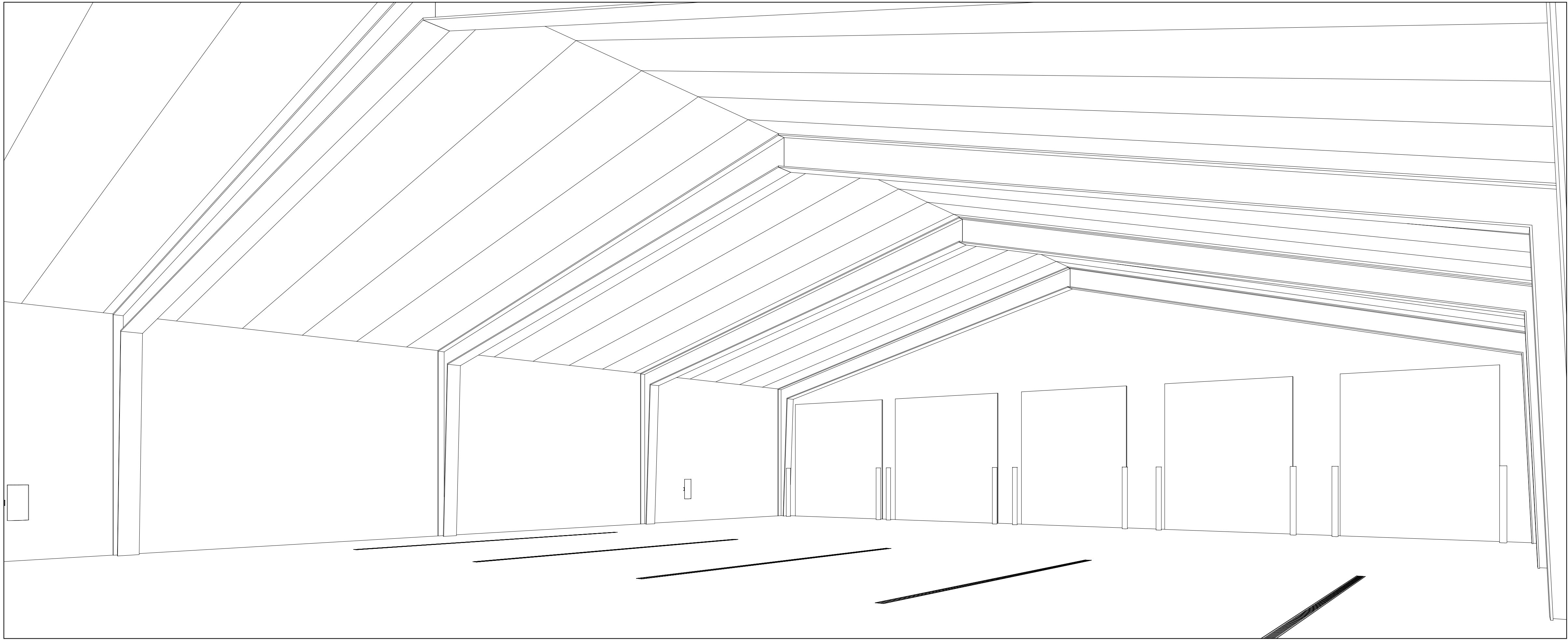
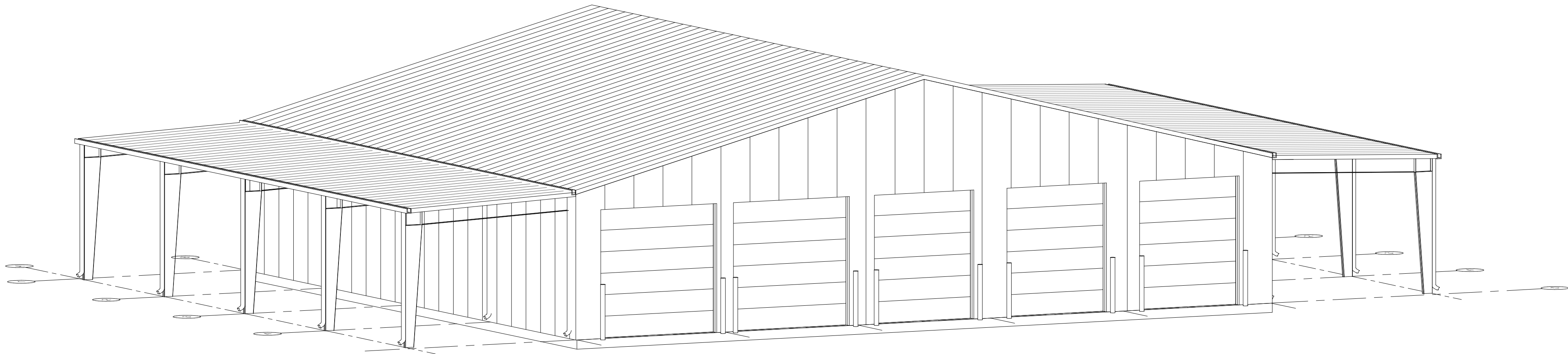


MAIZE USD 266 - BUS STORAGE BUILDING

MARCH 2026 BIDDING PACKAGE SUBMISSION



MABCD REVIEW
STAMP LOCATION

Schaefer.
architecture

Schaefer Architecture
257 N. Broadway
Wichita, KS, 67202
316.684.0171



DUDLEY WILLIAMS & ASSOCIATES, P.A.
STRUCTURAL ENGINEERS
MIDWEST ENGINEERING, INC.
MECHANICAL ENGINEERS
BAUGHMAN COMPANY, P.A.
CIVIL ENGINEERS

MAIZE USD 266
BUS STORAGE BUILDING
4750 NORTH 119TH STREET WEST
MAIZE, KS 67101

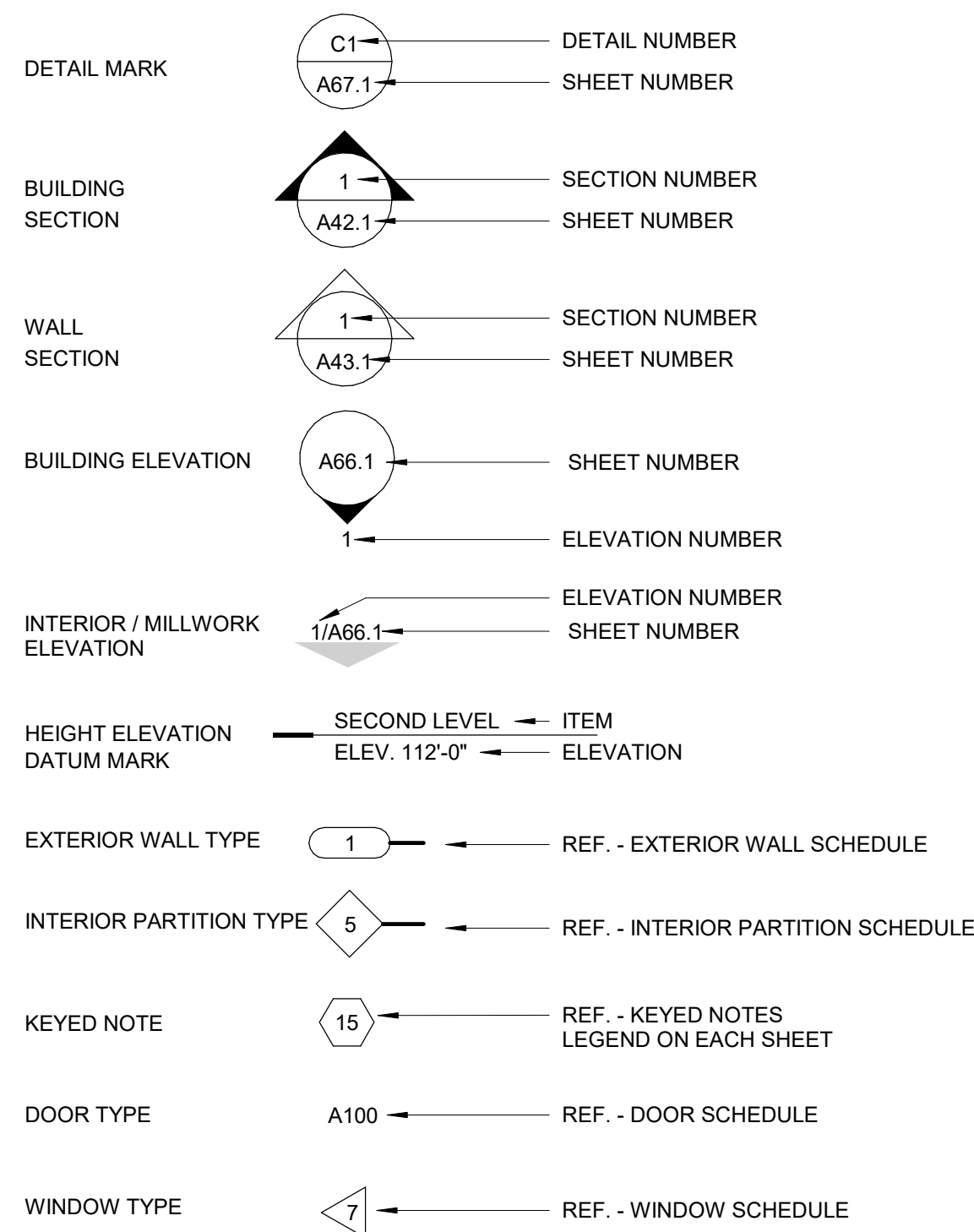
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REVISIONS

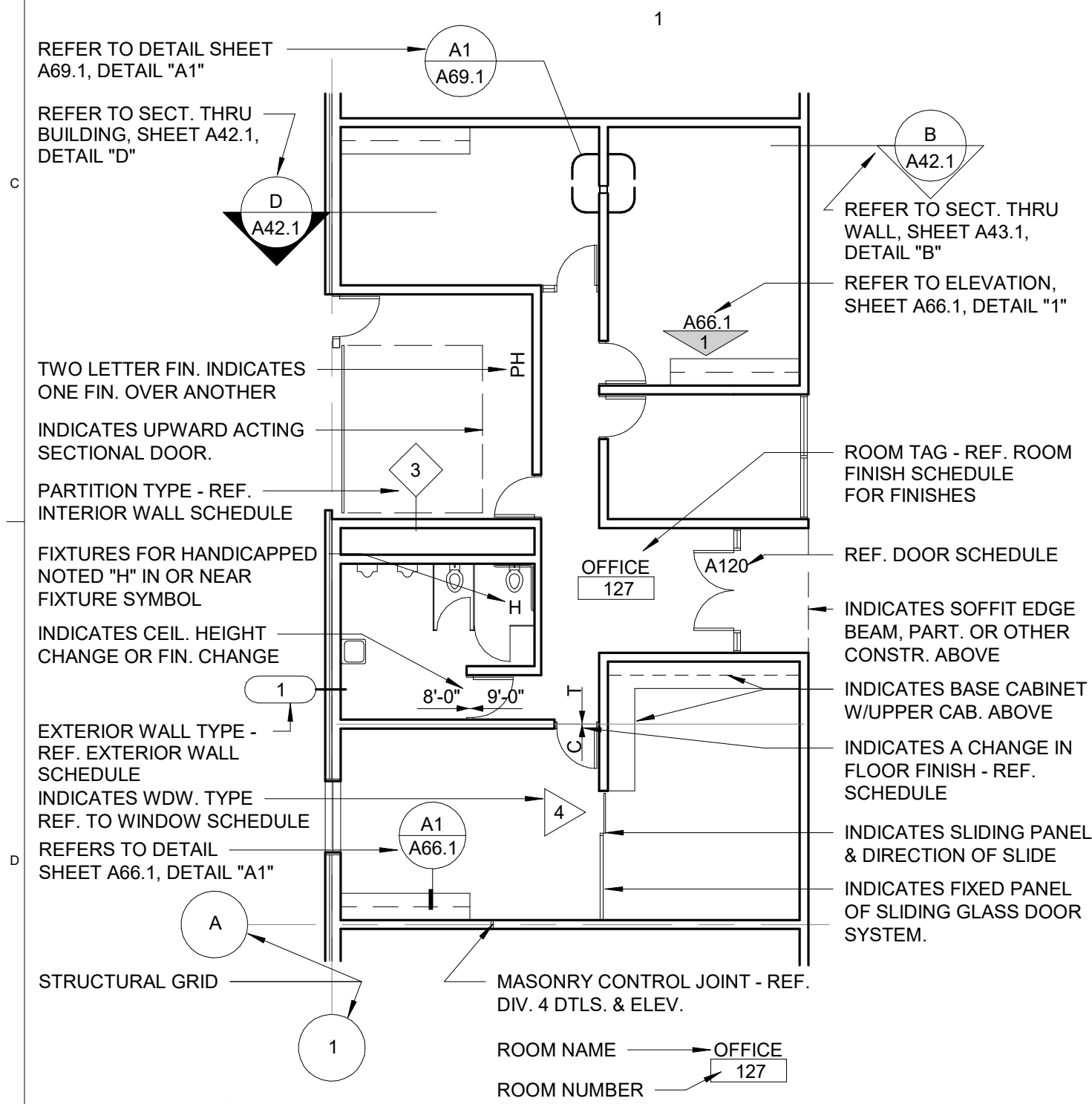
PROJECT NUMBER
5537.50
DATE
MARCH 2026
COVER

G10.0

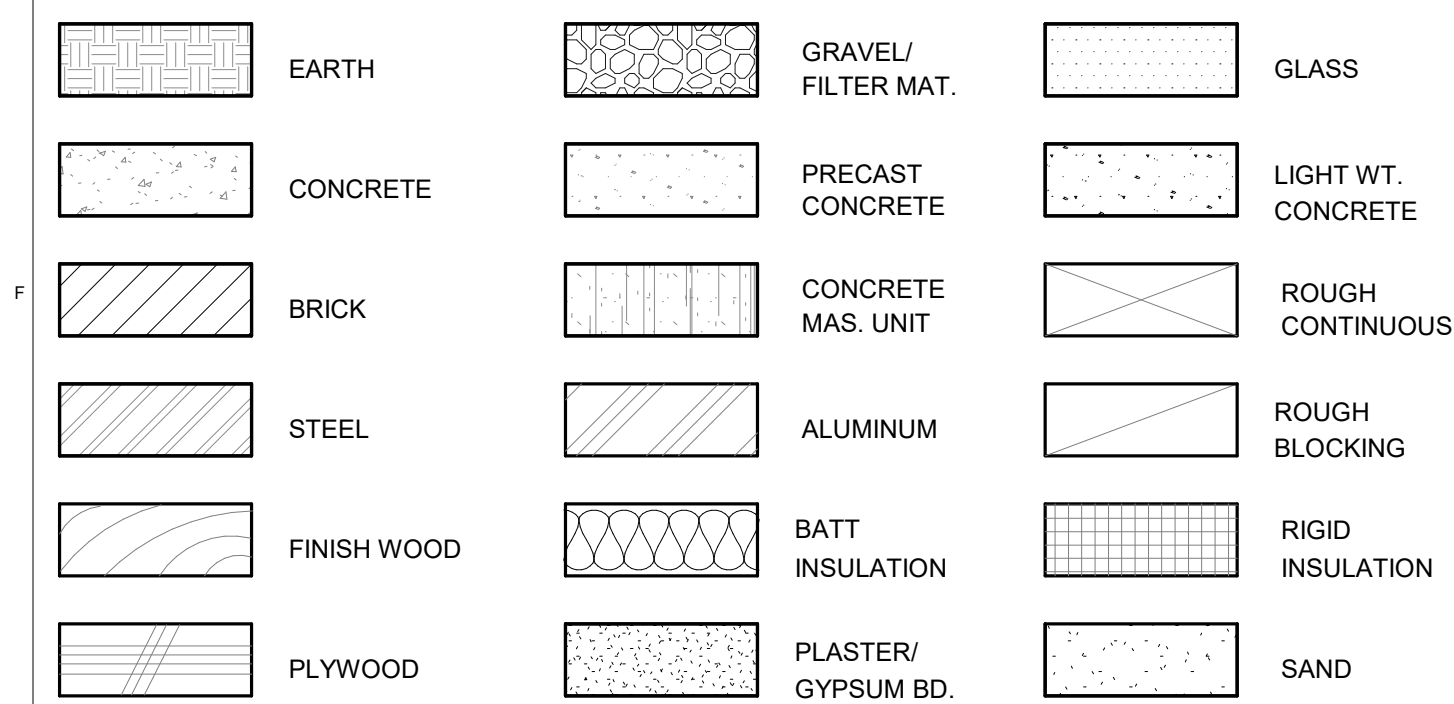
GRAPHIC SYMBOLS



DRAWING SYMBOLS KEY



GRAPHIC SYMBOLS HATCH



DESIGN TEAM

ARCHITECT

SCHAEFER ARCHITECTURE, INC.
257 N. BROADWAY
WICHITA, KS 67202
316.684.0171
www.schaefer-arch.com

CONTACT: GARY HOLLER, AIA
gholler@schaefer-arch.com

STRUCTURAL ENGINEER

DUDLEY WILLIAMS AND ASSOCIATES, P.A.
230 LAURA, SUITE 200
WICHITA, KS 67211-1514
316.263.7591
www.dwase.com

CONTACT: DESIREE MORRELL, PE
dmorrell@dwase.com

CIVIL ENGINEER

BAUGHMAN AND COMPANY, P.A.
315 SOUTH ELLIS
WICHITA, KS 67211
316.262.7271
www.baughmanco.com

CONTACT: PAT BAER, PE
pbaer@baughmanco.com

MECHANICAL ENGINEER

MIDWEST ENGINEERING, INC.
1210 E. 1st STREET
WICHITA, KS 67214
316.262.9300
www.mei-ks.com

CONTACT: BRAD WARD, PE
bward@mei-ks.com

CLIENT INFORMATION

MAIZE UNIFIED SCHOOL DISTRICT #266
905 W. ACADEMY AVE.
MAIZE, KS 67101
316.500.1931
www.usd266.com

CONTACT: DARIN AUGUSTINE
daugustine@usd266.com

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G14.1	CODE COMPLIANCE
G14.2	SURVEY

CIVIL

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C2.0 SITE GRADING PLAN AND EROSION CONTROL PLAN

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A42.1	BUILDING SECTIONS
A61.1	PROJECT DETAILS
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PROJECT DESCRIPTION

THE PROJECT CONSISTS OF A NEW VEHICLE (BUS) STORAGE BUILDING LOCATED BEHIND (EAST) THE EXISTING MAINTENANCE FACILITY AT 4750 N 119TH STREET WEST, MAIZE, KANSAS.

Schaefer Architecture
257 N. Broadway
Wichita, KS, 67202
316.684.0171



DUDLEY WILLIAMS & ASSOCIATES, P.A.
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MAIZE USD 266
BUS STORAGE BUILDING

4750 NORTH 19TH STREET WEST
MAIZE, KS 67101

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PROJECT NUMBER
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DATE
MARCH 2026

GENERAL INFORMATION

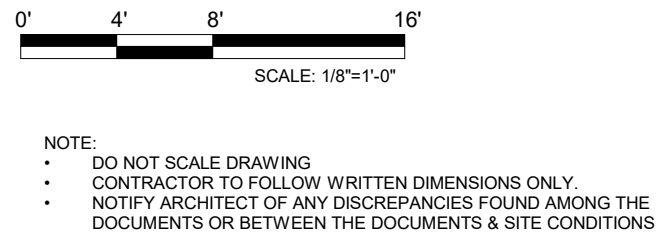
G10.1

A
B
C
D
E
F

PROJECT INFORMATION

TYPE OF CONSTRUCTION: NEW CONSTRUCTION
LOCATION: 4750 NORTH 119TH STREET WEST, MAIZE, KS 67101
OWNER: MAIZE USD
COUNTY: SEDGWICK
CITY: MAIZE, KS
ARCHITECT: SCHAEFER ARCHITECTURE, INC.
HANDICAP PROVISIONS: PATH OF TRAVEL, EXITS
WATER SUPPLY: CITY OF MAIZE
SEWAGE TREATMENT: CITY OF MAIZE
ELECTRICITY: EVERGY
USE: VEHICULAR (BUS) STORAGE
AUTHORITY HAVING JURISDICTION: STATE FIRE MARSHAL
LOCAL FIRE: CITY OF MAIZE
LOCAL CODE ENFORCEMENT: CITY OF MAIZE
NEW CONSTRUCTION: 6,449 S.F.

SHEET SCALE



PROJECT CERTIFICATION

ARCHITECT: SCHAEFER ARCHITECTURE, INC.
I CERTIFY THAT THE SUBMITTED PLANS FOR THE PROJECT REFERENCED ABOVE COMPLY WITH THE REQUIREMENTS OF THE FOLLOWING:

BUILDING
2024 INTERNATIONAL BUILDING CODE
2024 INTERNATIONAL FIRE CODE

PER 2006 IBC SECTION 104.11, THE KANSAS STATE FIRE MARSHAL ACCEPTS APPLICATION OF THE 2010 IBC TO THIS PROJECT AS PROVIDING AN EQUIVALENT LEVEL OF MINIMUM LIFE SAFETY AS REQUIRED BY THE KANSAS FIRE PREVENTION CODE.

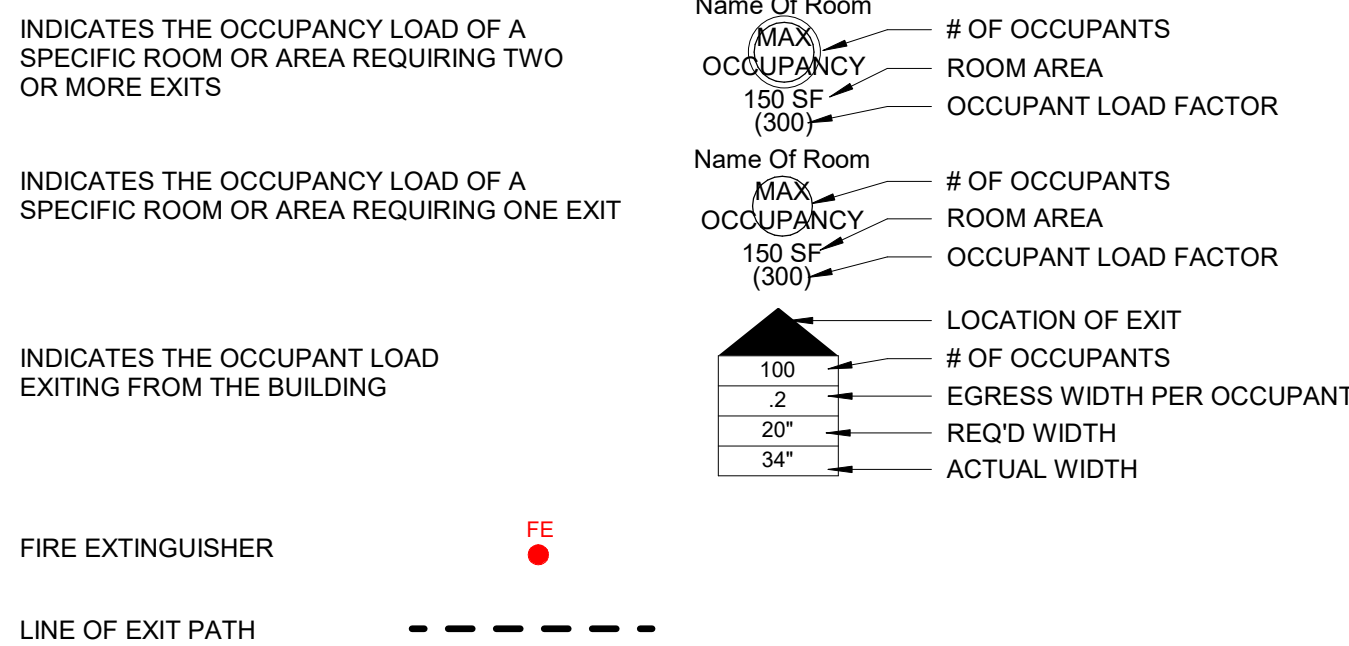
GENERAL NOTES

- ALL BUILDINGS ARE TO BE EQUIPPED WITH THE FOLLOWING:
HVAC SYSTEM: UNCONDITIONED
FUEL: UNCONDITIONED

FIRE SAFETY:
• FIRE EXTINGUISHER
- FIRE PREVENTION CODE KSA 31-133 AND 31-1354a. MOST BUILDINGS NEW OR EXISTING HAVE ONE OR MORE DEVIATIONS FROM ONE OF THE RECOGNIZED BUILDING CODES. STATE LAW MANDATES COMPLIANCE TO THE KANSAS FIRE PREVENTION CODE (KFPC) IN ALL OCCUPIED STRUCTURES.

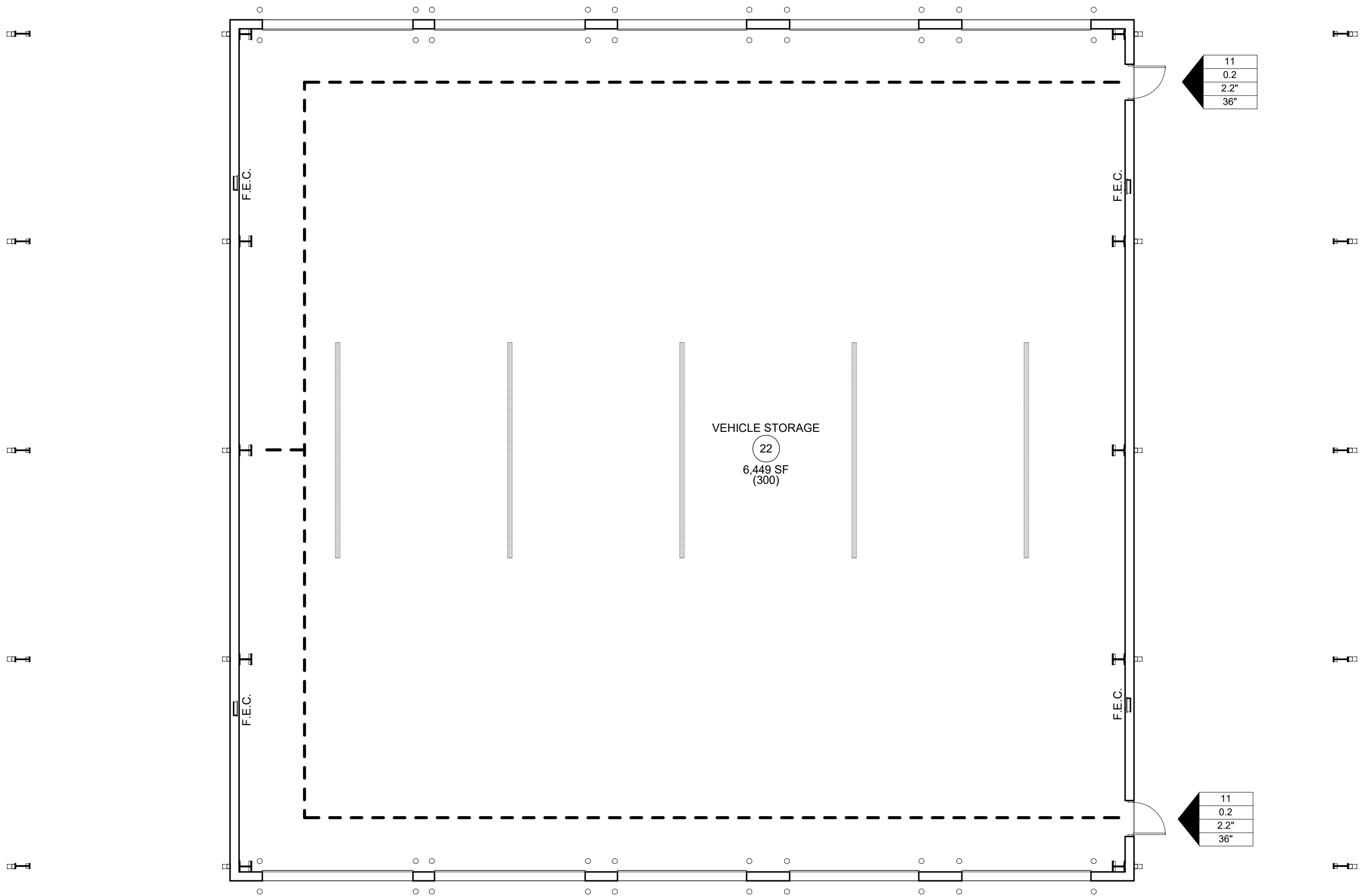
STATE LAW CAN BE SUMMARIZED AS:
a) EXISTING STRUCTURE CANNOT HAVE 'DISTINCTLY HAZARDOUS CONDITIONS' - THOSE THAT IMPEDE SPEEDY EXIT;
b) NEW CONSTRUCTION AND CHANGES IN OCCUPANCY SHALL COMPLY TO CURRENT STANDARDS;
c) ADDITION OR CHANGES IN EXISTING BUILDING CANNOT MAKE THE EXISTING CONDITION WORSE OR BLOCK EXITING.
- MOST CORRECTION TO DISTINCTLY HAZARDOUS CONDITIONS COMPENSATE FOR IMPAIRED EXITING. MANY COMPENSATORY MEASURES ACCEPTABLE TO THE KANSAS STATE FIRE MARSHAL'S OFFICE DO NOT UPGRADE THE BUILDING TO MEET THE LETTER OF A BUILDING, FIRE OR LIFE SAFETY CODE. EXISTING PORTIONS OF MANY BUILDINGS DO NOT COMPLY WITH A CERTAIN BUILDING CODE AND SHOULD BE SPECIFIED AS MEETING THE KANSAS FIRE PREVENTION CODE.

SYMBOL LEGEND

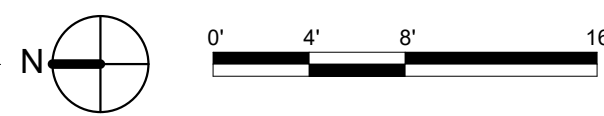


CODE - BUILDING INFORMATION

BUS STORAGE BUILDING		EXTERIOR WALL PROTECTION (TABLE 602)	
OCCUPANCY:	GROUP S-1	<5'	2
CONSTRUCTION TYPE:	II-B	>5', <10'	1
NON SEPERATED USE:	NONE	>10', <30'	0
		>30'	0
AREA PER TABLE 506:	17,500 SF / FLOOR	FIRE RESISTIVE REQUIREMEMNTS (TABLE 601)	
ACTUAL BLDG. SF:	6,449 SF	BUILDING ELEMENT	RATING (HOURS)
ALLOWABLE STORIES:	2	STRUCTURAL FRAME	0
ALLOWABLE HEIGHT:	55'-0"	BEARING WALLS - EXTERIOR	0
ACTUAL STORIES:	1	BEARING WALLS - INTERIOR	0
ACTUAL HEIGHT:	26'-6"	NON BEARING EXTERIOR WALLS	TABLE 602
		NON BEARING INTERIOR WALLS	0
		FLOORS	0
		ROOF	0



1 CODE PLAN - VEHICLE STORAGE
1/8" = 1'-0"



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CIVIL ENGINEERS

MAIZE USD 266
BUS STORAGE BUILDING

4750 NORTH 119TH STREET WEST
MAIZE, KS 67101

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PROJECT NUMBER
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DATE
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CODE COMPLIANCE

G14.1

MABCD REVIEW
STAMP LOCATION

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Wichita, KS, 67202
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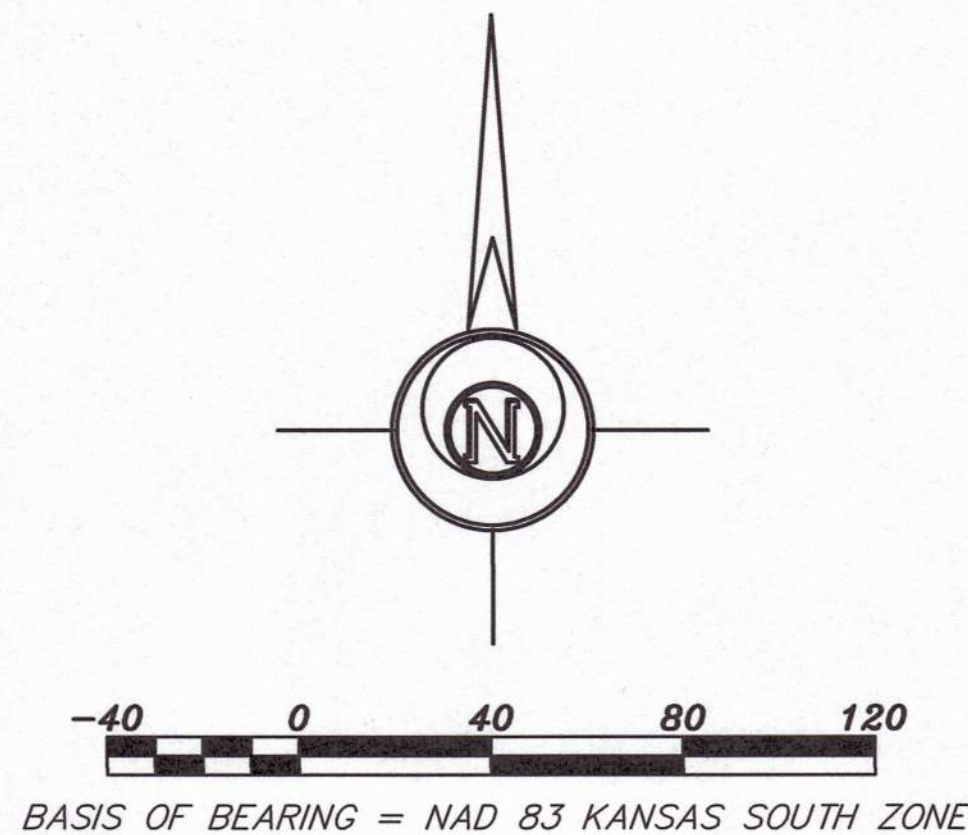
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BUS STORAGE BUILDING
4750 NORTH 119TH STREET WEST
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PROJECT NUMBER
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DATE
MARCH 2026
SURVEY

G14.2



- LEGEND**
- Sectional Monument Found
 - Fence
 - Benchmark
 - Fire Hydrant
 - Light Pole
 - Underground Electric Line

DESCRIPTION:
Topo Survey of a portion of the property owned by USD 266 Maize School District located in the Southwest Quarter of Section 19, Township 26 South, Range 1 West of the 6th Principal Meridian, Sedgwick County, Kansas.

BENCHMARK:
CP1 Magnail in Asphalt located 1,262 +/- feet South and 495 +/- feet East of the Northwest Corner of the Southwest Quarter of Section 30, Township 26 South Range 1 West of the 6th Principal Meridian. Elevation=1,369.81 (NAVD 88)
CP2 Magnail in Asphalt located 1,277 +/- feet South and 676 +/- feet East of the Northwest Corner of the Southwest Quarter of Section 30, Township 26 South Range 1 West of the 6th Principal Meridian. Elevation=1,371.06 (NAVD 88)
CP3 "x" cut in concrete located 1,208 +/- feet South and 495 +/- feet East of the Northwest Corner of the Southwest Quarter of Section 30, Township 26 South Range 1 West of the 6th Principal Meridian. Elevation=1,372.03 (NAVD 88)
CP4 "x" cut in concrete located 852 +/- feet South and 397 +/- feet East of the Southwest Corner of the Southwest Quarter of Section 30, Township 26 South Range 1 West of the 6th Principal Meridian. Elevation=1,368.79 (NAVD 88)
CP5 "x" cut in concrete located 955 +/- feet South and 650 +/- feet East of the Southwest Corner of the Southwest Quarter of Section 30, Township 26 South Range 1 West of the 6th Principal Meridian. Elevation=1,369.12 (NAVD 88)

DIGSAFE:
Utilities shown were located by respective owners. Any Utilities not shown are due to no response to Kansas One Call "1-800-DIG-SAFE" Ticket #25578802.
Utility locations shown hereon are based on information received from Kansas One-Call (1-800-DIG-SAFE) identified by Ticket #25578802 on November 12, 2025. Before any digging, contractor should verify utility locations by calling 1-800-DIG-SAFE.

SURVEYOR'S CERTIFICATE:
I hereby certify this plat to be a true, correct and complete representation of the property described hereon, as surveyed under my supervision, this 20th day of January, 2026.

Signature of Surveyor
PS-683
KANSAS
PROFESSIONAL SURVEYOR

Prepared For: USD #266 MAIZE SCHOOL DISTRICT	Description: A PORTION OF THE SW 1/4 SECTION 19, T26S, R1W SEDGWICK COUNTY, KANSAS
Prepared By: GSS HUTCHINSON 2908 North Plum St. 67502 Ph. 620-465-7032 (Main Office)	BRANCH OFFICES: MCPIERSON Ph. 620-341-4441 SALINA Ph. 785-404-6302 WICHITA Ph. 316-268-9933
Drawn By: KLL Checked By: DEG	Scale: 1"=40' Date: 01/20/2026
Date of Field Work: November 14, 2025 Sheet 1 of 1 Sheet(s)	Job No: G2025-1057

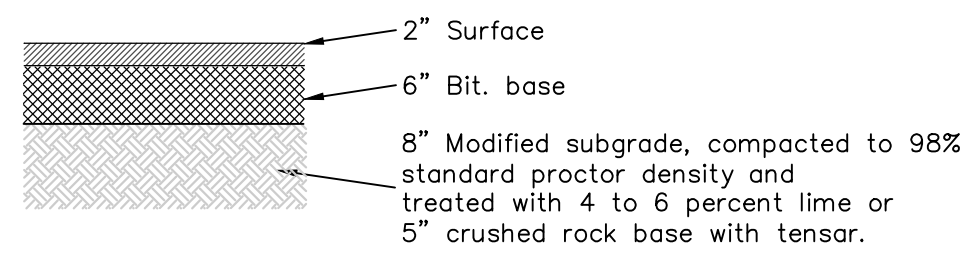
NW Cor. SW 1/4
Sec. 30, T26S, R1W
Fd. Magnail
from previous Carver,
Proj. No. 22141384
dated 5-1-2024

NORTH 119th STREET WEST
S 01°48'04" E 2,647.85' (M)
Asphalt

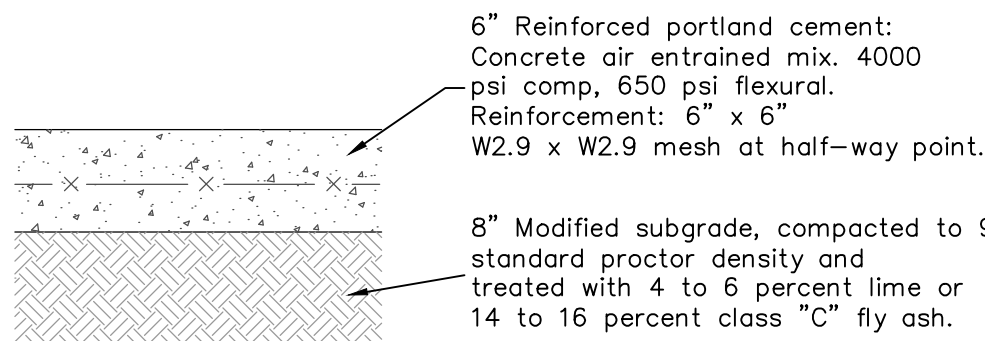
SW Cor. SW 1/4
Sec. 30, T26S, R1W
Fd. 1/2" Rebar w/ Garver LS 331 Cop
from previous MICO Engineering Inc
Proj. No. 2301010137
dated 6-29-23

Copyright © 2026, Schaefer Surveying Service, P.A. Saved 1/20/2026 1:01:34 PM by KLVON
Print Date & Time: Tuesday, January 20, 2026 3:08:47 PM Draw Path: \\A:\DOW\SSS\FILES\2025\1057_GSA.dwg\20251057.DWG

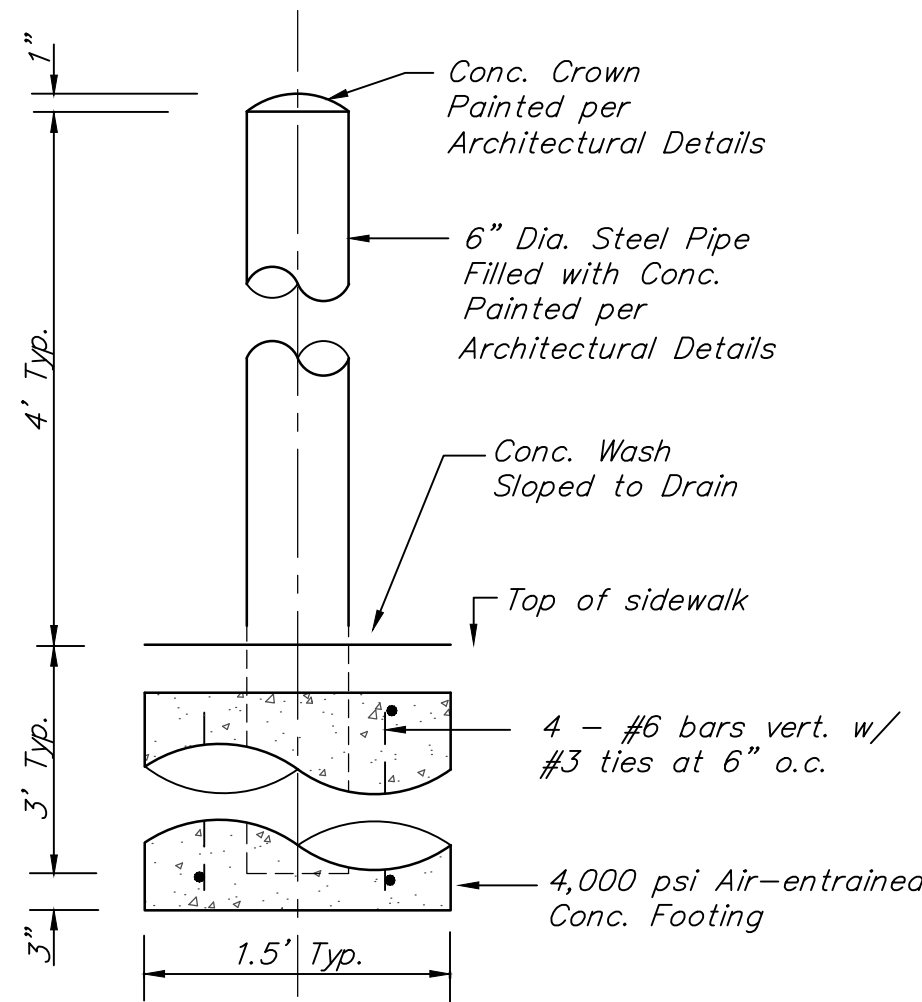
1 8" ASPHALT PAVEMENT
NOT TO SCALE



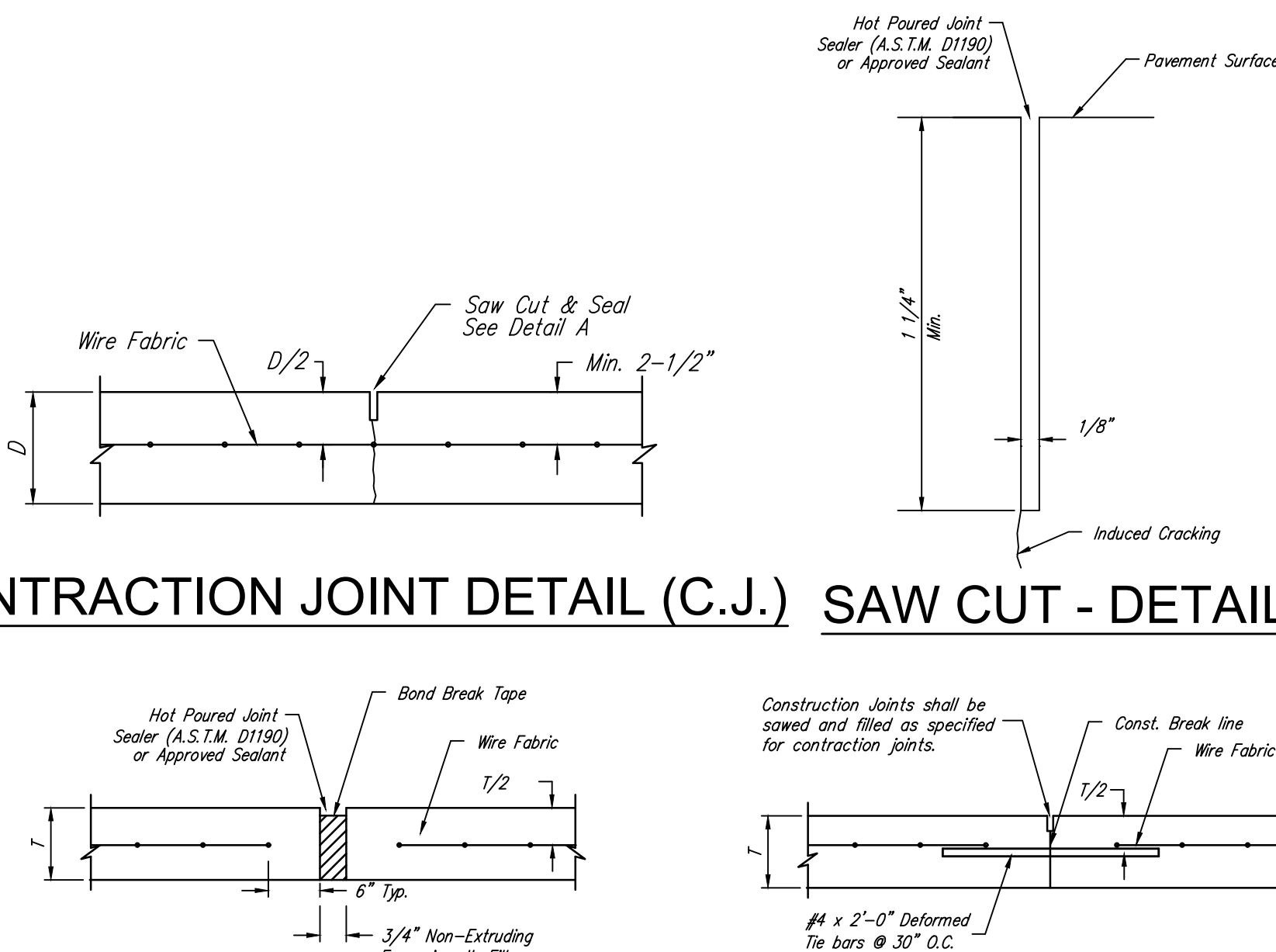
2 6" CONCRETE PAVEMENT
NOT TO SCALE



3 BOLLARD DETAIL
NOT TO SCALE



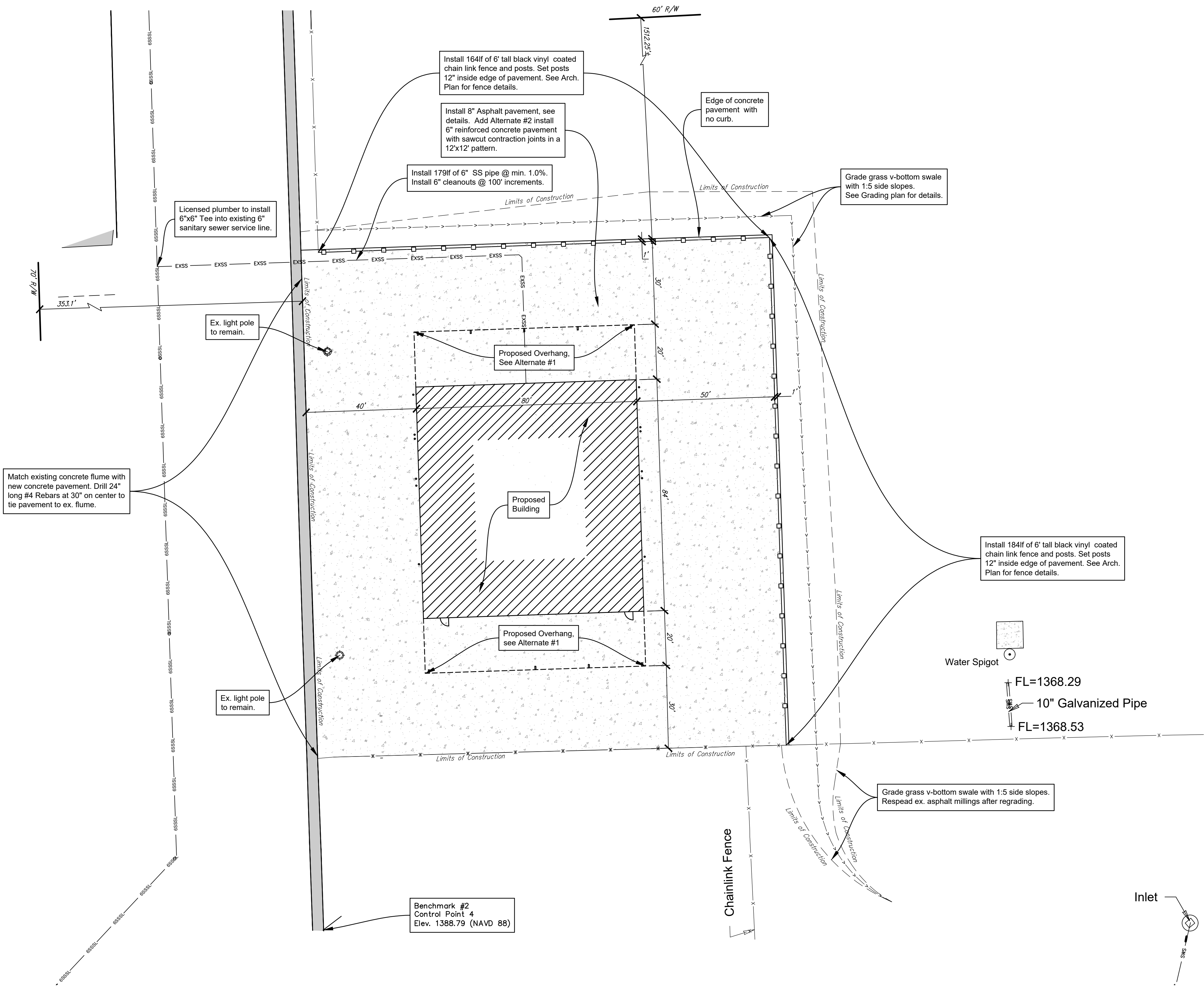
CONTRACTION JOINT DETAIL (C.J.) SAW CUT - DETAIL A



EXPANSION JOINT (E.J.)

CONSTRUCTION JOINT

3 CONCRETE PAVEMENT JOINT DETAILS
NOT TO SCALE



SITE DIMENSION PLAN
SCALE 50 0 50

SITE DEMOLITION PLAN
SCALE 20 0 20

SITE NOTES

- Existing utilities and their locations, as shown on the plans, represent the best information obtainable for design. Location information has been obtained from the various utility companies and is either from company record drawings or company provided field locations. The Contractor will be required to work around existing utilities which do not conflict with proposed construction.
- Traffic affected by construction on this project shall be handled in accordance with the latest edition of the Manual on Uniform Traffic Control Devices (MUTCD).
- The Contractor shall provide appropriate signage throughout the sequence of construction clearly stating where construction entrances are.
- The Contractor shall take all necessary precautions to protect the general public during the construction and demolition process. This may include the use of fences and signage.
- The Contractor shall notify adjacent landowners prior to proceeding with any construction work on landowner's property.
- Signaling and striping shall be installed in accordance with Local City Specifications and Codes. Striping shall be painted markings in conformance with the latest edition of the Standard Specifications for State Road and Bridge Construction by the Kansas Department of Transportation.
- Dimensions shown are to face of curb or edge of pavement unless otherwise noted.

DEMOLITION NOTES

- Existing utilities and their locations, as shown on the plans, represent the best information obtainable for design. Location information has been obtained from the various utility companies and is either from company record drawings or company provided field locations. The Contractor will be required to work around existing utilities which do not conflict with proposed construction.
- Traffic affected by construction on this project shall be handled in accordance with the latest edition of the Manual on Uniform Traffic Control Devices (MUTCD).
- The Contractor shall provide appropriate signage throughout the sequence of construction clearly stating where construction entrances are.
- The Contractor shall take all necessary precautions to protect the general public during the construction and demolition process. This may include the use of fences and signage.
- DEMOLITION
- The Contractor shall dispose of all site debris and trash in addition to all removed materials in a safe, legal, and responsible manner.
- The contractor shall remove all existing private signs and footings located on site as necessary for construction.
- All existing pavement on site shall be removed shall be removed and disposed of in a safe, legal, and responsible manner. Removal shall include base course and sub grade as necessary.
- The Contractor shall coordinate with local utility company on the removal of electric service lines, meters and poles located on site. Contractor shall ensure that all services are located and accounted for before disconnections begin to ensure that adjacent businesses are not affected.
- The Contractor shall coordinate with local gas utility on the removal of gas services and meters located on site. Contractor shall ensure that all services are located and accounted for before disconnections begin to ensure that adjacent businesses are not affected.

LEGAL DESCRIPTION

Part of Lot 1, Block A Pray-Woodman Addition, Sedgwick County, Kansas
4700 N. 119th Maize KS 67101.

SITE INFORMATION

Total Area:	5,529,926 sq. ft. (±126.95 acres)
Disturbed Area:	37,540 sq. ft. (±0.86acres)
Impervious Area, pre-const:	0 sq. ft. (±0.00 acres)
Impervious Area, post-const:	31,635 sq. ft. (±0.73 acres)

BENCHMARKS

- Site Benchmark - 1
Control Point #2, 566.5' East of west property Line, 5' north of the asph. drive, north of this area.
Elevation = 1371.06 (NAVD 88)
- Site Benchmark - 2
Control Point #1, at the south edge of the concrete flume at the Southwest corner of the construction site
Elevation = 1368.79 (NAVD 88)

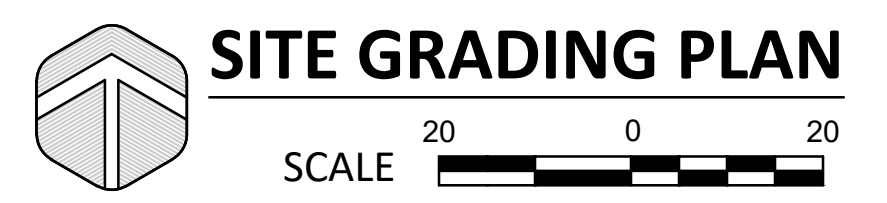
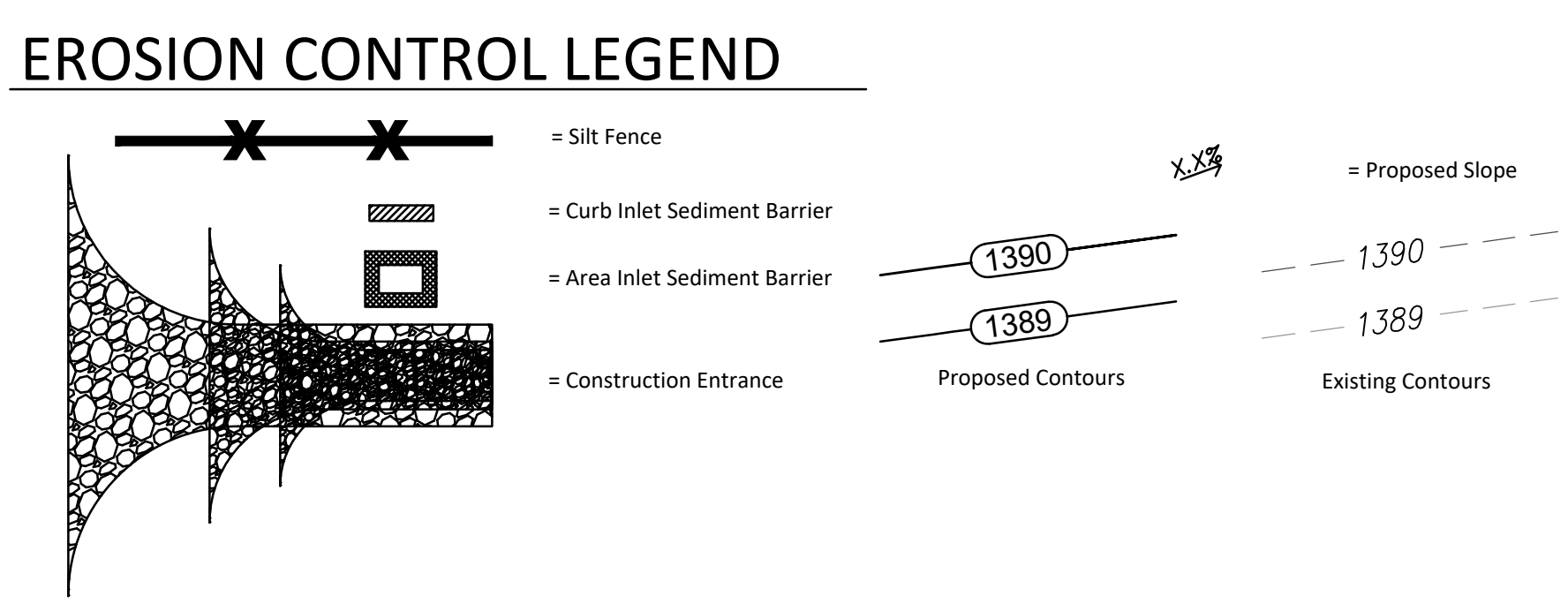
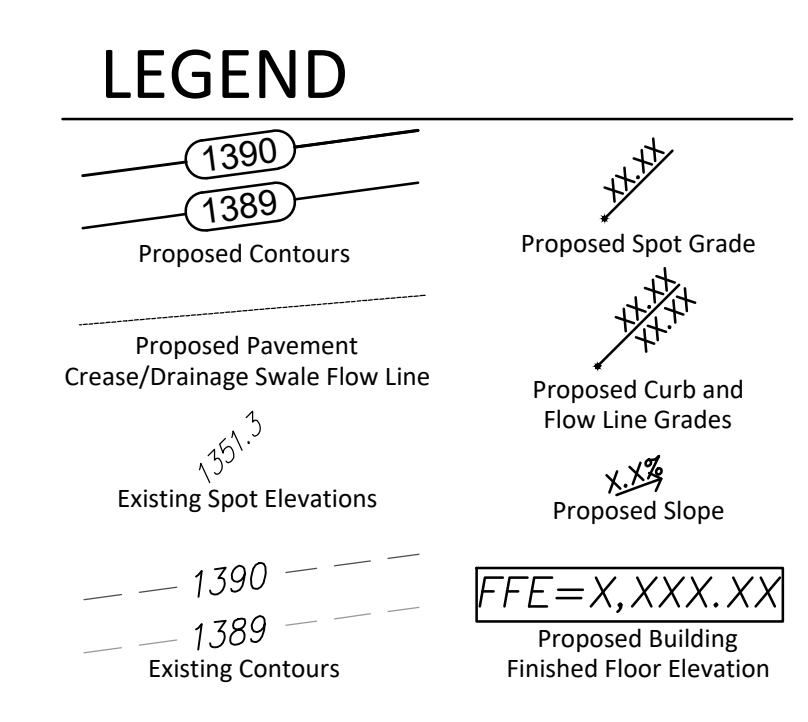
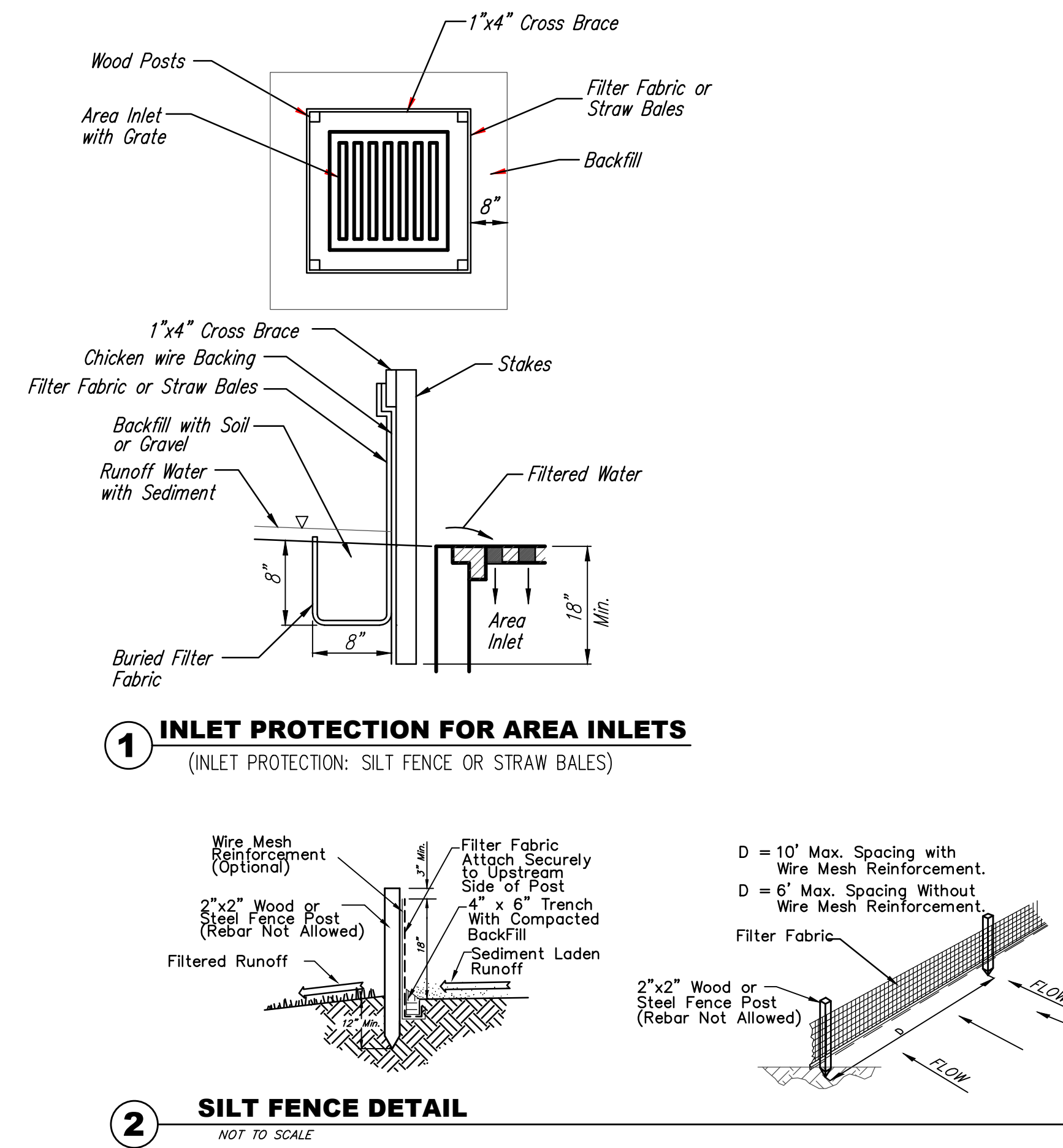
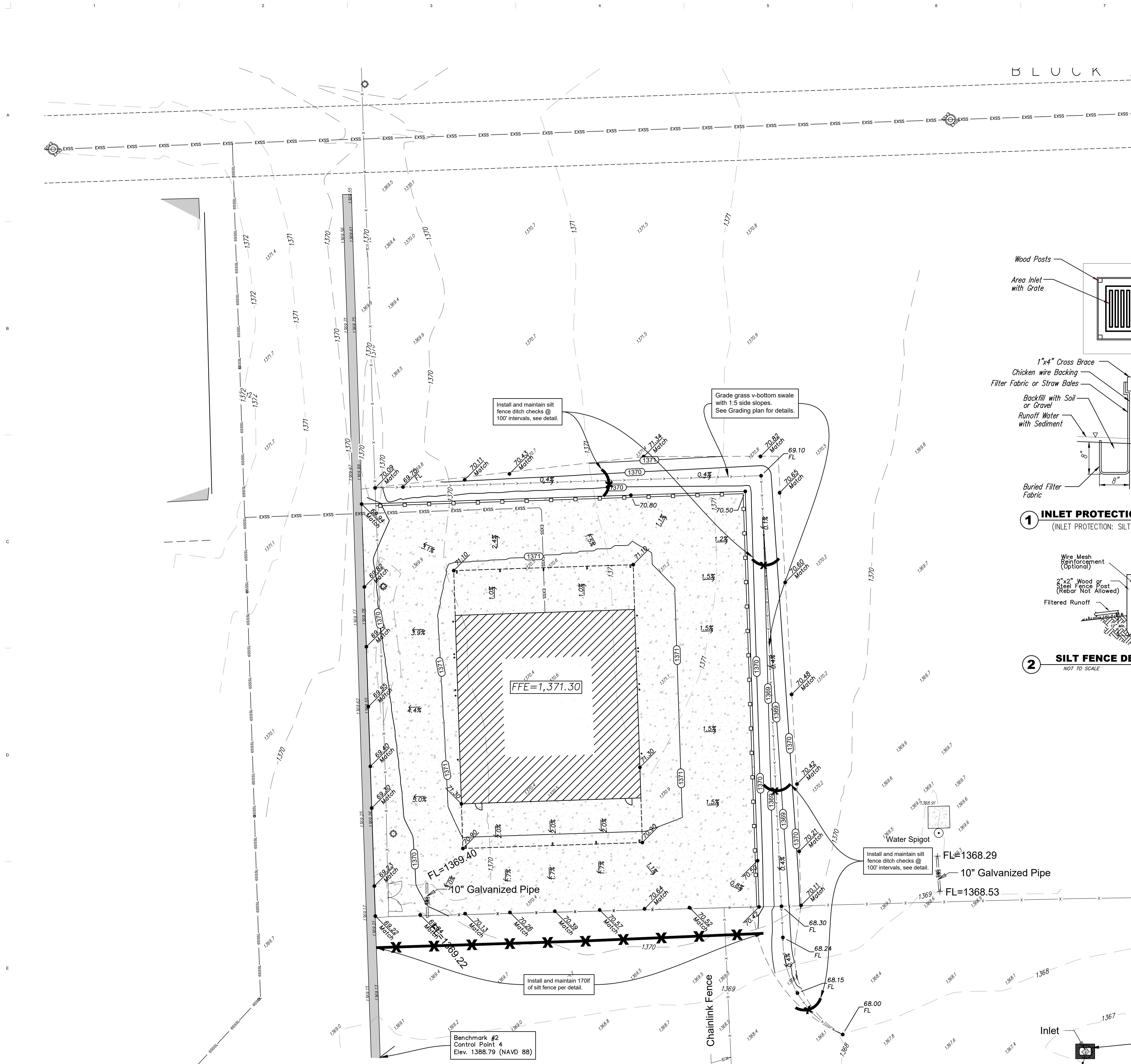
KANSAS ONE CALL

Contractor shall be required to provide notice to Kansas One Call at 287-2470 a minimum of three (3) working days prior to any excavation or work adjacent to utilities.
Kansas One Call 1-800-DIG-SAFE

UTILITY CONTACTS

The Contractor must notify the following in case of an emergency:	
Kansas Gas Service (Gas).....	1-888-482-4950
Black Hills Energy (Gas).....	1-800-303-0357
Energry (Electric).....	383-8650
Cox Communications (Telecommunication).....	262-4270
AT&T (Telecommunication).....	268-2759
City of Maize Water Dept. (Water).....	722-7561
City of Maize Sewer Maint.(San. Sewer).....	722-7561
City of Maize Storm Sewer Maint. (Storm Sewer).....	722-7561
City of Maize Traffic Maint.(Traf. Control).....	722-7561
Conoco Pipeline Co. (Petroleum).....	1-800-231-2551
Williams Pipeline Co. (Petroleum).....	529-6600
	or 1-800-324-9696
Phillips Pipeline Co. (Petroleum).....	1-800-766-8230
Sedgwick County Rural Electric CO-OP.....	542-3131





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Part of Lot 1, Block A Pray-Woodman Addition, Sedgwick County, Kansas
4700 N. 119th Maize KS 67101.

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work adjacent to utilities.
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UTILITY CONTACTS

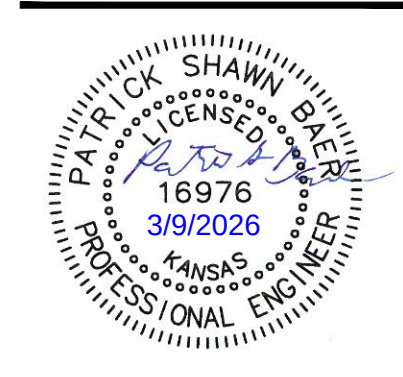
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Black Hills Energy (Gas).....	1-800-303-0357
Evergry (Electric).....	383-8650
Cox Communications (Telecommunication).....	262-4270
AT&T (Telecommunication).....	268-2759
City of Maize Water Dept. (Water).....	722-7561
City of Maize Sewer Maint. (San. Sewer).....	722-7561
City of Maize Storm Sewer Maint. (Storm Sewer).....	722-7561
City of Maize Traffic Maint. (Traf. Control).....	722-7561
Conoco Pipeline Co. (Petroleum).....	1-800-231-2551
Williams Pipeline Co. (Petroleum).....	529-6600
	or 1-800-324-9696
Phillips Pipeline Co. (Petroleum).....	1-800-766-8230
Sedgwick County Rural Electric CO-OP.....	542-3131

GRADING NOTES

- Existing utilities and their locations, as shown on the plans, represent the best information obtainable for design. Location information has been obtained from the various utility companies and is either from company record drawings or company provided field locations. The Contractor will be required to work around existing utilities which do not conflict with proposed construction.
- Traffic affected by the construction on this project shall be handled in accordance with the latest edition of the Local Manual on Uniform Traffic Control Devices.
- The Contractor shall verify all utility locations prior to construction of this project.
- Refer to Landscape Plan for final treatment of all disturbed areas.
- Signing and striping shall be installed in accordance with local City Specifications and Codes. Striping shall be painted markings in conformance with the latest edition of the Standard Specifications for State Road and Bridge Construction by the Kansas Department of Transportation.
- Paved Lot to be constructed with Paving as shown. See Architectural Site Plan and Geotechnical Report for Pavement and subgrade details.
- Proposed underground storm sewer mains, water mains/fire hydrants, or sanitary sewer mains, if any, shall be the contractor's responsibility. This portion of the project shall be designed and constructed under a separate plan set or private project, and undergo local City review and approval. The installation of any mains is required to be done by a bonded contractor, inspected and certified. The Contractor shall construct the mains only from Local City approved plans which will be provided to the General Contractor or can be provided by Baughman Company, P.A.
- The Contractor shall notify adjacent landowners prior to proceeding with any construction work on landowner's property.
- Cross-Slopes on sidewalks shall not exceed 1/4" per foot (or 2%). Notify Engineer of any discrepancies prior to forming of walks. Longitudinal slopes that exceed 5% will be considered ramps and will require handrails and edge protection per ADA.
- The final grade of all non-paved areas adjacent to the building shall be set at least 0.4' below FFE, unless otherwise noted.
- Contours and slope arrows are computer generated and are schematic in nature. Spot grades and notes from the stamped plans in addition to ADA guidelines shall govern construction.
- Proposed site grading shall not cause significant additional adverse impacts on adjacent property.



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MAIZE USD 266
BUS STORAGE BUILDING

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MAIZE, KS 67101

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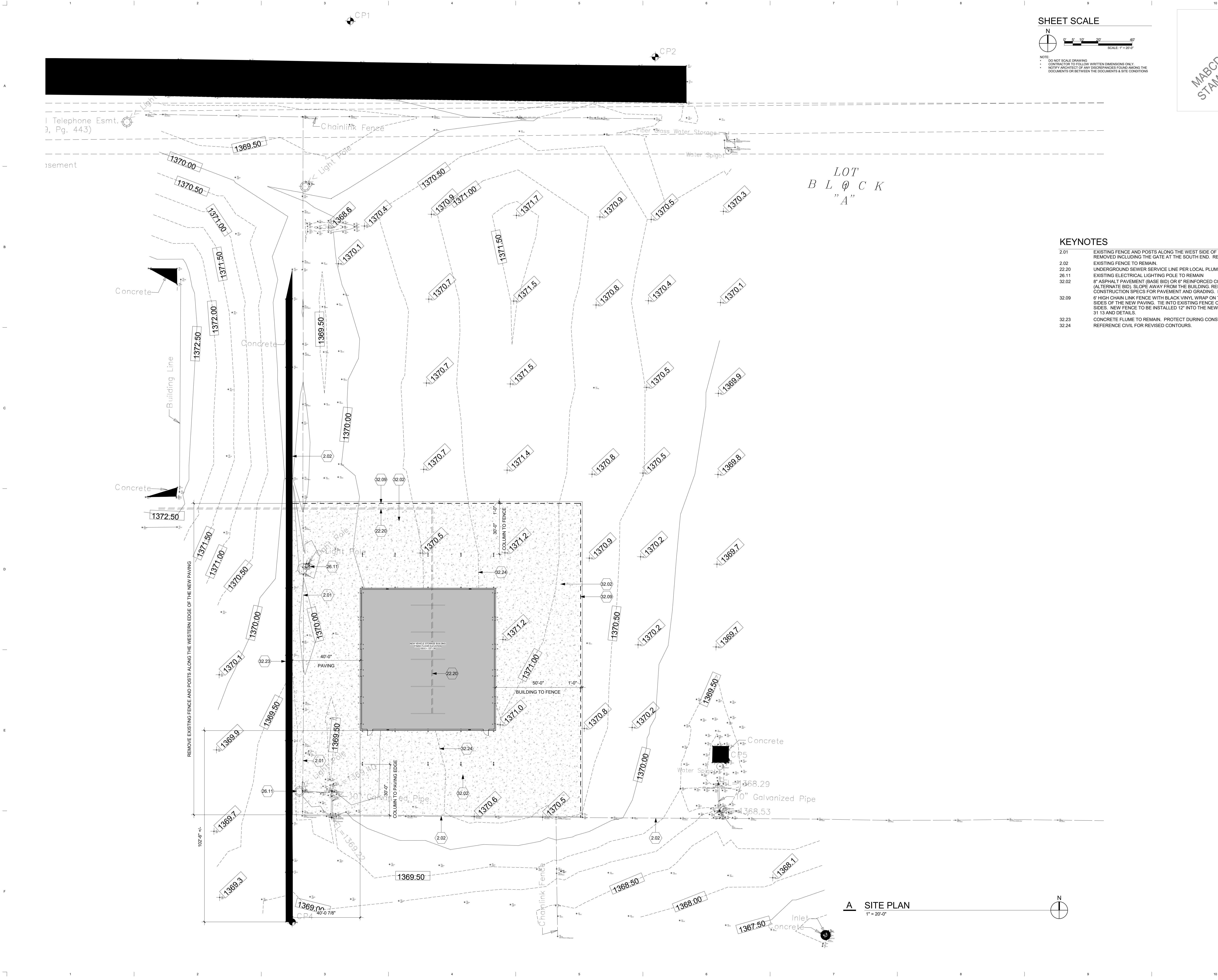
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PROJECT NUMBER
5537.50

DATE
MARCH 2026

SITE
GRADING &
EROSION
CONTROL
PLAN

C2.0



SHEET SCALE



NOTE:
- DO NOT SCALE DRAWING.
- CONTRACTOR TO FOLLOW WRITTEN DIMENSIONS ONLY.
- NOTIFY ARCHITECT OF ANY DISCREPANCIES FOUND AMONG THE DOCUMENTS OR BETWEEN THE DOCUMENTS & SITE CONDITIONS.

MABCD REVIEW
STAMP LOCATION

KEYNOTES

- 2.01 EXISTING FENCE AND POSTS ALONG THE WEST SIDE OF THE NEW PAVING TO BE REMOVED INCLUDING THE GATE AT THE SOUTH END. REF. CIVIL.
2.02 EXISTING FENCE TO REMAIN.
22.20 UNDERGROUND SEWER SERVICE LINE PER LOCAL PLUMBING CODES - REF. CIVIL.
26.11 EXISTING ELECTRICAL LIGHTING POLE TO REMAIN.
32.02 8" ASPHALT PAVEMENT (BASE BID) OR 6" REINFORCED CONCRETE PAVEMENT (ALTERNATE BID). SLOPE AWAY FROM THE BUILDING. REF. CITY OF WICHITA CONSTRUCTION SPECS FOR PAVEMENT AND GRADING. - REF. CIVIL.
32.09 6" HIGH CHAIN LINK FENCE WITH BLACK VINYL WRAP ON THE NORTH AND EAST SIDES OF THE NEW PAVING. TIE INTO EXISTING FENCE ON THE SOUTH AND EAST SIDES. NEW FENCE TO BE INSTALLED 12" INTO THE NEW PAVING - REF. SPEC. 32 31 13 AND DETAILS.
32.23 CONCRETE FLUME TO REMAIN. PROTECT DURING CONSTRUCTION. - REF. CIVIL.
32.24 REFERENCE CIVIL FOR REVISED CONTOURS.

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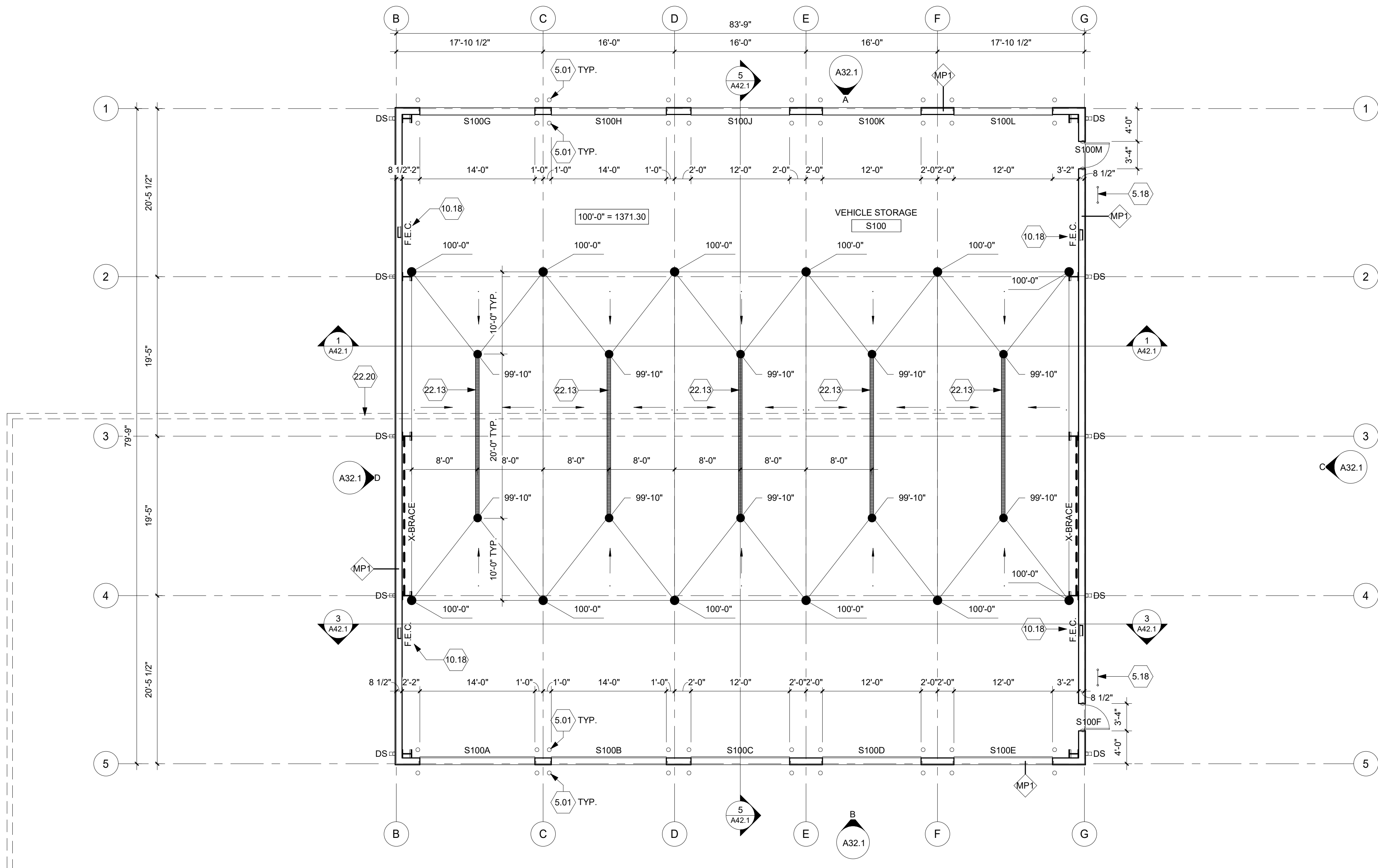
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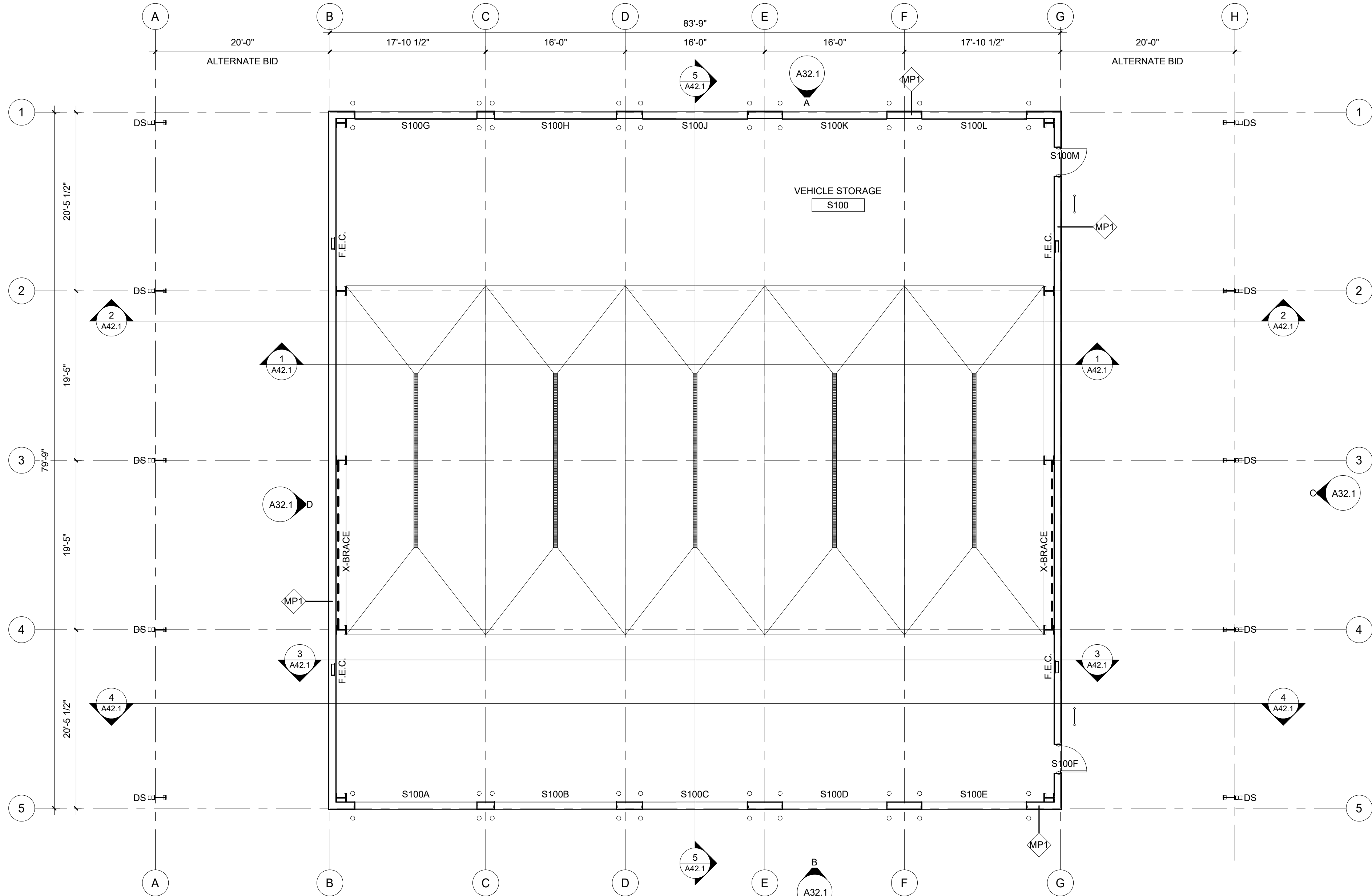
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ARCHITECTURAL
SITE PLAN

A20.0

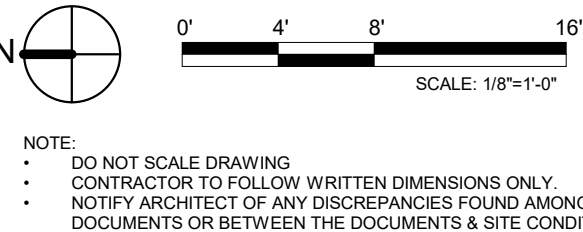


1 FLOOR PLAN - VEHICLE STORAGE (BASE BID)
1/8" = 1'-0"



2 FLOOR PLAN - VEHICLE STORAGE (ALT)
1/8" = 1'-0"

SHEET SCALE



GENERAL NOTES

- CONTRACTOR TO VERIFY EXISTING SITE AND BUILDING CONDITIONS PRIOR TO BIDDING
- REFERENCE SHEET 010.1 FOR GRAPHIC SYMBOL AND DRAWING SYMBOL DEFINITIONS
- HATCHED WALL WITH DARK (BLACK) PERIMETER LINES INDICATE NEW WALLS. SEE ENLARGED PLANS FOR ALL ACCESSORY LOCATIONS THAT REQUIRE IN-WALL BLOCKING FOR ATTACHMENT.
- ABBREVIATIONS:
 - CJ CONTROL JOINT - REF. DETAIL
 - FEC FIRE EXTINGUISHER AND BRACKET - REF. SPEC.
 - DS DOWNSPOUT (OVERSIZED)
 - NIC NOT IN CONTRACT, ITEM PROVIDED AND INSTALLED BY OWNER
 - VIF VERIFY IN FIELD ACTUAL CONDITIONS

KEYNOTES

- 5.01 6" TALL GALVANIZED STEEL PIPE BOLLARD FILLED WITH CONCRETE WITH SLEEVE - PAINT - REF. DETAIL A1/A61.1
- 5.18 STEEL FOOT SCRAPER - REF. SPEC. AND DETAIL E5/A61.1
- 10.18 FIRE EXTINGUISHER ON BRACKET - REF. SPEC.
- 22.13 4" INTERNAL WIDTH TRENCH DRAIN SLOPED AT 0.5% CONSTANT SLOPE TO THE WEST. SLOPE SLAB TO DRAIN AS SHOWN - REF. MECH.
- 22.20 UNDERGROUND SEWER SERVICE LINE PER LOCAL PLUMBING CODES - REF. CIVIL.

MABCD REVIEW
STAMP LOCATION

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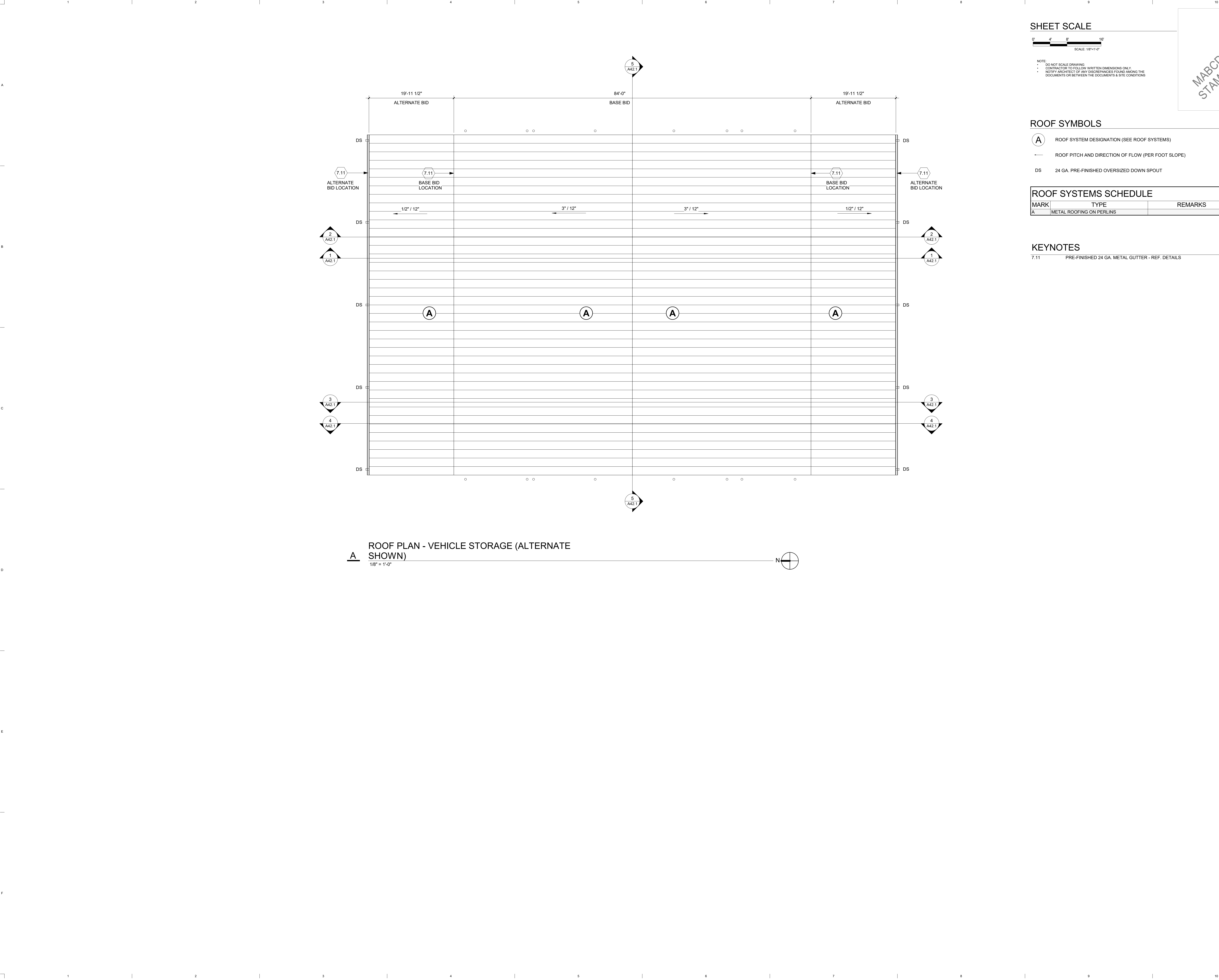
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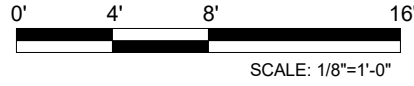
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FLOOR PLAN

A22.1



SHEET SCALE



NOTE:
• DO NOT SCALE DRAWING.
• CONTRACTOR TO FOLLOW WRITTEN DIMENSIONS ONLY.
• NOTIFY ARCHITECT OF ANY DISCREPANCIES FOUND AMONG THE DOCUMENTS OR BETWEEN THE DOCUMENTS & SITE CONDITIONS.

ROOF SYMBOLS

- A** ROOF SYSTEM DESIGNATION (SEE ROOF SYSTEMS)
- ROOF PITCH AND DIRECTION OF FLOW (PER FOOT SLOPE)
- DS 24 GA. PRE-FINISHED OVERSIZED DOWN SPOUT

ROOF SYSTEMS SCHEDULE

MARK	TYPE	REMARKS
A	METAL ROOFING ON PERLINS	

KEYNOTES

7.11 PRE-FINISHED 24 GA. METAL GUTTER - REF. DETAILS

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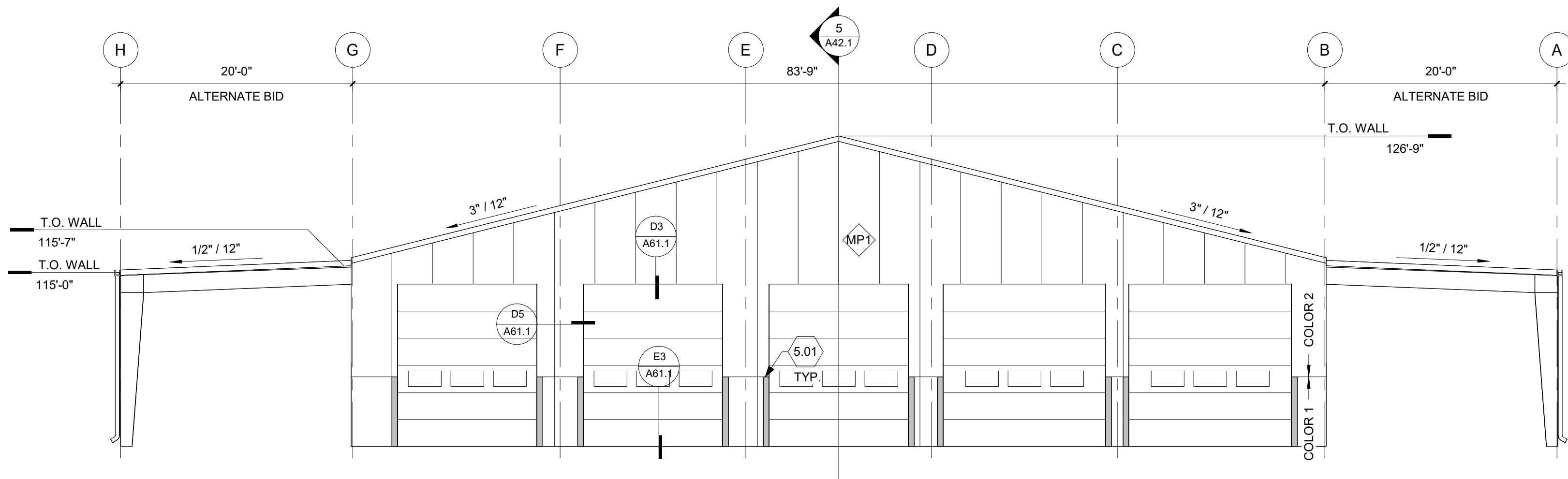
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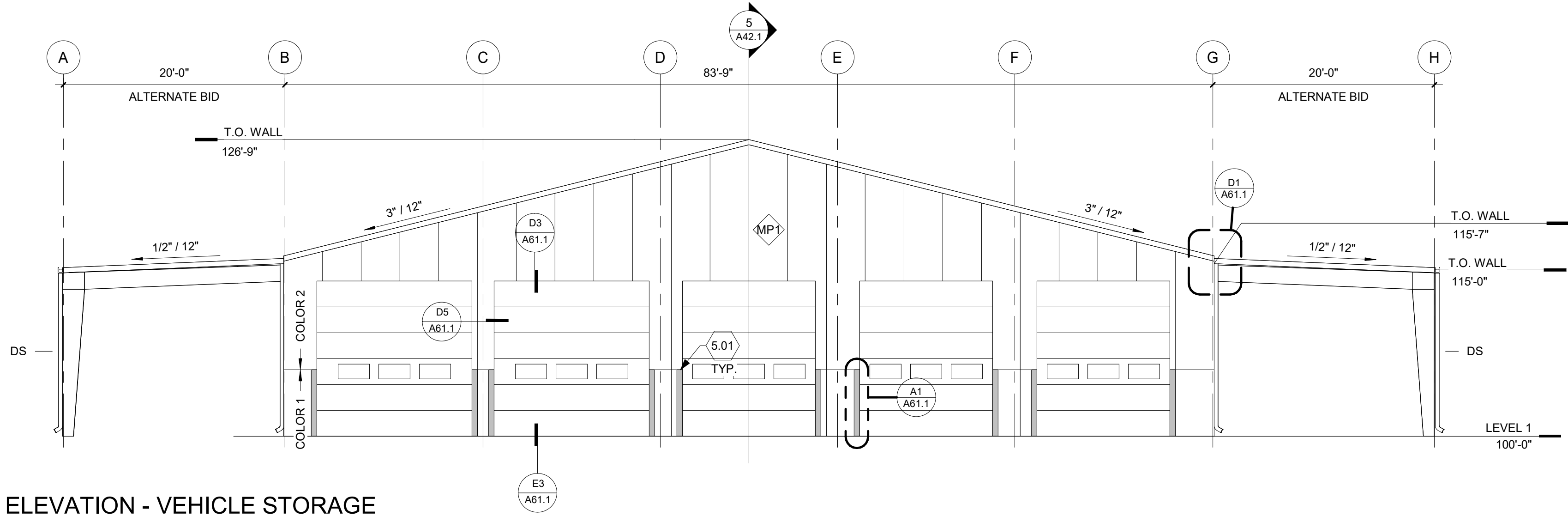
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ROOF PLAN

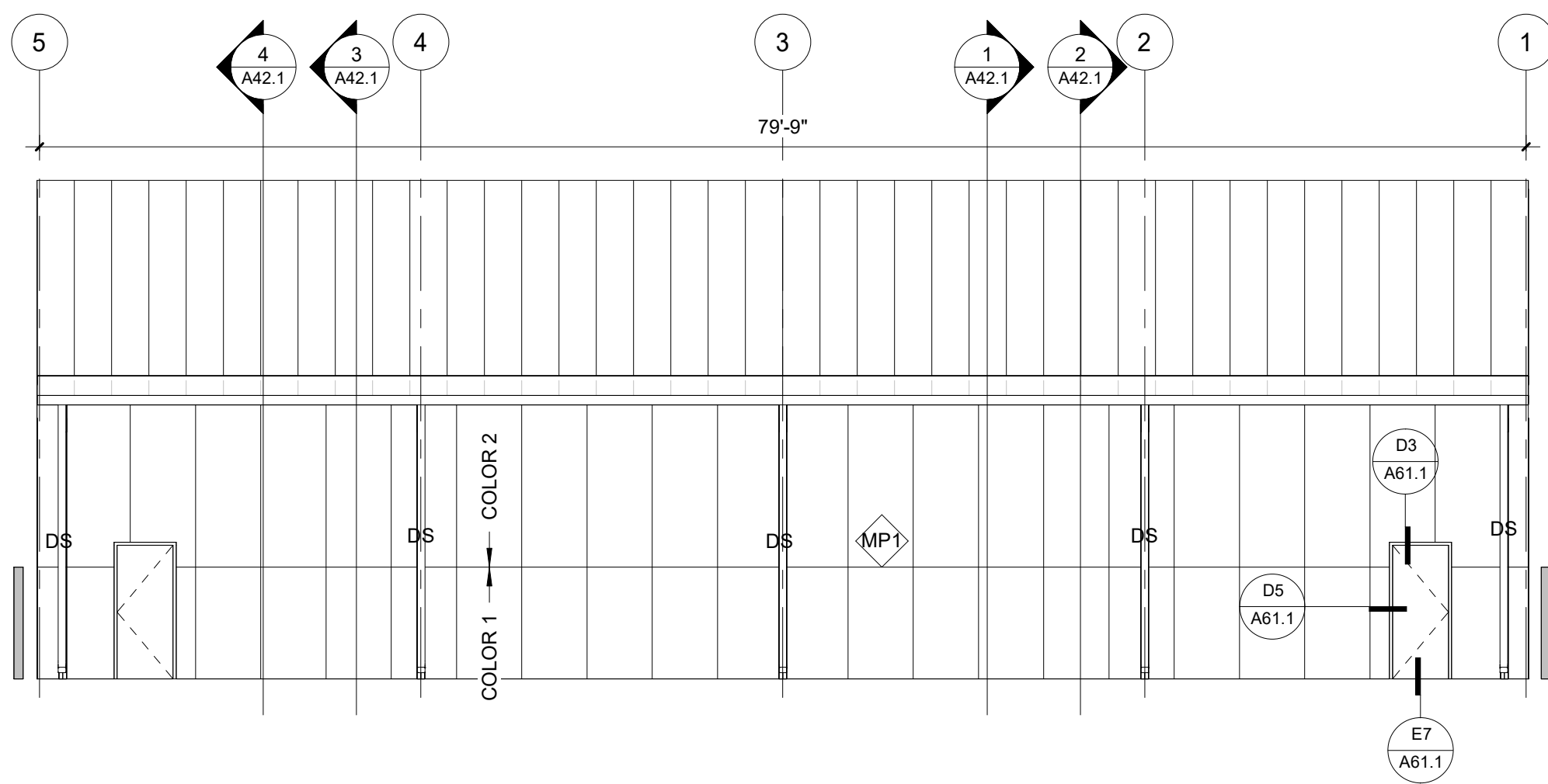
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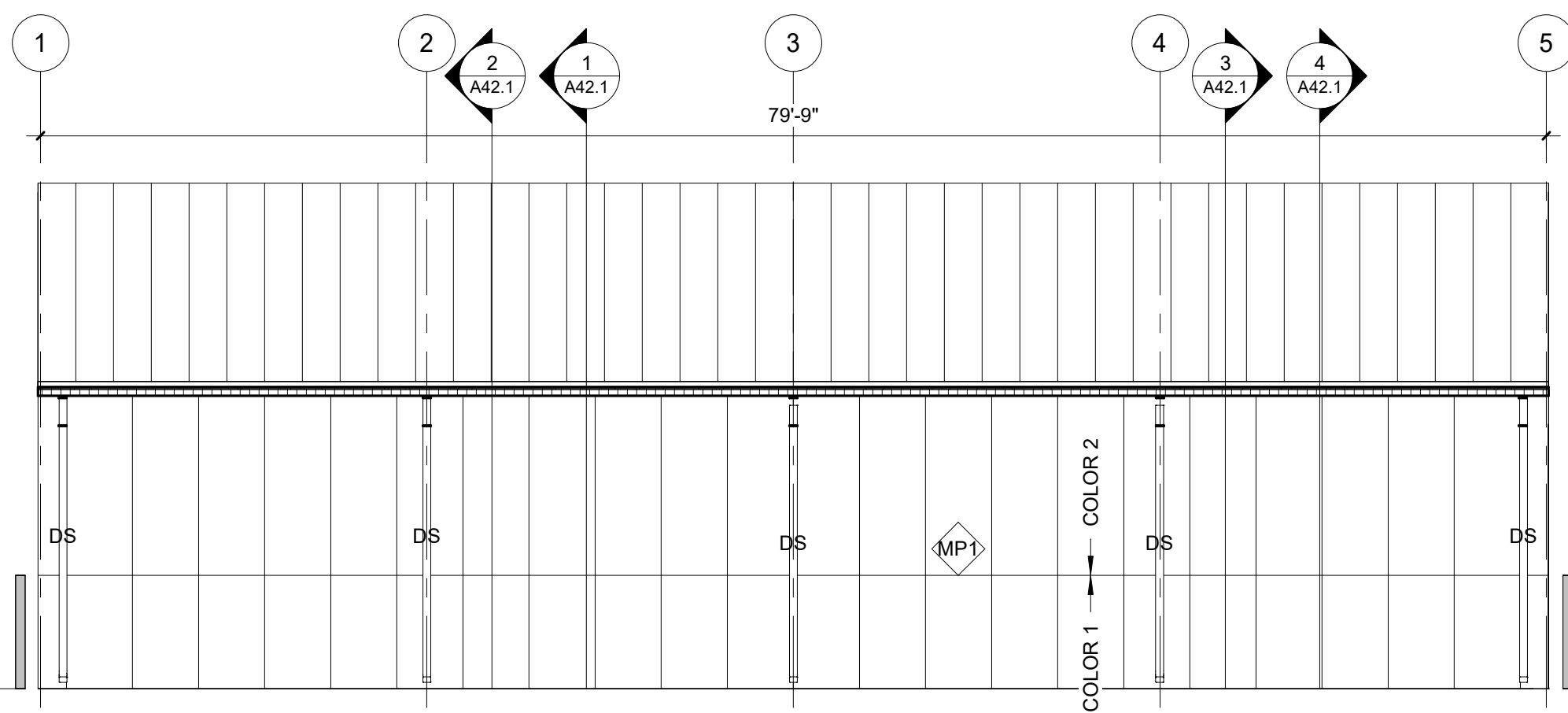
A EAST ELEVATION - VEHICLE STORAGE
1/8" = 1'-0"



B WEST ELEVATION - VEHICLE STORAGE
1/8" = 1'-0"



C SOUTH ELEVATION - VEHICLE STORAGE
1/8" = 1'-0"



D NORTH ELEVATION - VEHICLE STORAGE
1/8" = 1'-0"

MABCD REVIEW
STAMP LOCATION

GENERAL NOTES

- CONTRACTOR SHALL BE RESPONSIBLE TO BUILD TO FIELD CONDITIONS AND VERIFY ALL DIMENSIONS.
- FOR DOOR SCHEDULE AND FRAME ELEVATIONS REFER TO A72 SERIES SHEETS.
- MATERIALS LEGEND AS INDICATED:

PM1 - PRE-FINISHED METAL PANEL 1
PRE-FINISHED METAL PANEL, FLASHING, GUTTERS AND DOWNSPOUTS - REF. DETAILS AND SPEC.

KEYNOTES

5.01 6" TALL GALVANIZED STEEL PIPE BOLLARD FILLED WITH CONCRETE WITH SLEEVE - PAINT - REF. DETAIL A1/A61.1

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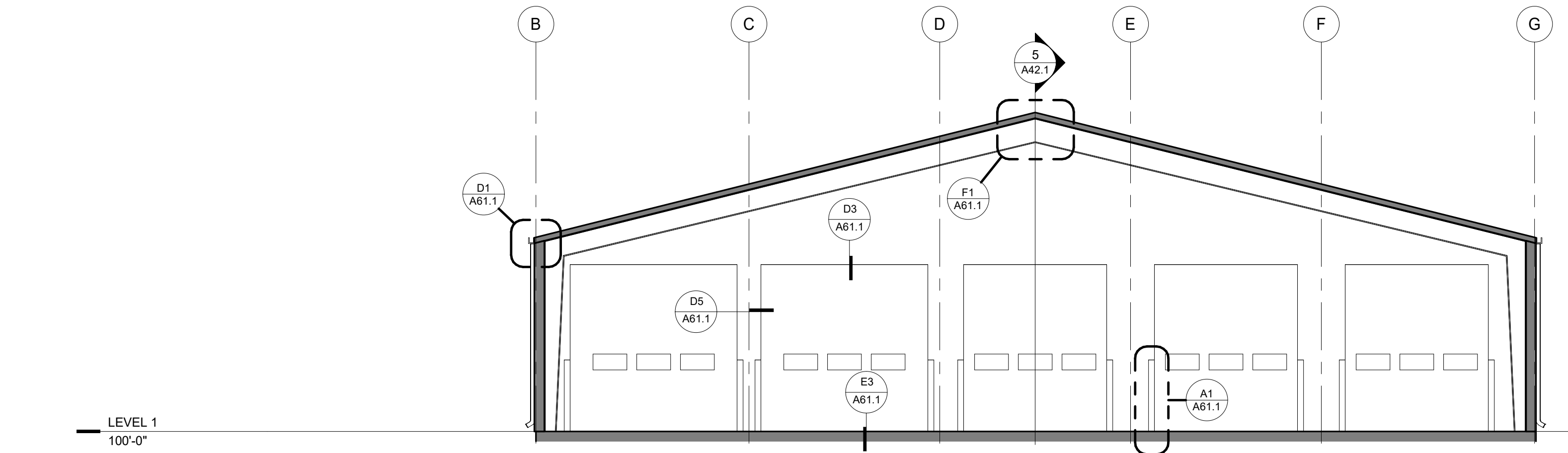
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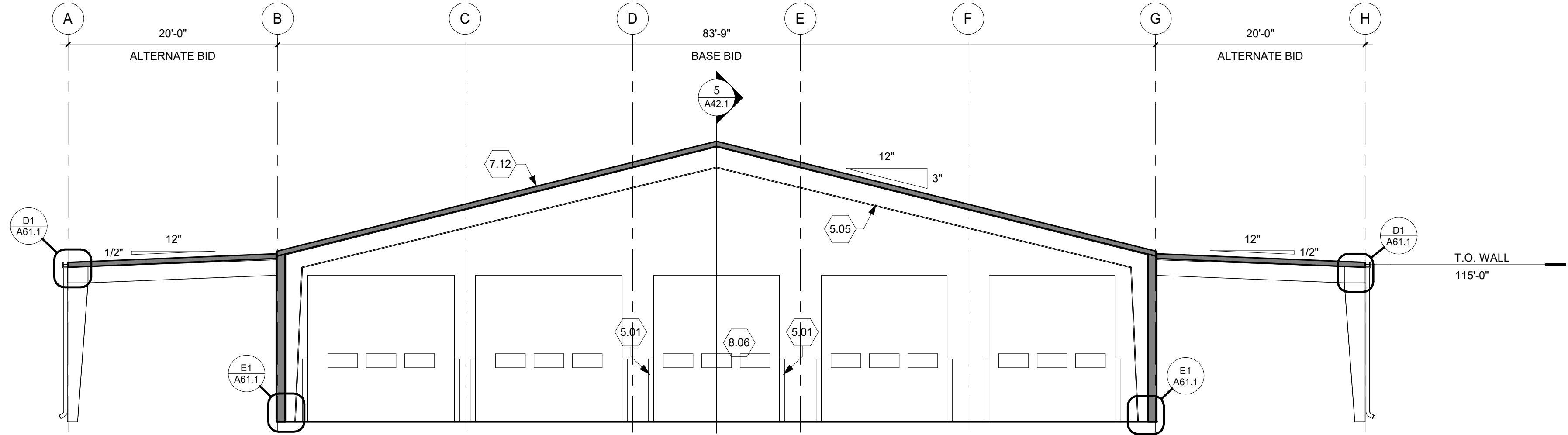
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BUILDING
ELEVATIONS

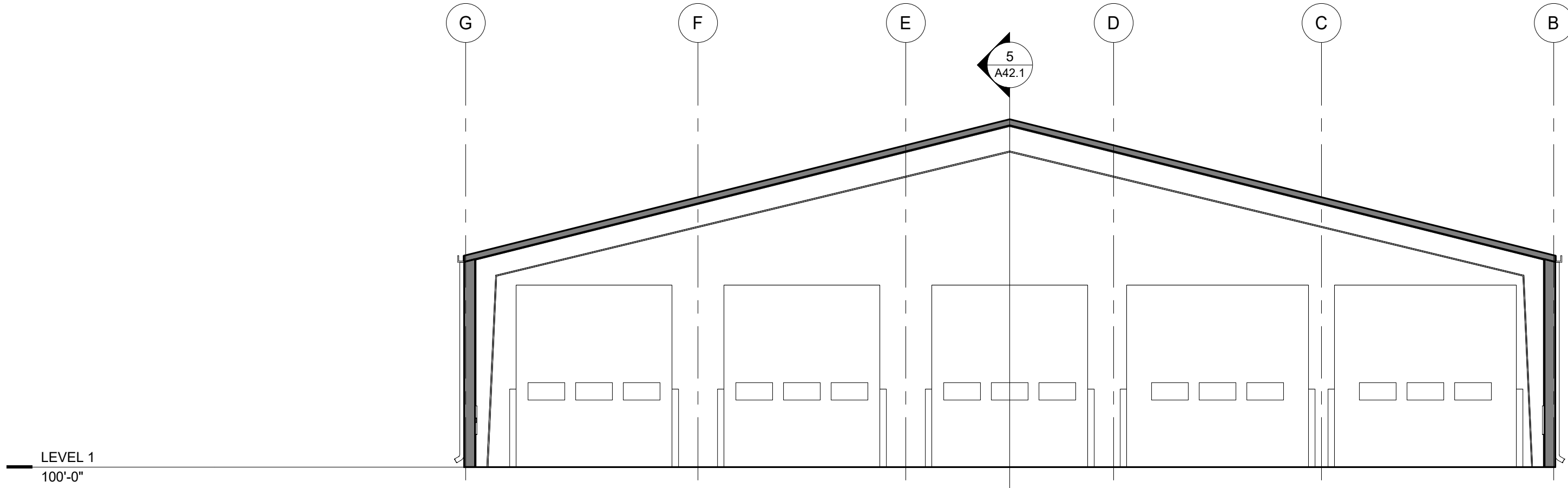
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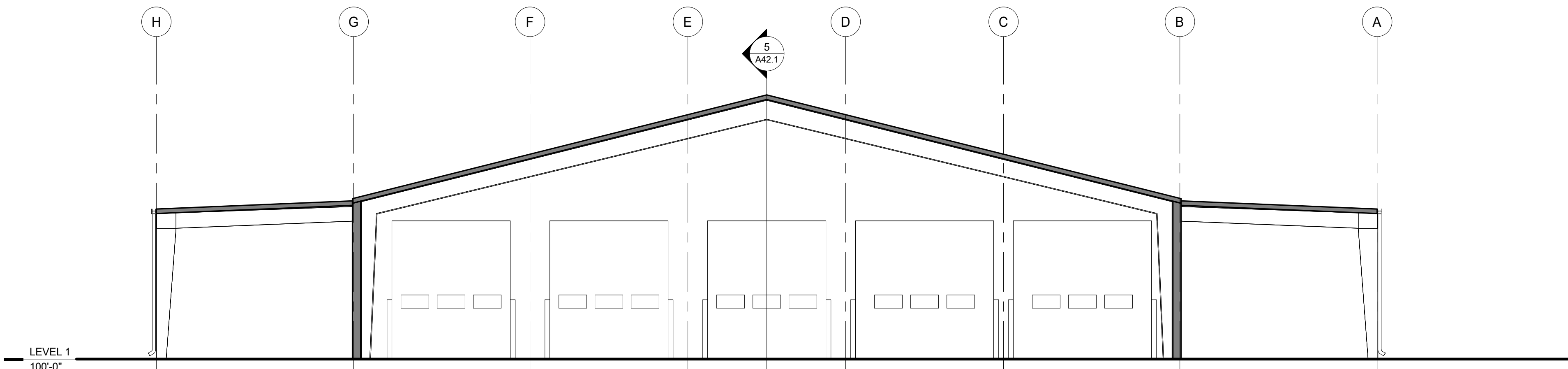
1 BUILDING SECTION (BASE BID)
1/8" = 1'-0"



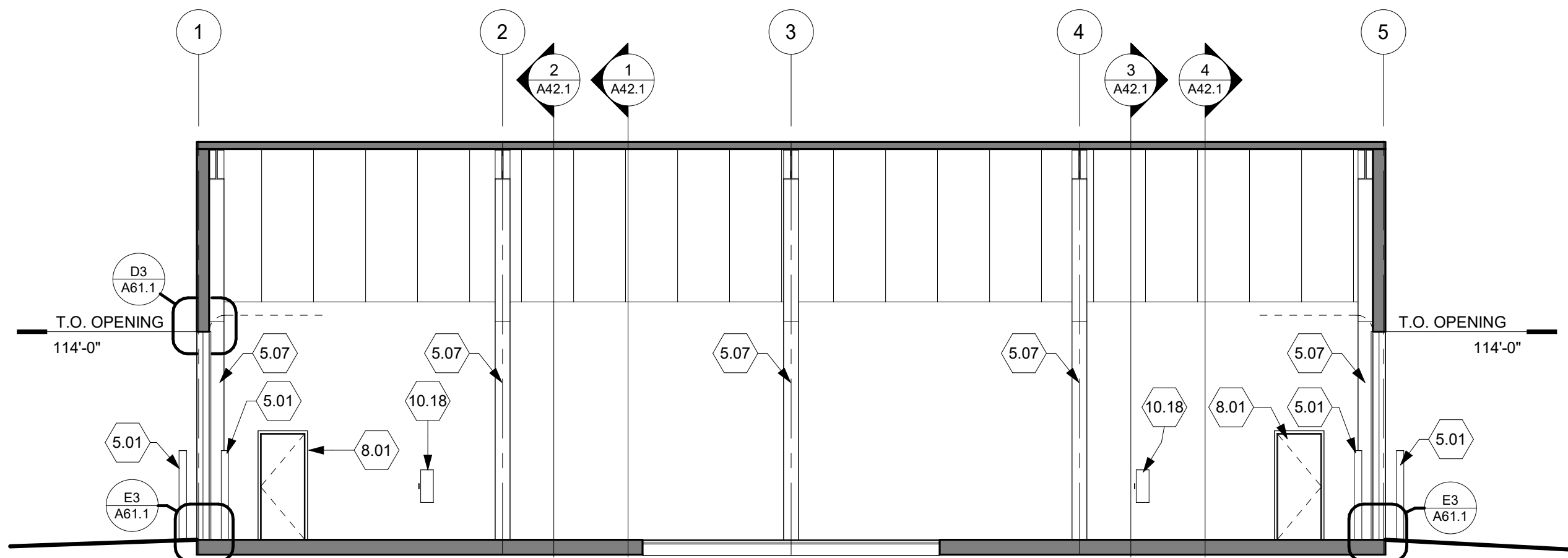
2 BUILDING SECTION (ALT)
1/8" = 1'-0"



3 BUILDING SECTION (BASE BID)
1/8" = 1'-0"



4 BUILDING SECTION (ALT)
1/8" = 1'-0"



5 BUILDING SECTION (BASE BID)
1/8" = 1'-0"

MABCD REVIEW
STAMP LOCATION

GENERAL NOTES

- CONTRACTOR TO VERIFY EXISTING SITE AND BUILDING CONDITIONS PRIOR TO BIDDING
- FLOOR ELEVATIONS ARE TO THE FACE OF CONCRETE WITHOUT FLOOR FINISH, UNLESS NOTED OTHERWISE
- ALL "TOP OF SLAB" DESIGNATIONS ARE TO ALIGN WITH TOP OF EXISTING SLAB AT ADJACENT EXISTING BUILDING SO THAT WHEN FLOOR FINISHES ARE APPLIED WALKING SURFACES ARE LEVEL.

KEYNOTES

- | | |
|-------|---|
| 5.01 | 6" TALL GALVANIZED STEEL PIPE BOLLARD FILLED WITH CONCRETE WITH SLEEVE - PAINT - REF. DETAIL A1/A61.1 |
| 5.05 | STEEL BEAM - PAINT WHERE EXPOSED - REF. STRUCT. |
| 5.07 | STEEL COLUMN - PAINT WHERE EXPOSED - REF. STRUCT. |
| 7.12 | ROOF SYSTEM - REF. ROOF PLAN |
| 8.01 | HOLLOW METAL DOOR AND FRAME - PAINT - REF. DOOR SCHEDULE, DETAILS AND SPEC. |
| 8.06 | OVERHEAD SECTIONAL DOOR - REF. DOOR SCHEDULE, DETAILS AND SPEC. |
| 10.18 | FIRE EXTINGUISHER ON BRACKET - REF. SPEC. |

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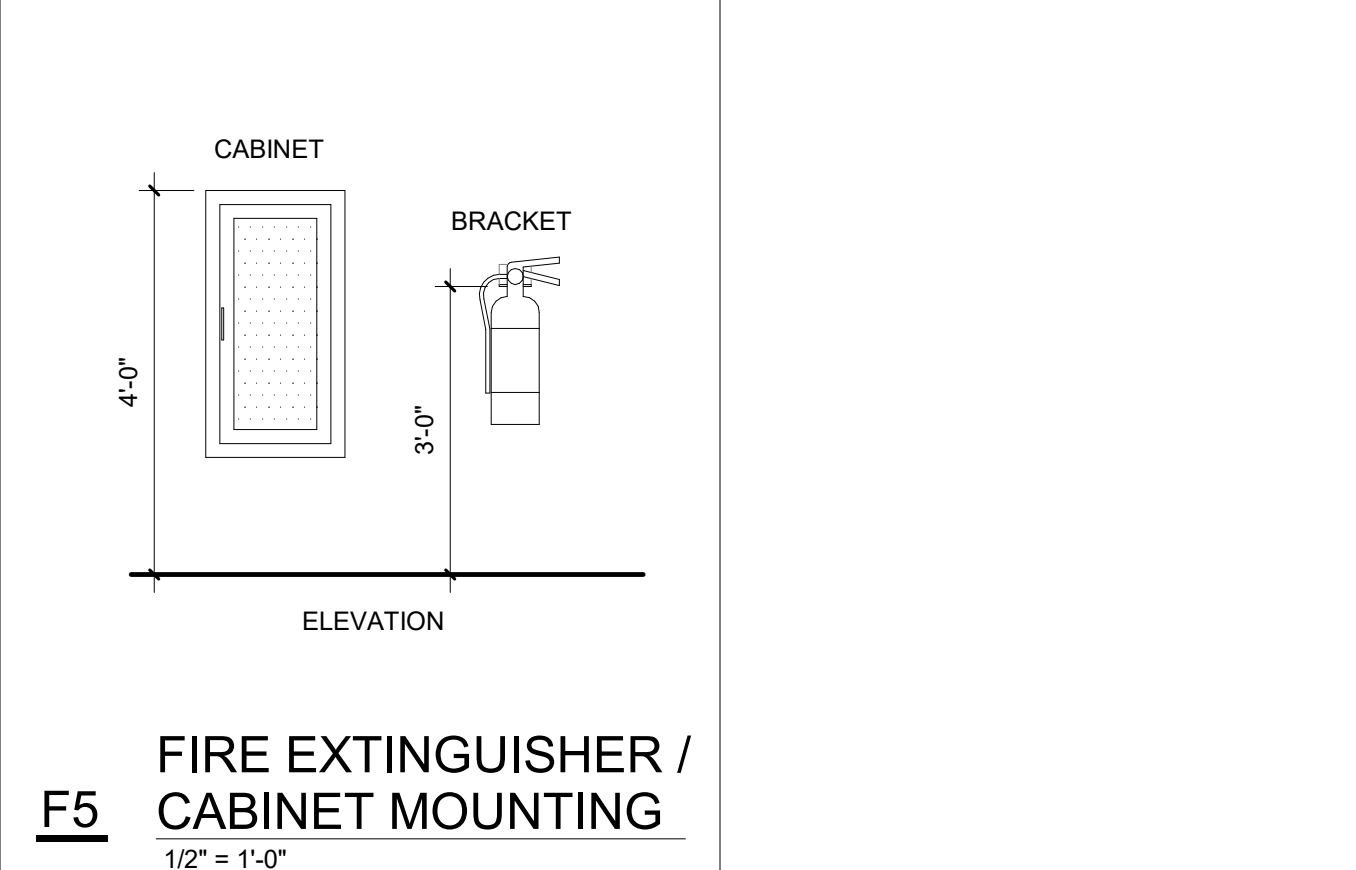
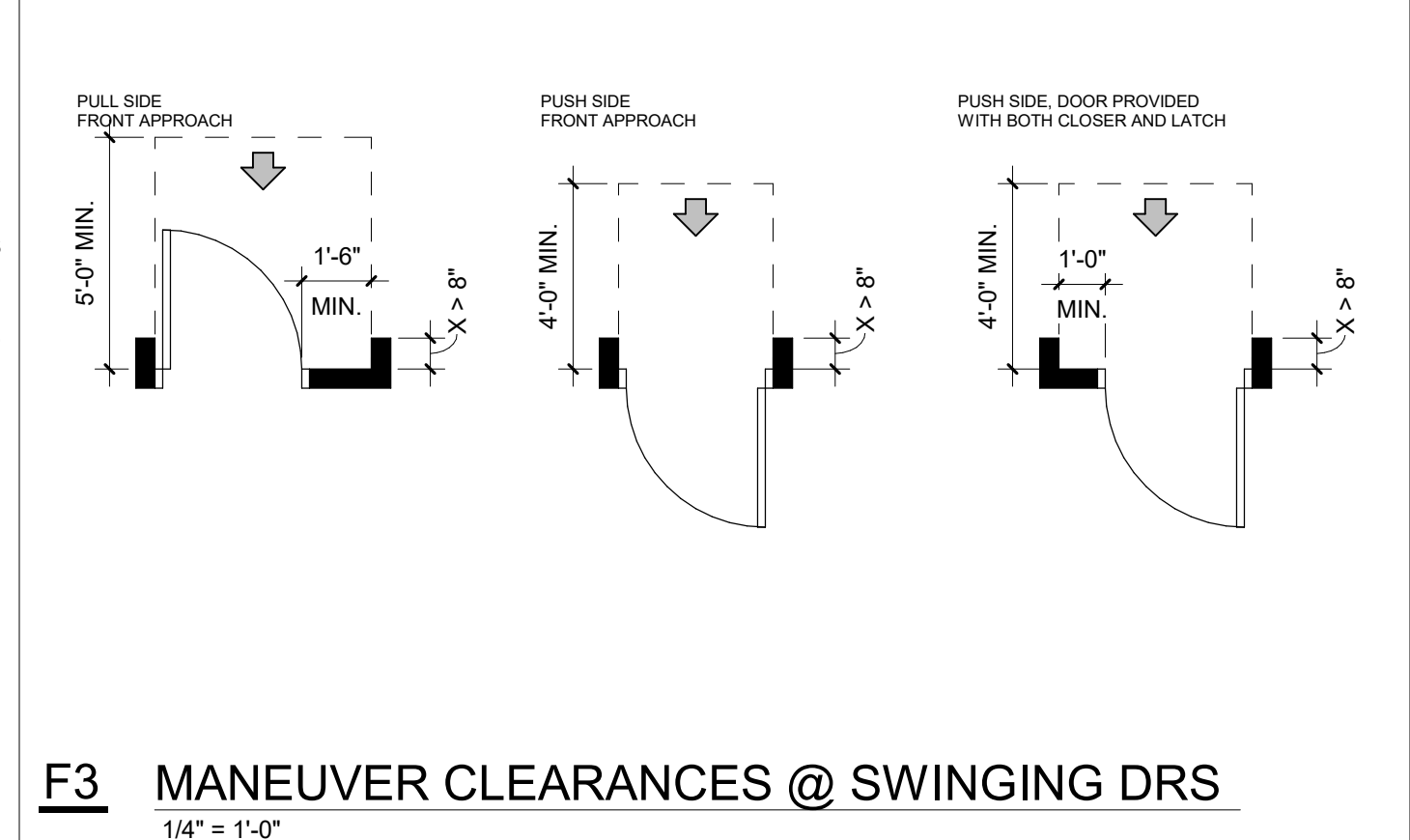
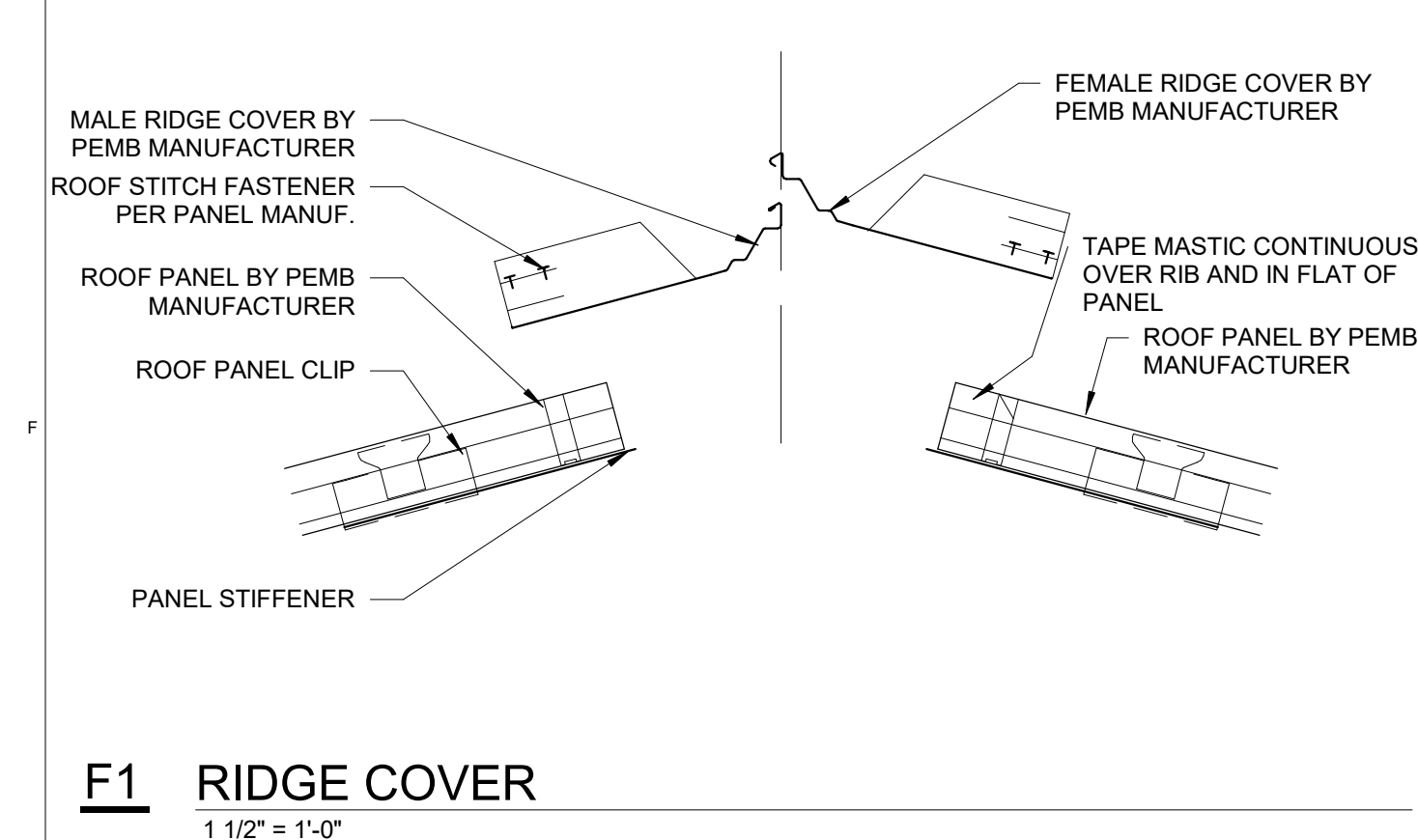
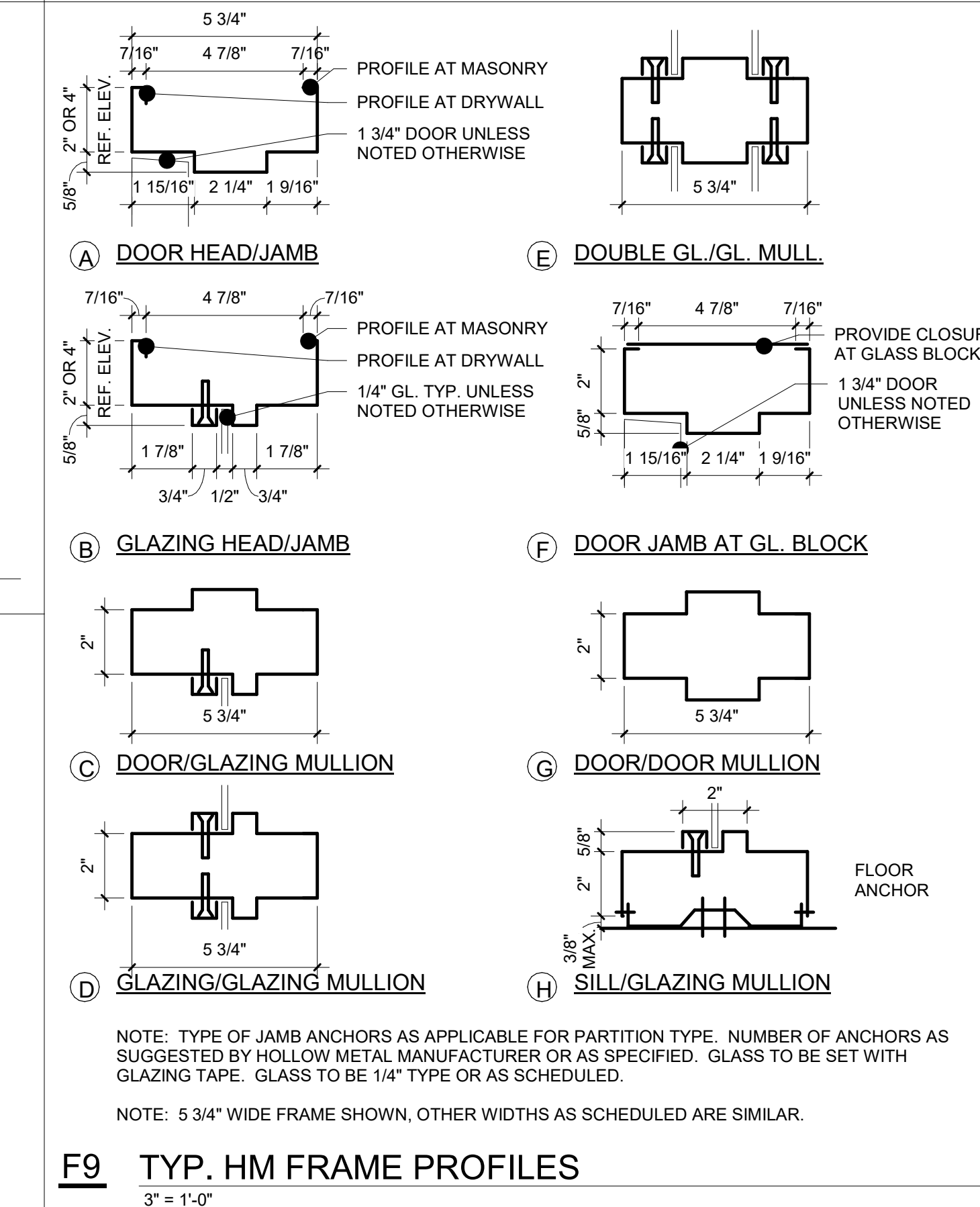
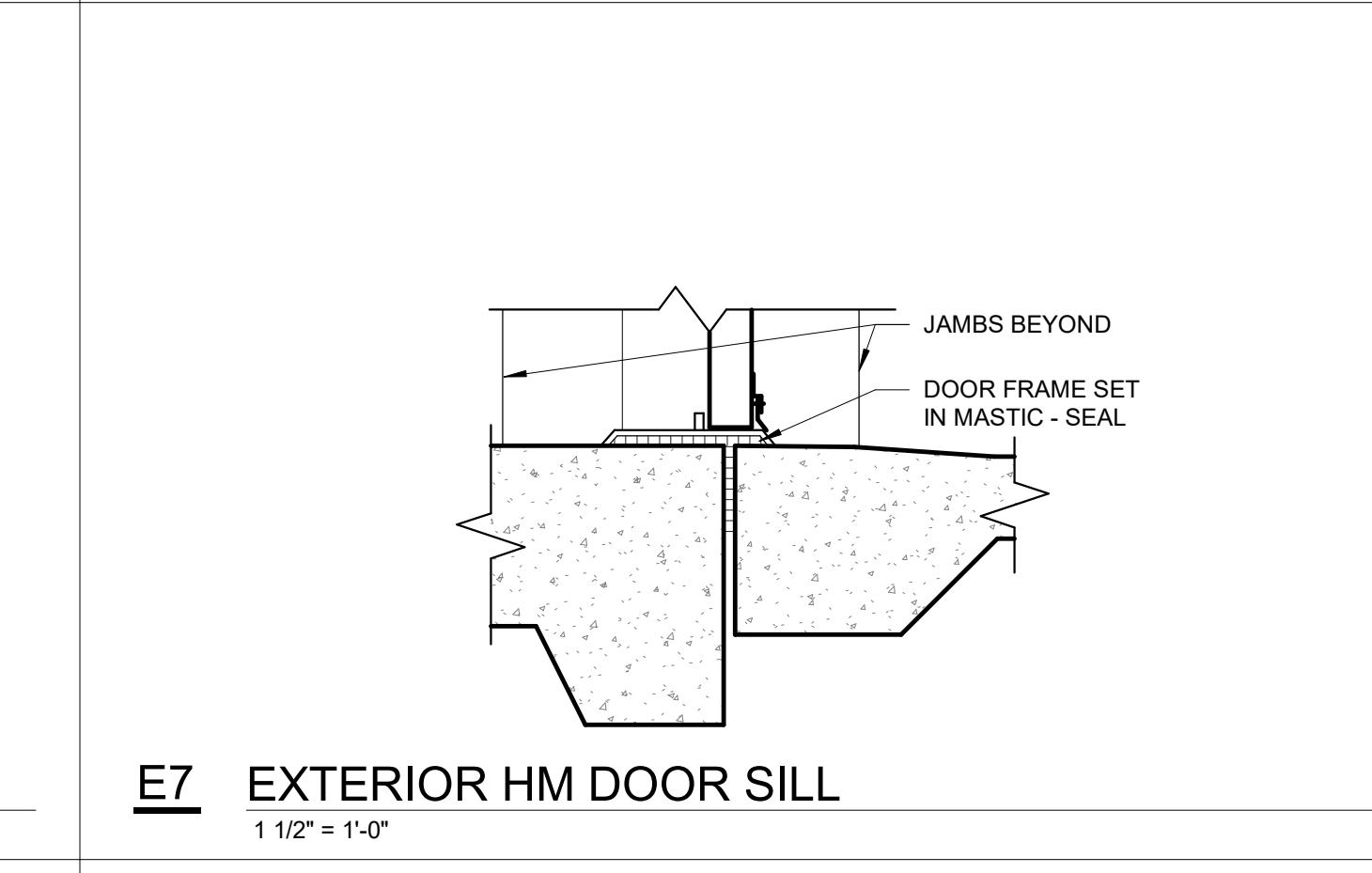
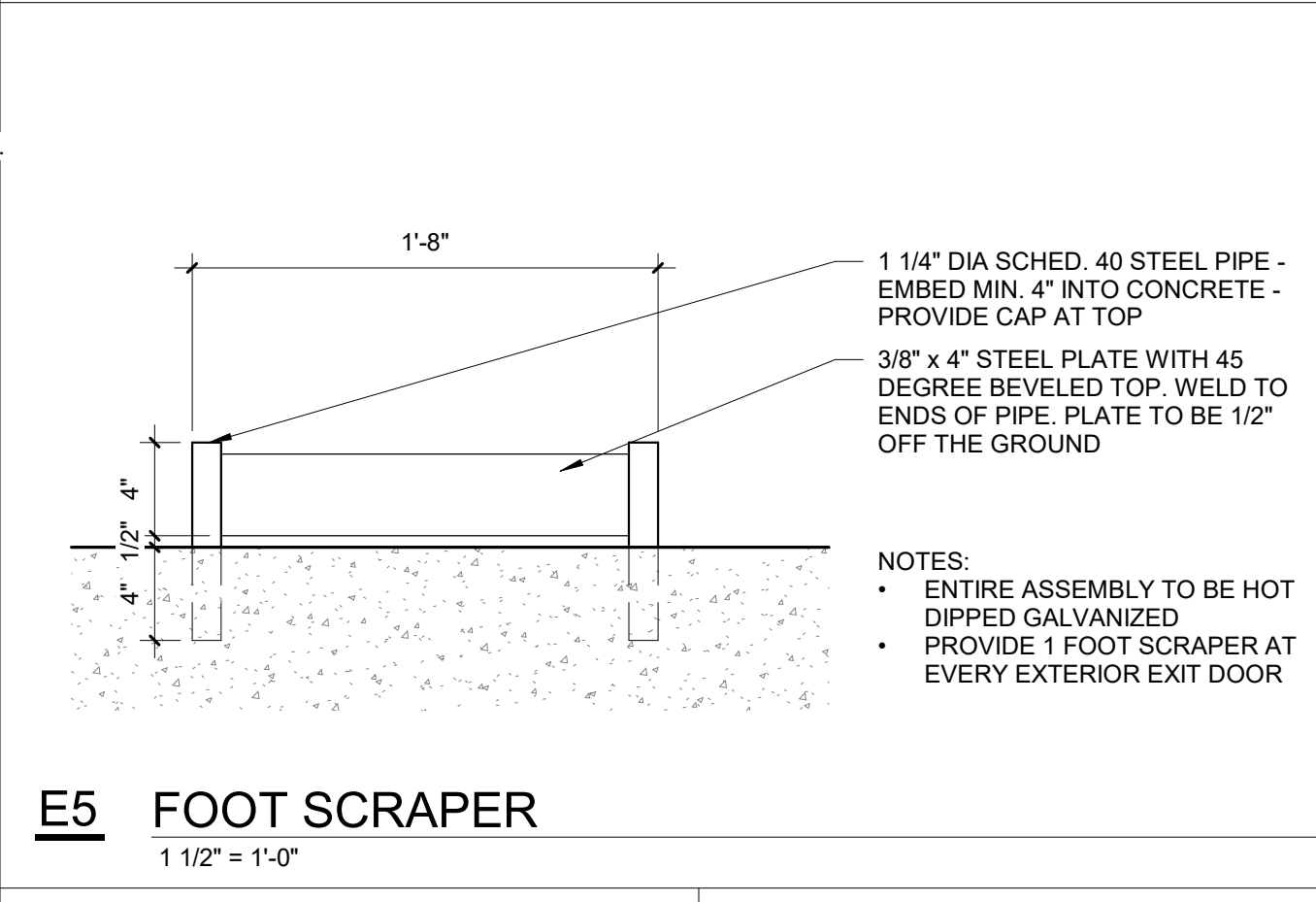
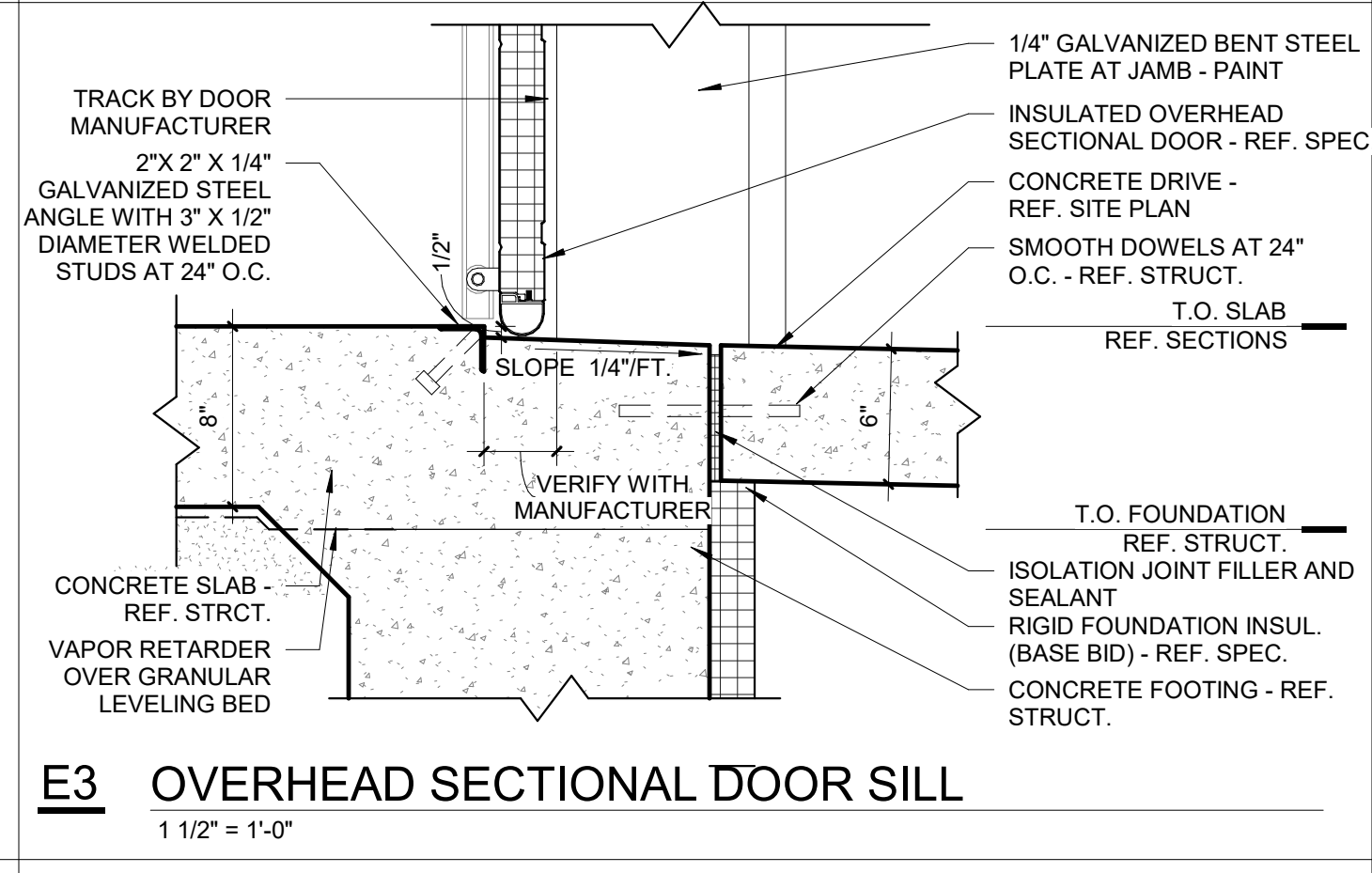
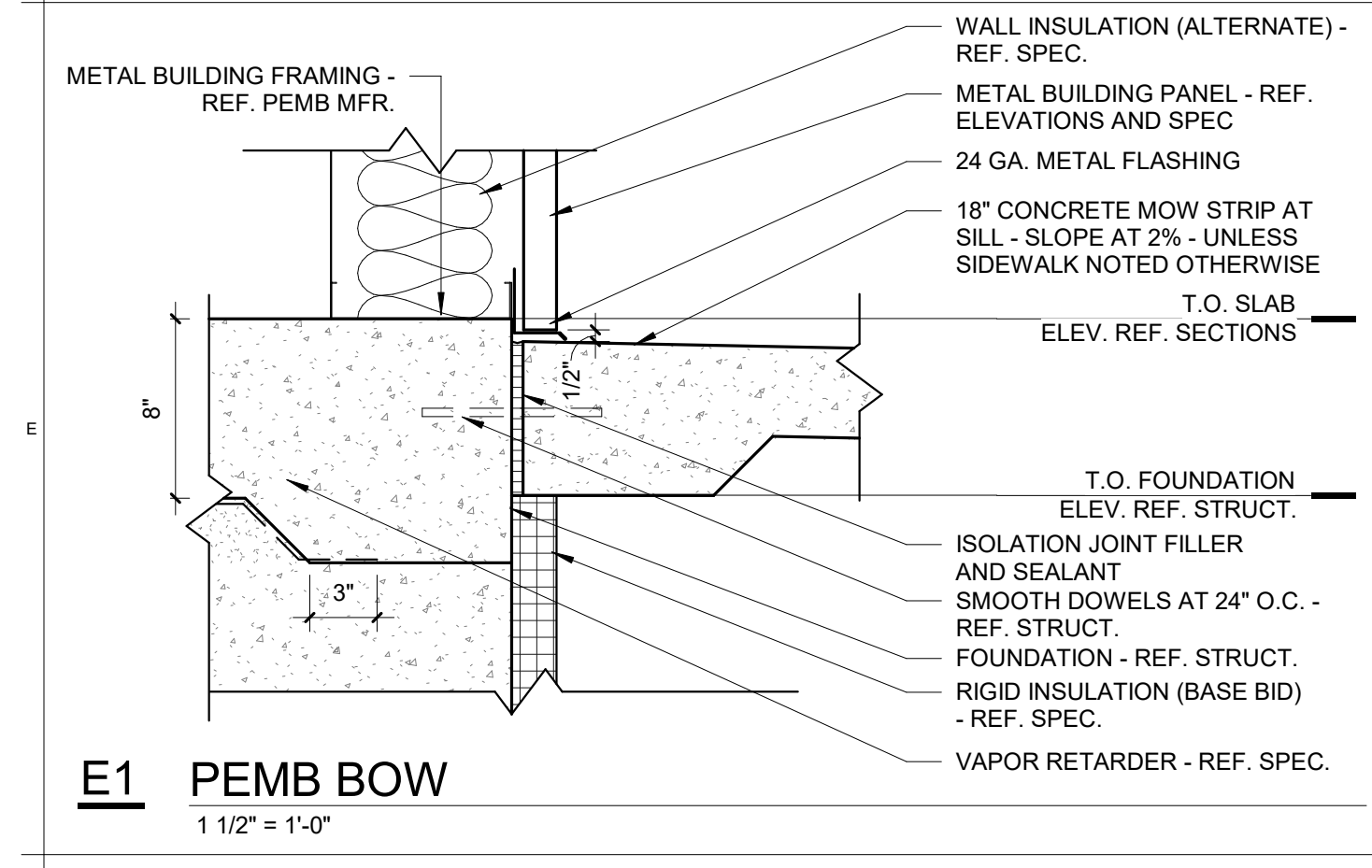
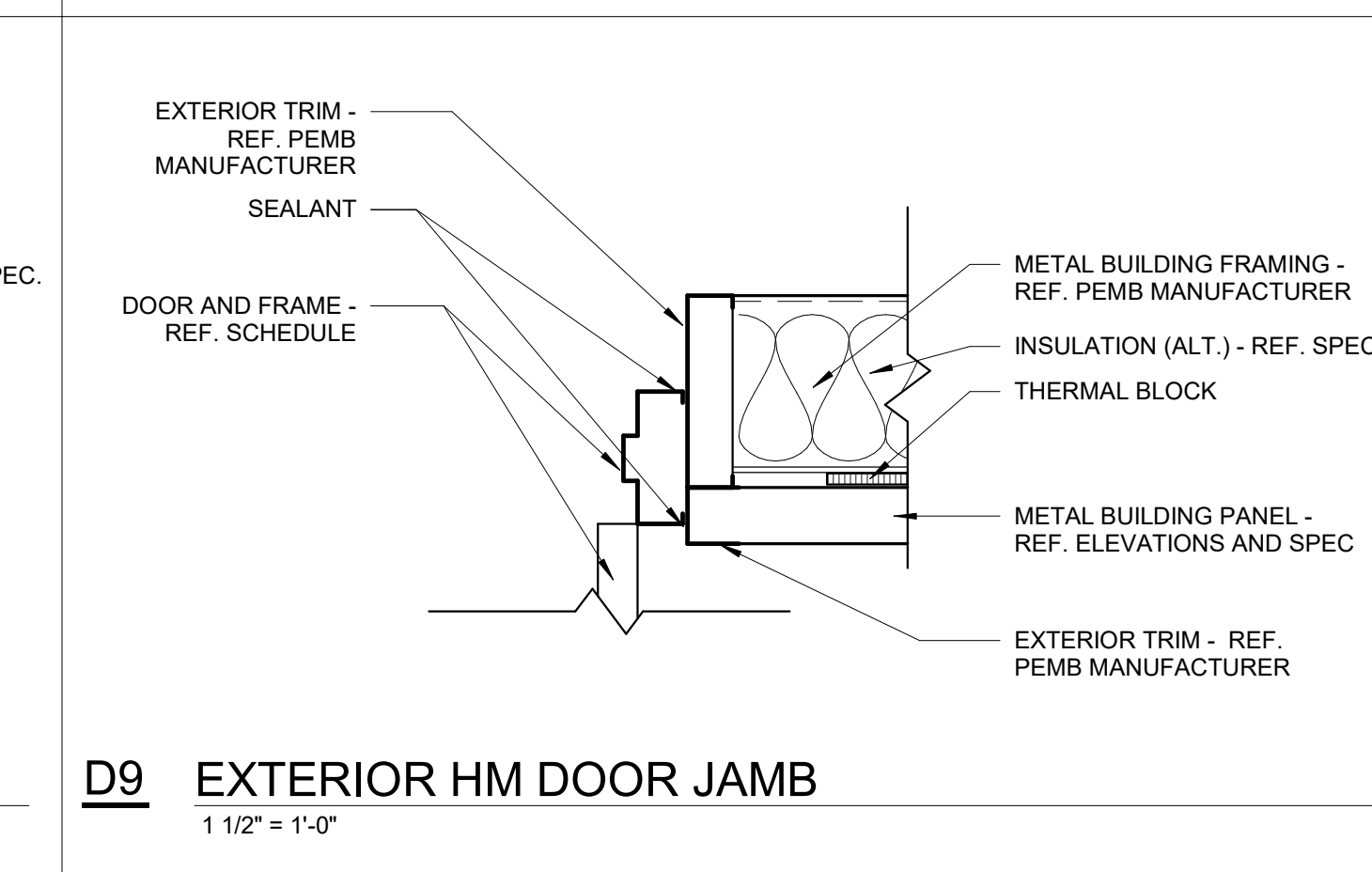
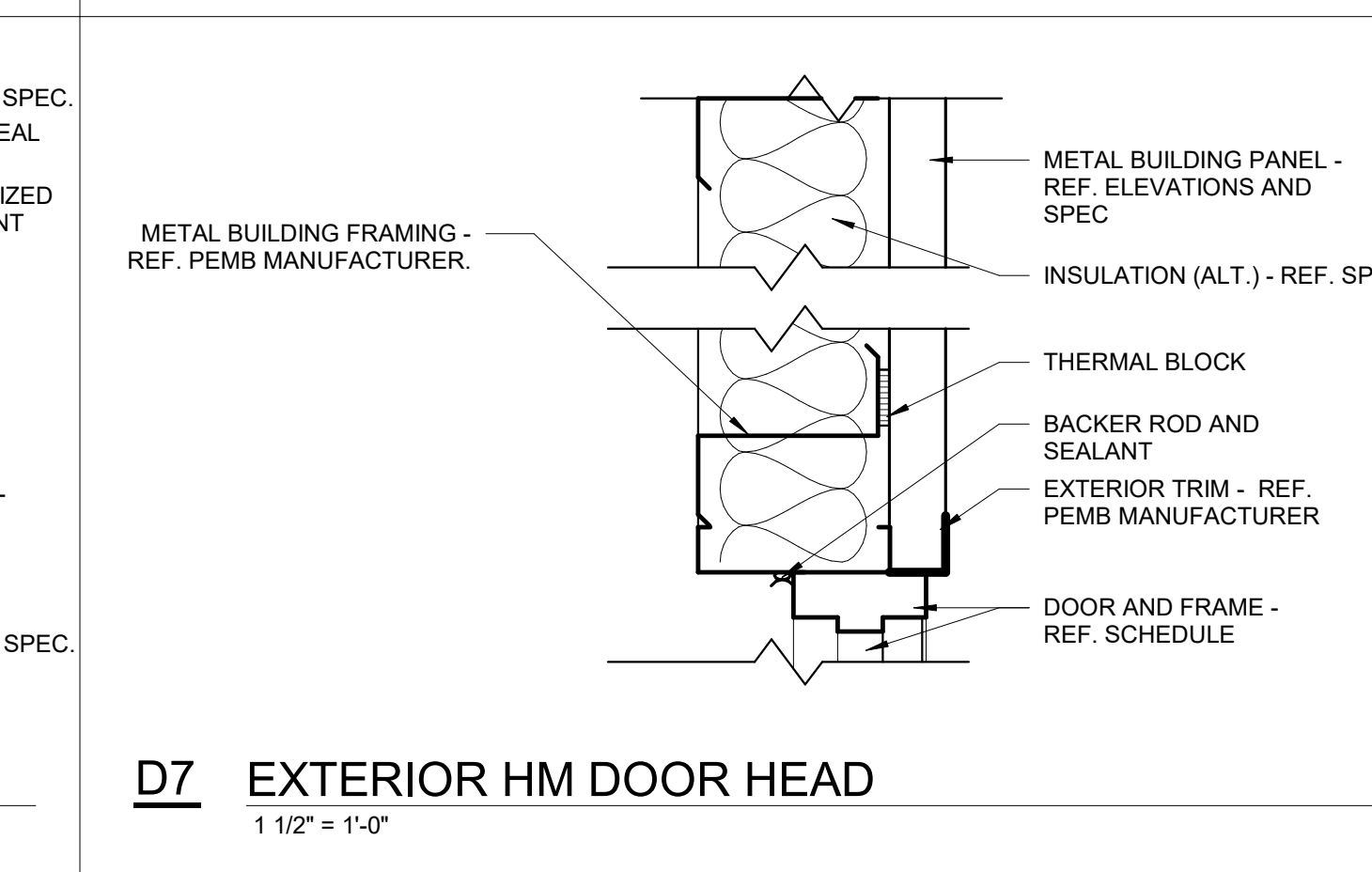
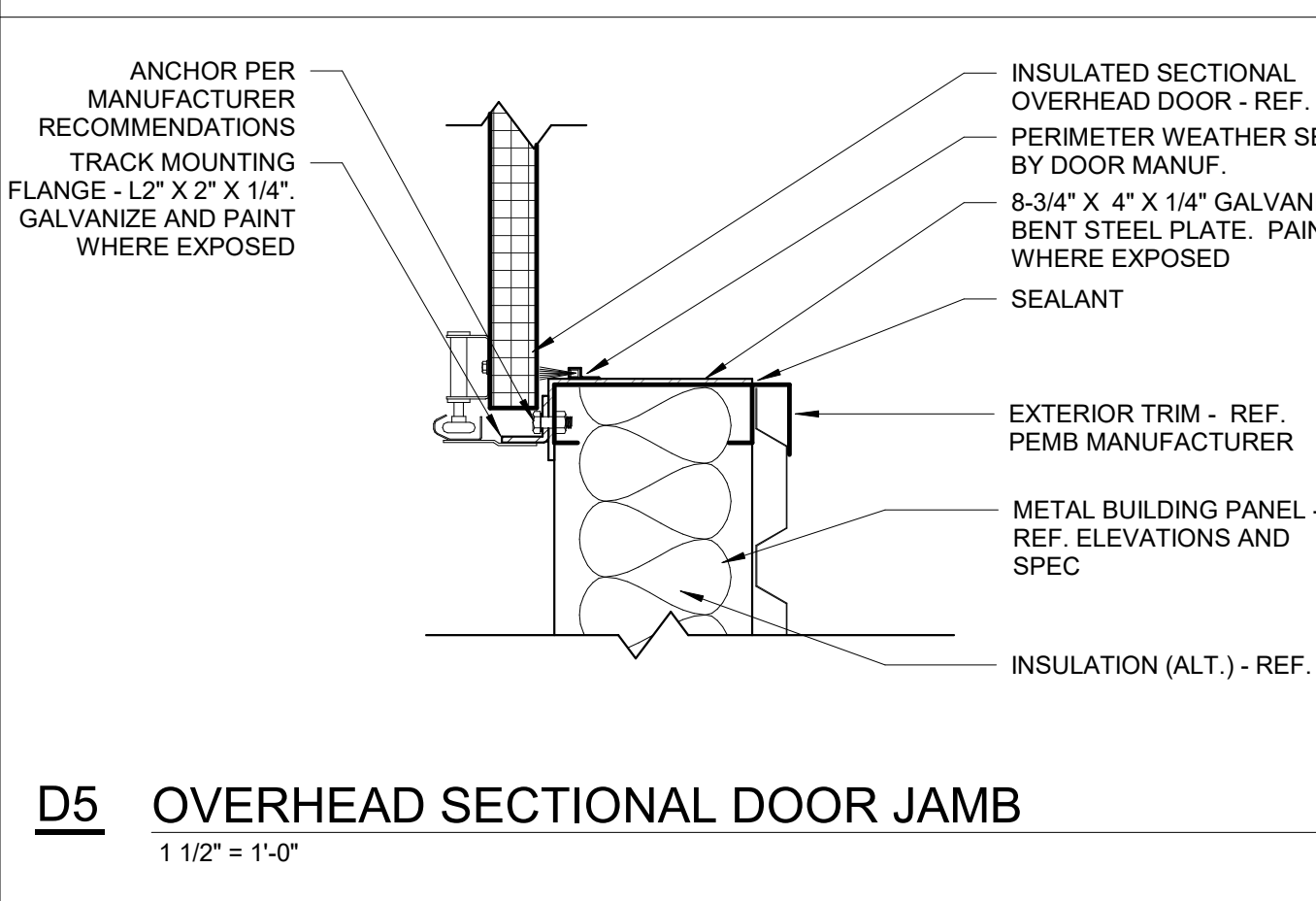
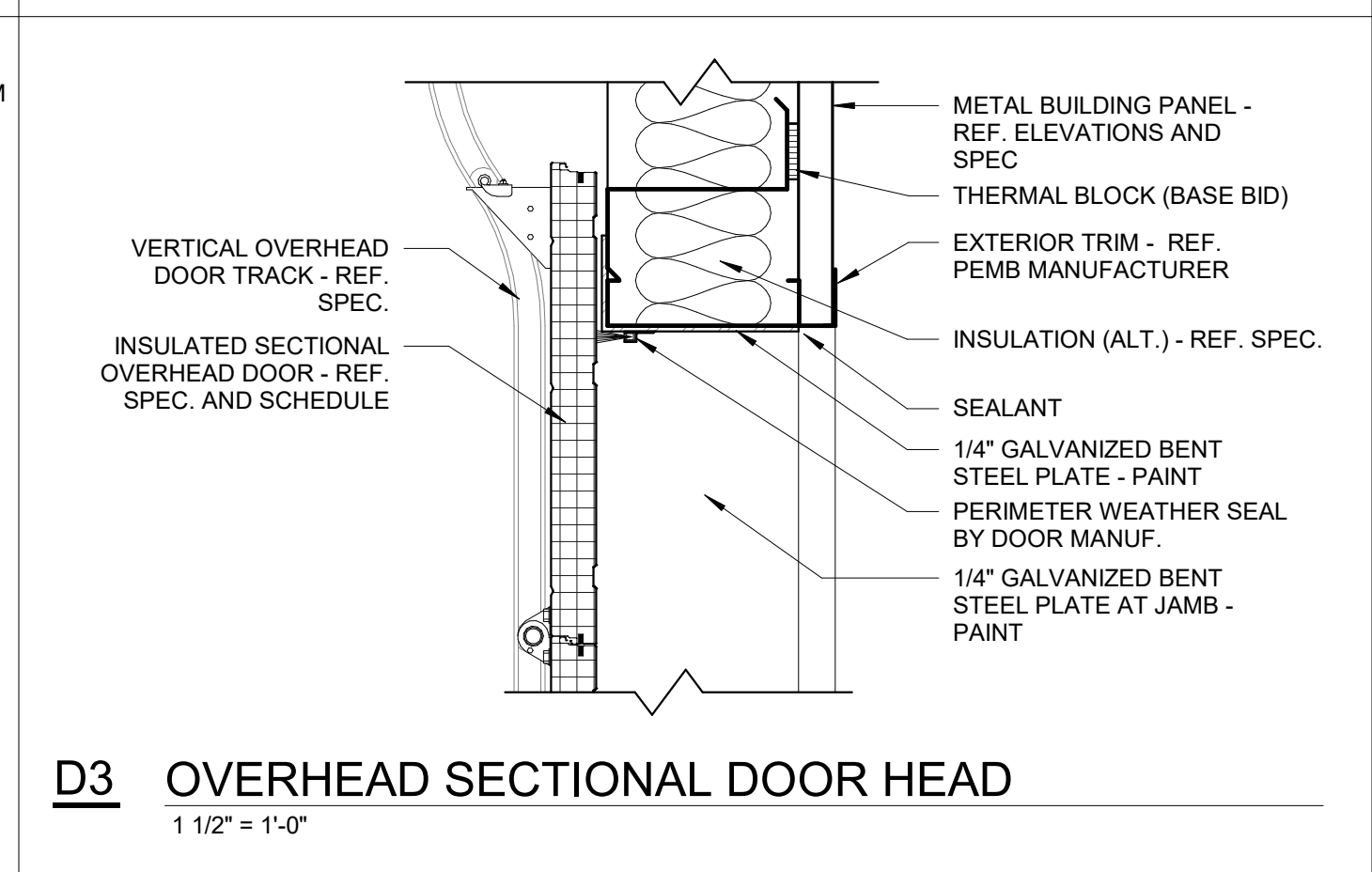
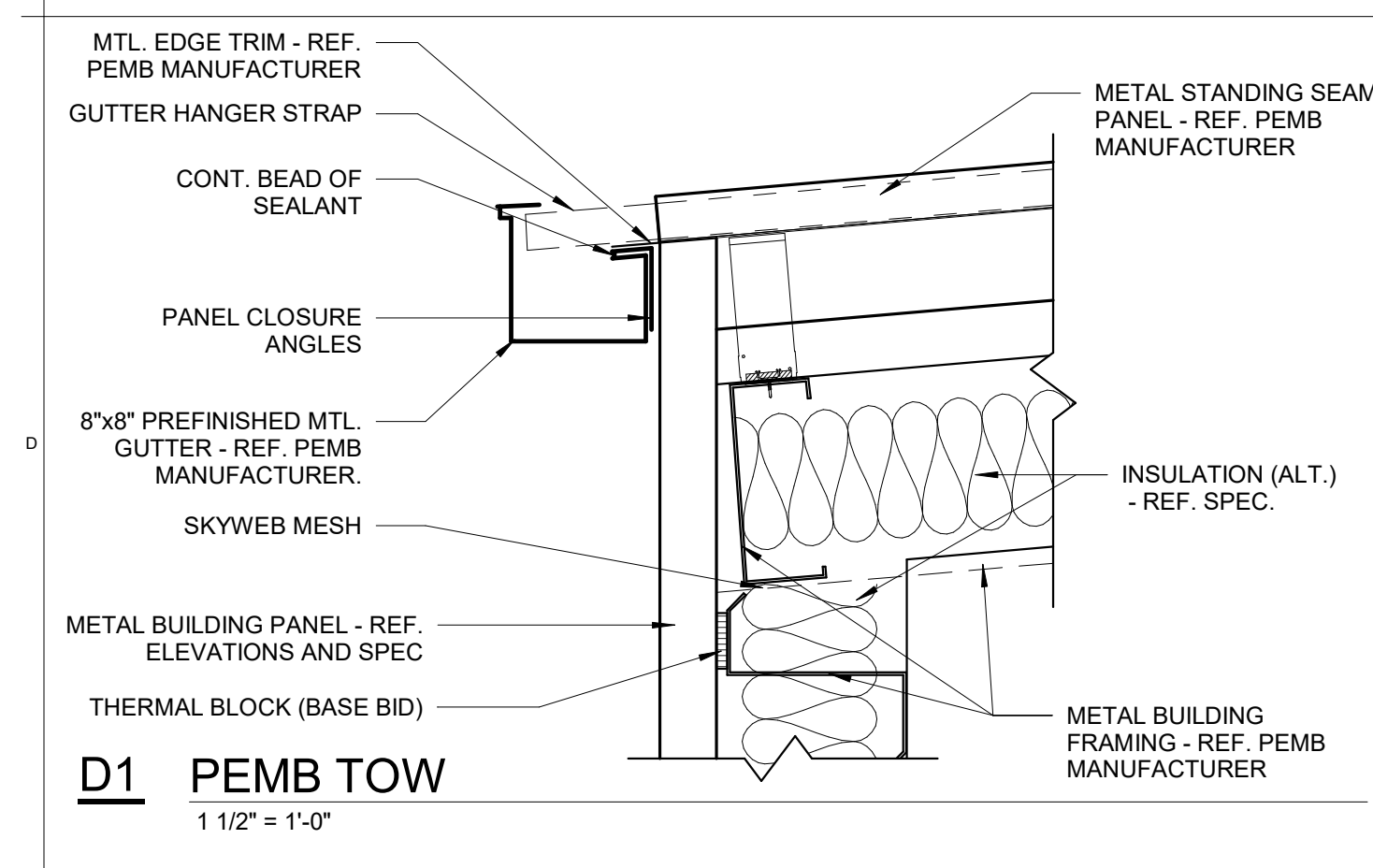
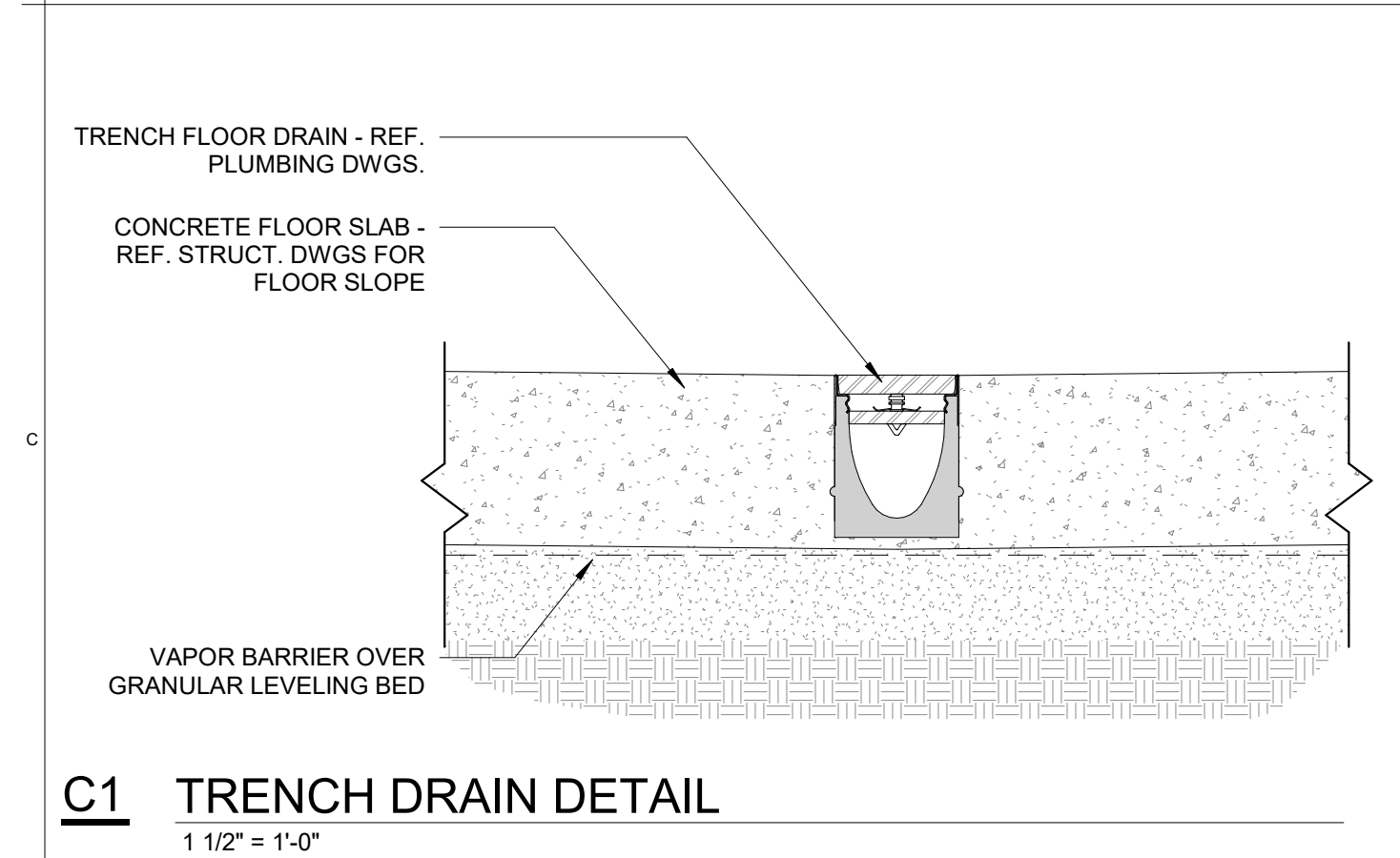
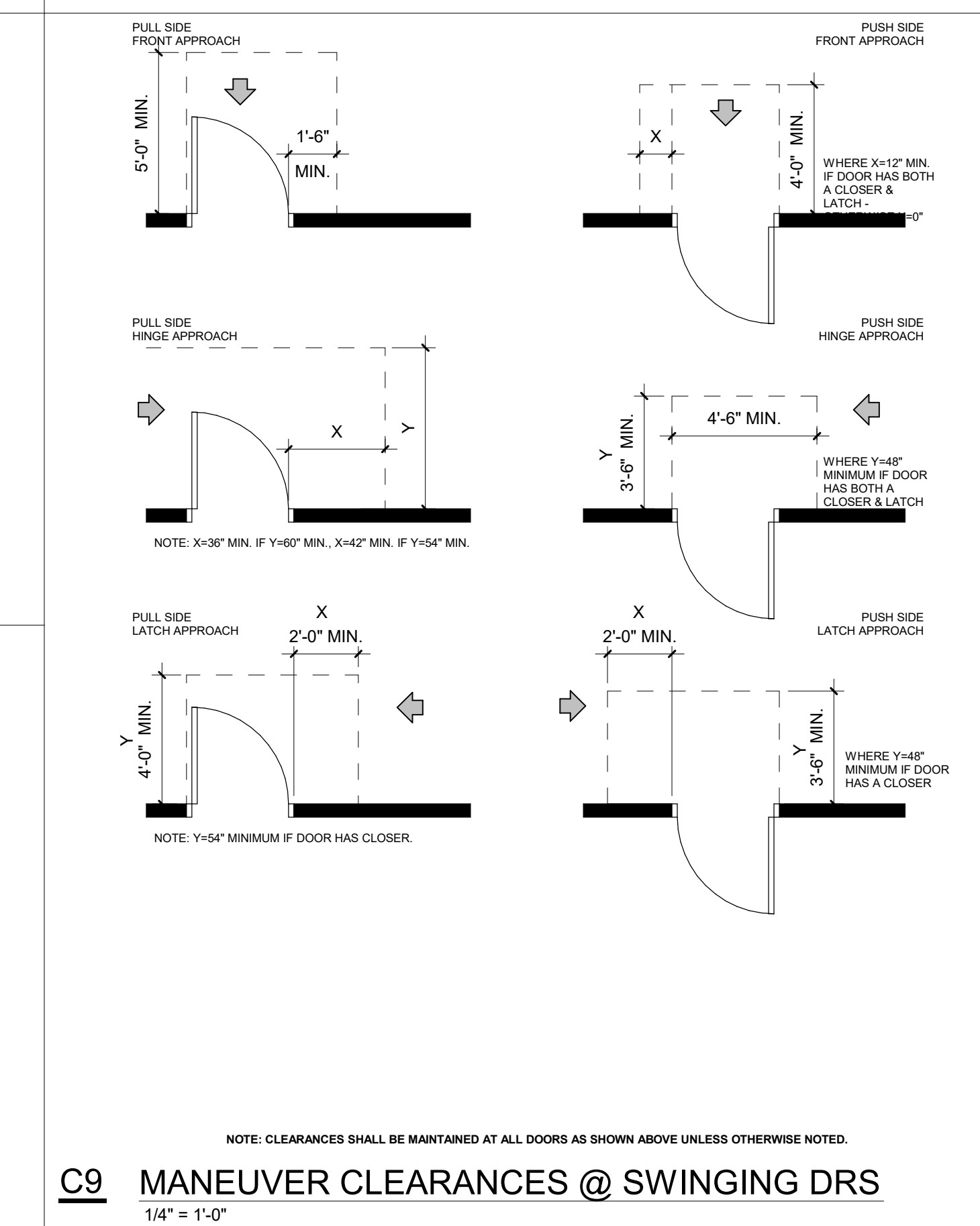
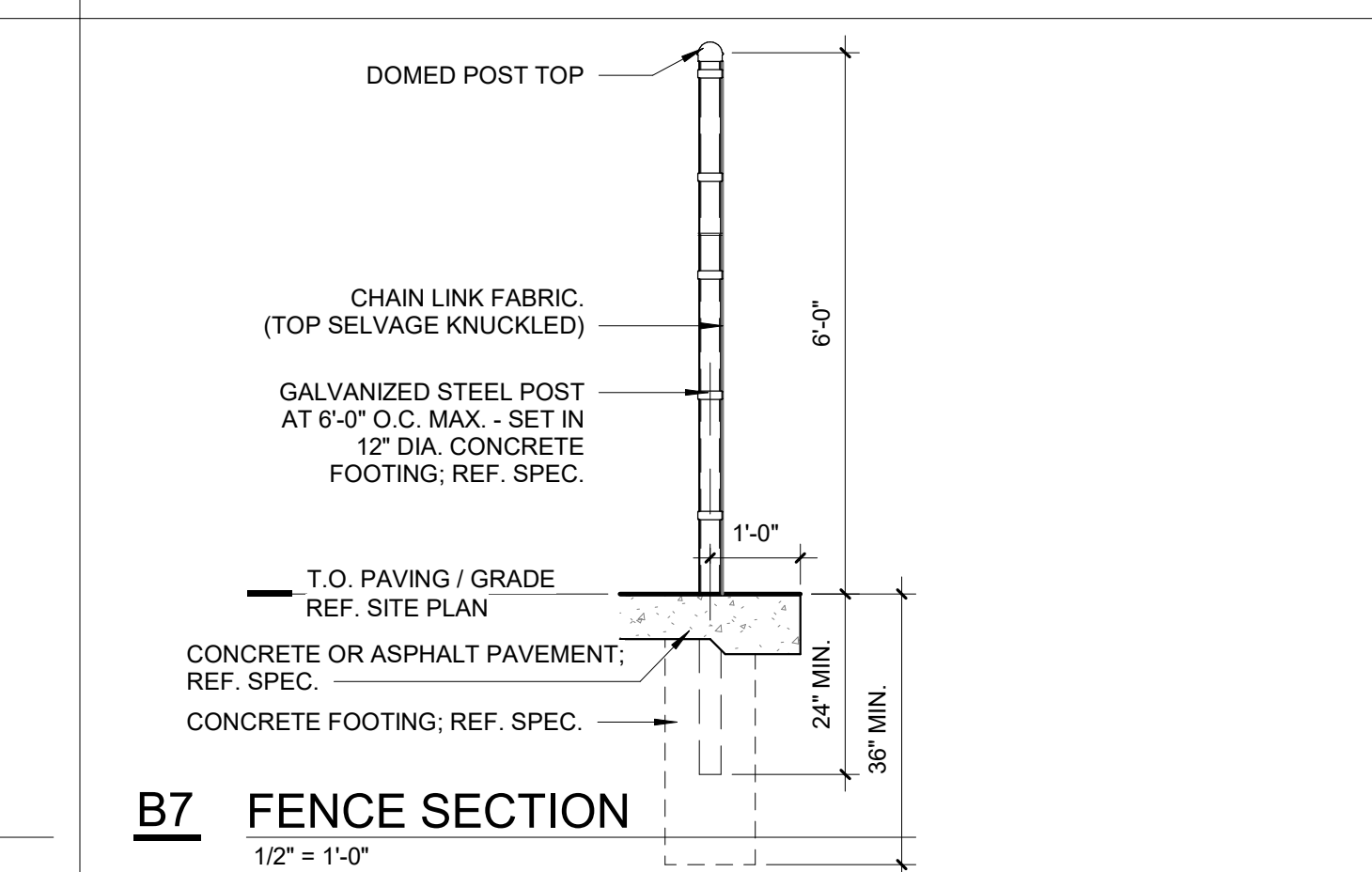
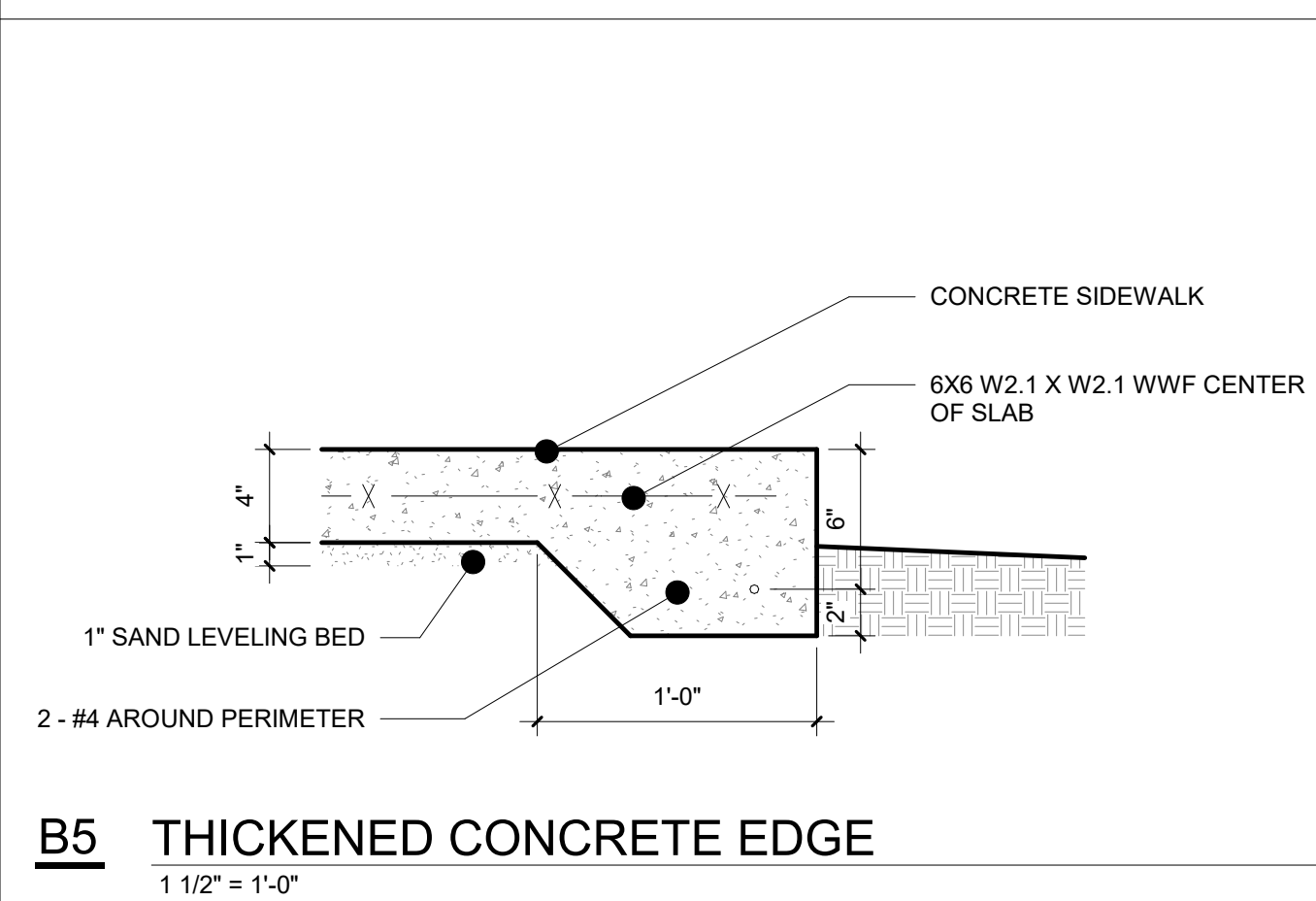
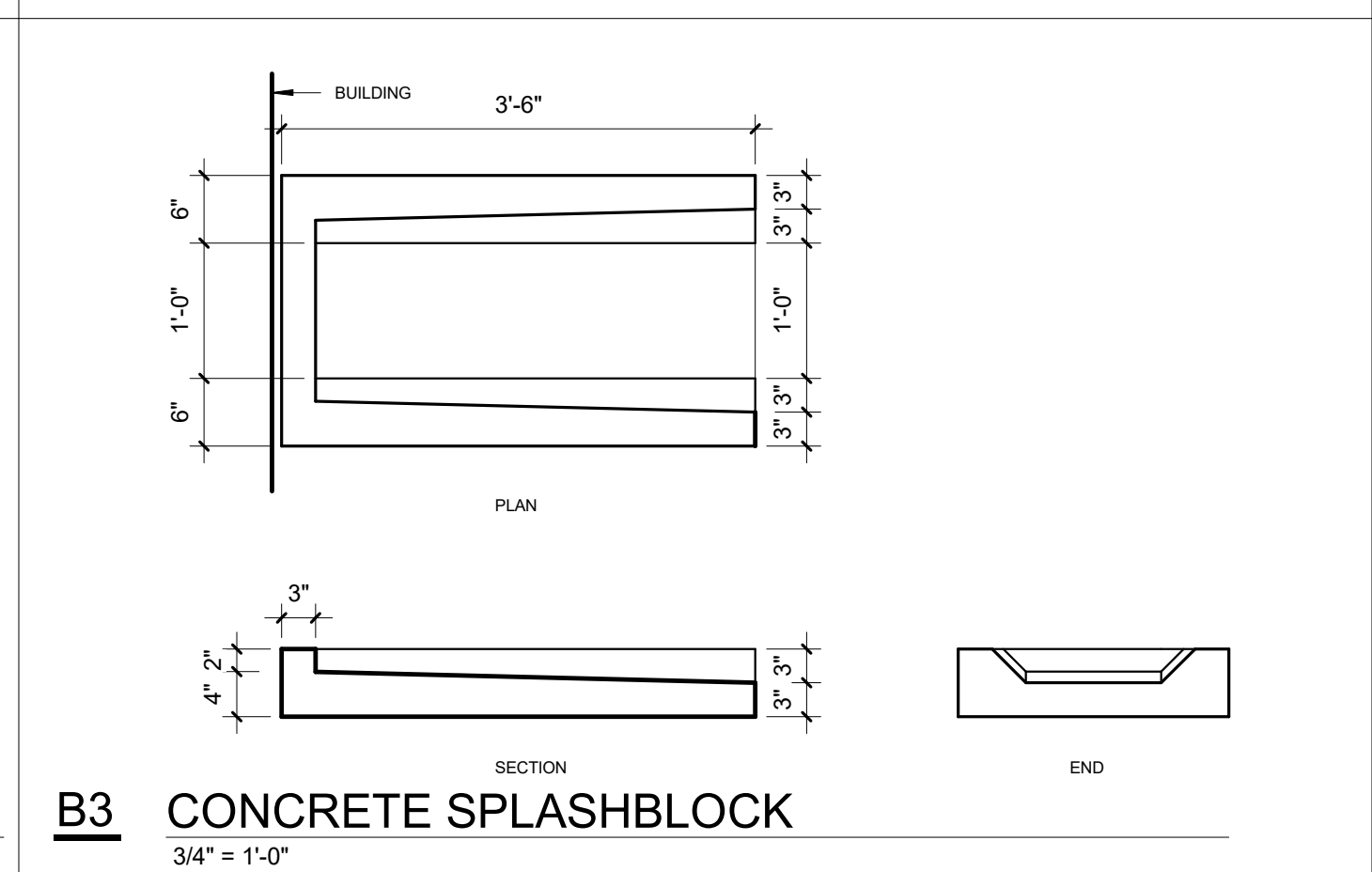
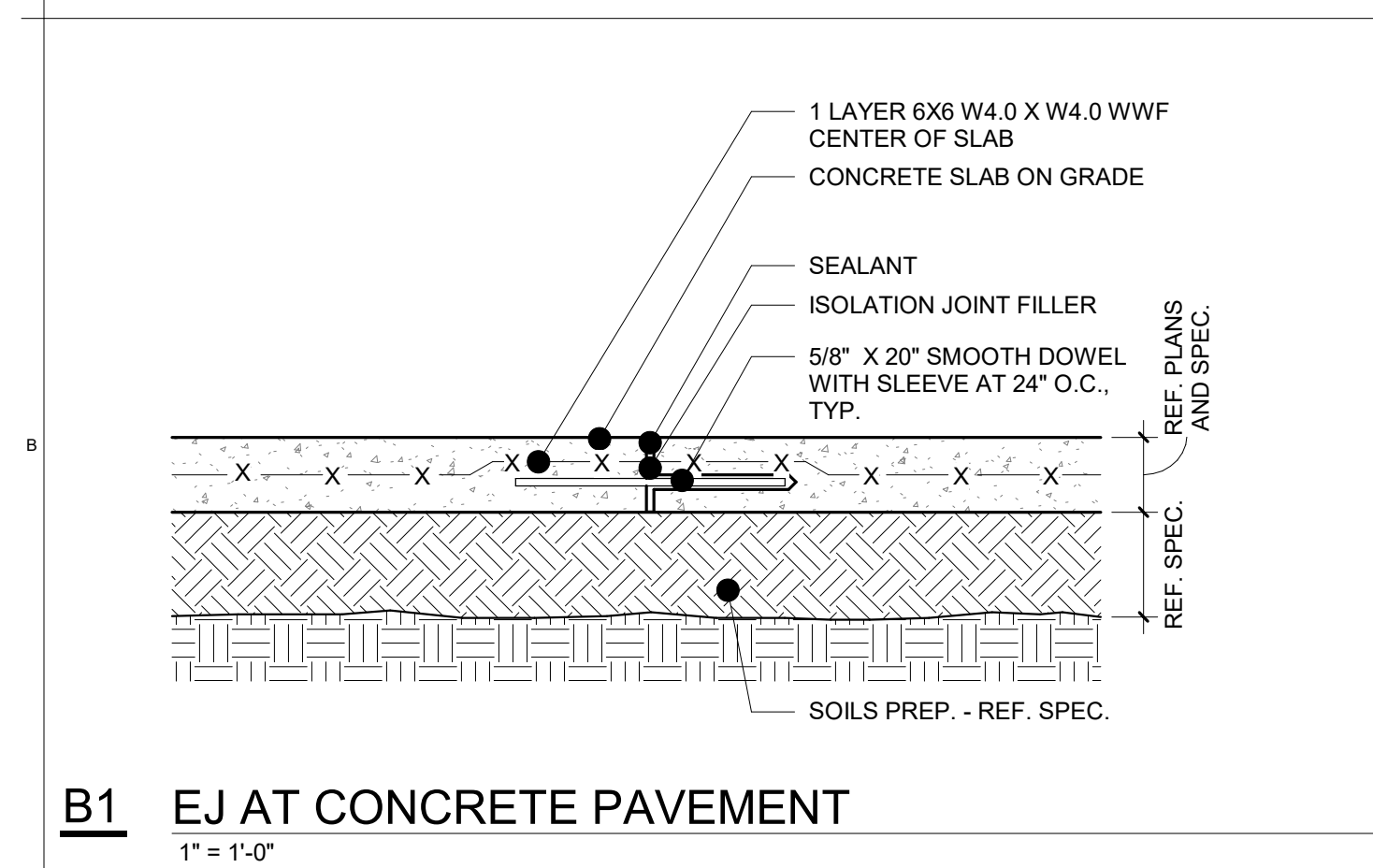
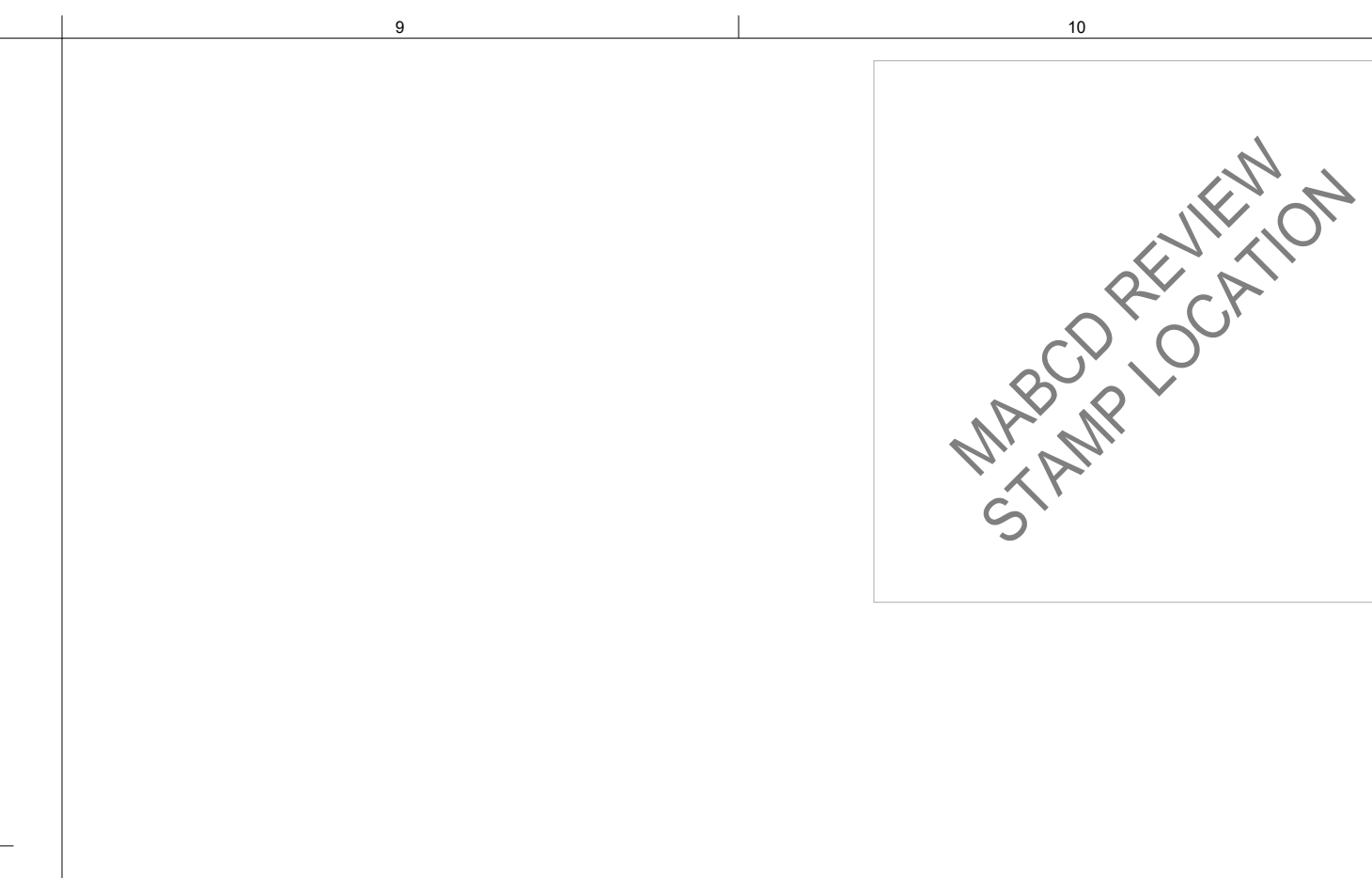
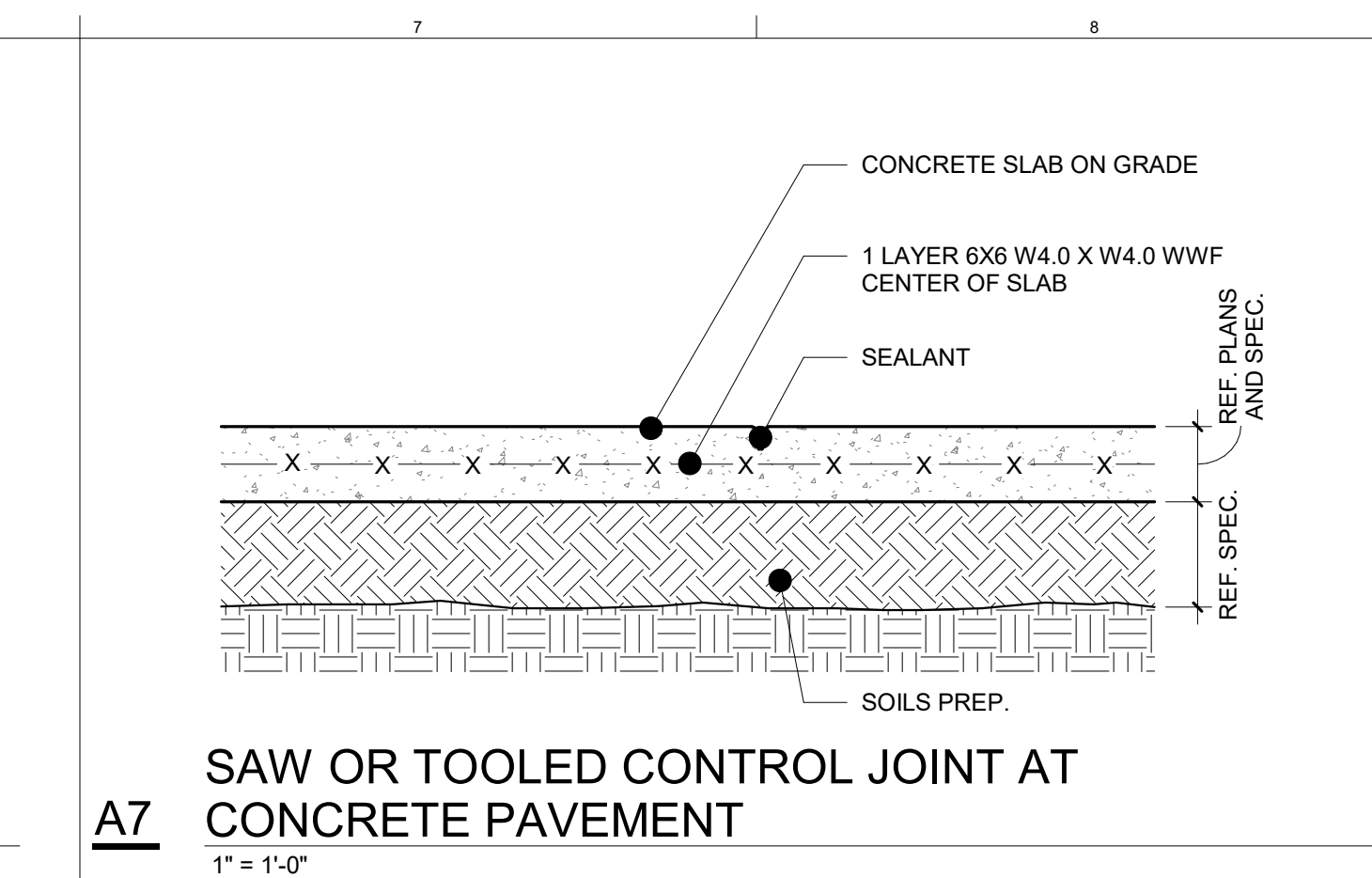
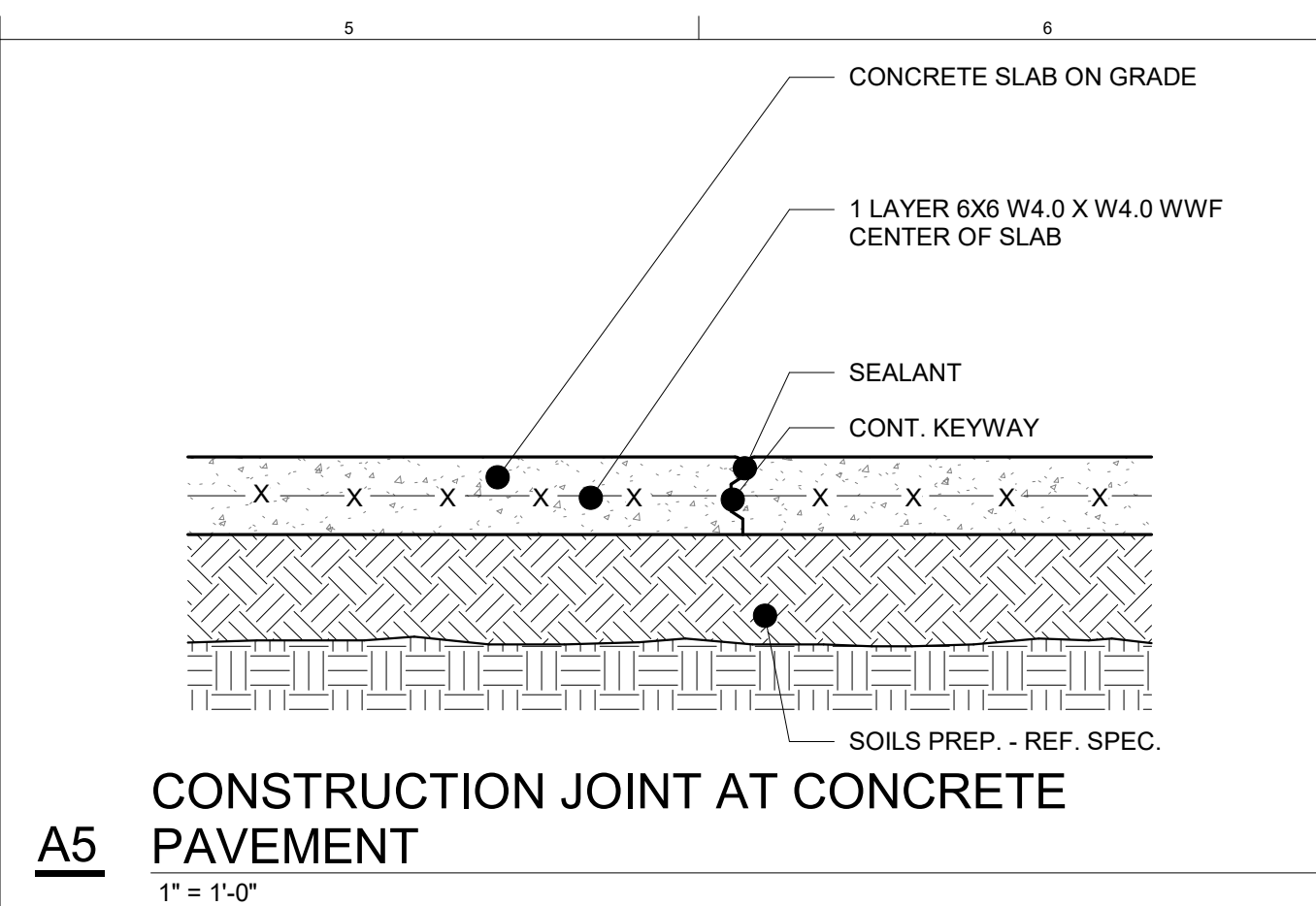
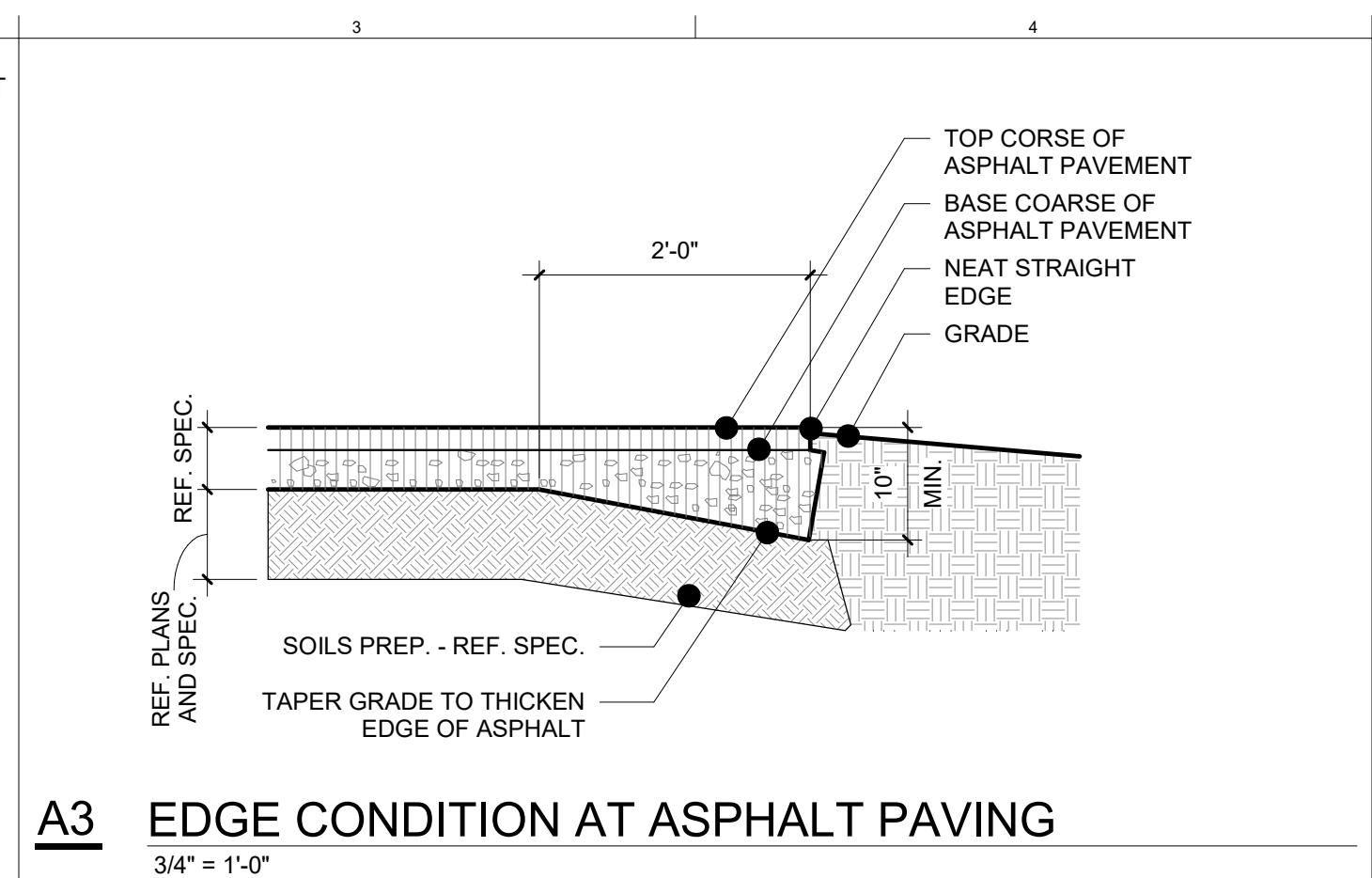
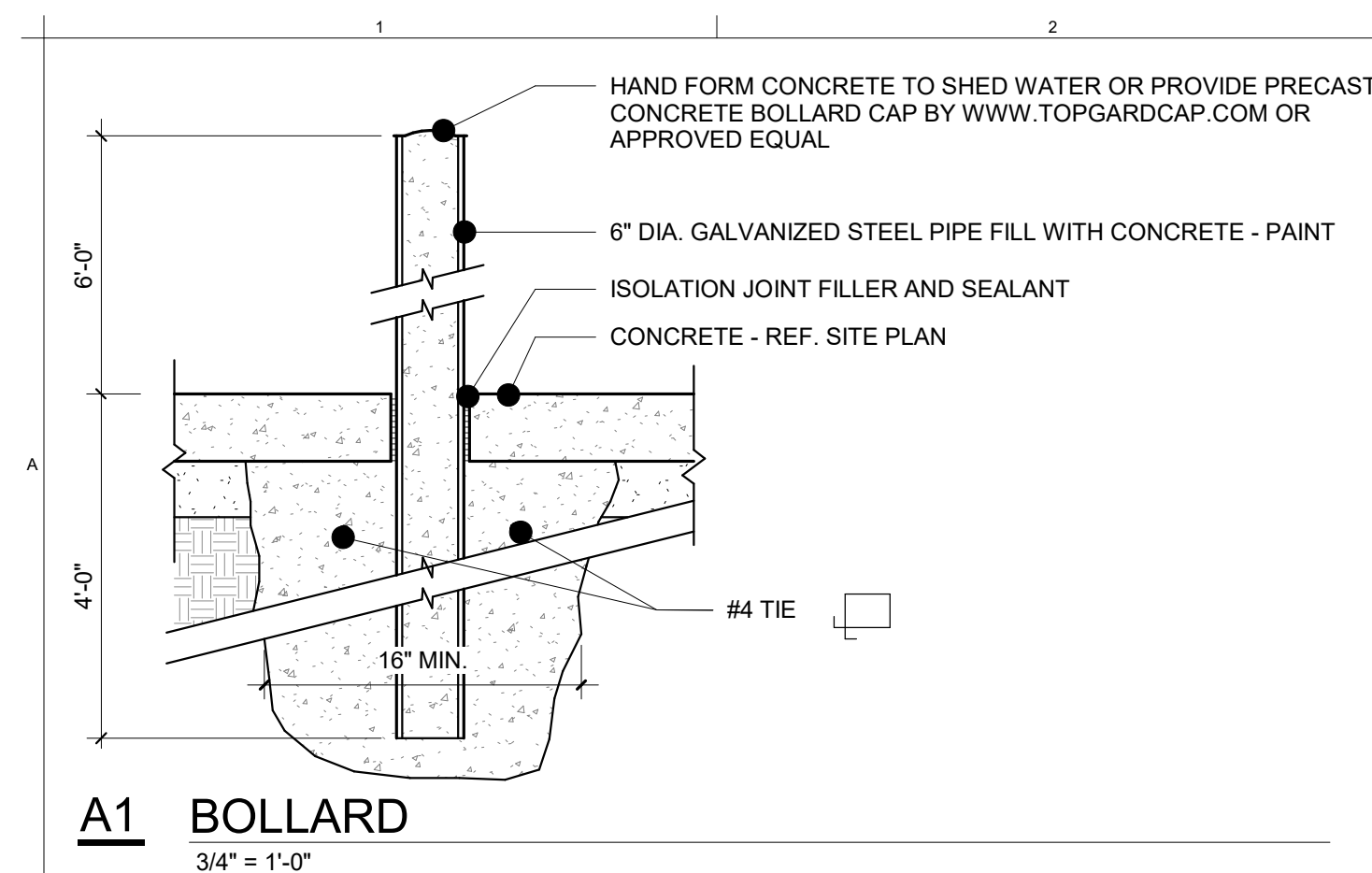
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BUILDING SECTIONS

A42.1



DOOR & FRAME SCHEDULE																
DOOR							FRAME					DETAILS				REMARKS
MARK	WIDTH	HEIGHT	MATERIAL	TYPE	GLASS	FIRE	DEPTH	MATERIAL	TYPE	GLASS	FIRE	JAMB	HEAD	SILL		
S100A	14'-0"	14'-0"	STL	OVHD	IN	--	0"	STL	--	--	--	D5/A61.1	D3/A61.1	E3/A61.1		
S100B	14'-0"	14'-0"	STL	OVHD	IN	--	0"	STL	--	--	--	D5/A61.1	D3/A61.1	E3/A61.1		
S100C	12'-0"	14'-0"	STL	OVHD	IN	--	0"	STL	--	--	--	D5/A61.1	D3/A61.1	E3/A61.1		
S100D	12'-0"	14'-0"	STL	OVHD	IN	--	0"	STL	--	--	--	D5/A61.1	D3/A61.1	E3/A61.1		
S100E	12'-0"	14'-0"	STL	OVHD	IN	--	0"	STL	--	--	--	D5/A61.1	D3/A61.1	E3/A61.1		
S100F	3'-0"	7'-2"	HM	F	--	--	5 3/4"	HM	HM-01	--	--	D9/A61.1	D7/A61.1	E7/A61.1		
S100G	14'-0"	14'-0"	STL	OVHD	IN	--	0"	STL	--	--	--	D5/A61.1	D3/A61.1	E3/A61.1		
S100H	14'-0"	14'-0"	STL	OVHD	IN	--	0"	STL	--	--	--	D5/A61.1	D3/A61.1	E3/A61.1		
S100J	12'-0"	14'-0"	STL	OVHD	IN	--	0"	STL	--	--	--	D5/A61.1	D3/A61.1	E3/A61.1		
S100K	12'-0"	14'-0"	STL	OVHD	IN	--	0"	STL	--	--	--	D5/A61.1	D3/A61.1	E3/A61.1		
S100L	12'-0"	14'-0"	STL	OVHD	IN	--	0"	STL	--	--	--	D5/A61.1	D3/A61.1	E3/A61.1		
S100M	3'-0"	7'-2"	HM	F	--	--	5 3/4"	HM	HM-01	--	--	D9/A61.1	D7/A61.1	E7/A61.1		

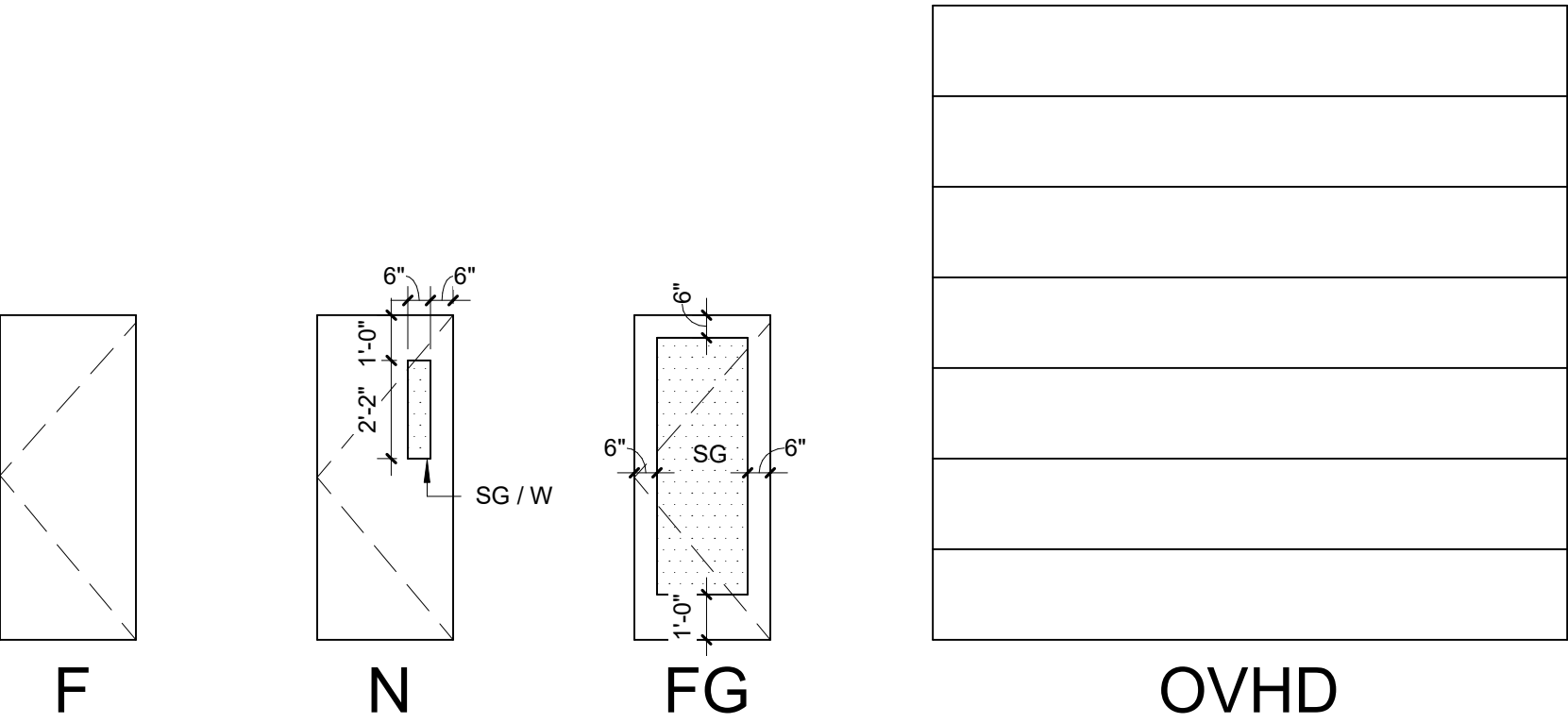
EXTERIOR WALL SCHEDULE										
TYPE MARK	STRUCTURAL MATERIAL	SHEATHING	WEATHER BARRIER	INSULATION		FINISH MATERIAL				REMARKS
				CAVITY	CONTINUOUS	EXTERIOR	INTERIOR			
MP1	Metal Building Girt - 8 1/2"	-	YES	ALTERNATE	-	1-1/2" PROFILE PANEL	-			

GENERAL NOTES

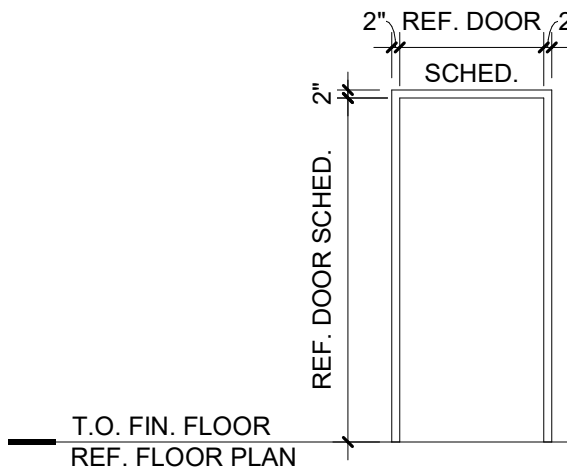
DOOR & FRAME MATERIAL:	GLAZING MATERIAL:
AL - ALUMINUM	IN - INSULATED
HM - HOLLOW METAL	LM - LAMINATED
WD - WOOD	SG - SAFETY GLASS
ST - STEEL	W - WIRE GLASS
SS - STAINLESS STEEL	CLR - CLEAR GLASS
FRP - FIBERGLASS REINFORCED POLYMER	FR- 3/4 HOUR FIRE RESISTIVE ASSEMBLY

- ALL DOORS ARE UNDERCUT AS SPECIFIED.
- SAFETY GLAZING LISTED IS REQUIRED ONLY IN LOCATIONS TO MEET CODE REQUIREMENTS EXCEPT WHEN SHOWN IN REMARKS COLUMN OR DRAWINGS.
- DOOR DIMENSIONS REFER TO OPENING DIMENSIONS OF FRAME.
- WOOD DOORS TO BE FACTORY FINISHED UNLESS NOTED OTHERWISE - REF. SPEC.
- HOLLOW METAL DOORS TO BE PAINTED UNLESS NOTED OTHERWISE - REF. SPEC.
- ALUMINUM DOORS, STOREFRONT AND CURTAIN WALL TO BE FACTORY FINISH UNLESS NOTED OTHERWISE - REF. SPEC.
- FURNISH AND INSTALL ALUMINUM STOREFRONT OR CURTAIN WALL FRAMES AND TRIM PER SPEC.
- FIELD VERIFY ALL DIMENSIONS OF OPENINGS TO RECEIVE NEW STOREFRONT, CURTAIN WALL OR DOOR FRAMES PRIOR TO FABRICATION. SIZES SHOWN ARE FOR ESTIMATING PURPOSES AND DESIGN INTENT, ACTUAL CONDITIONS MAY VARY. ADJUST DIMENSIONS TO INCLUDE NECESSARY SHIMS, SEALANTS AND FLASHINGS. G.C. TO COMMUNICATE DIMENSIONAL DISCREPANCIES TO ARCHITECT PRIOR TO FABRICATION.
- REPLACEMENT OF EXISTING WINDOW OR STOREFRONT:
 - KEEP DUST TO A MINIMAL AND CLEAN UP AFTER WORK IN THAT AFFECTED AREA'S UPON COMPLETION.
 - REPAIR ALL DAMAGED AREAS CAUSED BY WINDOW OR STOREFRONT REPLACEMENT INCLUDING (BUT NOT LIMITED TO) PLASTER OR GYP. BD. WALLS, WOOD TRIM, MILLWORK, CEILINGS, ETC. (VERIFY ALL EXISTING CONDITIONS PRIOR TO WORK).
 - FIELD VERIFY ALL DIMENSIONS OF EXISTING OPENINGS TO RECEIVE NEW WINDOWS OR STOREFRONT PRIOR TO FABRICATION.
 - CONTRACTOR SHALL REMOVE OLD UNITS AND INSTALL NEW WINDOW OR STOREFRONT UNITS IN THE SAME WORKING DAY.
- REMOVE ALL SEALANT FROM EXISTING SURFACES AFTER EXISTING WINDOW IS REMOVED. PREP. AREA TO RECEIVE NEW WINDOW OR STOREFRONT.

DOOR TYPES



FRAME TYPES



HM-01

MABCD REVIEW
STAMP LOCATION

Schaefer.
architecture

Schaefer Architecture
257 N. Broadway
Wichita, KS, 67202
316.684.0171



DUDLEY WILLIAMS & ASSOCIATES, P.A.
STRUCTURAL ENGINEERS

MIDWEST ENGINEERING, INC.
MECHANICAL ENGINEERS

BAUGHMAN COMPANY, P.A.
CIVIL ENGINEERS

MAIZE USD 266
BUS STORAGE BUILDING

4750 NORTH 119TH STREET WEST
MAIZE, KS 671 01

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REVISIONS



PROJECT NUMBER
5537.50

DATE
MARCH 2026

DOOR & FRAME
SCHEDULE

GENERAL STRUCTURAL NOTES

GENERAL CONTRACTOR SHALL REVIEW AND STAMP SHOP DRAWINGS BEFORE SUBMITTING FOR REVIEW. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. SEE THE ARCHITECTURAL DRAWINGS FOR EXACT DIMENSIONS OF OPENINGS IN THE WALLS, ROOF, AND FLOOR SYSTEMS. VERIFY ALL MECHANICAL AND ELECTRICAL OPENING SIZES AND LOCATIONS WITH THE RESPECTIVE CONTRACTORS. NO PIPES, SLEEVES, OR ETC. SHALL PASS THROUGH BEAMS OR COLUMNS UNLESS INDICATED ON THE PLAN. THE CONTRACTOR SHALL DESIGN, PROVIDE, AND MAINTAIN TEMPORARY BRACING, SHORING, GUYING, ETC. AND OTHER METHODS AS REQUIRED TO PREVENT ANY EXCESSIVE LOADING AND TO STABILIZE STRUCTURAL ELEMENTS DURING CONSTRUCTION. THESE METHODS SHALL REMAIN IN PLACE UNTIL ALL MEMBERS AND FINAL CONNECTIONS HAVE BEEN COMPLETED. THE GENERAL, MECHANICAL, AND ELECTRICAL CONTRACTORS SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL EMBEDS, INSERTS, ANCHORS, AND SUPPORTAL FRAMING REQUIRED FOR THE DESIGN OF ALL STRUCTURAL, MECHANICAL, AND ELECTRICAL SYSTEMS WHICH ARE NOT DETAILED ON THE STRUCTURAL DRAWINGS. DO NOT HANG OR ATTACH ANY ARCHITECTURAL, MECHANICAL, OR ELECTRICAL ELEMENTS OR SYSTEMS FROM METAL ROOF DECK OR PURLIN BRIDGING UNLESS SPECIFICALLY APPROVED OTHERWISE.

THE BUILDING STRUCTURAL SYSTEM IS DESIGNED PER THE INTERNATIONAL BUILDING CODE ~ 2024 EDITION. THE OWNER AND/OR CONTRACTOR SHALL PERFORM MATERIAL TESTING AND INSPECTION REQUIREMENTS FOR COMPLIANCE WITH THE GOVERNING BUILDING CODE, THE PROJECT SPECIFICATIONS, THE LOCAL BUILDING INSPECTION DEPARTMENT, AND THE FOLLOWING STRUCTURAL SPECIAL INSPECTION NOTES. STRUCTURAL STEEL ERECTION SHALL COMPLY WITH OSHA STANDARD 29 OF PART 1926, SUBPART R AND OTHER GOVERNING REGULATIONS. STRUCTURAL STEEL SUPPLIERS AND FABRICATORS SHALL INCORPORATE THE REQUIREMENTS OF THIS STANDARD INTO THE MATERIALS FABRICATED AND SUPPLIED ON THIS PROJECT.

THE FOUNDATION IS DESIGNED TO BEAR ON SUITABLE NATIVE CLAY SOILS OR COMPACTED STRUCTURAL FILL FOR AN ALLOWABLE BEARING PRESSURE OF 2000 PSF AS RECOMMENDED IN THE GEOTECHNICAL INVESTIGATION REPORT NO. 01175071 PREPARED BY TERRACON CONSULTANTS, INC., WICHITA, KANSAS AND DATED MAY 23, 2017. THE CONTRACTOR SHALL RETAIN A LICENSED GEOTECHNICAL ENGINEER TO VERIFY THAT THE EXISTING SOIL CONDITIONS WILL PROVIDE THE REQUIRED MINIMUM NET ALLOWABLE TOTAL LOAD BEARING PRESSURE. NOTIFY THE ARCHITECT/ENGINEER FOR DIRECTION IF THE EXISTING SOIL CONDITIONS ARE NOT CAPABLE OF PROVIDING THE DEFINED FOUNDATION DESIGN CRITERIA.

DESIGN LOADS

BUILDING STRUCTURE IS DESIGNED FOR THE FOLLOWING LOADS AND CRITERIA:

RISK CATEGORY:	II
DEAD:	WEIGHT OF MATERIALS AND CONSTRUCTION PLUS WEIGHT OF FIXED SERVICE EQUIPMENT ASSUMED ROOF DEAD LOAD: 5 PSF COLLATERAL DEAD LOAD: 5 PSF
LIVE:	ROOF LIVE LOAD (NON-REDUCIBLE): UNIFORM 20 PSF CONCENTRATED 300 LBS
SNOW:	GROUND SNOW LOAD: Pg = 23 PSF SNOW EXPOSURE FACTOR: Ce = 1.2 THERMAL FACTOR: Pt = 19.3 PSF (adj) FLAT ROOF SNOW LOAD: Pm = 23.0 PSF (adj) LOW-SLOPE ROOF MINIMUM SNOW LOAD: Pm = 23.0 PSF (adj) WINTER WIND PARAMETER: W2 = 25 DRIFT SURCHARGE SNOW LOADS: ASCE 7-22
RAIN:	15 MINUTE INTENSITY: i = 6.74 INHR 60 MINUTE INTENSITY: i = 3.36 INHR SEE ARCHITECTURAL FOR ROOF DRAIN LOCATIONS
WIND:	BASIC WIND SPEED (3-SECOND GUST): V = 110 MPH ULTIMATE Vasd = 85.2 MPH NOMINAL Kd = 0.95 C = 0.18
SEISMIC:	SEISMIC IMPORTANCE FACTOR: Ia = 1.0 SITE CLASS: D MAPPED SPECTRAL RESPONSE ACCELERATIONS: Sa = 0.12 S1 = 0.06 SPECTRAL RESPONSE COEFFICIENTS: Sds = 0.10 Sd1 = 0.05 SEISMIC DESIGN CATEGORY: B ANALYSIS PROCEDURE: STEEL BASIC SEISMIC FORCE RESISTING SYSTEM: EQUIVALENT LATERAL FORCE OR SEISMIC RESISTANCE RESPONSE MODIFICATION FACTOR: R = 3.0 SEISMIC RESPONSE COEFFICIENT: Ca = 0.033 DESIGN BASE SHEAR: V = Ca X W

STATEMENT OF STRUCTURAL SPECIAL INSPECTIONS

ONE OR MORE QUALIFIED INDEPENDENT TESTING AND INSPECTING AGENCIES SHALL PERFORM THE MATERIAL TESTING AND INSPECTION REQUIREMENTS AS OUTLINED IN THE PROJECT SPECIFICATIONS AND THIS SECTION. TESTING AND INSPECTION REPORTS SHALL BE FURNISHED TO THE BUILDING OFFICIAL, THE ARCHITECT, AND THE STRUCTURAL ENGINEER. REPORTS SHALL INDICATE THAT THE MATERIALS TESTED AND THE WORK INSPECTED ARE OR ARE NOT IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE REPORTED TO THE BUILDING OFFICIAL, THE ARCHITECT, AND THE STRUCTURAL ENGINEER.

THE TESTING AND INSPECTING AGENCIES SHALL SUBMIT A FINAL REPORT FOR EACH TYPE OF WORK STATING THAT ANY DISCREPANCIES NOTED IN THE TESTING AND INSPECTIONS HAVE BEEN CORRECTED AND THAT THE STRUCTURAL WORK WAS, TO THE BEST OF THEIR KNOWLEDGE, PERFORMED IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. THE TESTING AND INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF ANY RESPONSIBILITY FOR CONSTRUCTING THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND FOR CONTROLLING THE QUALITY OF CONSTRUCTION. PRIOR TO COMMENCEMENT OF THE WORK, EACH CONTRACTOR RESPONSIBLE FOR CONSTRUCTION OF THE TYPES OF WORK LISTED BELOW SHALL SUBMIT TO THE BUILDING OFFICIAL AND THE ARCHITECT A WRITTEN STATEMENT OF RESPONSIBILITY ACKNOWLEDGING AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SCHEDULING AND THE TIMELY NOTIFICATION TO THE TESTING AND INSPECTION AGENCIES OF THE NEED FOR MATERIAL TESTING OR INSPECTIONS. WORK WHICH REQUIRES TESTING OR INSPECTION SHALL BE READY FOR TESTING OR INSPECTION AT THE TIME OF THE TESTING AND INSPECTING AGENCY'S VISIT. NO WORK SHALL BE PERFORMED WHICH WOULD CONCEAL ITEMS TO BE TESTED OR INSPECTED UNTIL THE WORK HAS BEEN REVIEWED AND ACCEPTED.

THE FOLLOWING TYPES OF WORK REQUIRE SPECIAL INSPECTION (IBC - INTERNATIONAL BUILDING CODE 2024 EDITION):
1. INSPECTION OF FABRICATED ITEMS SHALL COMPLY WITH IBC SECTION 1704.2.5.
a. WHEN APPROVED BY THE BUILDING OFFICIAL, FABRICATORS FOLLOWING AN AGENCY AUDITED INTERNAL QUALITY CONTROL PRACTICE SHALL SUBMIT A CERTIFICATE OF COMPLIANCE STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.

2. TESTING AND INSPECTION OF STEEL CONSTRUCTION SHALL COMPLY WITH IBC SECTION 1705.2 AND AISI 360 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS.
a. THE CONTRACTOR SHALL MAINTAIN MATERIAL TEST REPORTS, MANUFACTURER'S CERTIFICATIONS, PRODUCT DATA SHEETS, WELDING PROCEDURE SPECIFICATIONS, WELDING PERSONNEL PERFORMANCE QUALIFICATION RECORDS, FABRICATOR/RECTOR QUALITY CONTROL MANUAL, FABRICATOR/RECTOR INSPECTOR QUALIFICATIONS AS SPECIFIED FOR REVIEW BY ARCHITECT/ENGINEER AS INDICATED IN AISI 360 CHAPTER N.

b. INSPECT AWS PERFORMANCE QUALIFICATION RECORDS FOR PERSONNEL PERFORMING SHOP FABRICATION OR FIELD ERECTION WELDING.
c. PERFORM INSPECTION OF THE ERECTED METAL BUILDING SYSTEM TO VERIFY COMPLIANCE WITH THE INSTALLATION DRAWINGS, INCLUDING RAFTER/BEAM FLANGE BRACES, COLUMN FLANGE BRACES, PURLIN AND GIRT LAPPED CONDITIONS, PURLIN AND GIRT BRIDGING, AND X-BRACING TIGHTENING.
d. PERFORM VISUAL INSPECTION OF ALL SHOP FABRICATION AND FIELD ERECTION WELDS.
e. PERFORM ULTRASONIC INSPECTION OF ALL PARTIAL OR COMPLETE JOINT PENETRATION WELDS DURING SHOP FABRICATION AND FIELD ERECTION.

f. PERFORM CONTINUOUS INSPECTION OF ALL FILLET WELDS GREATER THAN 5/16" DURING SHOP FABRICATION AND FIELD ERECTION.
g. PERFORM VISUAL INSPECTION OF ALL SNUG-TIGHTENED (TYPE S7) BOLTED CONNECTIONS.
h. OBSERVE THE PRE-INSTALLATION VERIFICATION TESTING REQUIRED FOR PRETENSIONED BOLTED CONNECTIONS DEFINED IN THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC) SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS, SECTION 7.

i. PERFORM VISUAL INSPECTION OF ALL BOLTED CONNECTIONS USING TENSION CONTROL BOLTS AT SLP-CRITICAL (TYPE SC) BOLTED CONNECTIONS AND PRETENSIONED (TYPE PT) BOLTED CONNECTIONS.
j. PERFORM VISUAL INSPECTION OF THE PLACEMENT OF ANCHOR RODS AND EMBED PLATES IN CONCRETE. VERIFY DIAMETER, GRADE, TYPE, LENGTH, AND EMBEDMENT OF ANCHORS PRIOR TO PLACING CONCRETE.

3. TESTING AND INSPECTION OF CONCRETE CONSTRUCTION SHALL COMPLY WITH IBC SECTION 1705.3 AND TABLE 1705.3.
a. VERIFY USE OF REQUIRED DESIGN MIX AND PERFORM SAMPLING AND TESTING OF CAST-IN-PLACE CONCRETE AS SPECIFIED.
b. PERFORM PERIODIC OBSERVATION OF FORMWORK AND REINFORCING FOR SIZE, COVER, SPACING, POSITIONING, LAP LENGTHS AND LOCATIONS.
c. PERFORM INSPECTION OF THE REINFORCING FOR STEEL, COVER, SPACING, POSITIONING, LAP LENGTHS AND LOCATIONS AT SLABS ON GRADE.
d. INSPECT ALL MECHANICALLY SPLICED REINFORCEMENT.

e. PERFORM PERIODIC INSPECTION OF CONCRETE CURING AND PROTECTION PROCEDURES, INCLUDING COMPLIANCE WITH THE HOT AND COLD WEATHER REQUIREMENTS DEFINED IN THE SPECIFICATIONS.
f. THE CONTRACTOR SHALL MAINTAIN RECORDS OF BATCH REPORTS AND DELIVERY TICKETS ON EACH LOAD OF CONCRETE DELIVERED TO THE PROJECT SITE FOR PERIODIC REVIEW BY THE ARCHITECT/ENGINEER.

4. TESTING AND INSPECTION OF POST-INSTALLED ANCHORS SHALL COMPLY WITH IBC SECTION 1705.1.1 AND TABLE 1705.3.
a. PERFORM AN INITIAL INSTALLATION INSPECTION FOR EACH TYPE AND SIZE OF POST-INSTALLED ANCHOR. ANY CHANGE IN THE PERSONNEL PERFORMING THE INSTALLATION SHALL REQUIRE AN INITIAL INSTALLATION INSPECTION.

b. PERFORM PERIODIC INSPECTIONS TO VERIFY THAT THE INSTALLATIONS CONTINUE TO BE PROPERLY PERFORMED.
c. INSTALLATION INSPECTIONS SHALL VERIFY ANCHOR TYPE, DIAMETER, EMBEDMENT DEPTH, SPACING, ADHESIVE TYPE AND EXPIRATION DATE, HOLE DIMENSIONS, BASE MATERIAL, HOLE CLEANING PROCEDURES, MAXIMUM IMPACT WRENCH TORQUE RATING, AND ADHERENCE TO THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

d. PERFORM VISUAL OBSERVATION OF ALL COMPLETED POST-INSTALLED ANCHOR INSTALLATIONS.
5. TESTING AND INSPECTION OF SOILS SHALL COMPLY WITH IBC SECTION 1705.6 AND TABLE 1705.6.
a. PERFORM SAMPLING, TESTING, AND INSPECTION OF THE SOIL TYPE, EXPOSED SUBGRADE, MOISTURE CONTENT, LIFT THICKNESS, AND COMPACTION AS SPECIFIED.

b. PERFORM PERIODIC TESTING AND INSPECTION OF THE SOILS AT THE FOUNDATION SYSTEM BEARING ELEVATION TO VERIFY THE REQUIRED SOIL BEARING CAPACITIES.

SUBGRADE PREPARATION AND EARTHWORK NOTES

SUBGRADE PREPARATION AND EARTHWORK SHALL BE PERFORMED UNDER THE DIRECTION OF THE GEOTECHNICAL ENGINEER. THE GEOTECHNICAL ENGINEER SHALL APPROVE SOIL MATERIALS, MONITOR EARTHWORK OPERATIONS AND THE PERFORMANCE OF TESTING DURING EARTHWORK PROCESS. SUBGRADE PREPARATION SHALL INCLUDE REMOVAL OF EXISTING SLABS, FOUNDATIONS, PAVEMENT, AND STRIPPING THE EXPOSED SURFACE TO A MINIMUM 6" DEPTH TO REMOVE SURFACE VEGETATION, ORGANIC MATERIAL, EXISTING FILL MATERIAL, AND OTHER DELETERIOUS MATERIAL. COHESIVE SOILS SHALL BE CLEAN, WELL-GRADED AGGREGATE MEETING THE ASTM D448 NO. 57 MATERIAL. AFTER SITE PREPARATION WORK HAS BEEN COMPLETED, PROOF ROLL THE EXPOSED SUBGRADE IN THE PRESENCE OF THE GEOTECHNICAL ENGINEER TO IDENTIFY ANY AREAS OF SOFT OR UNSTABLE MATERIAL. REMOVE ANY UNSATISFACTORY MATERIAL AND REPLACE WITH SUITABLE MATERIAL AS DIRECTED BY THE GEOTECHNICAL ENGINEER.

SCARIFY MOISTURE CONTENT AND COMPACT THE TOP 8 INCHES OF THE EXPOSED SUBGRADE PRIOR TO STARTING ENGINEERED FILL PLACEMENT OPERATIONS. MOISTURE CONDITION THE SCARIFIED SOILS TO AT LEAST 3 PERCENTAGE POINTS ABOVE OPTIMUM MOISTURE CONTENT AND COMPACT TO A MINIMUM OF 95% OF MAXIMUM DRY DENSITY AS DETERMINED BY STANDARD PROCTOR. ASTM D98.
PROVIDE A MINIMUM 4-INCH THICK ZONE OF GRANULAR BASE MATERIAL BELOW SLAB ON GRADE FLOOR AREAS. GRANULAR DRAINAGE BASE MATERIAL SHALL BE CLEAN, WELL-GRADED AGGREGATE MEETING THE ASTM D448 NO. 57 MATERIAL.
PROVIDE A MINIMUM 18-INCH THICK ZONE OF LVC MATERIAL BELOW THE ZONE OF GRANULAR BASE MATERIAL AT SLAB ON GRADE FLOOR AREAS. LOW VOLUME CHARGE (LVC) ZONE MATERIAL SHALL BE AN APPROVED SOIL, FREE OF ORGANIC MATERIAL, AND DELETERIOUS MATERIAL. COHESIVE SOILS SHALL HAVE A LIQUID LIMIT LESS THAN 40 AND A PLASTICITY INDEX BETWEEN 5 AND 15.
SILTY GRAVEL, SUCH AS NO. 40, IS AN ACCEPTABLE LVC MATERIAL.
REQUIRED GENERAL ENGINEERED FILL MATERIAL SHALL BE AN APPROVED SOIL, FREE OF ORGANIC MATERIAL AND DELETERIOUS MATERIAL WITH A LIQUID LIMIT LESS THAN 45 AND A PLASTICITY INDEX LESS THAN 20.
FILL MATERIAL SHALL BE PLACED IN MAXIMUM 9-INCH THICK LOOSE HORIZONTAL LIFTS AND SHALL BE COMPACTED TO AT LEAST 95% OF MAXIMUM DRY DENSITY PER STANDARD PROCTOR ASTM D999.
COHESIVE SOILS SHALL BE PLACED AT A MOISTURE CONTENT BETWEEN OPTIMUM AND 3 PERCENT ABOVE OPTIMUM MOISTURE CONTENT. THE SPECIFIED SOIL MOISTURE CONTENT SHALL BE MAINTAINED UNTIL THE FLOOR SLAB HAS BEEN PLACED.

CAST-IN-PLACE CONCRETE

CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES AND MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS:
FOOTINGS AND FOUNDATION WALLS: 3000 PSI WITH A MAX. W/C RATIO OF 0.55 (NON-AIR ENTRAINED)
INTERIOR FLOOR SLABS ON GRADE: 3500 PSI WITH A MAX. W/C RATIO OF 0.50
EXTERIOR EXPOSED CONCRETE: 4500 PSI WITH A MAX. W/C RATIO OF 0.45
SEE CIVIL DRAWINGS FOR EXTERIOR PAVEMENT

CONCRETE SHALL BE IN STRICT CONFORMANCE WITH THE CURRENT "ACI MANUAL OF CONCRETE PRACTICE". CEMENTITIOUS MATERIAL SHALL BE PORTLAND CEMENT CONFORMING TO ASTM C150 TYPE I OR II OR PORTLAND-LESTOME BLENDED HYDRAULIC CEMENT CONFORMING TO ASTM C595 TYPE I, F, LY ASH CONFORMING TO ASTM C518, TYPE C OR F MAY BE USED TO REPLACE UP TO 20% OF THE CEMENT.

AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL MEET ASTM C33. AGGREGATES SHALL BE PROPORTIONED SUCH THAT MIX DESIGN SHALL CONTAIN A MINIMUM OF 50% COARSE AGGREGATES BY GRADATION REQUIREMENTS SET FORTH IN ASTM C33. COARSE AGGREGATE SHALL MEET NO. 57 GRADING REQUIREMENTS.

EXTERIOR EXPOSED CONCRETE SHALL HAVE FROM 4 TO 7% ENTRAINED AIR.
NO ALUMINUM SHALL BE PLACED IN THE CONCRETE.
CONTRACTION JOINTS IN CONCRETE FLOORS SHALL BE SPACED TO DIVIDE THE SLAB INTO PANELS AS SHOWN. ALL SAW-CUT JOINTS IN SLAB ON GRADE FLOORS SHALL USE AN EARLY ENTRY DRY-CUTTING SAWING SYSTEM.

CONCRETE SHALL BE PROPORTIONED FOR A 2" TO 5" SLUMP RANGE AT THE POINT OF PLACEMENT.
CONCRETE SHALL CONTAIN A WATER-REDUCING ADMIXTURE MEETING ASTM C494, TYPE A OR F, AT A DOSAGE TO PROVIDE THE NECESSARY FLOWABILITY AND WORKABILITY WITHIN THE SPECIFIED SLUMP RANGE.
DESIGN, ERECT, SHORE, BRACE, AND MAINTAIN FORMWORK, ACCORDING TO ACI 301, TO SUPPORT ALL APPLIED LOADS DURING THE CONCRETE PLACEMENT.

CONCRETE PLACEMENT, CURING, AND HOT AND COLD WEATHER CONCRETING SHALL COMPLY WITH ACI 301, ACI 305R, ACI 305.1, ACI 306R, AND ACI 306.1.
CAST-IN-PLACE CONCRETE SHALL BE OBTAINED FOR TESTING PER ASTM C172 AND TESTED AS FOLLOWS:
a. OBTAIN ONE SET OF FOUR TEST CYLINDERS FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE LESS THAN 25 CUBIC YARDS, PLUS ONE SET OF FOUR TEST CYLINDERS FOR EACH ADDITIONAL 50 CUBIC YARDS OR REACTION THEREOF.

b. SLUMP: ONE TEST AT POINT OF DISCHARGE PER ASTM C143 FOR EACH SET OF TEST CYLINDERS TAKEN. PERFORM ADDITIONAL SLUMP TEST ON TRUCKLOADS WHEN CONSISTENCY SEEMS TO HAVE CHANGED.
c. CONCRETE TEMPERATURE: ONE TEST PER ASTM C1064 FOR EACH SET OF TEST CYLINDERS TAKEN OR HOURLY WHEN AIR TEMPERATURE IS BELOW 40°F OR ABOVE 90°F.

d. AIR CONTENT: VOLUMETRIC METHOD PER ASTM C173 OR PRESSURE METHOD PER ASTM C231 FOR EACH SET OF TEST CYLINDERS.
e. COMPRESSION TEST SPECIMENS: ONE SET OF FOUR STANDARD CYLINDERS PER ASTM C31 AT THE SPECIFIED FREQUENCY.
f. COMPRESSIVE STRENGTH TESTS: ONE SET OF FOUR CYLINDERS PER ASTM C39. TEST ONE CYLINDER AT 7-DAYS, TWO CYLINDERS AT 28-DAYS, AND HOLD ONE IN RESERVE TO BE TESTED AS DIRECTED.

PERSONNEL TRAINED AND CERTIFIED IN CONCRETE SAMPLING SHALL PERFORM ALL CONCRETE TESTING AND SAMPLING. TEST RESULTS SHALL BE SUBMITTED TO THE ARCHITECT, ENGINEER, AND CONTRACTOR WITHIN 24 HOURS OF COMPLETING TESTS. CONCRETE TESTING SHALL BE PERFORMED BY AN APPROVED TESTING AGENCY.
SUBMIT THE CONCRETE MIX DESIGN TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO BEGINNING CONSTRUCTION.

REINFORCING STEEL

WELDED WIRE REINFORCEMENT (WWR) SHALL MEET ASTM A1064. FURNISH IN FLAT SHEETS. LAP SPICE WELDED WIRE REINFORCEMENT THE CROSS WIRE SPACING PLUS 5 INCHES.
REINFORCING SHALL MEET ASTM A615 - 60,000, UNLESS NOTED OTHERWISE.
REINFORCING STEEL SHALL HAVE ADEQUATE COVERAGE AS INDICATED IN ACI 318 FOR THE GIVEN EXPOSURE.
REINFORCING SHALL BE CONTINUOUS AND LAPPED A MINIMUM OF 34 INCHES OR 36 BAR DIAMETERS WHICHEVER IS GREATER, UNLESS OTHERWISE NOTED.

REINFORCING SHALL BE DETAILED ACCORDING TO THE ACI DETAILING MANUAL AND PREPARED UNDER THE SUPERVISION OF A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF KANSAS.
PROVIDE CORNER LAP BARS TO MATCH IN SIZE AND SPACING OF ALL TRENCH FOOTING HORIZONTAL BARS.
PROVIDE 3-INCH SLAB BOLSTER WITH CONTINUOUS BOTTOM PLATE AT 4'-0" MAXIMUM CENTERS FOR POSITIONING FOOTING BOTTOM BARS.

PROVIDE BAR SUPPORTS FOR BARS IN SLABS ON GRADE AT A MAXIMUM OF 4'-0" IN EACH DIRECTION.
MARK EACH BUNDLE OF THE REINFORCING WITH WEATHERPROOF TAGS.
MECHANICAL SPICE COUPLERS SHALL BE CAPABLE OF DEVELOPING 125% OF THE SPECIFIED YIELD STRENGTH (Fy) OF THE BAR IN BOTH TENSION AND COMPRESSION. MECHANICAL SPICE COUPLERS ARE TO BE BAR-LOCK SICA SERIES BY DAYTON SUPERIOR CORPORATION, ZAP SCREWLOCK TYPE 2 SERIES BY BARSPLICE PRODUCTS INC., OR AN APPROVED EQUIVALENT.

SUBMIT THE REINFORCING STEEL SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO BEGINNING CONSTRUCTION.

SPECIAL REQUIREMENTS AT FOUNDATION SYSTEMS

1. SLEEVES OR BLOCKOUTS 10 INCHES OR GREATER IN ANY DIRECTION MUST BE APPROVED PRIOR TO INSTALLING ANY TRENCH FOOTING.
2. LOCATE SLEEVES OR BLOCKOUTS WHERE THE EDGE OF THE OPENING CLOSEST TO THE TOP OR BOTTOM OF THE TRENCH FOOTING IS 12 INCHES OR GREATER, UNLESS SPECIFICALLY APPROVED OTHERWISE.
3. IN TRENCH FOOTINGS, PROVIDE 2#5 HORIZONTAL BARS, ONE BAR EACH FACE, ABOVE AND BELOW OPENINGS, 4'-0" LONGER THAN THE OPENING DIMENSION AND CENTERED ON THE OPENING. PROVIDE ONE ADDITIONAL TRENCH FOOTING STIRRUP PLACED WITHIN 2 TO 3 INCHES EACH SIDE OF OPENINGS GREATER THAN 6 INCHES.
4. NO SLEEVES OR BLOCKOUTS SHALL OCCUR WITHIN THE COLUMNS OR PLASTERS CAST INTEGRAL WITH WALLS OR TRENCH FOOTINGS. NO SLEEVES OR BLOCKOUTS SHALL OCCUR DIRECTLY BELOW BEAM BEARING POINTS UNLESS SPECIFICALLY APPROVED OTHERWISE.
5. PROVIDE A MINIMUM OF 12 INCHES OF CONCRETE BETWEEN ADJACENT SLEEVES OR BLOCKOUTS UNLESS SPECIFICALLY APPROVED OTHERWISE.
6. CORED OPENINGS SHALL MEET THE SAME REQUIREMENTS AS SLEEVED OPENINGS.

STRUCTURAL STEEL

STRUCTURAL STEEL SHALL MEET THE LATEST AISI "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
STEEL PLATES AND SHAPES SHALL MEET ASTM A36, EXCEPT STRUCTURAL PIPING SHALL MEET ASTM A53, GRADE B, Fy = 35 KSI. UNHEADED ANCHOR RODS SHALL BE ASTM F1554, GRADE 36 OR ASTM F1554, GRADE 55 (SUPPLEMENT S1).
ANCHOR RODS SET IN CONCRETE SHALL BE FURNISHED WITH DOUBLE NUTS AND SET WITH A TEMPLATE. PROVIDE 3/8" PLATE WASHERS ABOVE ALL OVERSIZED HOLES IN COLUMN BASE PLATES. PROVIDE STANDARD SIZE HOLE IN PLATE WASHERS. PROVIDE STANDARD SIZE HOLES FOR ALL BOLTS AND ANCHORS IN STEEL FRAMING MEMBERS UNLESS NOTED OTHERWISE (1/16" LARGER HOLE THAN DIAMETER OF BOLT OR ANCHOR).
LARGER HOLE THAN DIAMETER OF BOLT OR ANCHOR).
WELDING SHALL CONFORM TO AWS D1.1, "STRUCTURAL WELDING CODE - STEEL". ALL WELDS SHALL BE AWS PREQUALIFIED WELDED JOINTS. NO UNAUTHORIZED WELDS WILL BE ACCEPTED.
EVALUATE MOISTURE CONTENT AND COMPACT THE TOP 8 INCHES OF THE EXPOSED SUBGRADE PRIOR TO STARTING ENGINEERED FILL PLACEMENT OPERATIONS. MOISTURE CONDITION THE SCARIFIED SOILS TO AT LEAST 3 PERCENTAGE POINTS ABOVE OPTIMUM MOISTURE CONTENT AND COMPACT TO A MINIMUM OF 95% OF MAXIMUM DRY DENSITY AS DETERMINED BY STANDARD PROCTOR. ASTM D98.

POST-INSTALLED ANCHORS

POST-INSTALLED ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. HOLES SHALL BE DRILLED WITH THE REQUIRED BIT TYPE AND SIZE TO PROVIDE THE MINIMUM EMBEDMENT LENGTH SPECIFIED IN THE STRUCTURAL DRAWINGS. HOLES SHALL BE CLEANED PRIOR TO INSTALLING THE ANCHOR WITH THE BRUSH AND COMPRESSED AIR METHOD OR SOLY GRAVEL, SUCH AS NO. 40, IS AN ACCEPTABLE LVC MATERIAL.
WITH THE MANUFACTURER'S PROPRIETARY DRILL BIT AND DUST EXTRACTION SYSTEM.
INSTALLATION OF POST-INSTALLED ANCHORS SHALL BE PERFORMED BY PERSONNEL, TRAINED BY THE ANCHOR AND/OR ADHESIVE MANUFACTURER FOR THE TYPE BEING USED.
POST-INSTALLED ANCHOR TYPES SHALL BE AS FOLLOWS:
UNLESS NOTED OTHERWISE, ADHESIVE ANCHORS SHALL BE ASTM A307, GRADE B, ASTM F1554, GRADE 36, OR AN APPROVED GREATER STRENGTH THREADED ROD. THREADED RODS SHALL BE PREPARED ACCORDING TO THE ADHESIVE MANUFACTURER'S SPECIFICATIONS AND THOROUGHLY CLEANED OF ALL SURFACE OILS.
ADHESIVE ANCHOR SYSTEMS INTO CONCRETE SHALL USE HILTI HIT-HY 200 V3 ADHESIVE OR AN APPROVED EQUAL. HILTI HIT-RE 500 V3, SIMPSON STRONG-TIE AT-3G, SIMPSON STRONG-TIE SET-3G, DEWALT AC208+, AND DEWALT PURE 220+ ARE APPROVED EQUAL ADHESIVE SYSTEMS INTO CONCRETE.

A PISTON PLUG INJECTION PROCEDURE APPROVED BY THE ADHESIVE MANUFACTURER SHALL BE USED FOR THE INJECTION OF ADHESIVE INTO ALL HOLES GREATER THAN 10 INCHES IN DEPTH.
SUBSTITUTION OF POST-INSTALLED ANCHORS OR REINFORCING BARS WILL NOT BE PERMITTED FOR EMBEDDED ELEMENTS SHOWN ON THE DRAWINGS UNLESS APPROVED IN ADVANCE BY THE ENGINEER.
THE INSTALLATION OF POST-INSTALLED ANCHORS AND REINFORCING BARS SHALL BE REVIEWED AND ACCEPTED BY THE FIELD TESTING AND INSPECTION AGENCY.

METAL BUILDING

METAL BUILDING SHALL BE DESIGNED FOR THE FOLLOWING LOADS AND CRITERIA:
DEAD LOAD: SELF WEIGHT OF METAL BUILDING SYSTEM
COLLATERAL LOAD: 5 PSF

SEE THE "DESIGN LOADS" SECTION FOR LIVE LOAD, SNOW LOAD, WIND LOAD, AND SEISMIC DESIGN CRITERIA.
CONSIDERATION SHALL BE GIVEN TO UNBALANCED SNOW LOADS, SNOW DRIFTING AND SLIDING AT CHANGES IN ROOF SLOPE.
GIVE CONSIDERATION SHALL BE GIVEN TO THE WEIGHT OF ANY SUSPENDED EQUIPMENT IN THE DESIGN OF THE STRUCTURAL MEMBERS. SEE THE MECHANICAL AND ELECTRICAL PLANS AND SPECIFICATIONS. VERIFY THE SIZE, WEIGHT, AND LOCATION OF ALL SUSPENDED ITEMS PRIOR TO DESIGNING AND FABRICATING THE METAL BUILDING STRUCTURE.

WIND LOADS SHALL BE APPLIED IN ACCORDANCE WITH THE REQUIREMENTS CONTAINED IN THE BODY OF THE INTERNATIONAL BUILDING CODE AND ASCE 7-22.
METAL BUILDING DESIGN SHALL MEET THE REQUIREMENTS OF THE LATEST MBMA BUILDING SYSTEMS MANUAL, THE INTERNATIONAL BUILDING CODE, THE AISI SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, AND THE AISI SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, AND THE PROJECT SPECIFICATIONS.

THE METAL BUILDING MANUFACTURER SHALL BE RESPONSIBLE FOR THE METAL BUILDING SYSTEM DESIGN AND SHALL SUBMIT A LETTER OF DESIGN CERTIFICATION SIGNED AND SEALED BY A QUALIFIED PROFESSIONAL ENGINEER.
THE METAL BUILDING MANUFACTURER SHALL SUBMIT COMPLETE DESIGN CALCULATIONS PREPARED, SIGNED, AND SEALED BY A QUALIFIED PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF KANSAS, ALONG WITH THE SHOP DRAWINGS FOR REVIEW PRIOR TO THE START OF ANY FOUNDATION WORK.

THE BUILDING SHALL BE DESIGNED TO LIMIT THE COMBINED RATIO OF ACTUAL STRESSES TO ALLOWABLE STRESSES TO BE 1.00 OR LESS.
DESIGN ASSEMBLIES TO WITHSTAND SERVICE LEVEL (ASD) DESIGN LOADS WITH DEFLECTIONS NO GREATER THAN THE FOLLOWING CRITERIA. FOR CALCULATION OF MAIN FRAME LATERAL DEFLECTIONS, SERVICE LEVEL WIND LOADS ARE TO BE DETERMINED FOR THE 50 YEAR MEAN RECURRENCE INTERVAL NOT THE 10-YEAR MEAN RECURRENCE INTERVAL WIND SPEED.

THE BUILDING SHALL BE DESIGNED TO LIMIT VERTICAL TOTAL LOAD DEFLECTION FOR ALL ROOF FRAMING MEMBERS TO L/240.
LATERAL LOAD DEFLECTIONS FOR MAIN FRAMES SHALL BE LIMITED TO H/120.
LATERAL LOAD DEFLECTIONS FOR GIRTS AND ENDWALL COLUMNS SHALL BE LIMITED TO L/240.

ALL COLUMNS AND FRAMES SHALL BE DESIGNED WITH PINNED END COLUMN BASES.
THE METAL BUILDING MANUFACTURER OR THE CONTRACTOR SHALL DESIGN THE DIAMETER AND LENGTH OF THE COLUMN ANCHOR RODS. CONCRETE DESIGN STRENGTH AND FOUNDATION DETAILS TO BE USED IN THE DESIGN OF ANCHOR RODS ARE INDICATED ON THE PLANS AND NOTES. SEE "STRUCTURAL STEEL" SECTION FOR ADDITIONAL INFORMATION AND REQUIREMENTS ON ANCHOR RODS.

VERIFY ALL BUILDING DIMENSIONS WITH THE METAL BUILDING MANUFACTURER. THE EXACT COLUMN CENTERLINE AND ANCHOR ROD LOCATION SHALL BE COORDINATED WITH THE BUILDING FOUNDATION BY THE CONTRACTOR AND THE METAL BUILDING SUPPLIER PRIOR TO THE START OF ANY FOUNDATION WORK.

CONCRETE COLUMN SIZE SHALL BE CHECKED AGAINST THE BASE PLATE SIZE AND ANCHOR ROD PATTERN TO ASSURE ADEQUATE SIZE TO INSTALL THE ANCHOR RODS WITHIN THE CONCRETE COLUMN REINFORCING CAGE AND THAT THE CONCRETE COLUMN SIZE IS GREATER THAN THE BASE PLATE SIZE.

THE METAL BUILDING MANUFACTURER SHALL PROVIDE THE TOP AND BOTTOM FLANGE LATERAL BRACING AT ROOF PURLINS SUPPORTING STANDING SEAM ROOFING SYSTEMS. PURLIN LATERAL BRACING SHALL BE INSTALLED TO MEET THE REQUIREMENTS OF THE AISI SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, THE GOVERNING BUILDING CODE, AND THE OWNER'S INSURANCE CARRIER.

THE METAL BUILDING MANUFACTURER SHALL PROVIDE THE FLANGE LATERAL BRACING AT WALL GIRTS AS REQUIRED BY DESIGN. WHERE INTERIOR DESIGN IS NOT PROVIDED, GIRTS SHALL HAVE METAL STRAP BRACING AT A MAXIMUM SPACING OF 8'-4".
X-BRACING SHALL CONSIST OF STEEL RODS OR OTHER STRUCTURAL STEEL SHAPES. CABLE BRACING WILL NOT BE ALLOWED.

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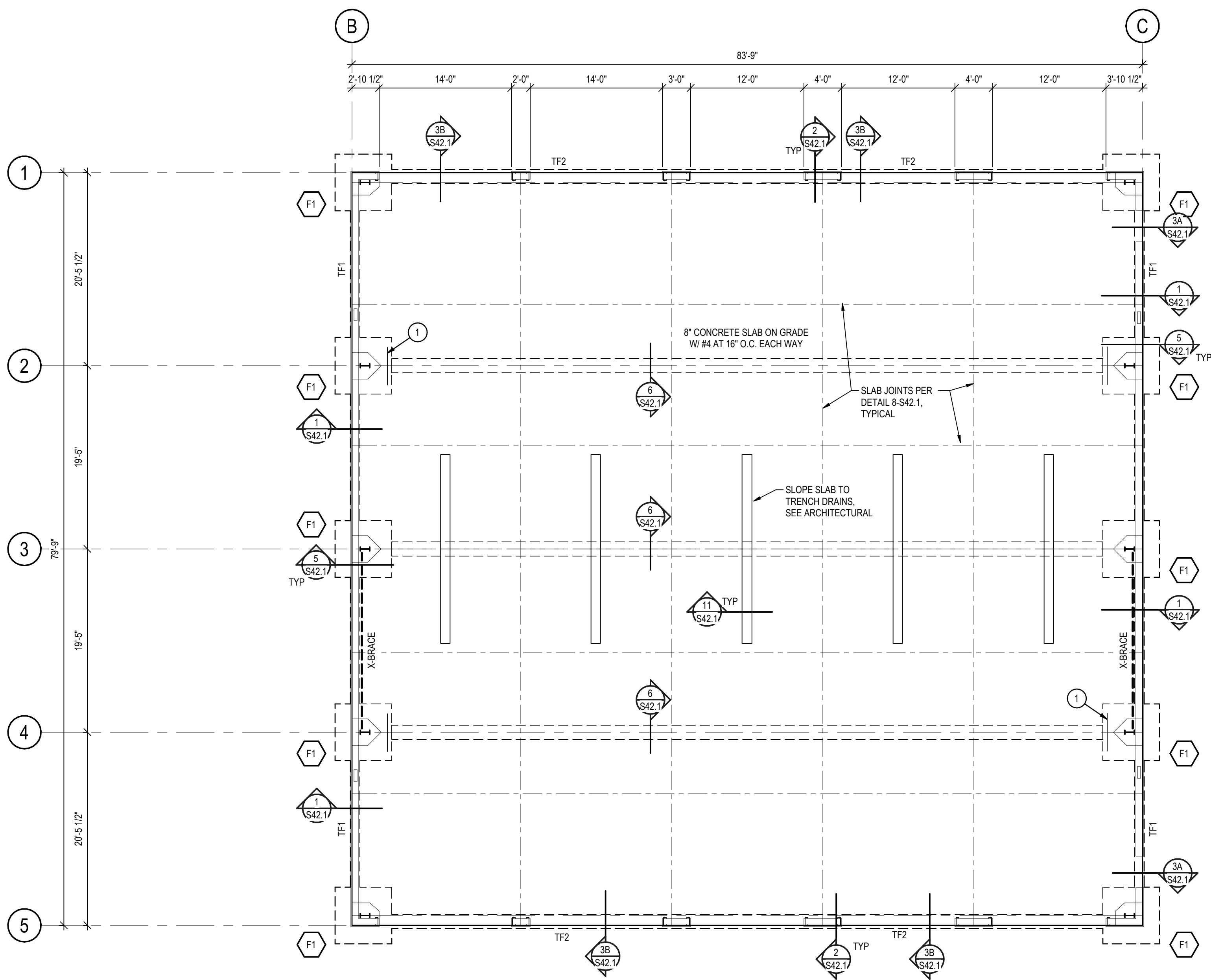
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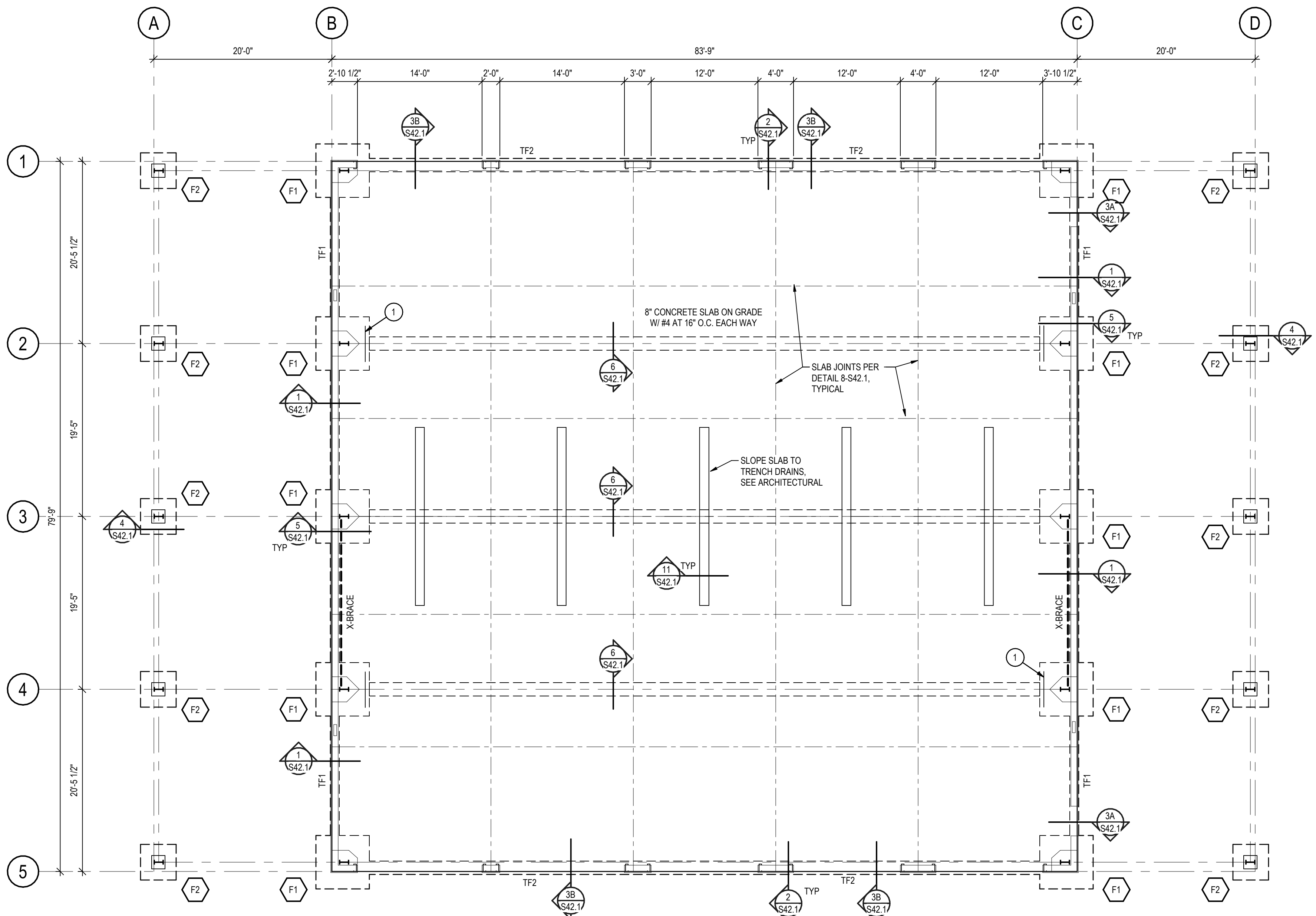
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REVISIONS

PROJECT NUMBER
5537.50
DATE
MARCH 2026
GENERAL STRUCTURAL NOTES



B FOUNDATION PLAN - BASE BID
1/8" = 1'-0"
0' 4' 8' 16'



A FOUNDATION PLAN - ALTERNATE
1/8" = 1'-0"
0' 4' 8' 16'

PLAN MARKS

- F# CONCRETE COLUMN AND FOOTING MARK, SEE SCHEDULE, SHEET S42.1
TF# TRENCH FOOTING MARK, SEE SCHEDULE, SHEET S42.1

PLAN NOTES

- SEE GENERAL STRUCTURAL NOTES, SHEET S10.1, FOR ADDITIONAL NOTES AND INFORMATION.
- PLAN DIMENSIONS ARE TO FACE OF METAL BUILDING GIRTS OR CENTERLINE OF COLUMN GRID, UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL FOR DIMENSIONS NOT INDICATED ON STRUCTURAL DRAWINGS.
- CENTER SCHEDULED COLUMN FOOTING BELOW METAL BUILDING COLUMN BASE PLATE, TYPICAL, UNLESS NOTED OTHERWISE.
- SEE DETAIL 9-S42.1 FOR TYPICAL CORNER BAR DETAIL.
SEE DETAIL 8-S42.1 FOR TYPICAL SLAB JOINT DETAILS.
- RECOMPACT SOIL PER SPECIFICATIONS AT ALL EXCAVATIONS FOR PIPING.
SEE DETAIL 10-S42.1 FOR DETAILS AT PIPING BELOW FOOTINGS.

KEYED PLAN NOTES

- 1 REINFORCE DISCONTINUOUS JOINTS AND REINTRANT CORNERS WITH 2-#4X4-0 AT 3' O.C. AT MID DEPTH OF SLAB, TYPICAL.

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VEHICLE STORAGE
FOUNDATION PLANS

