# APPENDIX 1

Geotechnical Investigation

Dated December 29, 2025



REPORT ON GEOTECHNICAL INVESTIGATION

2025 DEXTER SCHOOLS BOND PROGRAM

# DHS FIELDHOUSE AND SITE IMPROVEMENTS AND AL RITT STADIUM LIGHTING

DEXTER, MICHIGAN 48130

LATITUDE 42.324878° N
LONGITUDE 83.89651° W

G2 CONSULTING GROUP

Prepared for:

Granger Construction
6267 Aurelius Road
Lansing, Michigan 48911

G2 Project No. 253566S
December 29, 2025

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December 29, 2025

Mr. Greg Brand, P.E.
Senior Project Manager
Granger Construction
6267 Aurelius Road
Lansing, Michigan 48911

**RE: Report on Geotechnical Investigation**
2025 Dexter Schools Bond Program
DHS Fieldhouse and Site Improvements and Al Ritt Stadium Lighting
Dexter, Michigan 48130
G2 Project No. 253566

Dear Mr. Brand:

We have completed the geotechnical investigation associated with the 2025 Dexter Schools Bond Program, which includes construction of the proposed fieldhouse athletic building and site improvements at Dexter High School, as well as the construction of stadium lighting at Al Ritt field. This report presents the results of our observations and analyses, as well as recommendations related to earthwork, subgrade preparation, foundation design, bituminous pavement design, in-situ infiltration, and related construction considerations.

We appreciate the opportunity to be of service to Granger Construction and look forward to discussing the recommendations presented. In the meantime, if you have any questions regarding the report or any other matter pertaining to the project, please call us.

Sincerely,

**G2 Consulting Group, LLC**

Ryan P. Carpus, P.E.
Project Engineer

Jennifer M. Casey, P.E.
Project Consultant

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# EXECUTIVE SUMMARY

We understand the proposed project consists of constructing a new fieldhouse, pavements, a detention pond, and two (2) underground stormwater management structures at Dexter High School and new lighting at Al Ritt Stadium.

Approximately 2 to 10 inches of topsoil or 12 to 24 inches of crushed limestone aggregate are generally present at the ground surface of the soil borings. Within soil borings B-21 and B-22, B-28, B-31, B-33, B-37, and B-38, stiff to very stiff cohesive fill soils and very loose to medium compact granular fill soils underlie the topsoil and extend to approximate depths ranging from 3 to 6 feet. In general, the native soils consist of alternating strata of very loose to compact granular and stiff to very hard cohesive soils, extending to the explored depths ranging from 10 to 35 feet below grade. Groundwater was encountered at approximate depths ranging from 5 to 20 feet below existing grade within borings B-21 through B-29, B-31, and B-36 through B-39. Upon completion of drilling operations, the groundwater level within these borings was measured at depths ranging from 6 to 29 feet below grade. No measurable groundwater was observed during or upon completion of drilling operations within borings B-30 or B-32 through B-35. The rise in the groundwater level above the encountered elevation within soil borings B-22, B-23, and B-31 may be indicative of an artesian condition.

The fill soils noted above do not appear to have been placed in an engineered manner and have organic matter contents ranging from 4.3 to 7.0 percent. Considering the existing ground surface elevations and the fieldhouse FFE, we anticipate the fill soils within the influence of the fieldhouse will be removed while adjusting site grades to achieve the design subgrade elevation. In addition, based on proposed pavement finished grades, we anticipate the existing fill soils will be removed during grading operations in the vicinity of soil borings B-28 and B-31. Some existing fill soil may remain beneath the proposed pavement section within the vicinity of soil boring B-33. The fill within soil boring B-33 does not contain any notable organic matter and is generally considered suitable for support of the pavement, following the successful completion of a proof-roll/proof-compaction evaluation.

We recommend the fieldhouse be supported by shallow conventional strip and/or spread footings designed to bear at a minimum depth of 3-1/2 feet below finished grades within the native loose to medium compact granular soils, very stiff to hard cohesive soils, or engineered fill. Foundations bearing within the above mentioned soils can be designed based on a net allowable bearing capacity of 2,500 psf. Recommended soil design parameters for drilled pier foundations for the light poles are provided in the FOUNDATION RECOMMENDATIONS section of this report. If the recommendations outlined in this report are adhered to, total and differential settlements for the completed structures should be within 1 inch and ½ inch, respectively. We expect settlements of these magnitudes are within tolerable limits for the type of structures proposed.

No significant groundwater accumulations are anticipated in construction excavations for the proposed fieldhouse, utilities, or stormwater BMPs. We anticipate any groundwater seepage or surface runoff can be controlled with a properly constructed system of sumps and pumps.

Caving and/or sloughing of the granular soils will occur during drilled shaft excavation operations. In addition, groundwater was encountered at depths ranging from 5 to 7 feet below existing grade. Therefore, the drilled pier contractor should come to the site prepared to use temporary steel casing to maintain a stable excavation and seal off groundwater. The contractor should also come to the site prepared to break-up and remove any cobbles encountered in the excavation and may need a rock auger to excavate through the very hard cohesive soils.

Do not consider this summary separate from the entire text of this report, with all the conclusions and qualifications mentioned herein. Details of our analysis and recommendations are discussed in the following sections and in the Appendix of this report.

## PROJECT DESCRIPTION

We understand the proposed project includes construction of a new fieldhouse with associated site improvements at Dexter High School (DHS) and new stadium lighting at the Al Ritt Stadium near Creekside Intermediate School. The site improvements at DHS will include the construction of bituminous concrete pavements, site concrete, utilities, and structural best management practices (BMPs) consisting of a detention pond and two (2) underground stormwater management structures.

We understand the fieldhouse will be constructed to the south of the existing twin turf fields located within the southwest quadrant of the DHS campus. The fieldhouse will be a single-story, slab-on-grade structure with a height of 61 feet, an approximate footprint of 63,072 square feet, and a finished floor elevation (FFE) of 887-1/2 feet. The northern portion of the structure will include an interior mezzanine. Information regarding structural loading conditions associated with the fieldhouse was unavailable at the time of this report. It is assumed that the structure will have column loads ranging from 75 to 150 kips and wall loads ranging from 3 to 5 kips per foot.

Based on the topographic survey provided, existing site grades within the vicinity of the fieldhouse range from elevation 882 to 894 feet. As the fieldhouse will have a FFE of 887-1/2 feet, we anticipate existing grades will be lowered by as much as 6-1/2 feet and raised by as much as 5-1/2 feet to achieve the design subgrade elevation.

We understand that a bituminous concrete service drive and parking areas will be constructed around the perimeter of the fieldhouse. Information regarding anticipated traffic frequencies throughout the pavement areas was unavailable at the time of this report; however, we understand a heavy-duty pavement is proposed. Based on the provided Dexter High School Field House Schematic Design Grading Plan dated October 2025, Sheet C402, pavement grades west of the fieldhouse will be approximately 6 to 8 feet lower than existing grades, pavement grades east of the fieldhouse will be approximately 12 feet higher or 7 feet lower than existing grades, and pavement grades along the service drive leading to the parking areas will be similar to existing grade at the east and west ends of the drive and will be as much as 7 feet lower than existing grades along the remainder of the alignment.

Based on the drawings provided, we understand that four (4) new stadium light poles will be constructed along the perimeter of the Al Ritt Stadium football field. The light poles will have heights of 80 to 90 feet above grade. The loading conditions for the stadium lighting poles were not available to us at the time of this report; however, we assume the poles will be supported using drilled shaft foundations. Based on the "Electrical New Work Site Plan" drawing by Peter Basso Associates, dated October 31, 2025, we understand electrical conduit will be installed via directional drilling methods beneath the athletic field at a minimum depth of 10 feet below current grades.

G2 Consulting Group, LLC (G2) was provided a Geotechnical Report prepared by Driesenga & Associates, Inc. (D&A) Project No. 2520656.3A, dated August 12, 2025. The D&A report was completed during the preliminary design stage for the project. D&A soil borings SB-1, SB-2, and SB-3 lie within the vicinity of the proposed fieldhouse. These boring logs are included in the Appendix of this report are have been used in development of the recommendations contained herein.

If information related to existing/final site grades, structural loading conditions, finished floor elevation, and/or anticipated traffic frequencies change or become available, G2 should be notified to re-evaluate the recommendations provided herein, if necessary. The purpose of our exploration is to determine and evaluate the general subsurface conditions at the above-mentioned locations and to develop recommendations for earthwork, subgrade preparation, foundation design, bituminous pavement design, and in-situ infiltration, as well as construction considerations as they relate to the geotechnical conditions on site.

## SCOPE OF SERVICES

The field operations, laboratory testing, and engineering report preparation were performed under direction and supervision of a licensed professional engineer. Our services were performed according to generally accepted standards and procedures in the practice of geotechnical engineering in this area. Our scope of services for this project is as follows:

1. We performed a total of nineteen (19) soil borings throughout the above mentioned areas. Soil borings B-21 through B-26 per performed within the influence of the proposed fieldhouse extending to a depth of 25 feet each. Soil borings B-27 through B-35 were performed within the vicinity of the proposed pavement areas, detention pond, and underground stormwater management system, extending to depths ranging from 10 to 20 feet. Soil borings B-36 through B-39 were performed within the vicinity of the stadium light poles extending to depths ranging from 30 to 35 feet.

2. We performed laboratory testing on representative samples obtained from the soil borings. Laboratory testing included visual engineering classification, natural moisture content, organic matter content, Atterberg limits, and unconfined compressive strength determinations.

3. We prepared this engineering report. The report includes recommendations regarding earthwork, subgrade preparation, foundation design, bituminous pavement design, in-situ infiltration, and related construction considerations.

## FIELD OPERATIONS

G2, in conjunction with Granger Construction, selected the number, depths, and locations of the soil borings. The boring locations were determined in the field by a G2 representative using GPS assisted mobile technology and by measuring from known surface features using conventional taping methods. The approximate soil boring locations are shown on the Soil Boring Location Plans, Plate Nos. 1 and 2. A topographic survey detailing existing surface grades at the soil boring locations associated with the DHS fieldhouse and parking lot, titled "Topographic Survey" by Arbor Land Consultants and dated October 30, 2025, was used to interpolate ground surface elevations at the boring locations. If more precise ground surface elevations are required, we recommend the as-drilled ground surface elevations at the boring locations be determined using traditional survey techniques.

Prior to soil borings operations, we subcontracted a tree clearing company to provide access pathways to the soil boring locations within the wooded areas located near the southwest corner of the DHS campus. The soil borings were performed using an all-terrain vehicle (ATV) mounted drilling rig. Continuous 2-1/4-inch, inside-diameter, hollow-stem augers were used to extend the boreholes to the explored depths. Soil samples were collected at 2-1/2-foot intervals within the upper 10-feet and at 5-foot intervals thereafter. The samples were obtained using the Standard Penetration Test (SPT) method (ASTM D1586), which involves driving a 2-inch diameter split-spoon sampler into the soil with a 140-pound weight falling 30-inches. The sampler is generally driven in three (3) successive 6-inch increments with the number of blows for each increment recorded. The number of blows required to advance the sampler the last 12-inches is termed the Standard Penetration Resistance (N). The blow counts for each 6-inch increment and the resulting N-value are presented on the soil boring logs.

The soil samples were placed in sealed containers and brought to our laboratory for testing and classification. During field operations, the driller maintained logs of the subsurface conditions, including changes in stratigraphy and observed groundwater levels. The final boring logs are based on the field boring logs supplemented by laboratory soil classification and test results. The boreholes were backfilled with auger upon completion of drilling operations.

# LABORATORY TESTING

Representative soil samples were subjected to laboratory testing to determine soil parameters pertinent to subgrade preparation and foundation design. An experienced geotechnical engineer classified the samples in general conformance with the Unified Soil Classification System. Laboratory testing was conducted in conformance with the following ASTM Test Methods:

- "Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass" (ASTM D2216);
- "Standard Test Methods for Determining the Water (Moisture) Content, Ash Content, and Organic Material of Peat and Other Organic Soils" (ASTM D2974); and
- "Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils" (ASTM D4318).

The unconfined compressive strengths were determined using a spring-loaded hand penetrometer. The hand penetrometer estimates the unconfined compressive strength to a maximum of 4-1/2 tons per square foot (tsf) by measuring the resistance of the soil sample to the penetration of a spring-loaded cylinder.

The results of the moisture contents, organic matter contents, and unconfined compressive strengths are indicated on the Soil Boring Logs, Figure Nos. 21 through 39, at the depths the samples were obtained. The results of the Atterberg Limits tests are presented graphically in the appendix as Figure No. 40.

We will hold the soil samples for 60 days from the date of this report, at which time they will be discarded. If you would like the samples, please let us know prior to this date.

# SITE DESCRIPTION

## Dexter High School

The Dexter High School campus is located at 2200 N. Parker Road in Dexter, Michigan. The existing school can be accessed via two access drives leading eastward into the parcel and via an access drive extending southward from Shield Road. The existing site grades are highest near the intersection of Shield and N. Parker Road and gently slope downward as you approach the existing school. The site grades immediately surrounding the existing building are at higher elevations at the northwest end of the building near the entrance of the school and lower toward to the southeast end of the building.

The existing school building is a single to four-story structure with one-story partially below grade at the north end. The campus also includes several soccer fields, two (2) baseball diamonds, ten (10) tennis courts, and a detention pond, as well as associated bituminous concrete service drives and parking areas, and Portland cement concrete (PCC) sidewalks. The remainder of the campus is covered in manicured lawn, includes several mature trees, and isolated areas of dense vegetation.

Existing surface grades throughout the campus range from Elevation 856 to 894 feet. Site grades near the proposed fieldhouse and adjacent parking lots range from Elevation 882 to 894 feet, generally sloping down from north to south or northwest to southeast. Site grades along the proposed service drive generally range from Elevation 893 to 865 feet, generally sloping down in an easterly direction.

Based on historical aerial imagery, the parcel appears to have been used for agricultural purposes prior to construction of the Dexter High School Campus leading up to 1998. Sometime between 1998 and 2005, the Dexter

High School building and northern athletic facilities were constructed. We observe the southwestern wooded area was cleared and twin turf fields were constructed sometime around 2020. We observed crack sealing has been performed throughout the bituminous concrete areas since the original construction of the campus.



*Historical Aerial Imagery (1998)*



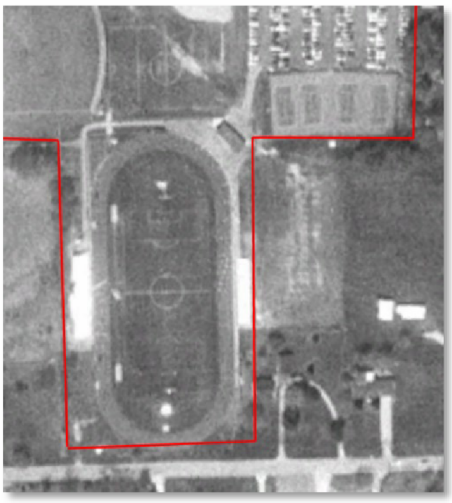
*Historical Aerial Imagery (2005)*



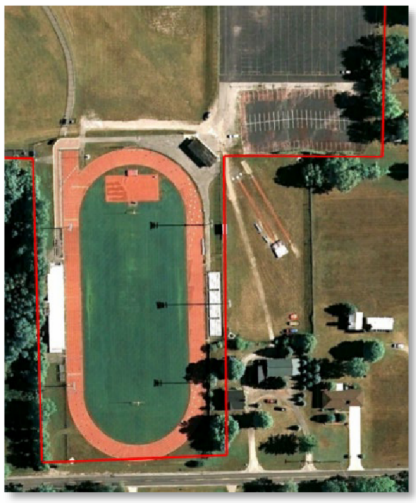
*Aerial Imagery (2025)*

## Al Ritt Stadium

The Al Ritt Stadium is located at 2615 Baker Road in Dexter, Michigan. The stadium is located south of Creekside Intermediate School. The site appears to be relatively flat, sloping down slightly from east to west. The existing football field, running track, bleacher structures, and maintenance building appear to have been constructed sometime prior to 1993. Then, sometime around 2007, the athletic fields north of the football field were demolished and replaced with bituminous concrete parking areas. It appears that the restroom/concessions building located on the north side of the stadium was constructed sometime between 2007 and 2013.



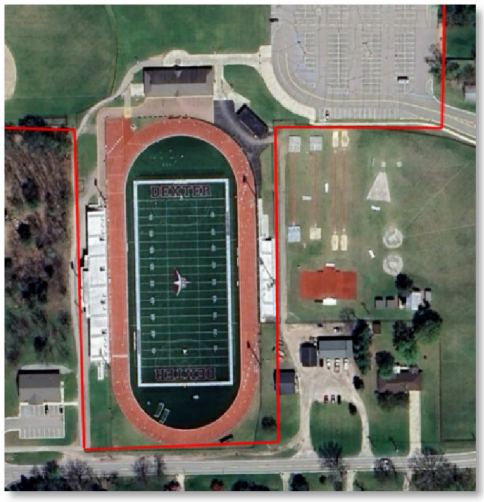
*Historical Aerial Imagery (1993)*



*Historical Aerial Imagery (2007)*




*Historical Aerial Imagery (2013)*



*Aerial Imagery (2025)*

# SOIL CONDITIONS

## DHS Fieldhouse Addition (B-21 through B-26 and D&A SB-1, SB-2, and SB-3)

Approximately 3 to 10 inches of topsoil is present at the ground surface of the soil borings. Within soil borings B-21 and B-22, clayey sand and silty clay fill soils underlie the topsoil and extend to an approximate depth of 3-1/2 feet. Alternating layers of native granular and cohesive soils consisting of sand, gravelly sand, silty sand, sandy silt, silty clay, sandy clay, and clayey silt, underlie the topsoil and fill soils and extend to the explored depth of 25 feet below existing grade.

The clayey sand fill is very loose in relative density with an SPT-N value of 2 blows per foot (bpf). The silty clay fill is very stiff in consistency with a moisture content of 19 percent and an unconfined compressive strength of 5,000 pounds per square foot (psf). The native cohesive soils within boring B-21 between 3-1/2 and 8-1/2 feet, within boring B-22 between 13-1/2 and 19-1/2 feet, within boring B-24 between 6 and 13-1/2 feet, and within B-25 between 18-1/2 and 24 feet are stiff in consistency with natural moisture contents between 10 and 21 percent and

unconfined compressive strength ranging from 2,000 to 3,000 psf. The remainder of the native cohesive soils are generally very stiff to hard in consistency with natural moisture contents ranging from 9 to 19 percent and unconfined compressive strength ranging from 4,000 to 9,000 psf. The native sandy clay present below elevations ranging from 867 to 858 feet within soil borings B-24 through B-26 is hard to very hard in consistency with natural moisture contents ranging from 8 to 10 percent, unconfined compressive strengths in excess of 9,000 psf, and SPT N-values ranging from 50 blows for 5 inches of penetration to 50 blows for 1 inch of penetration. The native granular soils are generally loose to medium compact in relative density with SPT N-values ranging from 6 to 27 bpf; however, isolated layers within soil boring B-23 between 23-1/2 and 25 feet and within boring B-25 between 14 and 18-1/2 feet are very loose in relative density with SPT N-values ranging from 3 to 4 bpf.

## DHS Fieldhouse Parking, Drive, and Structural BMPs (B-27 through B-35)

Approximately 2 to 7 inches of topsoil is present at the ground surface of borings B-27 through B-35. Within borings B-28, B-31, and B-32, granular and cohesive fill soils consisting of silty clay, sandy clay, and clayey sand underlie the topsoil and extend to approximate depths ranging from 3 and 6 feet. Notably, the silty clay fill within B-28 has an elevated organic matter content. Native granular soils, consisting of silty sand, clayey sand, and sandy silt, are present beneath the fill soils or topsoil within soil borings B-27, B-30, B-31, and B-35 and extend to depths ranging from 3 to 6 feet. Native cohesive soils, consisting of silty clay and sandy clay, generally underlie the upper granular soils and fill soils or topsoil within the remaining borings and extend to depths ranging from 3 feet to the explored depth of 20 feet. Granular soils consisting of clayey sand, sand, gravelly sand, and sandy gravel are present beneath the cohesive soils within soil borings B-27, B-29 through B-31, and B-33 and extend to the explored depth of 10 and 20 feet.

The clayey sand fill is medium compact in relative density with an SPT N-value of 15 bpf. The cohesive fill soils are stiff to very stiff in consistency with moisture contents ranging from 15 to 28 percent and unconfined compressive strengths ranging from 2,000 to 6,000 psf. Within B-28, the silty clay fill has organic matter contents ranging from 4 to 7 percent. The native cohesive soils within B-27 through B-35 are generally stiff to very stiff in consistency with natural moisture contents ranging from 10 to 19 percent and unconfined compressive strengths ranging from 2,500 to 7,000 psf; however, within B-35, the sandy clay between 3-1/2 and 6 feet is medium in consistency with a natural moisture content of 12 percent and an unconfined compressive strength of 1,500 psf. Within B-32, the silty clay has a liquid limit of 21 percent and a plasticity index of 6 percent. The native granular soils are generally loose to medium compact in relative density to approximate depths of 3 to 8-1/2 feet with SPT N-values ranging from 5 to 24 bpf. The native granular soil present below these depths are generally medium compact to compact with N-values ranging from 15 to 41 bpf.

## Al Ritt Stadium Lighting (B-36 through B-39)

Approximately 1 to 2 feet of crushed limestone aggregate is present at the ground surface of borings B-36 and B-37. Approximately 7 inches of topsoil is present at the ground surface of boring B-38. Within boring B-37 and B-38, granular fill soils consisting of silty sand and sand underlie the aggregate or topsoil and extend to approximate depths ranging from 4-3/4 to 6 feet. In general, native granular soils consisting of silty sand and sand are present beneath the fill soils or from the ground surface and extend to depths ranging from 8-1/2 to 13-1/2 feet. Silty clay and sandy clay underlie the fill soils within soil boring B-38 the native granular soils and generally extend to the explored depths ranging from 30 to 35 feet. A layer of gravelly sand is present below an approximate depth of 34-1/2 feet within soil boring B-39.

The silty sand fill is loose to medium compact in relative density with and SPT N-value ranging from 5 to 17 bpf. The native granular soils are very loose to loose in relative density within soil boring B-37 with SPT N-values ranging from 3 to 10 bpf. Elsewhere, the native granular soils are generally loose to medium compact in relative density

with N-values ranging from 6 to 23 bpf. However, the deeper native gravelly sand within boring B-39 is very compact in relative density with an SPT N-value in excess of 50 bpf. The native sandy clay present between the approximate depths of 8-1/2 and 13 feet within soil boring B-39 is very stiff in consistency with a natural moisture content of 13 percent and an unconfined compressive strength of 4,000 psf. Elsewhere, the native cohesive soils are hard to very hard in consistency with natural moisture contents ranging from 7 to 11 percent, unconfined compressive strengths of 9,000 psf or greater, and N-values ranging from 29 bpf to 50 blows for 2 inches of penetration.

### General

Although no environmental sampling protocol was used for the drilling operations, a hydrocarbon odor was noted in the native sandy clay within soil borings B-25 and B-33 between the approximate depths of 3 and 6 feet, which indicates the presence of possible subsurface contamination.

The stratification depths shown on the soil boring logs represent the soil conditions at the boring locations. Variations may occur between and away from the boring locations. Additionally, the stratigraphic lines represent the approximate boundaries between soil types. The transition may be more gradual than indicated. We have prepared the boring logs on the basis of the field log of the soil conditions encountered supplemented by laboratory classification and testing.

The Soil Boring Location Plans, Plate Nos, 1 and 2, and Soil Boring Logs, Figure Nos. 21 through 39, are presented in the Appendix. The soil profiles described above are generalized descriptions of the conditions encountered at the boring locations. General Notes Terminology defining the nomenclature used on the soil boring logs and elsewhere in this report are presented on Figure No. 41.

## GROUNDWATER CONDITIONS

### DHS Fieldhouse Addition (B-21 through B-26 and D&A SB-1, SB-2, and SB-3)

During drilling operations, groundwater was encountered at approximate depths ranging from 7 to 20 feet below existing grade within soil borings B-21 through B-26, corresponding to elevations ranging from 869 to 878-1/2 feet. The groundwater level was measured at depths ranging from 7 to 19-1/2 feet upon completion of drilling, which corresponds to elevations ranging from 872 to 878-1/2 feet and corresponds with the collapse depth of the borehole. With SB-1 and SB-2, no measurable groundwater was observed at completion of drilling above the collapse depth, which was higher than the encountered groundwater level within these borings.

### DHS Fieldhouse Parking, Drive, and Structural BMPs (B-27 through B-35)

During drilling operations, groundwater was encountered at approximate depths ranging from 10 to 19 feet below existing grade within soil borings B-27 through B-29 and B-31, corresponding to elevations ranging from 871 to 877-1/2 feet. The groundwater level was measured at depths ranging from 12 to 17-1/2 feet upon completion of drilling, which corresponds with elevations ranging from 866-1/2 to 880 feet and generally correspond with the collapse depths within these borings. No measurable groundwater was observed during or upon completion of drilling operations within borings B-30 or B-32 through B-35.

### Al Ritt Stadium Lighting (B-36 through B-39)

During drilling operations, groundwater was encountered at approximate depths ranging from 5 to 7 feet below existing grade within soil borings B-36 through B-39. The groundwater level was measured at depths ranging from

6 to 29 feet upon completion of drilling within B-36 through B-38, which corresponds with the collapse depth of the boreholes. No measurable groundwater was observed upon completion of drilling within soil boring B-39.

### General

The rise in the groundwater level above the encountered elevation within soil borings B-22, B-23, and B-31 may be indicative of an artesian condition.

Fluctuations in perched and long-term groundwater levels should be anticipated due to seasonal variations and following periods of prolonged precipitation. It should also be noted that groundwater observations made during drilling operations in predominantly cohesive soils are not necessarily indicative of the static groundwater level. This is due to the low permeability of such soils and the tendency of drilling operations to seal off the natural paths of groundwater flow.

## SITE PREPARATION RECOMMENDATIONS

### General

Existing fill soils are present within borings B-21, B-22, B-28, B-31, and B-33. The fill soils do not appear to have been placed in an engineered manner, include intermixed debris, and have organic matter contents ranging from 4.3 to 7.0 percent. Considering the existing ground surface elevations and the fieldhouse FFE, we anticipate the fill soils within the influence of the fieldhouse will be removed while adjusting site grades to achieve the design subgrade elevation, as illustrated on the Soil Boring Profile, Figure No. 42, in the Appendix. In addition, based on proposed pavement finished grades, we anticipate the existing fill soils will be removed during grading operations in the vicinity of soil borings B-28 and B-31. Some existing fill soil may remain beneath the proposed pavement section within the vicinity of soil boring B-33. The fill within soil boring B-33 does not contain any notable organic matter or debris and is generally considered suitable for support of the pavement, following the successful completion of a proof-roll/proof-compaction evaluation.

We anticipate earthwork operations will consist of the complete removal of trees, vegetation, and topsoil within the influence of the development areas, removal of any existing utilities within the influence of the fieldhouse, raising/lowering site grades to achieve design subgrade elevation, proof-rolling/proof-compacting the exposed subgrade, excavating for utilities, foundations, and structural BMPs, and preparing subgrade for floor slab and pavement support. We recommend all earthwork operations be performed in accordance with comprehensive specifications and be properly monitored in the field by qualified personnel under the direction of a licensed professional engineer.

At the start of earthwork operations, any existing trees, vegetation, and topsoil should be entirely removed within the proposed development areas. Any existing utilities and associated backfill within the structure footprint(s) should be completely removed and replaced with engineered fill. Where utilities lie outside the structures zone of influence, utilities to be abandoned can be filled with cementitious grout and left-in-place.

After site stripping, and cutting the site to rough grade, the exposed subgrade is expected to generally consist of native granular and cohesive soils. Prior to placement of engineered fill to raise site grades, where cohesive soils are present, soils should be proof-rolled with a fully loaded tandem-axle dump truck and be evaluated for stability. Unsuitable soils exhibiting excessive instability, such as severe rutting, should be improved with additional compaction or undercut to expose stable soils. Where granular soils are present, soils should be proof-compacted with a heavy smooth-drum vibratory roller and should be visually evaluated for instability and/or unsuitable soil conditions by a qualified geotechnical engineer or technician. We recommend 10 passes in two (2) perpendicular

directions during the proof-compaction operations. We recommend that the vibratory setting be turned off when within 25 feet of any existing structures. Any resulting undercut excavations should be evaluated for stability, improved as necessary, and should be backfilled with engineered fill.

Based on conducted laboratory testing, the moisture content of the surficial cohesive soils are approaching their respective plastic limit. Therefore, these subgrade soils may become unstable under repeated loading from construction equipment. The subgrade should not be exposed to prolonged periods of precipitation to prevent the subgrade from becoming unstable. We recommend earthwork operations be performed during the predominantly drier summer months.

### Engineered Fill

In general, we anticipate that the spoils generated from grade balancing within the influence of the fieldhouse and structural BMP excavations can be re-used as engineered fill elsewhere on site. However, soils including intermixed construction debris and/or organic matter should not be used as engineered fill throughout the proposed development areas, such as those observed within soil borings B-21 through B-23 and B-28.

Engineered fill should be free of organic matter, frozen soil, clods, or harmful material. The fill should be placed in uniform horizontal layers not more than 9-inches in loose thickness. The engineered fill should be compacted to achieve a dry density corresponding with 95 percent of the maximum dry density as determined by the Modified Proctor Compaction test (ASTM D1557).

To economically conduct earthwork operations at the site, imported fill, adhering to the aforementioned requirements, should consist of low plasticity clays or well-graded aggregates. Low plasticity clays, having a plasticity index less than 20 percent, should be placed within +3 or -1 percent of the optimum moisture content as determined by the Modified Proctor Test. For well-graded aggregates, such as MDOT Class II sand, we recommend the engineered fill be placed at ±2 percent of the optimum moisture content as determined by ASTM D1557.

We recommend the use of free-draining granular soils, such as MDOT Class II sand, within utility trenches and to backfill around below-grade walls. We recommend the use of engineered fill with a sufficient amount of fines (material passing the No. 200 sieve) in order to facilitate trenching and excavation techniques for strip and spread footing foundations.

## FOUNDATION RECOMMENDATIONS

Information regarding structural loading conditions associated with the fieldhouse and stadium light poles were unavailable at the time of this report. It is assumed that the fieldhouse will have column loads ranging from 75 to 150 kips and wall loads ranging form 3 to 6 kips per lineal foot. Once information regarding structural loading conditions becomes available, G2 should be notified to re-evaluate the recommendations provided herein, if necessary.

### Dexter High School Fieldhouse

Very loose clayey sand and very stiff silty clay fill is present within borings B-21 and B-22 near the northeast corner of the fieldhouse location. Considering the ground surface elevation at boring B-21 and B-22 range from 893 to 894 feet and the fieldhouse FFE is 887-1/2 feet, we anticipate the fill soils will be removed during grading operations to achieve the design subgrade elevation.

We recommend the fieldhouse be supported by shallow conventional strip and/or spread footings designed to bear at a minimum depth of 3-1/2 feet below finished grades within the native loose to medium compact granular soils, very stiff to hard cohesive soils, or engineered fill. Foundations bearing within the above mentioned soils can be designed based on a net allowable bearing capacity of 2,500 psf.

## Al Ritt Stadium Lighting

Loading conditions for the proposed light pole foundations was not available at the time of this report. However, based on our experience, we anticipate the light poles will be supported on drilled shaft foundations. We anticipate the drilled shafts will extend into the hard clay layer present at depths ranging from 6 to 13-1/2 feet in order to provide the necessary lateral support. The drilled shaft design is controlled by the overturning moment, thus bearing pressure imposed on the drilled shaft bottom is less than the recommended allowable bearing pressure of 9,000 psf. We recommend drilled shafts be designed using the following soil parameters:

| Boring No. | Depth (feet) | Soil | Cohesion (psf) | Angle of Friction (degrees) | Total Unit Weight (pcf) | Soil Modulus (pci) |
|---|---|---|---|---|---|---|
| B-36 | 0 to 3-1/2 | - | - | - | - | - |
| | 3-1/2 to 5 | Medium Compact Sand | 0 | 32 | 120 | 90 |
| | 5 to 8-1/2 | Medium Compact Sand | 0 | 32 | 120 | 60 |
| | 8-1/2 to 14-1/2 | Hard Clay | 4,500 | 0 | 135 | 700 |
| | 14-1/2 to 30 | Hard to Very Hard Clay | 8,000 | 0 | 140 | 800 |
| B-37 | 0 to 3-1/2 | - | - | - | - | - |
| | 3-1/2 to 6 | Loose Sand | 0 | 30 | 115 | 25 |
| | 6 to 13-1/2 | Loose Sand | 0 | 30 | 115 | 20 |
| | 13-1/2 to 17 | Hard Clay | 5,000 | 0 | 135 | 750 |
| | 17 to 30 | Hard to Very Hard Clay | 8,000 | 0 | 140 | 800 |
| B-38 | 0 to 3-1/2 | - | - | - | - | - |
| | 3-1/2 to 5 | Medium Compact Sand | 0 | 32 | 120 | 90 |
| | 5 to 6 | Medium Compact Sand | 0 | 32 | 120 | 60 |
| | 6 to 9-1/2 | Hard Clay | 4,500 | 0 | 135 | 700 |
| | 9-1/2 to 30 | Hard Clay | 5,000 | 0 | 135 | 750 |
| B-39 | 0 to 3-1/2 | - | - | - | - | - |
| | 3-1/2 to 6-1/2 | Loose Sand | 0 | 30 | 115 | 25 |
| | 6-1/2 to 8-1/2 | Loose Sand | 0 | 30 | 115 | 20 |
| | 8-1/2 to 13 | Very Stiff Clay | 2,000 | 0 | 130 | 300 |
| | 13 to 22 | Hard Clay | 5,000 | 0 | 135 | 750 |
| | 22 to 27 | Hard to Very Hard Clay | 8,000 | 0 | 140 | 800 |
| | 27 to 34-1/2 | Hard Clay | 5,000 | 0 | 135 | 750 |

## General

Continuous wall or strip footings should be at least 18 inches in width and isolated spread footings should be at least 36 inches in their least dimension. Exterior foundations must bear at a minimum depth of 3-1/2 feet below finished grade for protection against frost heave. Interior footings may bear at shallower depths provided they extend through fill soils, suitable bearing materials are available for support, and foundations are continuously protected from frost penetration during construction.

To achieve a change in the level of a strip footing, the footing should be gradually stepped at a grade no steeper than two horizontal units to one vertical unit (2H:1V). If required to construct foundations at different levels, adjacent spread foundation should be designed and constructed so the least lateral distance between the foundations is equivalent to or more than the difference in their bearing levels.

If the recommendations outlined in this report are adhered to, total and differential settlements for the completed structures should be within 1 inch and ½ inch, respectively. We expect settlements of these magnitudes are within tolerable limits for the type of structures proposed. We recommend all foundations be suitably reinforced to minimize the effects of differential settlements associated with local variations in subsoil conditions.

# FLOOR SLAB RECOMMENDATIONS

As previously stated, we anticipate the fill soils encountered within borings B-21 and B-22 will be removed in order to achieve the design subgrade elevation within the influence of the fieldhouse. The fieldhouse will have a FFE of 887-1/2 feet and we anticipate the floor slab will be supported by native granular/cohesive soils and/or engineered fill overlying native soils.

Following satisfactory completion of the subgrade preparation recommendations as described within the *SITE PREPARATION RECOMMENDATIONS* section of this report, a subgrade modulus (k) of 125 pounds per cubic inch (pci) may be used for design of the fieldhouse floor slab supported by the aforementioned soils.

We recommend at least 4-inches of clean free-draining material, such as 2NS sand or pea gravel, be placed between the subgrade and the bottom of the floor slab for use as a capillary break to reduce moisture transmission through the concrete floors and to reduce the potential for concrete curling. If moisture sensitive floor coverings are planned or if greater protection against vapor transmission is desired, a vapor barrier consisting of 10-mil plastic sheeting, or equivalent, may be placed on the sand layer beneath floor slabs. However, additional floor slab curing techniques will be required especially if floor slab placement occurs in the winter months to prevent floor slab curling. The floor slab should be isolated from the foundation system to allow for independent movement.

# PAVEMENT RECOMMENDATIONS

## General

We understand that a bituminous concrete service drive and parking areas will be constructed near the fieldhouse. Information regarding anticipated traffic frequencies was unavailable at the time of this report; however, we understand a heavy-duty pavement section will be constructed. It is assumed that traffic will primarily consist of PCVs and school buses with occasional waste management, emergency response, and delivery vehicles. Once information regarding anticipated traffic frequencies becomes available, G2 should be notified to re-evaluate the recommendations provided herein.

We anticipate some subgrade instability will be encountered throughout the pavement areas depending on the time of year of construction. We recommend limiting undercuts to a depth of 12 inches below proposed aggregate base, compacting the exposed subgrade, and backfilling with MDOT 21AA dense-graded aggregate. If a stable subgrade cannot be achieved at a maximum depth of 12 inches below the proposed subgrade, we recommend placing a geogrid reinforcement, such as TENSAR Tri-Ax geogrid (Type III) or equivalent, at a depth of 12 inches below proposed subgrade and backfilling with MDOT 21AA aggregate. The geogrid should extend to a minimum of 5 feet beyond the edges of the unstable area. Any undercut areas within predominantly clayey soils should be connected with finger drains to the closest catch basins to drain water from within the granular undercut backfill material.

## Pavement Design

Provided the site recommendations in the *SITE PREPARATION RECOMMENDATIONS* section of this report are adhered to, we recommend a subgrade resilient modulus of 6,000 pounds per square inch (psi) be used for the anticipated subgrade soils. Based on the anticipated traffic frequencies, we have designed a heavy-duty bituminous concrete pavement section based on an estimated 300,000 equivalent single axle loads (ESALs) over a 20-year design life. For evaluation purposes, we estimated a serviceability loss of 2.2, a standard deviation of 0.45, and a reliability of 90 percent. Based on the results of our analysis, we recommend the following minimum pavement design cross-section:

**Typical Heavy-Duty Flexible Pavement Section**

| Material | Thickness | Structural Coefficient |
|---|---|---|
| MDOT 5EML Bituminous Wearing Course | 2 inches | 0.42 |
| MDOT 4EML Bituminous Leveling Course | 3 inches | 0.42 |
| MDOT 21AA Aggregate Base (dense-graded) | 10 inches | 0.14 |
| | | Total SN = 3.5 |

Large front-loading refuse trucks can impose significant concentrated wheel loads within trash dumpster pick-up areas and loading and delivery areas. This type of loading can result in rutting of bituminous concrete pavements and ultimately in failure. Therefore, we recommend reinforced Portland cement concrete pavement be used in this area and be large enough to accommodate the entire truck during pick-up operations.

All pavement materials are specified within the 2020 Standard Specifications for Construction from the Michigan Department of Transportation. The bituminous pavement materials are described in Section 501 and 904 and can be assigned a structural coefficient number of 0.42. The Portland cement concrete pavement materials are described in Section 601. Any imported MDOT 21AA dense-graded aggregate base course can be assigned a structural coefficient number of 0.14. The aggregate materials for the subbase are described in Section 902. We recommend that bituminous concrete utilize grade PG 68-22 binder, with no more than 17 percent of the overall binder content from reclaimed asphalt pavement (RAP) within the top wearing course layer.

## Pavement Drainage and Maintenance

Proper drainage is an important consideration for pavement design, especially in consideration of the cohesive soils. The pavement and subgrade should be properly sloped to promote effective surface and subsurface drainage and prevent water ponding. We recommend edge drains be installed around the perimeter of the pavement areas.

Regular timely maintenance should be performed on the pavement to reduce the potential deterioration associated with moisture infiltration through surface cracks. The owner should be prepared to seal the cracks with a hot-applied elastic crack filler as soon as possible after cracking develops and as often as necessary to block the passage of water to the subgrade soils.

# STRUCTURAL BMP RECOMMENDATIONS

We understand that structural BMPs consisting of a detention pond and two (2) underground stormwater management structures will be constructed adjacent to the fieldhouse. The detention pond will be constructed on the east side of the fieldhouse and the two (2) underground stormwater management structures will be constructed below the parking areas on the east and west sides of the fieldhouse. The invert elevations of the BMPs were unavailable at the time of this report. Once this information becomes available, G2 should be notified to re-evaluate the recommendations provided herein. We understand the intent of the BMPs are to capture and detain stormwater runoff until redirected to the natural water cycle by means of subsurface infiltration.

Soil borings B-27 and B-28 were drilled within the vicinity of the proposed west underground stormwater structure, soil boring B-29 was drilled within the proposed detention pond, and soil borings B-30 and B-31 were drilled in the vicinity of the east underground stormwater structure. Following grade cuts, the soils within borings B-27 and B-28 are cohesive in nature, which can be considered relatively impermeable and do not promote subsurface infiltration. Site grades will be raised by several feet in the area of the detention pond. Predominantly cohesive soils are also present within soil boring B-29 to an approximate depth of 8-1/2 feet, or approximately 11-1/2 feet below proposed grade. We do not anticipate the proposed detention pond will extend below 11-1/2 feet; therefore, the detention pond should be expected to retain water and an overflow structure will be required. Sand and gravelly sand are present within soil borings B-30 and B-31 at depths of 2-1/2 to 6 feet below proposed finished grade and extend to depths ranging from 7-1/2 to 20 feet below finished grade. We anticipate these soils will allow for subsurface infiltration beneath the eastern parking lot.

Per Washtenaw County guidelines, each BMP must have an invert elevation three (3) feet above the seasonally high groundwater table. The granular soils within borings B-30 and B-31 could promote subsurface infiltration, provided the BMP invert is sufficiently above the seasonally high groundwater table. Once the BMP invert elevations become available, we recommend that supplemental test pit excavations and in-situ infiltration testing be performed at each BMP location.

# CONSTRUCTION CONSIDERATIONS

## Open Cut Excavations

We anticipate caving and/or sloughing of the surficial granular soils during shallow foundation excavation operations. As such, the foundation contractor should come prepared to over-excavate and form foundation excavations within the granular soils. Where cohesive soils are present, we anticipate foundations can be earth formed. If earth formed foundations are utilized, we recommend foundation excavation and concrete placement operations be conducted on the same day to minimize potential for cave-ins or stormwater run-off into the open excavations.

Based on the observations within soil borings B-21 through B-35, groundwater is present below Elevations 871 to 878-1/2 feet. However, based on the proposed FFE of 887-1/2 feet, we do not anticipate construction excavations for the proposed fieldhouse, utilities, or BMPs will extend below the groundwater table. We anticipate any groundwater seepage or surface runoff can be controlled with a properly constructed system of sumps and pumps.

We recommend maximum slope inclinations of 2H:1V within the very loose to loose granular soils, 1-1/2H:1V within the medium compact granular and medium cohesive soils, 1H:1V within the compact granular and stiff cohesive

soils, and 3/4H:1V within the very stiff to hard cohesive soils for excavations extending below a depth of 5 feet. Where seepage from excavation cuts is observed, the slopes will need to be flattened sufficiently to achieve stability, but in no case left steeper than 3H:1V at and below the seepage level.

All excavations should be safely sheeted, shored, sloped, or braced in accordance with MI-OSHA requirements. If material is stored or equipment is operations near an excavation, lower angle slopes or stronger shoring must be used to resist the extra pressure due to the superimposed loads.

## Drilled Piers

Caving and/or sloughing of the granular soils will occur during drilled shaft excavation operations. In addition, groundwater was encountered at depths ranging from 5 to 7 feet below existing grade. Therefore, the drilled pier contractor should come to the site prepared to extend temporary steel casing through the granular soils and into the underlying cohesive soils to maintain a stable excavation and seal off groundwater during construction operations. The contractor should also come to the site prepared to break-up and remove any cobbles encountered in the excavation and may need a rock auger to excavate through the very hard cohesive soils. Under no circumstances should concrete fall through standing water at the base of the excavation. Therefore, if groundwater cannot be controlled with temporary casing, concrete must be placed by tremie method.

Once drilling is completed to the design depth, reinforcing steel should be set and concrete placed until a positive head of concrete has been established within the casing. This positive concrete head should be maintained while pulling the casing to prevent sloughing of the granular soils into the fresh concrete. After concrete has been placed to an appropriate grade, the casing may be removed and concrete placement operations completed. The telescoped casing, if used, must be removed in the opposite order in which it is installed (i.e. – from the inside out).

To reduce lateral movement of the drilled pier foundations, the contractor must place the concrete for the pier in intimate contact with undisturbed soil. Fill any voids or enlargements in the drilled pier shaft excavation with concrete at the time of drilled pier concrete placement. We recommend using a concrete mix design with a slump of 5 to 7 inches for free fall placement and 7 to 9 inches for tremie placement to reduce the potential for concrete arching and provide a workable material.

We recommend using a short temporary top form, such as a Sonotube, to form the top portion of the drilled pier. The use of this top form is a beneficial aid to the correct placement and orientation of the anchor bolts.

## Directional Drilling Installation

We understand electrical conduits will be installed via direction drilling beneath the existing football field at Al Ritt Stadium. In general, we anticipate the soil conditions will be conducive to directional drilling installation methods; however, some degree of difficulty may be encountered as the directional drilling cutting head approaches the interface between the very stiff to hard cohesive and very loose to loose granular soils. We recommend an experienced contractor with the resources to provide appropriate designs as well as provide the proper equipment to penetrate the anticipated soils perform the installation operations.

Drilling fluid should be used during drilling and back-reaming operations. Drilling fluid should be composed of environmentally clean water and should be amended using appropriate additives for the specific soil types to be encountered. Potentially hazardous materials should not be used in drilling fluid. Based on the soil conditions at the site, we recommend the fluid pressure and pumping rate for pilot drilling and pre-ream be commensurate with the subsurface conditions and the drilling fluid viscosity and density be appropriate for the anticipated variations of the subsurface conditions.

We recommend the drilling contractor periodically evaluate the specific gravity, unit weight, and sand content of the drilling fluid to mitigate the potential for the buildup of excess pressure behind the cutting head.

## GENERAL COMMENTS

We have formulated the evaluations and recommendations presented in this report relative to site preparation and foundation construction on the basis of data provided to us relating to the project location and surface grade for the proposed site(s). Any significant change in this data should be brought to our attention for review and evaluation with respect to prevailing subsurface conditions. Furthermore, if changes occur in the design, location, or concept of the project, conclusions and recommendations contained in this report are not valid unless G2 reviews the changes. G2 will then confirm the recommendations presented herein or make changes in writing.

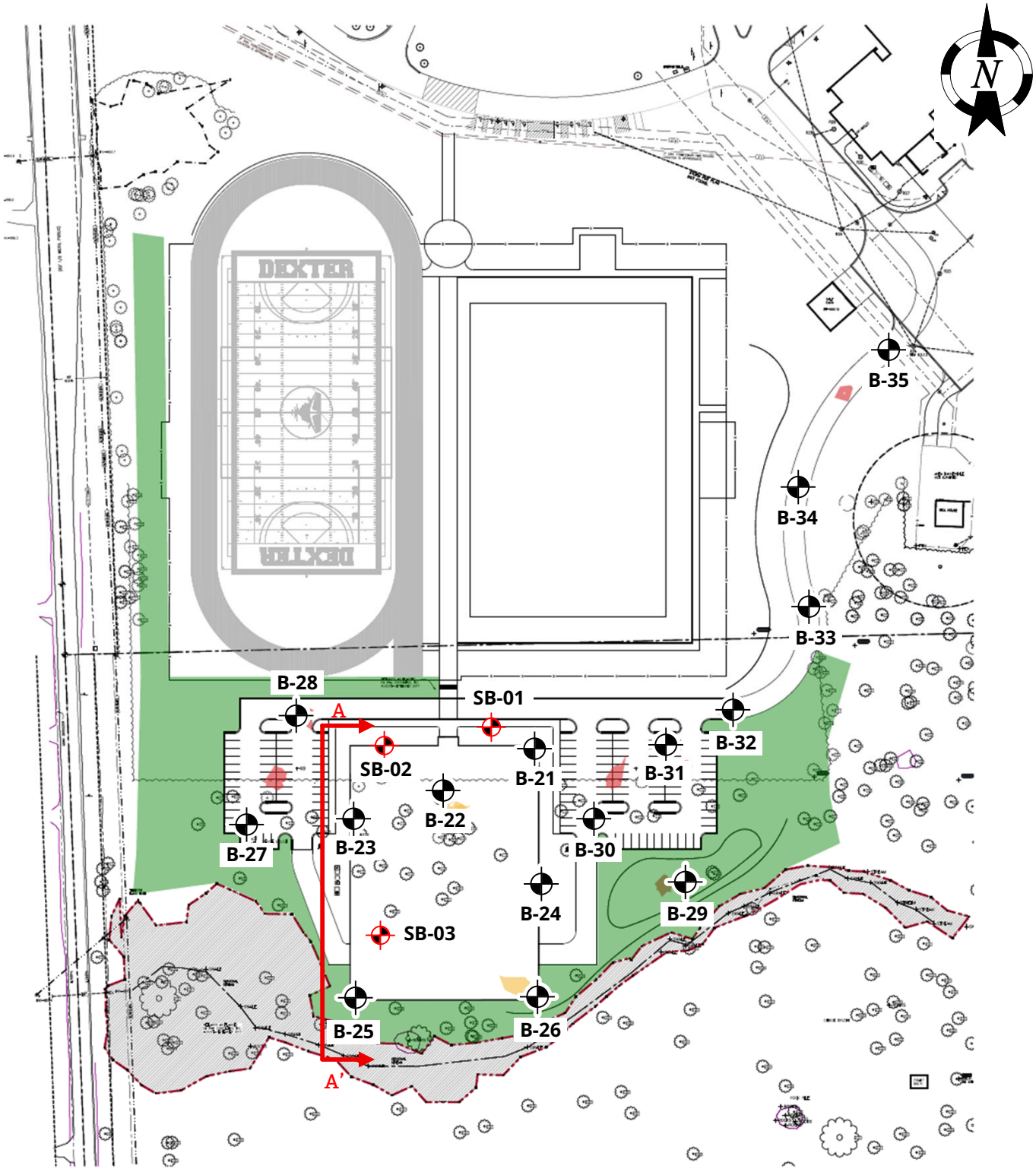
The scope of the present investigation was limited to evaluation of subsurface conditions for the support of proposed structures and other aspects of the proposed improvements. No chemical, environmental, or hydrogeological testing or analyses were included in the scope of this investigation.

We base the analyses and recommendations submitted in this report upon the data from the soil borings performed at the approximate locations shown on the Soil Boring Location Plans, Plate Nos. 1 and 2. This report does not reflect variations that may occur between the actual boring locations and the actual structure locations. The nature and extent of any such variations may not become clear until the time of construction. If significant variations then become evident, it may be necessary for us to re-evaluate our report recommendations.

We recommend G2 Consulting Group, LLC observe all geotechnical related work, including subgrade preparation, foundation construction, and engineered fill placement. G2 Consulting Group, LLC will perform the appropriate testing to confirm the geotechnical conditions given in the report are found during construction.

# APPENDIX


| | |
|---|---|
| SOIL BORING LOCATION PLANS | PLATE NOS. 1 AND 2 |
| SOIL BORING LOGS | FIGURE NOS. 21 THROUGH 39 |
| SOIL BORING PROFILE A-A' | FIGURE NO. 40 |
| ATTERBERG LIMITS TEST RESULTS | FIGURE NO. 41 |
| GENERAL NOTES TERMINOLOGY | FIGURE NO. 42 |
| DRIESENGA & ASSOCIATES, INC. SOIL BORING LOGS | |




## Legend

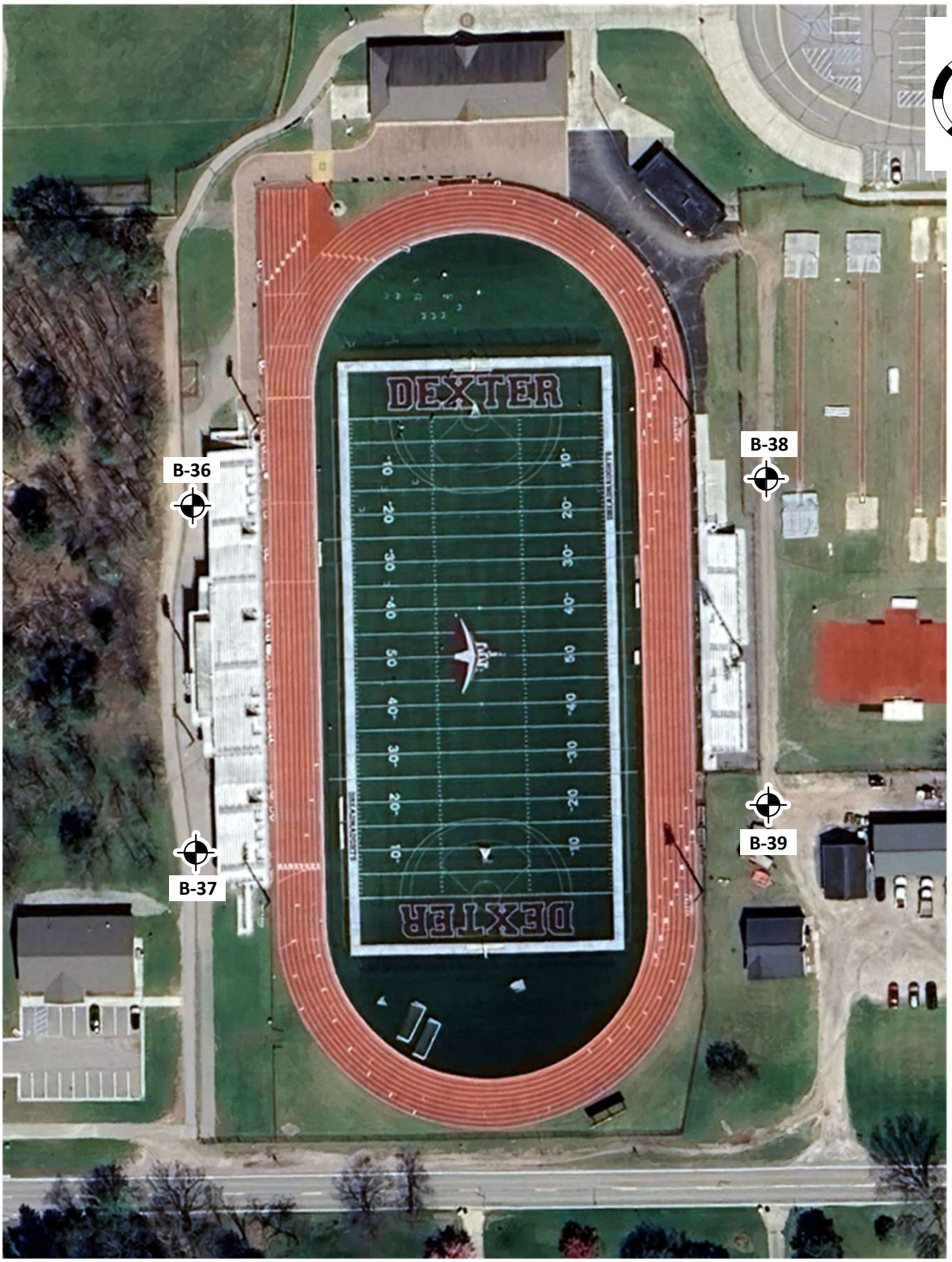
- Soil Borings performed by 2G Drilling between December 1, 2025 and December 8, 2025
- Preliminary soil sorings performed by others

## Soil Boring Location Plan

2025 Dexter Bond Program
DHS Fieldhouse and Site Improvements
2200 N. Parker Road
Dexter, Michigan 48130

2G CONSULTING GROUP Smart.Results.Fast.

| | |
|---|---|
| Project No. 253566 | |
| Drawn by: TSH | |
| Date: 1-5-2026 | Plate No. 1 |
| Scale: NTS | |




## Legend

Soil Borings perform by 2G Drilling between December 1, 2025 and December 8, 2025

**Soil Boring Location Plan**

2025 Dexter Bond Program
Al Ritt Stadium Lighting
2521 Baker Road
Dexter, Michigan 48130

2G CONSULTING GROUP Smart.Results.Fast.

| | |
|---|---|
| Project No. 253566 | |
| Drawn by: TSH | |
| Date: 1-5-2026 | Plate No. 2 |
| Scale: NTS | |

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31903° Longitude: -83.89737°

# Soil Boring No. B-21

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 894.0 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Silty Sand (10 inches) 0.8 | | | | | | | |
| | | Fill: Very Loose Brown Clayey Sand; intermixed wood chips | | S-01 | 3 1 1 | 2 | | | |
| | | 3.5 | | | | | | | |
| 889.0 | | Stiff Brown Silty Clay with trace sand and gravel | 5 | S-02 | 2 5 4 | 9 | 20.7 | | 2500* |
| | | | | S-03 | 2 5 3 | 8 | 13.8 | | 2500* |
| | | 8.5 | | | | | | | |
| 884.0 | | Loose to Medium Compact Brown Sand with trace silt and gravel | 10 | S-04 | 3 5 4 | 9 | | | |
| 879.0 | | | 15 | S-05 | 4 13 14 | 27 | | | |
| | | 18.5 | | | | | | | |
| 874.0 | | Medium Compact Brown Gravelly Sand | 20 | S-06 | 14 8 10 | 18 | | | |
| | | 22.5 | | | | | | | |
| 869.0 | | Medium Compact Brown Clayey Sand with trace silt and gravel 25.0 | 25 | S-07 | 3 4 8 | 12 | | | |
| | | End of Boring @ 25 ft | | | | | | | |
| 864.0 | | | 30 | | | | | | |

Total Depth: 25 ft
Drilling Date: December 1, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
20 feet during drilling operations; wet cave at 19-1/2 feet after auger removal

Notes:
Borehole collapsed at 19-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 21

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31895° Longitude: -83.89776°

# Soil Boring No. B-22

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 893.0 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Sandy Clay (4 inches) 0.3 | | | | | | | |
| | | Fill: Very Stiff Brown Silty Clay with trace sand and gravel; intermixed roots | | S-01 | 2 4 6 | 10 | 19.0 | | 5000* |
| | | 3.5 | | | | | | | |
| 888.0 | | | 5 | S-02 | 3 7 8 | 15 | 11.8 | | 7000* |
| | | | | S-03 | 3 7 10 | 17 | 12.9 | | 9000* |
| | | Very Stiff to Hard Brown Silty Clay with trace sand and gravel; occasional sand seams | | | | | | | |
| 883.0 | | | 10 | S-04 | 2 4 7 | 11 | 10.6 | | 5000* |
| | | 13.5 | | | | | | | |
| 878.0 | | | 15 | S-05 | 1 2 4 | 6 | 12.1 | | 2000* |
| | | Stiff Gray Sandy Clay with trace silt and gravel | | | | | | | |
| | | 19.5 | | | | | | | |
| 873.0 | | | 20 | S-06 | 2 4 6 | 10 | 13.3 | | 4000* |
| | | Very Stiff Grayish Brown Sandy Clay with trace silt and gravel; occasional sand layers | | | | | | | |
| | | 23.5 | | | | | | | |
| | | Medium Compact Gray Gravelly Sand 25.0 | | | | | | | |
| 868.0 | | | 25 | S-07 | 4 7 9 | 16 | | | |
| | | End of Boring @ 25 ft | | | | | | | |
| 863.0 | | | 30 | | | | | | |

Total Depth: 25 ft
Drilling Date: December 1, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
20 feet during drilling operations; wet cave at 18-1/2 feet after auger removal

Notes:
Borehole collapsed at 18-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 22

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31879° Longitude: -83.89816°

# Soil Boring No. B-23

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 892.0 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Silty Clay (4 inches) 0.3 | | | | | | | |
| | | Hard Brown Silty Clay with trace sand and gravel; intermixed roots | | S-01 | 7<br>12<br>14 | 26 | 17.5 | | 9000* |
| | | 3.0 | | | | | | | |
| 887.0 | | Hard Brown Sandy Clay with trace silt and gravel; occasional sand seams | 5 | S-02 | 10<br>11<br>17 | 28 | 10.8 | | 9000* |
| | | 7.0 | | S-03 | 6<br>6<br>9 | 15 | | | |
| 882.0 | | Medium Compact Brown Sand with trace silt and gravel; occasional clayey sand layers | 10 | S-04 | 4<br>6<br>7 | 13 | | | |
| | | 14.5 | | | | | | | |
| 877.0 | | Hard Gray Sandy Clay with trace gravel | 15 | S-05 | 1<br>17<br>25 | 42 | 9.6 | | 9000* |
| | | 19.0 | | | | | | | |
| 872.0 | | Medium Compact Gray Clayey Sand with trace silt and gravel | 20 | S-06 | 3<br>7<br>6 | 13 | | | |
| | | 23.5 | | | | | | | |
| 867.0 | | Very Loose Gray Clayey Sand with trace silt and gravel 25.0 | 25 | S-07 | 1<br>1<br>2 | 3 | | | |
| | | End of Boring @ 25 ft | | | | | | | |
| 862.0 | | | 30 | | | | | | |

Total Depth: 25 ft
Drilling Date: December 3, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
20 feet during drilling operations; wet cave at 18-1/2 feet after auger removal

Notes:
Borehole collapsed at 18-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 23

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.3186° Longitude: -83.89735°

# Soil Boring No. B-24

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: 886.0 ft

| ELEV. (ft) | PRO-FILE | | DEPTH (ft) |
|---|---|---|---|
| | | Topsoil: Dark Brown Clayey Sand (6 inches) — 0.5 | |
| | | Medium Compact Reddish Brown Clayey Sand with trace silt and gravel — 3.5 | |
| 881.0 | | Medium Compact Brown Clayey Sand with trace silt and gravel — 6.0 | 5 |
| 876.0 | | Stiff Brown Sandy Clay with trace silt and gravel — 13.5 | 10 |
| 871.0 | | Compact Gray Sandy Silt — 18.5 | 15 |
| 866.0 | | Hard Gray Sandy Clay with trace silt and gravel; occasional cobbles (Hardpan) — 25.0 | 20 |
| 861.0 | | End of Boring @ 25 ft | 25 |
| 856.0 | | | 30 |

## SOIL SAMPLE DATA

| SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|
| S-01 | 3 7 8 | 15 | | | |
| S-02 | 4 7 9 | 16 | | | |
| S-03 | 3 5 6 | 11 | 10.3 | | 3000* |
| S-04 | 4 7 9 | 16 | 14.0 | | 2000* |
| S-05 | 8 13 18 | 31 | | | |
| S-06 | 16 50/5" | --- | 8.2 | | 9000* |
| S-07 | 47 50/5" | --- | 8.6 | | 9000* |

Total Depth: 25 ft
Drilling Date: December 2, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
15 feet during drilling operations; wet cave at 15 feet after auger removal

Notes:
Borehole collapsed at 15 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 24

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31825° Longitude: -83.89815°

# Soil Boring No. B-25

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 885.5 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Sandy Clay (7 inches) 0.6 | | | | | | | |
| | | | | S-01 | 6 7 9 | 16 | 9.4 | | 9000* |
| | | Hard Mottled Brown and Gray Sandy Clay with trace silt and gravel | | | | | | | |
| 880.5 | | (Mild hydrocarbon odor) | 5 | S-02 | 7 7 8 | 15 | 15.9 | | 8000* |
| | | 6.0 | | S-03 | 3 7 8 | 15 | | | |
| 875.5 | | Medium Compact Brown Sand with trace silt and gravel | 10 | S-04 | 15 12 13 | 25 | | | |
| | | 14.0 | | | | | | | |
| 870.5 | | Very Loose Gray Sandy Silt | 15 | S-05 | 1 1 3 | 4 | | | |
| | | 18.5 | | | | | | | |
| 865.5 | | Stiff Gray Sandy Clay with trace silt and gravel; occasional clayey sand seams | 20 | S-06 | 3 3 4 | 7 | 14.5 | | 3000* |
| | | 24.0 | | | | | | | |
| 860.5 | | Hard Gray Sandy Clay with trace silt and gravel (Hardpan) 25.0 | 25 | S-07 | 70 50/1" | --- | 8.6 | | 9000* |
| | | End of Boring @ 25 ft | | | | | | | |
| 855.5 | | | 30 | | | | | | |

Total Depth: 25 ft
Drilling Date: December 2, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
7 feet during drilling operations; wet cave at 7 feet after auger removal

Notes:
Borehole collapsed at 7 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 25

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31825° Longitude: -83.89735°

# Soil Boring No. B-26

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 882.0 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Clayey Sand (7 inches) 0.6 | | | | | | | |
| | | Medium Compact Brown Clayey Sand with trace silt and gravel | | S-01 | 4 5 7 | 12 | | | |
| | | 3.0 | | | | | | | |
| 877.0 | | Loose Brown Silty Sand with trace gravel | 5 | S-02 | 6 5 4 | 9 | | | |
| | | 6.0 | | | | | | | |
| | | Loose Brown Clayey Sand with trace silt and gravel | | S-03 | 2 3 4 | 7 | | | |
| | | 8.0 | | | | | | | |
| 872.0 | | Compact Brown Gravelly Sand with trace silt | 10 | S-04 | 5 10 21 | 31 | | | |
| | | 13.5 | | | | | | | |
| 867.0 | | Very Stiff Gray Clayey Silt | 15 | S-05 | 8 11 16 | 27 | 15.4 | | 6000* |
| | | 19.0 | | | | | | | |
| 862.0 | | Hard Gray Sandy Clay with trace silt and gravel; occasional sand layers | 20 | S-06 | 14 23 25 | 48 | 10.0 | | 9000* |
| | | 24.0 | | | | | | | |
| 857.0 | | Hard to Very Hard Gray Sandy Clay with trace silt and gravel (Hardpan) 25.0 | 25 | S-07 | 26 50/5" | --- | 10.2 | | 9000* |
| | | End of Boring @ 25 ft | | | | | | | |
| 852.0 | | | 30 | | | | | | |

Total Depth: 25 ft
Drilling Date: December 2, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
7 feet during drilling operations; wet cave at 8-1/2 feet after auger removal

Notes:
Borehole collapsed at 8-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 26

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/9/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31879° Longitude: -83.89861°

# Soil Boring No. B-27

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 892.0 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Sandy Clay (5 inches) 0.4 | | | | | | | |
| | | Compact Brown Silty Sand with trace gravel; occasional sandy clay layers | | S-01 | 11 17 28 | 45 | | | |
| | | 3.5 | | | | | | | |
| 887.0 | | Hard Mottled Brown and Gray Sandy Clay with trace silt and gravel | 5 | S-02 | 11 11 8 | 19 | 17.1 | | 9000* |
| | | | | S-03 | 6 10 12 | 22 | 9.5 | | 9000* |
| | | 8.0 | | | | | | | |
| 882.0 | | Very Stiff Brown Sandy Clay with trace silt and gravel; occasional sand seams | 10 | S-04 | 5 8 10 | 18 | 12.1 | | 7000* |
| | | 13.5 | | | | | | | |
| 877.0 | | Stiff Gray Sandy Clay with trace silt and gravel | 15 | S-05 | 2 2 4 | 6 | 10.0 | | 2500* |
| | | 18.5 | | | | | | | |
| 872.0 | | Loose Gray Clayey Sand with trace silt and gravel 20.0 | 20 | S-06 | 4 4 6 | 10 | | | |
| | | End of Boring @ 20 ft | | | | | | | |
| 867.0 | | | 25 | | | | | | |
| 862.0 | | | 30 | | | | | | |

Total Depth: 20 ft
Drilling Date: December 3, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
17-1/2 feet during drilling operations; wet cave at 17-1/2 feet after auger removal

Notes:
Borehole collapsed at 17-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 27

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31914° Longitude: -83.89836°

# Soil Boring No. B-28

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 892.5 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Silty Clay (6 inches) 0.5 | | | | | | | |
| | | Fill: Very Stiff Mottled Brown and Gray Silty Clay with trace sand and gravel; intermixed organic matter (Organic Matter Content = 4.3%) | | S-01 | 4 10 7 | 17 | 17.6 | | 6000* |
| | | 3.5 | | | | | | | |
| 887.5 | | Fill: Stiff Very Dark Brown Silty Clay; intermixed peat (Organic Matter Content = 7.0%) | 5 | S-02 | 3 4 4 | 8 | 28.4 | | 2000* |
| | | 6.0 | | S-03 | 2 3 4 | 7 | 11.4 | | 6000* |
| 882.5 | | Very Stiff Brown Sandy Clay with trace silt and gravel | 10 | S-04 | 3 4 7 | 11 | 10.2 | | 6000* |
| | | 13.5 | | S-05 | 4 4 4 | 8 | 11.9 | | 3000* |
| 877.5 | | Stiff Gray Sandy Clay with trace silt and gravel | 15 | | | | | | |
| | | 18.5 | | S-06 | 5 6 6 | 12 | 12.4 | | 3000* |
| 872.5 | | Stiff Gray Clayey Silt 20.0 | 20 | | | | | | |
| | | End of Boring @ 20 ft | | | | | | | |
| 867.5 | | | 25 | | | | | | |
| 862.5 | | | 30 | | | | | | |

Total Depth: 20 ft
Drilling Date: December 1, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
15 feet during drilling operations; wet cave at 16-3/4 feet after auger removal

Notes:
Borehole collapsed at 17 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 28

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31862° Longitude: -83.89674°

# Soil Boring No. B-29

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 881.0 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Sandy Clay (7 inches) 0.6 | | | | | | | |
| | | Stiff Brown Sandy Clay with trace silt and gravel; occasional clayey sand layers | | S-01 | 6<br>10<br>9 | 19 | 14.7 | | 3000* |
| | | 3.0 | | | | | | | |
| 876.0 | | Loose to Medium Compact Brown Clayey Sand with trace silt and gravel; occasional cobbles | 5 | S-02 | 5<br>12<br>12 | 24 | | | |
| | | | | S-03 | 4<br>5<br>5 | 10 | | | |
| | | 8.5 | | | | | | | |
| 871.0 | | | 10 | S-04 | 6<br>14<br>14 | 28 | | | |
| | | | | | | | | | |
| 866.0 | | Medium Compact to Compact Brown Gravelly Sand with trace silt and clay | 15 | S-05 | 10<br>20<br>16 | 36 | | | |
| | | | | | | | | | |
| 861.0 | | 20.0 | 20 | S-06 | 12<br>17<br>17 | 34 | | | |
| | | End of Boring @ 20 ft | | | | | | | |
| 856.0 | | | 25 | | | | | | |
| 851.0 | | | 30 | | | | | | |

Total Depth: 20 ft
Drilling Date: December 2, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
10 feet during drilling operations; wet cave at 14-3/4 feet after auger removal

Notes:
Borehole collapsed at 15 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 29

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31879° Longitude: -83.8970°

# Soil Boring No. B-30

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 886.0 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Silty Sand (6 inches) 0.5 | | | | | | | |
| | | Loose Brown Clayey Sand with trace silt and gravel | | S-01 | 6<br>5<br>3 | 8 | | | |
| | | 3.0 | | | | | | | |
| 881.0 | | Stiff Brown Sandy Clay with trace silt and gravel | 5 | S-02 | 4<br>4<br>3 | 7 | 12.2 | | 2000* |
| | | 6.0 | | S-03 | 8<br>11<br>17 | 28 | | | |
| 876.0 | | | 10 | S-04 | 12<br>19<br>21 | 40 | | | |
| 871.0 | | Medium Compact to Compact Brown Sand with trace silt and gravel | 15 | S-05 | 9<br>10<br>13 | 23 | | | |
| | | 18.5 | | | | | | | |
| 866.0 | | Compact Brown Sandy Gravel with trace silt 20.0 | 20 | S-06 | 10<br>25<br>12 | 37 | | | |
| | | End of Boring @ 20 ft | | | | | | | |
| 861.0 | | | 25 | | | | | | |
| 856.0 | | | 30 | | | | | | |

Total Depth: 20 ft
Drilling Date: December 2, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 14 feet after auger removal

Notes:
Borehole collapsed at 14 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 30

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31904° Longitude: -83.89682°

# Soil Boring No. B-31

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 892.0 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Sandy Clay (2 inches) 0.2 | | | | | | | |
| | | Fill: Stiff Brown Sandy Clay with trace silt and gravel; occasional clayey sand layers | | S-01 | 5 6 6 | 12 | 15.1 | | 3000* |
| | | 3.0 | | | | | | | |
| 887.0 | | Medium Compact Brown Sandy Silt with trace gravel | 5 | S-02 | 5 7 8 | 15 | | | |
| | | 6.0 | | | | | | | |
| | | Very Stiff Brown Silty Clay with trace sand and gravel | | S-03 | 7 8 16 | 24 | 11.7 | | 7000* |
| | | 8.5 | | | | | | | |
| 882.0 | | Compact Brown Gravelly Sand with trace silt | 10 | S-04 | 12 17 16 | 33 | | | |
| | | 13.5 | | S-05 | 5 7 11 | 18 | | | |
| 877.0 | | Medium Compact Brown Clayey Sand with trace silt and gravel; occasional gravelly sand layers | 15 | | | | | | |
| | | 20.0 | 20 | S-06 | 2 6 9 | 15 | | | |
| 872.0 | | End of Boring @ 20 ft | | | | | | | |
| 867.0 | | | 25 | | | | | | |
| 862.0 | | | 30 | | | | | | |

Total Depth: 20 ft
Drilling Date: December 3, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
19 feet during drilling operations; wet cave at 12 feet after auger removal

Notes:
Borehole collapsed at 12 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 31

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31915° Longitude: -83.89652°

# Soil Boring No. B-32

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 891.0 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Sandy Clay (4 inches) 0.3 | | | | | | | |
| | | Hard Brown Silty Clay with trace sand and gravel (LL = 21, PI = 6) | | S-01 | 8 7 7 | 14 | 12.1 | | 9000* |
| 886.0 | | 4.5 | 5 | S-02 | 8 12 9 | 21 | | | |
| | | Medium Compact Brown Silty Sand with trace gravel 6.0 | | | | | | | |
| | | Hard Brown Sandy Clay with trace silt and gravel 8.0 | | S-03 | 8 8 10 | 18 | 10.0 | | 9000* |
| 881.0 | | Compact Brown Clayey Sand with trace silt and gravel 10.0 | 10 | S-04 | 12 20 21 | 41 | | | |
| | | End of Boring @ 10 ft | | | | | | | |
| 876.0 | | | 15 | | | | | | |
| 871.0 | | | 20 | | | | | | |
| 866.0 | | | 25 | | | | | | |
| 861.0 | | | 30 | | | | | | |

Total Depth: 10 ft
Drilling Date: December 2, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 9-1/2 feet after auger removal

Notes:
Borehole collapsed at 9-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 32

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31952° Longitude: -83.89623°

# Soil Boring No. B-33

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: 887.0 ft | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Clayey Sand (7 inches) 0.6 | | | | | | | |
| | | Fill: Medium Compact Brown Clayey Sand with trace silt; occasional silty sand layers | | S-01 | 7<br>8<br>7 | 15 | | | |
| | | 3.0 | | | | | | | |
| 882.0 | | Very Stiff Brown Sandy Clay with trace silt and gravel; hydrocarbon odor | 5 | S-02 | 4<br>4<br>9 | 13 | 10.7 | | 7000* |
| | | 6.0 | | | | | | | |
| | | Very Stiff Reddish Brown Sandy Clay with trace silt and gravel | | S-03 | 2<br>4<br>5 | 9 | 19.1 | | 5000* |
| | | 9.0 | | | | | | | |
| 877.0 | | Medium Compact Brown Sand with trace silt and gravel 10.0 | 10 | S-04 | 1<br>7<br>10 | 17 | | | |
| | | End of Boring @ 10 ft | | | | | | | |
| 872.0 | | | 15 | | | | | | |
| 867.0 | | | 20 | | | | | | |
| 862.0 | | | 25 | | | | | | |
| 857.0 | | | 30 | | | | | | |

Total Depth: 10 ft
Drilling Date: December 4, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 8-1/2 feet after auger removal

Notes:
Borehole collapsed at 8-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 33

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.31992° Longitude: -83.89621°

# Soil Boring No. B-34

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: 883.0 ft | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Sandy Clay (6 inches) 0.5 | | | | | | | |
| | | Very Stiff Brown Sandy Clay with trace silt and gravel | | S-01 | 4 4 3 | 7 | 16.9 | | 5000* |
| | | 3.8 | | | | | | | |
| 878.0 | | Loose Brown Silty Sand with trace gravel | 5 | S-02 | 3 3 2 | 5 | | | |
| | | 6.0 | | | | | | | |
| | | Medium Compact Brown Clayey Sand with trace silt and gravel | | S-03 | 5 7 10 | 17 | | | |
| 873.0 | | 10.0 | 10 | S-04 | 8 12 10 | 22 | | | |
| | | End of Boring @ 10 ft | | | | | | | |
| 868.0 | | | 15 | | | | | | |
| 863.0 | | | 20 | | | | | | |
| 858.0 | | | 25 | | | | | | |
| 853.0 | | | 30 | | | | | | |

Total Depth: 10 ft
Drilling Date: December 4, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 7-1/2 feet after auger removal

Notes:
Borehole collapsed at 7-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 34

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.32027° Longitude: -83.89586°

# Soil Boring No. B-35

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| ELEV. (ft) | PRO-FILE | GROUND SURFACE ELEVATION: 865.0 ft | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Silty Sand (6 inches) 0.5 | | | | | | | |
| | | Medium Compact Brown Silty Sand with trace gravel | | S-01 | 10<br>10<br>12 | 22 | | | |
| | | 3.5 | | | | | | | |
| 860.0 | | Medium Brown Sandy Clay with trace silt and gravel | 5 | S-02 | 4<br>3<br>4 | 7 | 11.5 | | 1500* |
| | | 6.0 | | | | | | | |
| | | Very Stiff Brown Silty Clay with trace sand and gravel | | S-03 | 6<br>12<br>10 | 22 | 12.2 | | 6000* |
| | | 8.5 | | | | | | | |
| 855.0 | | Stiff Brown Sandy Clay with trace silt and gravel 10.0 | 10 | S-04 | 5<br>6<br>9 | 15 | 10.9 | | 3000* |
| | | End of Boring @ 10 ft | | | | | | | |
| 850.0 | | | 15 | | | | | | |
| 845.0 | | | 20 | | | | | | |
| 840.0 | | | 25 | | | | | | |
| 835.0 | | | 30 | | | | | | |

Total Depth: 10 ft
Drilling Date: December 4, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 8-3/4 feet after auger removal

Notes:
Borehole collapsed at 9 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 35

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.32640° Longitude: -83.89043°

# Soil Boring No. B-36

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

- Crushed Limestone Aggregate: Gray Gravelly Sand with trace silt — 0.0 to 2.0
- Medium Compact Brown Silty Sand with trace gravel — 2.0 to 3.5
- Loose to Medium Compact Brown Sand with trace silt and gravel — 3.5 to 8.5
- Hard Gray Silty Clay with trace sand and gravel — 8.5 to 12.5
- Hard Gray Sandy Clay with trace silt and gravel (Hardpan) — 12.5 to 30.0

## SOIL SAMPLE DATA

| DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|
| 0 | S-01 | 8 6 7 | 13 | | | |
| 5 | S-02 | 2 3 3 | 6 | | | |
| 5 | S-03 | 9 10 13 | 23 | | | |
| 10 | S-04 | 6 11 18 | 29 | 8.7 | | 9000* |
| 15 | S-05 | 3 8 50/3" | --- | 7.8 | | 9000* |
| 20 | S-06 | 14 50/5" | --- | 7.9 | | 9000* |
| 25 | S-07 | 100/4" | --- | 11.0 | | |
| 30 | S-08 | 61 50/3" | --- | 8.3 | | 9000* |

Total Depth: 30 ft End of Boring @ 30 ft
Drilling Date: December 8, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
5 feet during drilling operations; wet cave at 15 feet after auger removal

Notes:
Borehole collapsed at 15 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 36

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.32583° Longitude: -83.89041°

# Soil Boring No. B-37

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| DEPTH (ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Crushed Limestone Aggregate: Gray Gravelly Sand with trace silt — 1.0 | | | | | | | |
| | | Fill: Loose Brown Silty Sand — 2.3 | | S-01 | 5<br>4<br>4 | 8 | | | |
| 5 | | Fill: Loose Very Dark Brown Silty Sand (Organic Matter Content = 6.5%) — 4.8 | 5 | S-02 | 3<br>2<br>3 | 5 | | | |
| | | | | S-03 | 2<br>4<br>6 | 10 | | | |
| 10 | | Very Loose to Loose Brown Sand with trace silt and gravel | 10 | S-04 | 3<br>1<br>2 | 3 | | | |
| | | — 13.5 | | S-05 | 20<br>26<br>34 | 60 | 8.8 | | 9000* |
| 15 | | | 15 | | | | | | |
| | | Hard Gray Silty Clay with trace sand and gravel; occasional cobbles | | S-06 | 25<br>38<br>50/6" | --- | 7.7 | | 9000* |
| 20 | | | 20 | | | | | | |
| | | — 22.5 | | S-07 | 25<br>44<br>50/2" | --- | 9.2 | | 9000* |
| 25 | | (Poor recovery @ S-07) Hard to Very Hard Gray Sandy Clay with trace silt and gravel (Hardpan) | 25 | | | | | | |
| | | — 30.0 | | S-08 | 31<br>50/5" | --- | 11.1 | | 9000* |
| 30 | | | 30 | | | | | | |

Total Depth: 30 ft End of Boring @ 30 ft
Drilling Date: December 8, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
7 feet during drilling operations; wet cave at 6 feet after auger removal

Notes:
Borehole collapsed at 6 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 37

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/7/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.32643° Longitude: -83.88910°

# Soil Boring No. B-38

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

- 0.0 – 0.6: Topsoil: Dark Brown Silty Sand (7 inches)
- 0.6 – 3.0: Fill: Medium Compact Brown Sand with trace silt and gravel
- 3.0 – 6.0: Fill: Medium Compact Gray and Brown Silty Sand; occasional clay clods
- 6.0 – 30.0: Hard Gray Sandy Clay with trace gravel
  - (Poor recovery @ S-05)

## SOIL SAMPLE DATA

| DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|
| | S-01 | 6 6 8 | 14 | | | |
| 5 | S-02 | 5 8 9 | 17 | | | |
| | S-03 | 4 8 15 | 23 | 8.6 | | 9000* |
| 10 | S-04 | 6 13 40 | 53 | 7.1 | | 9000* |
| 15 | S-05 | 20 25 27 | 52 | | | |
| 20 | S-06 | 17 26 32 | 58 | 8.8 | | 9000* |
| 25 | S-07 | 21 20 33 | 53 | 7.2 | | |
| 30 | S-08 | 30 33 39 | 72 | 7.0 | | |

Total Depth: 30 ft End of Boring @ 30 ft
Drilling Date: December 4, 2025
Inspector:
Contractor: 2G Drilling
Driller: A. Guzdzial

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
5 feet during drilling operations; wet cave at 29 feet after auger removal

Notes:
Borehole collapsed at 29 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 38

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/9/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.32586° Longitude: -83.88908°

# Soil Boring No. B-39

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

Loose Reddish Brown Silty Sand

6.5

Loose Brown Sand with trace silt

8.5

Very Stiff Brown Sandy Clay with trace silt and gravel

13.0

Hard to Very Hard Gray Sandy Clay with trace silt and gravel; occasional cobbles

## SOIL SAMPLE DATA

| DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|
| | S-01 | 5<br>4<br>3 | 7 | | | |
| 5 | S-02 | 3<br>3<br>4 | 7 | | | |
| | S-03 | 3<br>3<br>5 | 8 | | | |
| 10 | S-04 | 9<br>14<br>16 | 30 | 12.9 | | 4000* |
| 15 | S-05 | 13<br>26<br>35 | 61 | 9.6 | | 9000* |
| 20 | S-06 | 27<br>33<br>36 | 69 | 8.5 | | 9000* |
| 25 | S-07 | 55<br>50/5" | --- | 9.1 | | 9000* |
| 30 | S-08 | 19<br>31<br>42 | 73 | 6.6 | | 9000* |

Total Depth: 35 ft
Drilling Date: December 8, 2025
Inspector:
Contractor: 2G Drilling
Driller: Brendan

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
6-1/2 feet during drilling; dry cave at 11 feet after auger removal

Notes:
Borehole collapsed at 11 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 39a

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/9/26

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.32586° Longitude: -83.88908°

# Soil Boring No. B-39

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Hard to Very Hard Gray Sandy Clay with trace silt and gravel; occasional cobbles *(continued)* | | | | | | | |
| 35 | | 34.5<br>Very Compact Gray Gravelly Sand with trace silt<br>35.0 | 35 | S-09 | 9<br>27<br>50/3" | --- | 8.1 | | 9000* |
| | | End of Boring @ 35 ft | | | | | | | |
| 40 | | | 40 | | | | | | |
| 45 | | | 45 | | | | | | |
| 50 | | | 50 | | | | | | |
| 55 | | | 55 | | | | | | |
| 60 | | | 60 | | | | | | |

Total Depth: 35 ft
Drilling Date: December 8, 2025
Inspector:
Contractor: 2G Drilling
Driller: Brendan

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

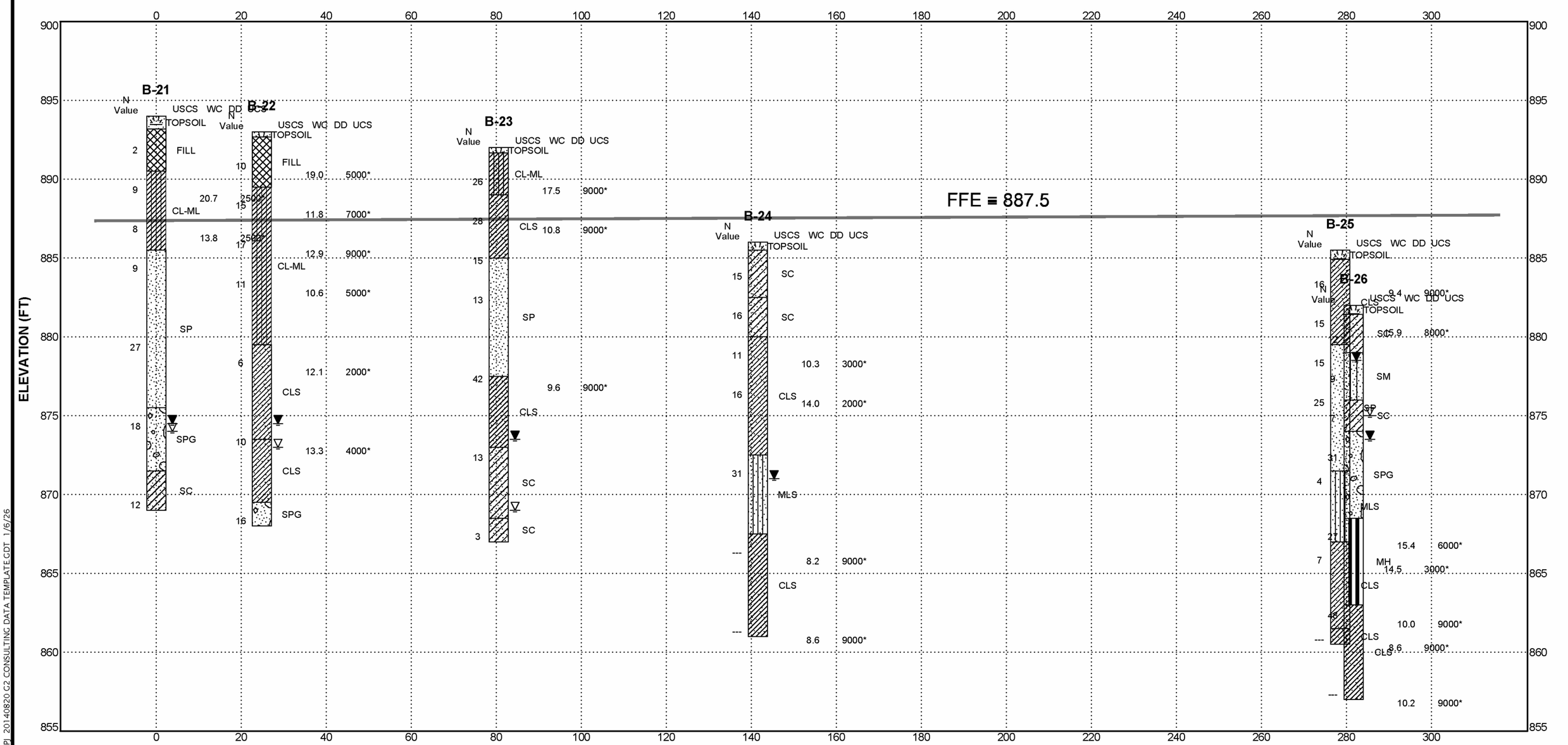
Water Level Observation:
6-1/2 feet during drilling; dry cave at 11 feet after auger removal

Notes:
Borehole collapsed at 11 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 39b

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 1/9/26




## Legend

- ▽ Encountered Groundwater Level
- ▼ Groundwater Level at Completion

G2 CONSULTING GROUP

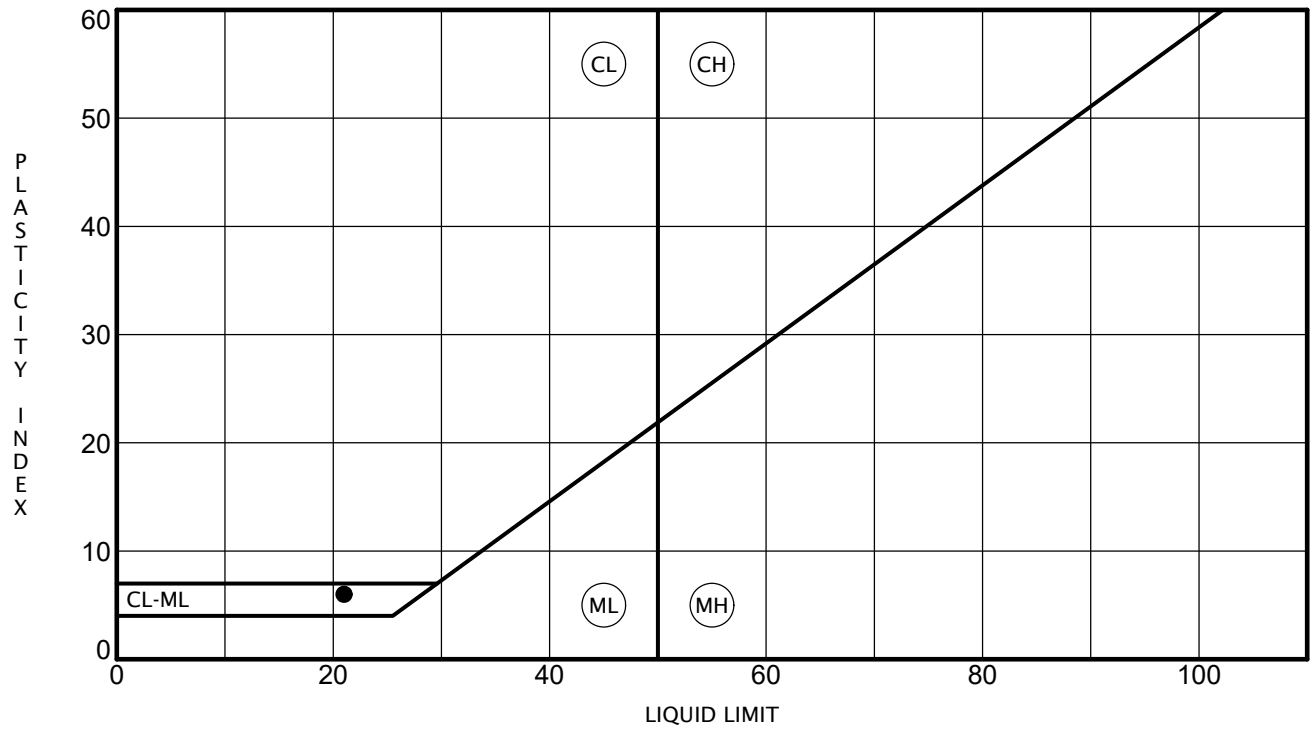
# SOIL BORING PROFILE A-A'

Project Name: Dexter Schools 2025 Bond Program - Dexter
Project Location: Fieldhouse & Al Ritt Stadium Improvements
Dexter, Michigan

G2 Project No.: 253566

Figure No. 40

SOIL BORING PROFILE WITH WATER LEVELS 253566.GPJ 20140820 G2 CONSULTING DATA TEMPLATE.GDT 1/6/26




| | Specimen Identification | | LL | PL | PI | Fines | M % | Classification |
|---|---|---|---|---|---|---|---|---|
| ● | B-32 | S-01 | 21 | 15 | 6 | | 12 | Brown Silty Clay |

CONSULTING GROUP

# ATTERBERG LIMITS RESULTS

Project Name: Dexter Schools 2025 Bond Program - Dexter Fieldhouse & Al Ritt Stadium Improvements

Project Location: Dexter, Michigan

G2 Project No.: 253566

Figure No. 41

US_ATTERBERG_LIMITS 253566.GPJ 20140820 G2 CONSULTING DATA TEMPLATE.GDT 1/6/26

# GENERAL NOTES TERMINOLOGY

Unless otherwise noted, all terms herein refer to the Standard Definitions presented in ASTM D 653.

## PARTICLE SIZE

| | | |
|---|---|---|
| Boulders | | - greater than 12 inches |
| Cobbles | | - 3 inches to 12 inches |
| Gravel | - Coarse | - 3/4 inches to 3 inches |
| | - Fine | - No. 4 to 3/4 inches |
| Sand | - Coarse | - No. 10 to No. 4 |
| | - Medium | - No. 40 to No. 10 |
| | - Fine | - No. 200 to No. 40 |
| Silt | | - 0.005mm to 0.074mm |
| Clay | | - Less than 0.005mm |

## CLASSIFICATION

The major soil constituent is the principal noun, i.e. clay, silt, sand, gravel. The second major soil constituent and other minor constituents are reported as follows:

| **Second Major Constituent (percent by weight)** | **Minor Constituent (percent by weight)** |
|---|---|
| Trace - 1 to 12% | Trace - 1 to 12% |
| Adjective - 12 to 35% | Little - 12 to 23% |
| And - over 35% | Some - 23 to 33% |

## COHESIVE SOILS

If clay content is sufficient so that clay dominates soil properties, clay becomes the principal noun with the other major soil constituent as modifier, i.e. sandy clay. Other minor soil constituents may be included in accordance with the classification breakdown for cohesionless soils, i.e. silty clay, trace sand, little gravel.

| **Consistency** | **Unconfined Compressive Strength (psf)** | **Approximate Range of (N)** |
|---|---|---|
| Very Soft | Below 500 | 0 - 2 |
| Soft | 500 - 1,000 | 3 - 4 |
| Medium | 1,000 - 2,000 | 5 - 8 |
| Stiff | 2,000 - 4,000 | 9 - 15 |
| Very Stiff | 4,000 - 8,000 | 16 - 30 |
| Hard | 8,000 - 16,000 | 31 - 50 |
| Very Hard | Over 16,000 | Over 50 |

Consistency of cohesive soils is based upon an evaluation of the observed resistance to deformation under load and not upon the Standard Penetration Resistance (N).

## COHESIONLESS SOILS

| **Density Classification** | **Relative Density %** | **Approximate Range of (N)** |
|---|---|---|
| Very Loose | 0 - 15 | 0 - 4 |
| Loose | 16 - 35 | 5 - 10 |
| Medium Compact | 36 - 65 | 11 - 30 |
| Compact | 66 - 85 | 31 - 50 |
| Very Compact | 86 - 100 | Over 50 |

Relative Density of cohesionless soils is based upon the evaluation of the Standard Penetration Resistance (N), modified as required for depth effects, sampling effects, et

## SAMPLE DESIGNATIONS

AS - Auger Sample – Cuttings directly from auger flight
BS - Bottle or Bag Samples
S - Split Spoon Sample - ASTM D 1586
LS - Liner Sample with liner insert 3 inches in length
ST - Shelby Tube sample - 3 inch diameter unless otherwise noted
PS - Piston Sample - 3 inch diameter unless otherwise noted
RC - Rock Core - NX core unless otherwise noted

STANDARD PENETRATION TEST (ASTM D 1586) - A 2.0 inch outside-diameter, 1-3/8 inch inside-diameter split barrel sampler is driven into undisturbed soil by means of a 140-pound weight falling freely through a vertical distance of 30 inches. The sampler is normally driven three successive 6-inch increments. The total number of blows required for the final 12 inches of penetration is the Standard Penetration Resistance (N).