsdale, NY

Project No: 3218

Report Date: 7-14-2025

USCS based on dilatancy & plasticity per ASTM D2488 if no limits data and % fines > 5%, and estimated D10 if necessary.

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0.0	7.0	12.1	6.6	23.8	29.7	20.8

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1.5	100.0		
1	93.0		
.75	93.0		
.375	86.5		
#4	80.9		
#10	74.3		
#20	64.1		
#40	50.5		
#60	37.7		
#100	28.7		
#200	20.8		

* (no specification provided)

Soil Description
Brown silty sand with gravel

Atterberg Limits
PL= LL= PI=

Coefficients
D₉₀= 12.9054 D₈₅= 7.9620 D₆₀= 0.6724
D₅₀= 0.4164 D₃₀= 0.1640 D₁₅=
D₁₀= C_u= C_c=

Classification
USCS= SM AASHTO=

Remarks

Source of Sample: B-8
Sample Number: S-2

Depth: 2-4 ft.

Date:

SKYLANDS TESTING, LLC

Sparta, NJ

Client: Soiltesting, Inc.
Project: Edgemont School District
Scarsdale, NY

Project No: 3218

Report Date: 7-14-2025

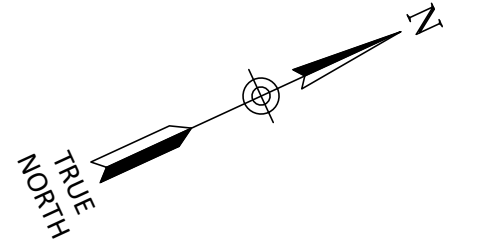
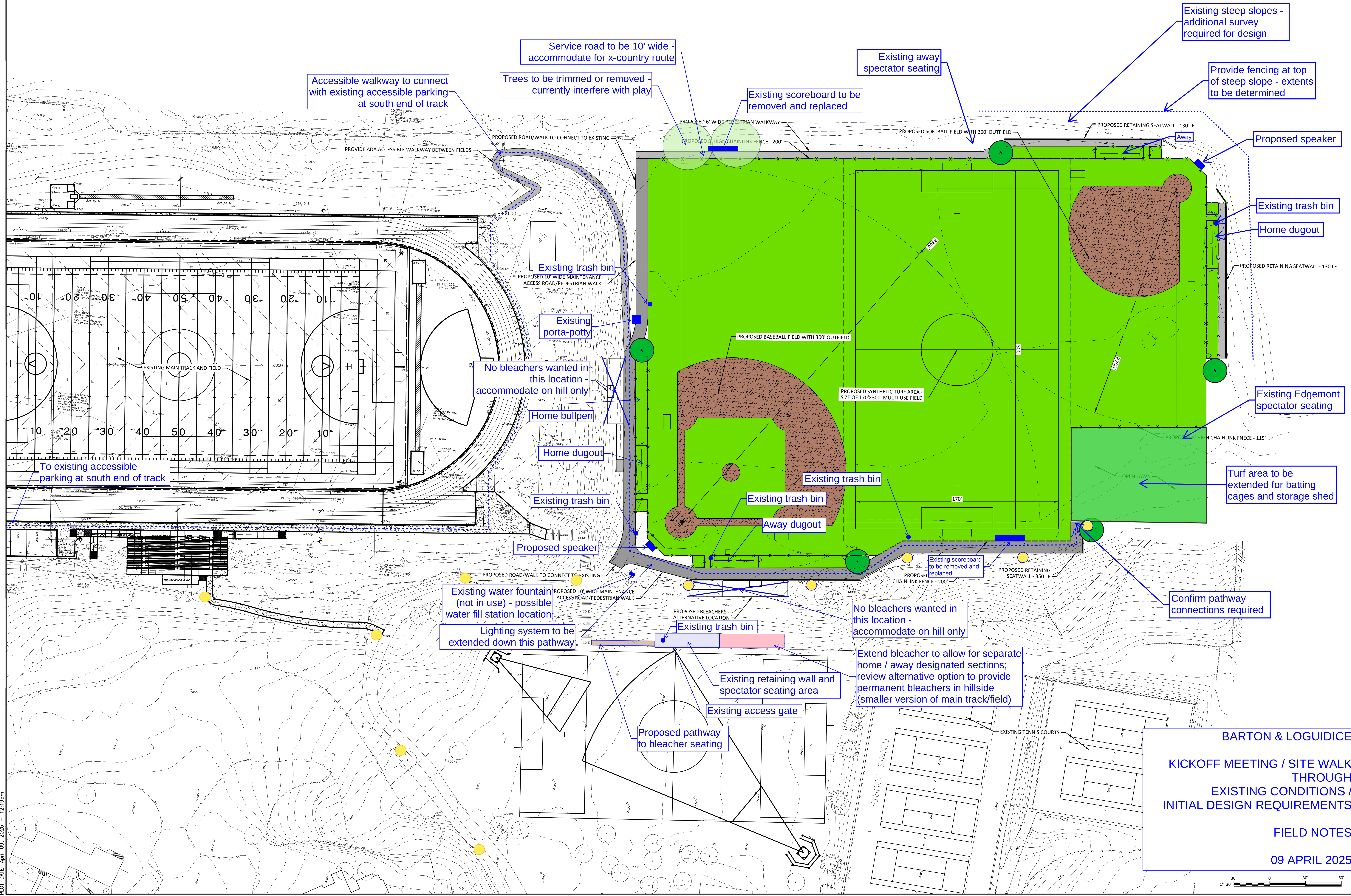


APPENDIX E

Provided Documents by The Client

PLOT DATE: April 09, 2025 - 12:19pm

FILE: Z:\BL-Vault\ID2\18217AD2-1C71-4823-8927-99D5C4054147\0\3335000-3335999\3335707\L\L\2062010 C100 Site Plans_EHS Fields (ID 3335707).dwg



Existing steep slopes - additional survey required for design

Service road to be 10' wide - accommodate for x-country route

Accessible walkway to connect with existing accessible parking at south end of track

Trees to be trimmed or removed - currently interfere with play

Existing scoreboard to be removed and replaced

Existing away spectator seating

Provide fencing at top of steep slope - extents to be determined

Proposed speaker

Existing trash bin

Home dugout

Existing Edgemont spectator seating

Turf area to be extended for batting cages and storage shed

Confirm pathway connections required

No bleachers wanted in this location - accommodate on hill only

Extend bleacher to allow for separate home / away designated sections; review alternative option to provide permanent bleachers in hillside (smaller version of main track/field)

Existing retaining wall and spectator seating area

Existing access gate

Proposed pathway to bleacher seating

Existing water fountain (not in use) - possible water fill station location

Lighting system to be extended down this pathway

Proposed speaker

Existing trash bin

Home dugout

Home bullpen

No bleachers wanted in this location - accommodate on hill only

Existing porta-potty

Existing trash bin

Proposed 10' wide maintenance access road/pedestrian walk

Proposed 6' wide pedestrian walkway

Proposed 8' high chainlink fence - 200'

Proposed softball field with 200' outfield

Proposed retaining seatwall - 130 LF

Away

Proposed retaining seatwall - 130 LF

Proposed baseball field with 300' outfield

Proposed synthetic turf area - size of 170'x300' multi-use field

Existing trash bin

Existing trash bin

Away dugout

Proposed scoreboard to be removed and replaced

Proposed chainlink fence - 200'

Proposed retaining seatwall - 350 LF

BARTON & LOGUIDICE
KICKOFF MEETING / SITE WALK
THROUGH
EXISTING CONDITIONS /
INITIAL DESIGN REQUIREMENTS

FIELD NOTES

09 APRIL 2025

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Landscape Architect/Engineer
Barton & Loguidice
247A ROUTE 100, SUITE 2007, SOMERS, NEW YORK 10589
914.232.3646 PHONE WWW.BARTONANDLOGUIDICE.COM

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FULLER & ANGELO P.C.
ARCHITECTS & PLANNERS

PROJECT TITLE
EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL FIELD SITE IMPROVEMENTS &
RECONSTRUCTION OF EXISTING BASEBALL FIELD
300 WHITE OAK LANE, SCARSDALE, NY 10583

DRAWING TITLE
SITE LAYOUT PLAN
300 WHITE OAK LANE, SCARSDALE, NY 10583

FOR BID
06-19-2025
DATE
S.D. SUBMISSION
ISSUED TO

SHEET SIZE
30"x42"
SCALE
AS NOTED

DRAWING NO.
C102

DRAWN BY
B & L

FILE NO.
XX



Bidder Sign In Sheet

Phase 2c - E Bldg Resource/LGI Reno & Turf Field

AOR: Fuller & D'Angelo

EUFSD - EHS - A Building Renovation

Date: 7-Apr-26

TRITON
CONSTRUCTION

Print Name:

Company

Phone Number:

Email Address:

1	John Barone	IAK-robe/asbestos	917 577 8819	JBARONE@IAKonline.com
2	Maek Puckhardt	Maek Construction	(203) 797-8788	chiefestimator@maekconstruction.com
3	DARKO NOJKOVIC	NEOPANTA EST.	518-406-5345	DNJOVKOVIC@NYCAPITAL.COM (HBAZEMENT)
4	Enoi Derwisoski	UNIMAR LLC	973 992 8529	Enoi.Derwisoski@UNIMARLLC.COM
5	Matt Laotato	Land Tek	631-691-2381	estimating@landtekgroup.com
6	Sarabjit Singh	New Safetyway Contracting	646-269-6877	newsafetyway1@yahoo.com
7	Tom Hart	TEI Fire Protection	718-890-7945	thomas.hart@teici.com
8	Al Bozell	ADY HomeMant	914 495 3030	LTORRES@ALBOZZELLI.COM
9	Leah Costabile	De Rosa	845 392-2865	LCOSTABILE@KMARV.COM
10	MATT DE ROSA	DE ROSA SPORTS	914 341 1506	JOHN@DEROSSASPORTS.COM
11	Eugene Smith	NABEC Electric	914 941 2244	es@nabecelectric.com
12	Leah Jones	BEL	917 861 3022	ljones@leahandloganinc.com
13	Liam Regan	BEL	845 399 7754	lregan@brutonandloganinc.com
14	WILLIAM J. MEANS	FIN	914-592-4444	william@fullerandangelo.com
15	Bob Camilli	Camilli's Son	914-769-1164	Peter.camilli@verizon.net
16				

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

CONTRACTOR'S REQUEST FOR INTERPRETATION NO. 1 F&D RFI NO: 004 (F&D USE)

1.1 NAME OF PROJECT: **High School Resource & Building E Renovations & Athletic Field Reconstruction**

1.2 NAME OF OWNER: **Edgemont Union Free School District**

FACILITY: **Edgemont High School**

1.3 DATE: 3/26/26

1.4 A/E PROJECT NO: **24529.06**

1.5 ARCHITECT: **Fuller and D'Angelo, P.C.**

45 Knollwood Road, Elmsford, NY 10523

Tel: 914-592-4444; Fax: 914-592-1717

+WilliamM@fullerdangelo.com

Refer to Section 00 2113 Par 1.13 for additional requirements.

FROM (CO. NAME): Worth Construction Co., inc.

CONTACT NAME: Peter Smith Tel: 203/797-8788

Tel: _____ E-mail: chiefestimator@worthconstruction.com

SUBJECT: Milestone Schedule

DISCIPLINE/TRADE: _____

DWG./SPEC. REFERENCE: Spec Section 01 1010

QUESTION: Milestone Schedule is based on a Tentative Bid Aware of 11/10/2025

Phase 1 Preconstruction/Administrative time frame starts at November 10, 2026.

Please provide an updated Milestone Schedule that includes and updated RFI Deadline.

___ FIELD CONDITION _____

___ DRAWING/SPEC _____

___ DISCREPANCY _____

___ OWNER CHANGE _____

___ CLARIFICATION _____

___ CONTRACTOR'S SUGGESTION (IF APPLICABLE): _____

FULLER AND D'ANGELO, P.C.
ARCHITECTS AND PLANNERS

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION
RFI Form

ANSWER

See Addendum #3.

ARCHITECT'S SIGNATURE:  **DATE:** **4/9/2026**

Note: review and any responses to this request for information by the architect/engineer is strictly for design intent only and does not constitute acknowledgement or acceptance of any cost or schedule implications unless specifically presented by the contractor. By submission of this request for information, the contractor assumes all responsibility in the absence of an approved change order or work directive.

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

SECTION 00 2115
RFI FORM

FULLER AND D'ANGELO, P.C.
ARCHITECTS AND PLANNERS

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

CONTRACTOR'S REQUEST FOR INTERPRETATION NO. _____ F&D RFI NO: **005** (F&D USE)

1.1 NAME OF PROJECT: **High School Resource & Building E Renovations & Athletic Field Reconstruction**

1.2 NAME OF OWNER: **Edgemont Union Free School District**

FACILITY: **Edgemont High School**

1.3 DATE: 04/02/2026

1.4 A/E PROJECT NO: **24529.06**

1.5 ARCHITECT: **Fuller and D'Angelo, P.C.**

45 Knollwood Road, Elmsford, NY 10523

Tel: 914-592-4444; Fax: 914-592-1717

+WilliamM@fullerdangelo.com

Refer to Section 00 2113 Par 1.13 for additional requirements.

FROM (CO. NAME): The LandTek Group

CONTACT NAME: Matthew Lautato Tel: (631) 691-2381

Tel: _____ E-mail: estimatinggroup@landtekgroup.com

SUBJECT: Milestone Schedle

DISCIPLINE/TRADE: Site Contract #5

DWG./SPEC. REFERENCE: Addendum #1 Milestone Schedule

QUESTION: 1) The milestone schedule inlcuded in addedndum #1 looks to be for a different project. Please clarify

2) If the project schedule included in addendum #1 is incorrect, then the RFI responses in addendum #2 that say to refer to the milestone schedule are also incorrect.

___ FIELD CONDITION _____

___ DRAWING/SPEC _____

___ DISCREPANCY _____

___ OWNER CHANGE _____

___ CLARIFICATION _____

___ CONTRACTOR'S SUGGESTION (IF APPLICABLE): _____

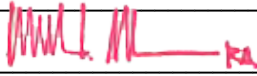
FULLER AND D'ANGELO, P.C.
ARCHITECTS AND PLANNERS

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION
RFI Form

ANSWER

See Addendum #3.

ARCHITECT'S SIGNATURE:



DATE: **4/9/2026**

Note: review and any responses to this request for information by the architect/engineer is strictly for design intent only and does not constitute acknowledgement or acceptance of any cost or schedule implications unless specifically presented by the contractor. By submission of this request for information, the contractor assumes all responsibility in the absence of an approved change order or work directive.

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

CONTRACTOR'S REQUEST FOR INTERPRETATION NO. 01 F&D RFI NO: 006 (F&D USE)

1.1 NAME OF PROJECT: High School Resource & Building E Renovations & Athletic Field Reconstruction

1.2 NAME OF OWNER: Edgemont Union Free School District

FACILITY: Edgemont High School

1.3 DATE: 4/8/26

1.4 A/E PROJECT NO: 24529.06

1.5 ARCHITECT: Fuller and D'Angelo, P.C.

45 Knollwood Road, Elmsford, NY 10523

Tel: 914-592-4444; Fax: 914-592-1717

+WilliamM@fullerdangelo.com

Refer to Section 00 2113 Par 1.13 for additional requirements.

FROM (CO. NAME): Pierotti Corp.

CONTACT NAME: Tony Monaco Tel: 914-233-9990

Tel: _____ E-mail: tonym@pierotticorp.com

SUBJECT: Windows, storefronts. doors

DISCIPLINE/TRADE: _____

DWG./SPEC. REFERENCE: 08116

QUESTION: See the attached

___ FIELD CONDITION ___

___ DRAWING/SPEC ___

___ DISCREPANCY ___

___ OWNER CHANGE ___

___ CLARIFICATION ___

___ CONTRACTOR'S SUGGESTION (IF APPLICABLE): _____

FULLER AND D'ANGELO, P.C.
ARCHITECTS AND PLANNERS

Pierotti Corp RFI #01 - A/E Project No: 24529.06

Edgemont High School

4/8/26

RFI-1 4/7/2026

1. A725 Window Types notes 135° corner conditions at SF1 and SF1a. Floor Plan A110 shows SF1 and SF1a as inline storefronts, no corner conditions. Please clarify.
2. Please confirm interior windows SF1 thru SF3 will be non-thermal 2" x 4-1/2" frames.
3. Section 084313 paragraph 2.4.D Refers to Section 081116 for swing doors. Section 081116 paragraph 2.2.A Refers to Section 084313 for interior doors.

Paragraph 2.4 (081116) specifies interior aluminum doors. Please clarify the proper specification for interior aluminum storefront doors (110, 112, 122, 123). **&109**

1. **See Addendum #3.**
2. **Confirmed.**
3. **Interior aluminum doors are SL-15**



4/9/2026

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

CONTRACTOR'S REQUEST FOR INTERPRETATION NO. 1 F&D RFI NO: 007 (F&D USE)

1.1 NAME OF PROJECT: **High School Resource & Building E Renovations & Athletic Field Reconstruction**

1.2 NAME OF OWNER: **Edgemont Union Free School District**

FACILITY: **Edgemont High School**

1.3 DATE: 4/8/2026

1.4 A/E PROJECT NO: **24529.06**

1.5 ARCHITECT: **Fuller and D'Angelo, P.C.**

45 Knollwood Road, Elmsford, NY 10523

Tel: 914-592-4444; Fax: 914-592-1717

+WilliamM@fullerdangelo.com

Refer to Section 00 2113 Par 1.13 for additional requirements.

FROM (CO. NAME): UniMak, LLC

CONTACT NAME: Ivo Dostinoski Tel: 973-233-4647

Tel: 973-478-4925 E-mail: estimating@unimakllc.com

SUBJECT: _____

DISCIPLINE/TRADE: _____

DWG./SPEC. REFERENCE: A725

QUESTION: A725 Window Types notes 135° corner conditions at SF1 and SF1a.

Floor Plan A110 shows SF1 and SF1a as inline storefronts, no corner conditions.

Please clarify.

___ FIELD CONDITION _____

___ DRAWING/SPEC _____

___ DISCREPANCY _____

___ OWNER CHANGE _____

___ CLARIFICATION _____

___ CONTRACTOR'S SUGGESTION (IF APPLICABLE): _____

FULLER AND D'ANGELO, P.C.
ARCHITECTS AND PLANNERS

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION
RFI Form

ANSWER

See Addendum #3.

ARCHITECT'S SIGNATURE:



DATE: 4/9/2026

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EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

CONTRACTOR'S REQUEST FOR INTERPRETATION NO. 02 F&D RFI NO: 008 (F&D USE)

1.1 NAME OF PROJECT: High School Resource & Building E Renovations & Athletic Field
Reconstruction

1.2 NAME OF OWNER: Edgemont Union Free School District

FACILITY: Edgemont High School

1.3 DATE: 4/9/26

1.4 A/E PROJECT NO: 24529.06

1.5 ARCHITECT: Fuller and D'Angelo, P.C.

45 Knollwood Road, Elmsford, NY 10523

Tel: 914-592-4444; Fax: 914-592-1717

+WilliamM@fullerdangelo.com

Refer to Section 00 2113 Par 1.13 for additional requirements.

FROM (CO. NAME): Pierotti Corp.

CONTACT NAME: Tony Monaco Tel: 914-233-9990

Tel: _____ E-mail: tonym@pierotticorp.com

SUBJECT: Deck infills

DISCIPLINE/TRADE: structural steel

DWG./SPEC. REFERENCE: AR103 and A625

QUESTION: See the attached

___ FIELD CONDITION _____

___ DRAWING/SPEC _____

___ DISCREPANCY _____

___ OWNER CHANGE _____

___ CLARIFICATION _____

___ CONTRACTOR'S SUGGESTION (IF APPLICABLE): _____

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ARCHITECTS AND PLANNERS

EDGEMONT SCHOOL DISTRICT HIGH SCHOOL RESOURCE BUILDING & BUILDING E RENOVATIONS & ATHLETIC FIELD RECONSTRUCTION

300 WHITE OAK LANE, SCARSDALE, NY 10583

SED PROJECT NUMBER:

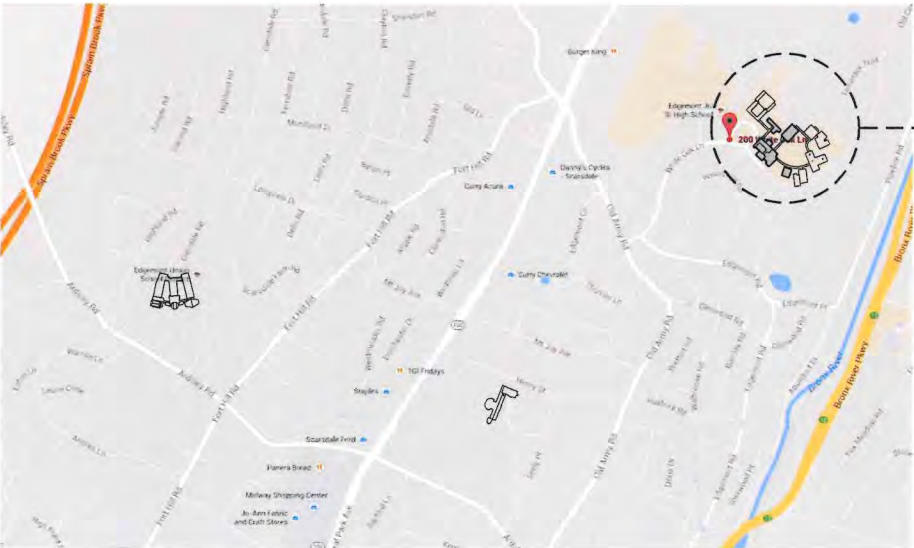
HIGH SCHOOL RESOURCE BUILDING: 66-04-06-03-0-010-009

HIGH SCHOOL BUILDING E: 66-04-06-03-0-016-005



Marek Szurawski
Director of Projects / Estimator
marek@strongholdny.com

Stronghold Industries, LLC
41 Commerce Drive
Carmel, NY 10512
Phone: (914) 235-8280 x215
Fax: (845) 319-6452
www.strongholdny.com



EDGEMONT HIGH SCHOOL
300 WHITE OAK LANE, SCARSDALE, NY 10583



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TEL 914.592.4444 FAX 914.592.1717
WWW.FULLERDANGELO.COM

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CONSULTANTS:

ENVIRONMENTAL :
ADELAIDE

ENVIRONMENTAL HEALTH
1511 ROUTE 22 - SUITE C-24
BREWSTER, NEW YORK, 10509
TEL: 845.278.7710 fax: 845.278.7750

MECHANICAL, PLUMBING AND ELECTRICAL:
LANDMARK FACILITIES GROUP, INC (LFG)

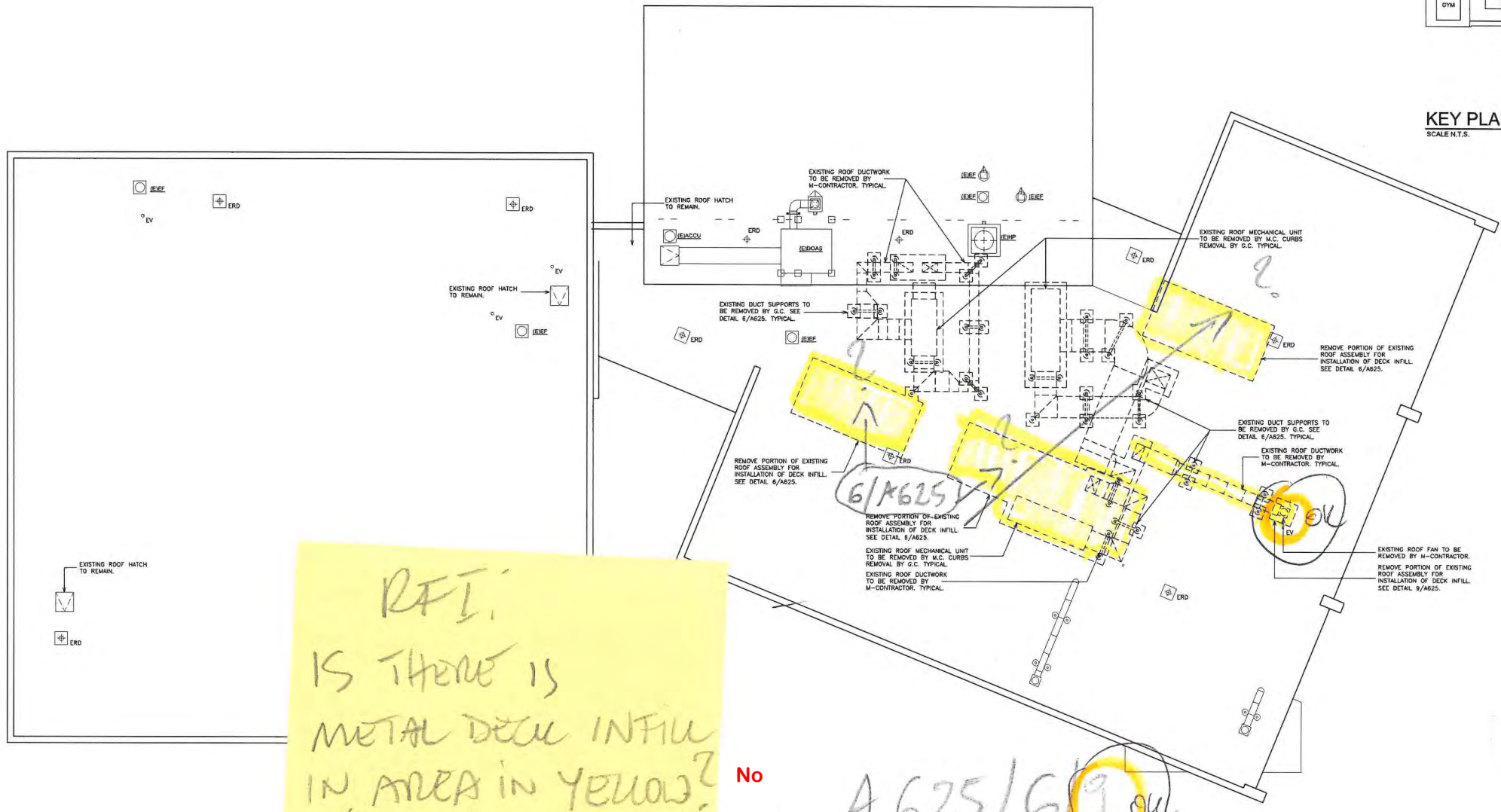
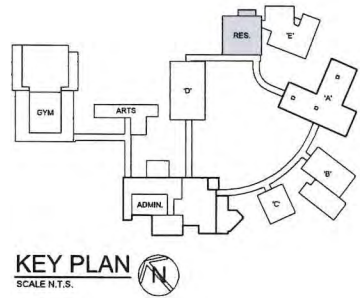
252 EAST AVENUE
NORWALK, CT 06855
TEL: (203) 866-4626
www.lfginc.com

CIVIL ENGINEER:

**Barton
&Loguidice**

247 ROUTE 100, SUITE 2000 D
SOMERS, NY 10589
TEL: 914.232.3646
www.bartonandloguidice.com

03-24-2026	BID
11-24-2025	S.E.D. SUBMISSION
DATE	ISSUED TO
SHEET SIZE 30"x42"	DRAWING NO.
SCALE AS NOTED	T-1
DRAWN BY F & D	FILE NO. 24529.06



1 ROOF REMOVAL PLAN
SCALE: 1/8" = 1'-0"
0' 2' 4' 8' 16' 32'

RFI:
IS THERE IS
METAL DECK INFILL
IN AREA IN YELLOW?
6/A625 SAYS (NO)
BUT
NOTE ON THIS DRAWING SAYS
DECK INFILL

No

A 625/619 OK

L4x4x5/16 (20')x 1

2" 20 GA GALV. MET. DECK

ADVISE

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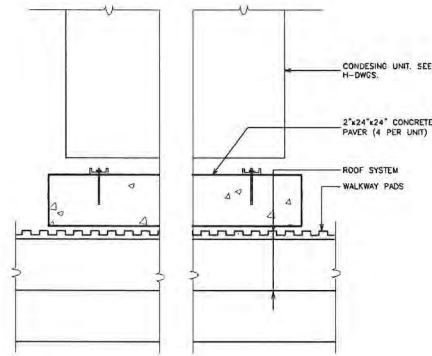
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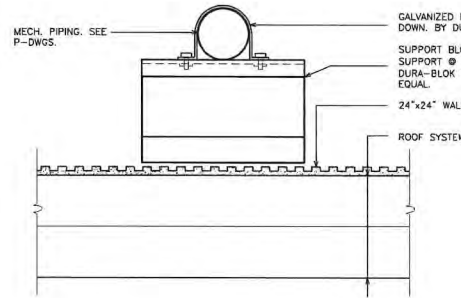
S.E.D. CONTROL NUMBER:
RESOURCE BUILDING
06-04-06-03-0-010-009
BUILDING E
06-04-06-03-0-010-005

PROJECT TITLE: SCHOON DISTRICT
HIGH SCHOOL RESOURCE BUILDING E
RECONSTRUCTION/REPAIRS
NO WHITE OAK LANE, SCARSDALE, NY 11583
DRAWING TITLE:
ROOF REMOVAL PLAN

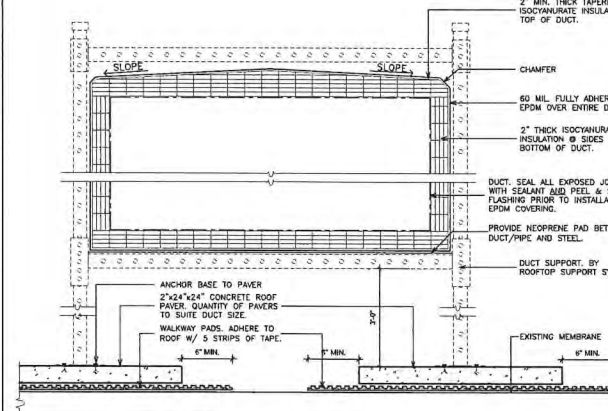
02-24-2020 BID
11-24-2023 S.E.D. SUBMISSION
DATE ISSUED TO
SHEET SIZE 30"x42"
SCALE AS NOTED
DRAWN BY F & D
FILE NO. 24529.06
DRAWING NO. AR103



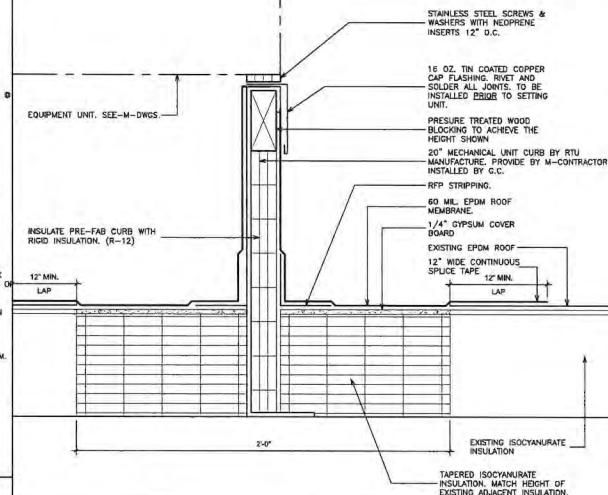
1 DETAIL- CONCRETE ROOF PAVER SUPPORT
SCALE: 3" = 1'-0"



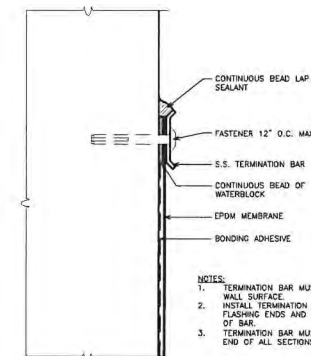
2 PIPE SUPPORT
SCALE: 3" = 1'-0"



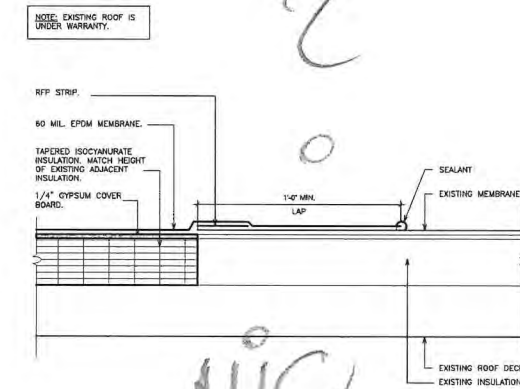
3 DETAIL- DUCT INSULATION & SUPPORT
SCALE: 1 1/2" = 1'-0"



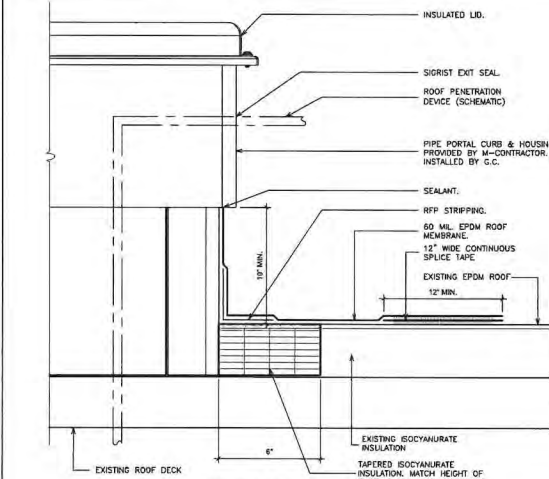
4 DETAIL- STRUCTURAL MECHANICAL CURB
SCALE: N.T.S.



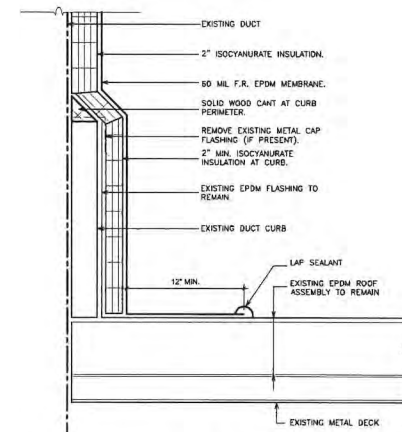
5 DETAIL- TERMINATION BAR FLASHING TERMINATION
SCALE: 6" = 1'-0"



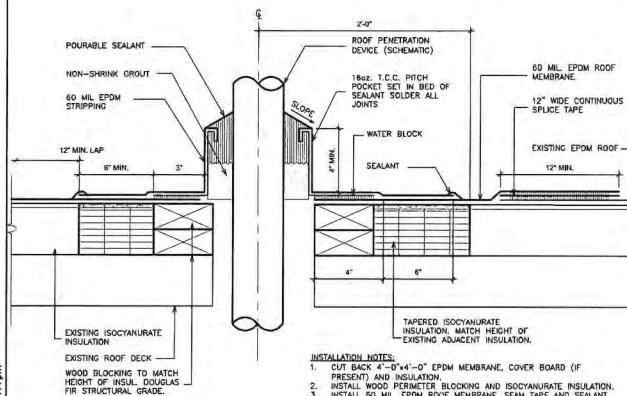
6 TYPICAL NEW TO EXIST. SPLICE DETAIL
SCALE: 3" = 1'-0"



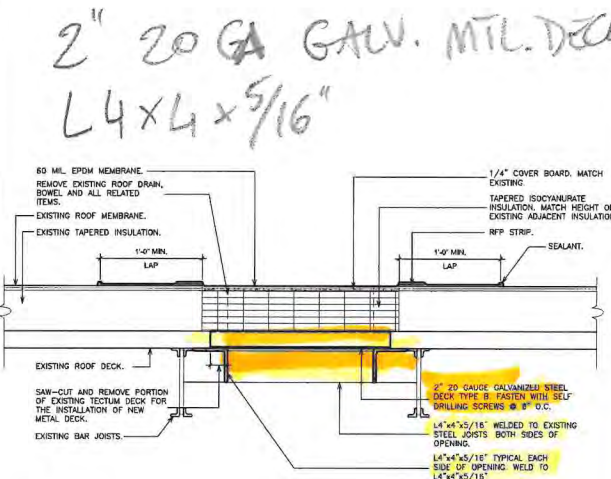
7 DETAIL- PIPE PORTAL @ EXISTING ROOF
SCALE: N.T.S.



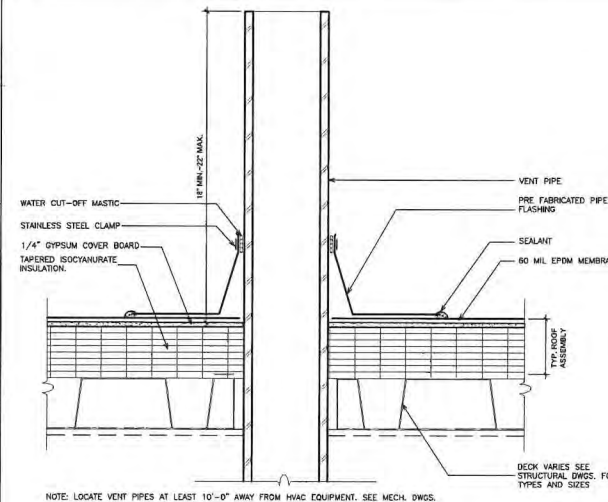
11 REVISED DUCT CURB
SCALE: N.T.S.



8 DETAIL- PITCH POCKET @ EXISTING ROOF
SCALE: N.T.S.



9 DECK INFILL
SCALE: 1 1/2" = 1'-0"



10 DETAIL- PIPE FLASHING
SCALE: N.T.S.

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ARCHITECTS
PLANNERS
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ELMWOOD PARK, NY 11754
TEL: 815-898-4444
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S.E.D. CONTROL NUMBER:
RESOURCE BUILDING
66-04-06-03-0-010-009
BUILDING E
66-04-06-03-0-010-005

PROJECT TITLE:
HIGH SCHOOL RENOVATION & ATHLETIC FIELD
300 WHITE OAK LANE, BOARDSDALE, NY 10803
PROJECT NO.:
DRAWING NO.:
ROOF DETAILS

5/28/2018
11/28/2025
DATE
SHEET SIZE
30"x42"
SCALE
AS NOTED
DRAWN BY
F & D
ISSUED TO
S.E.D. SUBMISSION
ISSUED TO
DRAWING NO.
A625
FILE NO.
24529.06

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

CONTRACTOR'S REQUEST FOR INTERPRETATION NO. 2 F&D RFI NO: 009 (F&D USE)

1.1 NAME OF PROJECT: High School Resource & Building E Renovations & Athletic Field
Reconstruction

1.2 NAME OF OWNER: Edgemont Union Free School District

FACILITY: Edgemont High School

1.3 DATE: 4/8/2026

1.4 A/E PROJECT NO: 24529.06

1.5 ARCHITECT: Fuller and D'Angelo, P.C.

45 Knollwood Road, Elmsford, NY 10523

Tel: 914-592-4444; Fax: 914-592-1717

+WilliamM@fullerdangelo.com

Refer to Section 00 2113 Par 1.13 for additional requirements.

FROM (CO. NAME): UniMak, LLC

CONTACT NAME: Ivo Dostinoski Tel: 973-233-4647

Tel: 973-478-4925 E-mail: estimating@unimakllc.com

SUBJECT: _____

DISCIPLINE/TRADE: _____

DWG./SPEC. REFERENCE: _____

QUESTION: Please confirm interior windows SF1 thru SF3 will be non-thermal
2" x 4-1/2" frames.

___ FIELD CONDITION _____

___ DRAWING/SPEC _____

___ DISCREPANCY _____

___ OWNER CHANGE _____

___ CLARIFICATION _____

___ CONTRACTOR'S SUGGESTION (IF APPLICABLE): _____

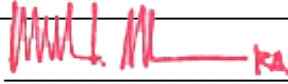
FULLER AND D'ANGELO, P.C.
ARCHITECTS AND PLANNERS

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION
RFI Form

ANSWER

Confirmed.

ARCHITECT'S SIGNATURE:



DATE: 4/9/2026

Note: review and any responses to this request for information by the architect/engineer is strictly for design intent only and does not constitute acknowledgement or acceptance of any cost or schedule implications unless specifically presented by the contractor. By submission of this request for information, the contractor assumes all responsibility in the absence of an approved change order or work directive.

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

CONTRACTOR'S REQUEST FOR INTERPRETATION NO. 3 F&D RFI NO: 010 (F&D USE)

1.1 NAME OF PROJECT: **High School Resource & Building E Renovations & Athletic Field Reconstruction**

1.2 NAME OF OWNER: **Edgemont Union Free School District**

FACILITY: **Edgemont High School**

1.3 DATE: 4/8/2026

1.4 A/E PROJECT NO: **24529.06**

1.5 ARCHITECT: **Fuller and D'Angelo, P.C.**

45 Knollwood Road, Elmsford, NY 10523

Tel: 914-592-4444; Fax: 914-592-1717

+WilliamM@fullerdangelo.com

Refer to Section 00 2113 Par 1.13 for additional requirements.

FROM (CO. NAME): UniMak, LLC

CONTACT NAME: Ivo Dostinoski Tel: 973-233-4647

Tel: 973-478-4925 E-mail: estimating@unimakllc.com

SUBJECT: _____

DISCIPLINE/TRADE: _____

DWG./SPEC. REFERENCE: 084313/081116

QUESTION: Section 084313 paragraph 2.4.D Refers to Section 081116 for swing doors.

Section 081116 paragraph 2.2.A Refers to Section 084313 for interior doors.

Paragraph 2.4 (081116) specifies interior aluminum doors. Please clarify the

proper specification for interior aluminum storefront doors (110, 112, 122, 123). &109

___ FIELD CONDITION _____

___ DRAWING/SPEC _____

___ DISCREPANCY _____

___ OWNER CHANGE _____

___ CLARIFICATION _____

___ CONTRACTOR'S SUGGESTION (IF APPLICABLE): _____

FULLER AND D'ANGELO, P.C.
ARCHITECTS AND PLANNERS

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION
RFI Form

ANSWER

Interior aluminum swing doors are SL-15.

ARCHITECT'S SIGNATURE:



DATE: **4/9/2026**

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EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

CONTRACTOR'S REQUEST FOR INTERPRETATION NO. 001 F&D RFI NO: 011 (F&D USE)

1.1 NAME OF PROJECT: High School Resource & Building E Renovations & Athletic Field Reconstruction

1.2 NAME OF OWNER: Edgemont Union Free School District

FACILITY: Edgemont High School

1.3 DATE: 4/8/2026

1.4 A/E PROJECT NO: 24529.06

1.5 ARCHITECT: Fuller and D'Angelo, P.C.

45 Knollwood Road, Elmsford, NY 10523

Tel: 914-592-4444; Fax: 914-592-1717

+WilliamM@fullerdangelo.com

Refer to Section 00 2113 Par 1.13 for additional requirements.

FROM (CO. NAME): Chenango Contracting Inc.

CONTACT NAME: Florence McGraw Tel: 607-729-8500

Tel: _____ E-mail: fmcgraw@chenangocontracting.com

SUBJECT: Acceptable Synthetic Turf Manufacturers and Systems

DISCIPLINE/TRADE: Synthetic Turf Contractor

DWG./SPEC. REFERENCE: 321813 paragraph 2.01

QUESTION: We would like to submit Doubleplay Natural Grass & Clay with thermagreen 15

(SportLite HD15SL) pad as an equal for the base bid and the alternate with envirofill we would

suggest our vertex (FTVT)1.75" custom pile weight and the thermagreen 23 (SportLite HD23SL)

This is a Baseball/ Softball Field is the FIFA Third-Party Performance Testing is required?

FIELD CONDITION

X DRAWING/SPEC 321813

DISCREPANCY

OWNER CHANGE

CLARIFICATION

CONTRACTOR'S SUGGESTION (IF APPLICABLE):

FULLER AND D'ANGELO, P.C.
ARCHITECTS AND PLANNERS

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION
RFI Form

ANSWER

Substitutions will not be reviewed during the bidding phase.

ARCHITECT'S SIGNATURE:



DATE: 4/9/2026

Note: review and any responses to this request for information by the architect/engineer is strictly for design intent only and does not constitute acknowledgement or acceptance of any cost or schedule implications unless specifically presented by the contractor. By submission of this request for information, the contractor assumes all responsibility in the absence of an approved change order or work directive.

DOUBLEPLAY Baseball

TECHNICAL SPECIFICATION



Natural Grass

PROPERTY	VALUE	UNITS	METHOD
Product Stock Code	FTVT-50		
Pile Yarn Type	UV-resistant polyethylene		n/a
Yarn Structure 1	Ridged Monofilament		n/a
Yarn Denier 1	14500	Denier	D1907
Tape Thickness 1	360	Microns	
Yarn Structure 2	Slit-Film		
Yarn Denier 2	5000	Denier	D1907
Tape Thickness 2	100+	Microns	
Pile Height	2	inches	D5823
Pile Weight	39	oz/yd ²	D5848
Primary Backing Weight	7+	oz/yd ²	D5848
Secondary Backing Weight (Perforated)	16+	oz/yd ²	D5848
Total Carpet Weight	62	oz/yd ²	D5848
Stitch Gauge	3/4 inch centers		D5793
Tuft Bind	8+	lbs/force	D1335
Grab Tear Length	>200	lbs/force	D5034
Grab Tear Width	>200	lbs/force	D5034
Pill Burn Test	Pass		D2859
Impact Attenuation (Gmax)	<200	gmax	F1936
Water Permeability	>40	inch/hour	F1551; DIN 18-035
Ambient Rubber	2.6	Lbs/ft ²	
Sand	3.65	Lbs/ft ²	
Total Product Weight	962	oz/yd ²	

Issue Date: 9/14/2021

Disclaimer: Variation of +/-5% on above listed property values is within normal manufacturing tolerances



DOUBLEPLAY Baseball

TECHNICAL SPECIFICATION



Natural Clay

PROPERTY	VALUE	UNITS	METHOD
Product Stock Code	XT-50-3/8		
Pile Yarn Type	UV-resistant polyethylene		n/a
Yarn Structure	Slit-Film		n/a
Yarn Denier	10000+	Denier	D1577
Tape Thickness	100+	Microns	D3218
Pile Height	2	inches	D5823
Pile Weight	42	oz/yd ²	D5848
Primary Backing Weight	7+	oz/yd ²	D5848
Secondary Backing Weight (Perforated)	16+	oz/yd ²	D5848
Total Carpet Weight	65	oz/yd ²	D5848
Stitch Gauge	3/8 inch centers		D5793
Tuft Bind	8+	lbs/force	D1335
Grab Tear Length	>200	lbs/force	D5034
Grab Tear Width	>200	lbs/force	D5034
Pill Burn Test	Pass		D2859
Water Permeability	>40	inch/hour	DIN 18-035
Ambient Rubber	1.5	Lbs/ft ²	
Sand	5.4	Lbs/ft ²	
Total Product Weight	1059	oz/yd ²	

Issue Date: 12/12/2022

Disclaimer: Variation of +/-5% on above listed property values is within normal manufacturing tolerances

When the priority is to replicate the expected ball/surface interactions of a natural playing surface, this will result in higher g-max/HIC values. As a result, this field area takes exception to commonly recommended g-max/HIC tolerances



SportLite™, developed by ThermaGreen Environmental, is the Shock & Drainage Pad for top quality synthetic turf systems. Our pad is manufactured in North America, approved under the NAFTA agreement, and is produced by re-purposing virgin non-contaminated post-industrial cross-linked (closed cell) PEX foam. It is bonded to a non-woven geotextile to create a pad that accommodates the various demands of different sports.

ThermaGreen Sport products meet energy absorption standards under ASTM F355-10a for HIC and G_{MAX} . SportLite™ does not contain or use any toxic substances (i.e. CFC & HCFC) during the manufacturing process; it has passed California Proposition 65 and the Synthetic Precipitation Leaching Procedure (SPLP) for metals, and is compliant under NSF 61 US/Canadian drinking water quality. It is inert, environmentally friendly, resistant to mold and most chemicals (including petroleum products), and helps you obtain LEED credits!

TESTED PROPERTY	TESTED METHOD	TYPICAL VALUES	
		SI units	Imperial units

FOAM / GEOTEXTILE COMPOSITE			
Mass per Unit Area	ASTM D3776M-09a	2.7 kg/m ²	0.57 lb/ft ²
Overall Density	ASTM D3575-08-W (A)	139.8 kg/m ³	8.7 lb/ft ³
Ply Adhesion (200mm wide specimen)	ASTM D7005-03 mod	0.2 kN/m	1.3 lbf/in
Thermal Stability	ASTM D3575-08-S	< 0.24 %	< 0.24 %
Thermal Resistance	ISO 11092-1993ISO	15.01 r(m·K/W)	1.69 (R value)

PERFORMANCE PROPERTIES			
Compression Set	ASTM D3575-14	2.36%	2.36%
Compressive Strength at 25% Deflection	ASTM D3575-14	99 kPa	14 psi
Water Permeability ⁽¹⁾	ASTM D4491-99a (2014)	884 l/min/m ²	21.7 gal/min/ft ²
Shock Attenuation ⁽²⁾	ASTM F355-10a	89 - 97 G_{MAX}	340 - 410 HIC

GEOTEXTILE (prior to lamination)			
Grab Tensile Strength	ASTM D4632	400 N	90 lb
Grab Tensile Elongation	ASTM D4632	75 %	75 %
Tear Resistance	ASTM D4533	130 N	30 lb
Puncture Resistance	ASTM D6241	1100 N	250 lb
Permittivity	ASTM D4491	2.0 s ⁻¹	2.0 s ⁻¹

DURABILITY AND ENDURANCE PROPERTIES		
Residual properties after 4000 hours water immersion		
Compression Set	ASTM D5322-98 / D3575-08-B	100% retained
Compressive Strength at 25% Deflection	ASTM D5322-98 / D3575-08-D	> 75% retained
Residual properties after 4000 hours aging at 85°C/185°F		
Compressive Strength at 25% Deflection	ASTM D3045-92 / D3575-08-B	100% retained
Compression Deflection	ASTM D3045-92 / D3575-08-D	> 75% retained

DIMENSIONS			
Thickness +/-2 mm (0.08")	ASTM D5199-01	15 mm	5/8 in
Roll Weight	-	233 kg	513 lbs
Roll Width	-	1.22 m	4 ft
Roll Length (custom roll lengths available) +/-1%	-	68.58 m	225 ft
Roll Area	-	83.67 m ²	900 ft ²

(1) Flow rate under 51mm hydraulic head

(2) Shock attenuation measured using Triax "A" "Missile Tests - Results will vary depending on actual field configuration and final cross-section design

The technical information is provided by ThermaGreen and to the best of our knowledge, is true and accurate; ThermaGreen assumes no legal responsibility for use or reliance of data.

SportLite™, developed by ThermaGreen Environmental, is the Shock & Drainage pad for top quality synthetic turf systems. Our pad is manufactured in North America, approved under the NAFTA agreement, and is produced by re-purposing virgin non-contaminated post-industrial cross-linked (closed cell) PEX foam. It is bonded to a non-woven geotextile to create a pad that accommodates the various demands of different sports.

ThermaGreen Sport products meet energy absorption standards under ASTM F355-10a for HIC and G_{MAX} . SportLite™ does not contain or use any toxic substances (i.e. CFC & HCFC) during the manufacturing process; it has passed California Proposition 65 and the Synthetic Precipitation Leaching Procedure (SPLP) for metals, and is compliant under NSF 61 US/Canadian drinking water quality. It is inert, environmentally friendly, resistant to mold and most chemicals (including petroleum products), and helps you obtain LEED credits!

TESTED PROPERTY	TESTED METHOD	TYPICAL VALUES	
		SI units	Imperial units

FOAM / GEOTEXTILE COMPOSITE			
Mass per Unit Area	ASTM D3776M-09a	3.9 kg/m ²	0.8 lb/ft ²
Overall Density	ASTM D3575-08-W (A)	168 kg/m ³	10.5 lb/ft ³
Ply Adhesion (200mm wide specimen)	ASTM D7005-03 mod	0.2 kN/m	1.3 lbf/in
Thermal Stability	ASTM D3575-08-S	< 0.5 %	< 0.5 %

PERFORMANCE PROPERTIES			
Compression Set	ASTM D3575-08-B	8.2 %	8.2 %
Compressive Strength at 25% Deflection	ASTM D3575-08-D	150 kPa	22 psi
In-Plane Flow Rate ⁽¹⁾	ASTM D4716M-14	16.8 l/min/m	1.36 gal/min/ft
Water Permeability	ASTM D4491-99a (2014) el	1189/min/m ²	29.2 gal/min/ft ²
Shock Attenuation ⁽²⁾	ASTM F355-10a	81 - 96 G_{MAX}	190 -218 HIC

GEOTEXTILE (prior to lamination)			
Grab Tensile Strength	ASTM D4632	400 N	90 lb
Grab Tensile Elongation	ASTM D4632	75 %	75 %
Tear Resistance	ASTM D4533	130 N	30 lb
Puncture Resistance	ASTM D6241	1100 N	250 lb
Permittivity	ASTM D4491	2.0 s ⁻¹	2.0 s ⁻¹

DURABILITY AND ENDURANCE PROPERTIES		
Residual properties after 4000 hours water immersion		
Compression Set	ASTM D5322-98 / D3575-08-B	100% retained
Compressive Strength at 25% Deflection	ASTM D5322-98 / D3575-08-D	> 75% retained
Residual properties after 4000 hours aging at 85°C/185°F		
Compressive Strength at 25% Deflection	ASTM D3045-92 / D3575-08-B	100% retained
Compression Deflection	ASTM D3045-92 / D3575-08-D	> 75% retained

DIMENSIONS			
Thickness +/-2 mm (0.08")	ASTM D5199-01	23 mm	7/8 in
Roll Weight	-	304 kg	672 lbs
Roll Width	-	1.22 m	4 ft
Roll Length (custom roll lengths available) +/-1%	-	62.48 m	205 ft
Roll Area	-	76.22 m ²	820 ft ²

(1) Hydraulic gradient 1% (0.01), compression strength of 10kPa (1.5 psi)

(2) Shock attenuation measured using Triax "A" Missile Tests - Results will vary depending on actual field configuration and final cross-section design

The technical information is provided by ThermaGreen and to the best of our knowledge, is true and accurate; ThermaGreen assumes no legal responsibility for use or reliance of data.

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

SECTION 00 2115
RFI FORM

FULLER AND D'ANGELO, P.C.
ARCHITECTS AND PLANNERS

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION

RFI Form

CONTRACTOR'S REQUEST FOR INTERPRETATION NO. ____ F&D RFI NO: **012** (F&D USE)

1.1 NAME OF PROJECT: **High School Resource & Building E Renovations & Athletic Field Reconstruction**

1.2 NAME OF OWNER: **Edgemont Union Free School District**

FACILITY: **Edgemont High School**

1.3 DATE: 04/10/2026

1.4 A/E PROJECT NO: **24529.06**

1.5 ARCHITECT: **Fuller and D'Angelo, P.C.**

45 Knollwood Road, Elmsford, NY 10523

Tel: 914-592-4444; Fax: 914-592-1717

+WilliamM@fullerdangelo.com

Refer to Section 00 2113 Par 1.13 for additional requirements.

FROM (CO. NAME): The LandTek Group

CONTACT NAME: Matthew Lautato Tel: (631) 691-2381

Tel: _____ E-mail: estimatinggroup@landtekgroup.com

SUBJECT: Scope Clarification

DISCIPLINE/TRADE: Site Contract #5

DWG./SPEC. REFERENCE: ES100, FE100, FE101, C103

QUESTION: 1) Who is responsible the electrical trenching/backfill shown on ES100, FE100, and FE101? Also, who is responsible for the furnish and install of light poles and bases?

2) Is there a detail / model specified for the water combobox shown on C103?

3) During the pre-bid there was mention of soil stabilization / lawn maintenance that would carry over into 2027. Please clarify maintenance schedule and scope.

___ FIELD CONDITION _____

___ DRAWING/SPEC _____

___ DISCREPANCY _____

___ OWNER CHANGE _____

___ CLARIFICATION _____

___ CONTRACTOR'S SUGGESTION (IF APPLICABLE): _____

FULLER AND D'ANGELO, P.C.
ARCHITECTS AND PLANNERS

EDGEMONT SCHOOL DISTRICT
HIGH SCHOOL RESOURCE & BUILDING E RENOVATIONS
& ATHLETIC FIELD RECONSTRUCTION
RFI Form

ANSWER

1. **Site Contractor**

1a **Electrical Contractor furnishes and installs light poles. EC furnishes light pole bases installed by Site Contractor.**

2. **Will be included in Addendum #4.**

3. **Will be included in Addendum #4.**

ARCHITECT'S SIGNATURE:



DATE: **4/10/2026**

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