



January 21, 2026

Rainforth Grau Architects Project No. 2022-1545.00

PTN: 68585-238

ADDENDUM NO. 1

Henderson Elementary School - Lunch Shelter
Lodi, California
Lodi Unified School District

RGA, a studio of HMC Architects

1. ALL WORKMANSHIP, MATERIALS, APPLIANCES AND EQUIPMENT which may be included in the following items shall be the same relative quantity as described for similar work set forth in the original or main specifications of which these Addendum items shall be considered a part.
2. ADDENDUM SECTIONS (included with this Addendum)
The following modify or supplement the issued bid documents.
Section 13 34 00 – ADD 13 34 00, FABRICATED ENGINEERED STRUCTURES
3. ADDENDUM DRAWINGS (included with this Addendum)
The following modify or supplement the issued bid documents.
AD 1.01 ADD Sheet A1.1.2
AD 1.02 ADD Sheet LS1.0
AD 1.03 ADD Sheet LS1.1
AD 1.04 ADD Sheet LS1.2
AD 1.05 ADD Sheet LS1.3
AD 1.06 ADD Sheet LS3.0
AD 1.07 ADD Sheet LS3.1
AD 1.08 ADD Sheet LS3.4

4. PROJECT MANUAL

A. Section 00 11 16, Notice to Bidders

1. Article 2: Add note, "Include full removal of concrete foundations, stairs, and any other building utilities that are in the way of the new shade shelter at VRA/Independence @ Henderson. Also, remove gas stub to dead end in a box in a safe area outside project area, coordinating with PG&E and the District."

B. Section 00 41 13, Bid Form and Proposal

1. ADD line item for "ADDITIVE ALTERNATE 1, REPLACE ADDITIONAL URINALS."

C. Section 13 34 00, Fabricated Engineered Structures

1. ADD Section 13 34 00 included with this addendum.

5. DRAWINGS

A. Sheet A1.1.0, Site Plan and Code Information:

1. Detail 1, Site Plan: Add light post to be connected from Building B to be located along existing fence line approximately halfway between B and the public right of way for safe dispersal purposes. Light post to be based on Gardco ECF-S-32L-1A-NW-G2-AR-3-120-HB-IMRI3-HB-TLRPC-BZ, 120 volts, 106 Watts with 4" x 20' straight square aluminum pole with finish to match head. Install on 24" diameter concrete post base 24" above grade with 48" embedment and (8) #5 bars vertical reinforcing steel bars and #4 (MIN.) steel hoops @ 12" o.c."

B. Sheet A1.1.2, Lunch Shelter Exhibit

1. ADD Sheet AD 1.01 included with this addendum.

C. Sheet A2.1.1, Enlarged Floor Plans and Fixture Schedule

1. Detail 3: Add Note to read, "Additive Alternate 1: Include as an additive alternate, the full removal of existing raised floor and full removal of two additional existing urinals."

2. Detail 4: Add Note to read, "Additive Alternate 1: Include as an additive alternate, the repair and patching of the floor where raised floor was removed and installation of two new urinals similar to UR-1 and installation of one additional urinal screen and post."

D. Sheet LS1.0, General Info

1. ADD Sheet AD 1.02 included with this addendum.

E. Sheet LS1.1, General Info

1. ADD Sheet AD 1.03 included with this addendum.

F. Sheet LS1.2, DSA 103

1. ADD Sheet AD 1.04 included with this addendum.

G. Sheet LS1.3, DSA 103

1. ADD Sheet AD 1.05 included with this addendum.
 - H. Sheet LS3.0, 30' Wide Rectangular Hip Foundation Plan
 1. ADD Sheet AD 1.06 included with this addendum.
 - I. Sheet LS3.1, 30' Wide Rectangular Hip Framing & Connection Details
 1. ADD Sheet AD 1.07 included with this addendum.
 - J. Sheet LS3.4, 30' Wide Rectangular Hip Standing Seam Roofing Plan
 1. ADD Sheet AD 1.08 included with this addendum.
6. BID QUESTIONS
- A. Is this project subject to a Project Labor Agreement (PLA) or CWTA?
Response: No.
 - B. What is the project estimate?
Response: The project estimate is \$900,000.
 - C. Please specify how many days the construction will take (also clarify whether it's calendar days or working days).
Response: Per 00 01 20 List of Schedules, construction to take place between March 13, 2026, thru July 23, 2026 with approved for After Hours, Nights, Weekends & Holiday Work; also per 00 52 13 Agreement, Time for Completion: It is hereby understood and agreed that the Work under this Contract shall be completed by 7/23/2026 ("Contract Time") from the date specified in the District's Notice to Proceed.

* * * END OF ADDENDUM * * *

HENDERSON SCHOOL LUNCH SHELTER
LODI, CALIFORNIA
ADDENDUM 1
21 JANUARY 26

SECTION 13 34 00 - FABRICATED ENGINEERED STRUCTURES

PART 1 - GENERAL

1.01 SUMMARY

- A. Section Includes: Fabricated engineered structures including structural framing, secondary framing, and concrete footings of the following type :
1. Shade structures with metal roofing panels.

1.02 PREINSTALLATION MEETINGS

- A. Preinstallation Conference: Conduct conference at Project site.
1. Review methods and procedures related to fabricated engineered structures including, but not limited to, the following:
 - a. Preparatory work performed by other trades.
 - b. Construction schedule. Verify availability of materials and erector's personnel, equipment, and facilities needed to make progress and avoid delays.
 - c. Required tests, inspections, and certifications.
 - d. Unfavorable weather and forecasted weather conditions and impact on construction schedule.
 - e. Flashings, special details, drainage, penetrations, curbs, and condition of other construction that may affect the work.

1.03 ACTION SUBMITTALS

- A. Product Data: For each type of fabricated engineered structure component.
- B. Shop Drawings:
1. Include plans, elevations, sections, structural connection details, and attachment details.
 2. Detail fabrication, assembly, and attachment of metal roofing panels.
- C. Samples for Initial Selection: For factory-applied finishes.

1.04 INFORMATIONAL SUBMITTALS

- A. Design Data Submittal:
1. Include analysis data indicating compliance with performance requirements and design data signed and sealed by the qualified professional engineer responsible for their preparation.
- B. Sample Warranties: For special warranties.

RG+
22-1545

FABRICATED ENGINEERED STRUCTURES
13 34 00 - 1

HENDERSON SCHOOL LUNCH SHELTER
LODI, CALIFORNIA
ADDENDUM 1
21 JANUARY 26

1.05 QUALITY ASSURANCE

- A. Manufacturer Qualifications: A qualified manufacturer.
 - 1. Engineering Responsibility: Preparation of comprehensive engineering analysis and Shop Drawings by a professional engineer who is legally qualified to practice in jurisdiction where Project is located.
- B. Erector Qualifications: An experienced erector who specializes in erecting and installing work similar in material, design, and extent to that indicated for this Project and who is acceptable to manufacturer.
- C. Flashing and Sheet Metal Standard: Comply with SMACNA's "Architectural Sheet Metal Manual" unless more stringent requirements are specified or shown on Drawings.
- D. Welding Qualifications: Qualify procedures and personnel according to the following:
 - 1. AWS D1.1/D1.1M, "Structural Welding Code - Steel."
 - 2. AWS D1.3, "Structural Welding Code - Sheet Steel."
- E. Ready-Mix-Concrete Manufacturer Qualifications: A firm experienced in manufacturing ready-mixed concrete products and that complies with ASTM C 94/C 94M requirements for production facilities and equipment.

1.06 DELIVERY, STORAGE, AND HANDLING

- A. Deliver components so as not to be damaged or deformed. Package for protection during transportation and handling.

1.07 WARRANTY

- A. Special Warranty: Manufacturer and fabricator agree to repair or replace components of fabricated engineered structures that fail in materials or workmanship within specified warranty period.
 - 1. Failures include, but are not limited to, the following:
 - a. Structural failures including framework.
 - b. Deterioration of metals, metal finishes, and other materials beyond normal weathering.
 - 1. Fabricated Engineered Structure Warranty Period: Ten years from date of Substantial Completion.
- B. Special Warranty on Panel Finishes: Manufacturer's standard form in which manufacturer agrees to repair finish or replace metal panels that show evidence of deterioration of factory-applied finishes within specified warranty period.
 - 1. Finish Warranty Period: 20 years from date of Substantial Completion.

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22-1545

FABRICATED ENGINEERED STRUCTURES
13 34 00 - 2

HENDERSON SCHOOL LUNCH SHELTER
LODI, CALIFORNIA
ADDENDUM 1
21 JANUARY 26

PART 2 - PRODUCTS

2.01 MANUFACTURERS

- A. Basis-of-Design Fabricated Engineered Structure: Subject to compliance with requirements, provide ICON Shelter Systems, Inc. structure assembly or comparable assembly by an approved manufacturer.
- B. Source Limitations: Obtain system components, including primary and secondary framing and metal support assemblies, from single source from single manufacturer.

2.02 SYSTEM DESCRIPTION

- A. Provide a complete, integrated set of mutually dependent components and assemblies that form a structure capable of withstanding structural and other loads, thermally induced movement, and exposure to weather without failure.
- B. Structural Framing: Manufacturer's standard, consisting of primary frame, capable of supporting design loads.
- C. Secondary-Frame Type: Manufacturer's standard purlins, joists, and girts.
- D. Eave Height: As indicated on Drawings.
- E. Roof Slope: As indicated on Drawings.

2.03 PERFORMANCE REQUIREMENTS

- A. Structural Performance: Fabricated engineered structures shall withstand the effects of gravity loads and the following loads and stresses within limits and under conditions indicated.
 - 1. Design Loads: As indicated on Drawings and in accordance with DSA Pre-Check Design Criteria.
 - 2. Deflection Limits: No greater than the following:
 - a. Purlins and Rafters: Vertical deflection as required by roofing panel manufacturer.
 - b. Design secondary-framing system to accommodate deflection of primary framing and construction tolerances, and to maintain clearances at openings.
- B. Seismic Performance: Fabricated engineered structures shall withstand the effects of earthquake motions determined according to ASCE/SEI 7.
- C. Thermal Movements: Allow for thermal movements from ambient and surface temperature changes by preventing buckling, opening of joints, overstressing of components, failure of joint sealants, failure of connections, and other detrimental

RG+
22-1545

FABRICATED ENGINEERED STRUCTURES
13 34 00 - 3

HENDERSON SCHOOL LUNCH SHELTER
LODI, CALIFORNIA
ADDENDUM 1
21 JANUARY 26

effects. Base calculations on surface temperatures of materials due to both solar heat gain and nighttime-sky heat loss.

1. Temperature Change: 120 deg F, ambient; 180 deg F, material surfaces.

- D. Wind-Uplift Resistance: Provide assemblies that comply with UL 580 for wind-uplift-resistance class indicated.

1. Uplift Rating: Meet performance requirements indicated in accordance with ASCE-7, but not less than UL 60.

2.04 STRUCTURAL STEEL FRAMING

- A. Structural Steel: Comply with AISC 360, "Specification for Structural Steel Buildings."

- B. Bolted Connections: Comply with RCSC's "Specification for Structural Joints Using High-Strength Bolts."

- C. Cold-Formed Steel: Comply with AISI's "North American Specification for the Design of Cold-Formed Steel Structural Members" for design requirements and allowable stresses.

- D. Primary Framing: Manufacturer's standard primary-framing system, designed to withstand required loads and specified requirements.

- E. Secondary Framing: Manufacturer's standard secondary framing, including purlins, girts, eave struts, flange bracing, base members, gable angles, clips, headers, jambs, and other miscellaneous structural members. Unless otherwise indicated, fabricate framing from metallic-coated steel sheet.

- F. Miscellaneous Structural Members: Manufacturer's standard sections fabricated from zinc-coated (galvanized) steel; designed to withstand required loads.

- G. Steel Pipe: ASTM A53/A53M.

- H. Anchors, Fasteners, Fittings, Hardware, and Installation Accessories: Complying with performance requirements indicated and suitable for exposure conditions, supporting structure, anchoring substrates, and installation methods indicated. Corrosion-resistant or noncorrodible units; weather-resistant, tamperproof, vandal- and theft-resistant, of one of the following materials:

1. Zinc coated steel.
2. Stainless steel.

- I. Finish: Manufacturer's standard primed and top-coated decorative powder-coat finish complying with finish manufacturer's written instructions.

RG+
22-1545

FABRICATED ENGINEERED STRUCTURES
13 34 00 - 4

HENDERSON SCHOOL LUNCH SHELTER
LODI, CALIFORNIA
ADDENDUM 1
21 JANUARY 26

2.05 SHEET METAL FLASHING

- A. Metallic-Coated Steel Sheet: Restricted flatness steel sheet, metallic coated by the hot-dip process and prepainted by the coil-coating process to comply with ASTM A 755/A 755M.
 - 1. Zinc-Coated (Galvanized) Steel Sheet: ASTM A 653/A 653M, G90 coating designation; structural quality.
 - 2. Exposed Finish: Fluoropolymer.
 - a. Color: As indicated by manufacturer's designations.

2.06 METAL ROOF PANELS

- A. Provide factory-formed metal roof panels designed to be installed by lapping side edges of adjacent panels and mechanically attaching panels to supports using exposed fasteners in side laps. Include accessories required for weathertight installation.
 - 1. Metallic-Coated Steel Sheet: Zinc-coated (galvanized) steel sheet complying with ASTM A653/A653M, G90 (Z275) coating designation, or aluminum-zinc alloy-coated steel sheet complying with ASTM A792/A792M, Class AZ50 (Class AZM150) coating designation; structural quality. Prepainted by the coil-coating process to comply with ASTM A755/A755M.
 - a. Nominal Thickness: Manufacturers Standard.
 - b. Exterior finish: Two-coat fluoropolymer.
 - c. Color: As selected by Architect from Manufacturer full range.

2.07 CONCRETE MATERIALS

- A. Furnish formwork and formwork accessories according to ACI 301 (ACI 301M).
- B. Forms for Cylindrical Columns, Pedestals, and Supports: Metal, glass-fiber-reinforced plastic, paper, or fiber tubes that produce surfaces not exceeding specified formwork surface class.
 - 1. Provide forms with sufficient wall thickness to resist plastic concrete loads without detrimental deformation.
- C. Concrete Reinforcement:
 - 1. Reinforcing Bars: ASTM A 615/A 615M, deformed.
- D. Portland Cement: ASTM C 150, Type V.
- E. Normal-Weight Aggregate: ASTM C 33, graded, 1-inch nominal maximum aggregate size.
- F. Water: ASTM C 94/C 94M.

HENDERSON SCHOOL LUNCH SHELTER
LODI, CALIFORNIA
ADDENDUM 1
21 JANUARY 26

2.08 CONCRETE MIXTURES

- A. Normal-Weight Concrete: Prepare design mixes, proportioned according to ACI 301 (ACI 301M), as follows:
 - 1. Minimum Compressive Strength: 5000 psi at 28 days.
- B. Ready-Mixed Concrete: Measure, batch, mix, and deliver concrete according to ASTM C 94/C 94M, and furnish batch ticket information.

2.09 ACCESSORIES

- A. General: Provide accessories as standard fabricated engineered structure system manufacturer and as specified. Fabricate and finish accessories at the factory to greatest extent possible, by manufacturer's standard procedures and processes. Comply with indicated profiles and with dimensional and structural requirements.

2.10 FABRICATION

- A. General: Design components and field connections required for erection to permit easy assembly.
 - 1. Mark each piece and part of the assembly to correspond with previously prepared erection drawings, diagrams, and instruction manuals.
 - 2. Fabricate structural framing to produce clean, smooth cuts and bends. Punch holes of proper size, shape, and location. Members shall be free of cracks, tears, and ruptures.
- B. Primary Framing: Shop fabricate framing components to indicated size and section, with baseplates, bearing plates, stiffeners, and other items required for erection welded into place. Cut, form, punch, drill, and weld framing for bolted field assembly.
- C. Secondary Framing: Shop fabricate framing components to indicated size and section by roll forming or break forming, with baseplates, bearing plates, stiffeners, and other plates required for erection welded into place. Cut, form, punch, drill, and weld secondary framing for bolted field connections to primary framing.

2.11 SHEET METAL FABRICATION

- A. General: Custom fabricate flashing and sheet metal to comply with recommendations in SMACNA's "Architectural Sheet Metal Manual" that apply to design, dimensions, geometry, metal thickness, and other characteristics of item indicated. Fabricate items at the shop to greatest extent possible.
 - 1. Obtain field measurements for accurate fit before shop fabrication.
 - 2. Form flashing and sheet metal without excessive oil canning, buckling, and tool marks and true to line and levels indicated, with exposed edges folded back to form hems.

RG+
22-1545

FABRICATED ENGINEERED STRUCTURES
13 34 00 - 6

HENDERSON SCHOOL LUNCH SHELTER
LODI, CALIFORNIA
ADDENDUM 1
21 JANUARY 26

3. Conceal fasteners and expansion provisions where possible. Exposed fasteners are not allowed on faces exposed to view.
- B. Sealed Joints: Form nonexpansion but movable joints in metal to accommodate elastomeric sealant.
- C. Expansion Provisions: Where lapped expansion provisions cannot be used, form expansion joints of intermeshing hooked flanges, not less than 1 inch deep, filled with butyl sealant concealed within joints.
- D. Fabricate cleats and attachment devices from same material as accessory being anchored or from compatible, noncorrosive metal.
- E. Hanging Gutters: Fabricate to cross section indicated, complete with end pieces, outlet tubes, and other accessories as required. Fabricate in minimum 96-inch- long sections. Furnish flat-stock gutter spacers and gutter brackets fabricated from same metal as gutters, of size recommended by SMACNA but not less than twice the gutter thickness. Fabricate expansion joints, expansion-joint covers, and gutter accessories from same metal as gutters.
 1. Expansion Joints: Butt type with cover plate.
 2. Gutters: Fabricate from one of the following materials:
 - a. Galvanized Steel: 0.022 inch thick.
 - b. Aluminum-Zinc Alloy-Coated Steel: 0.022 inch thick.

2.12 PIPE DOWNSPOUTS

- A. Downspouts: Fabricate round downspouts complete with mitered elbows. Furnish with metal hangers, from same material as downspouts, and anchors. Hot dip galvanize after fabrication.
 1. Fabricate from the following material:
 - a. Schedule 40 steel pipe.
 2. Finish: Manufacturer's standard powder coat finish.

2.13 SOURCE QUALITY CONTROL

- A. Special Inspection: Owner will engage a qualified special inspector to perform source quality control inspections and to submit reports.

PART 3 - EXECUTION

3.01 EXAMINATION

- A. Examine substrates, areas, and conditions, with erector present, for compliance with requirements for installation tolerances and other conditions affecting performance of the Work.

RG+
22-1545

FABRICATED ENGINEERED STRUCTURES
13 34 00 - 7

HENDERSON SCHOOL LUNCH SHELTER
LODI, CALIFORNIA
ADDENDUM 1
21 JANUARY 26

- B. Before erection proceeds, survey elevations and locations of concrete- and masonry-bearing surfaces and locations of anchor rods, bearing plates, and other embedments to receive structural framing, with erector present, for compliance with requirements and manufacturer's tolerances.
- C. Proceed with erection only after unsatisfactory conditions have been corrected.

3.02 PREPARATION

- A. Provide temporary shores, guys, braces, and other supports during erection to keep structural framing secure, plumb, and in alignment against temporary construction loads and loads equal in intensity to design loads. Remove temporary supports when permanent structural framing, connections, and bracing are in place unless otherwise indicated.

3.03 CONCRETE

- A. Design, construct, erect, brace, and maintain formwork according to ACI 301 (ACI 301M).
- B. Place and secure anchorage devices and other embedded items required for adjoining work attached to or supported by cast-in-place concrete. Use setting drawings, templates, diagrams, instructions, and directions furnished with items to be embedded.
- C. Comply with CRSI's "Manual of Standard Practice" for fabricating, placing, and supporting reinforcement.
- D. Comply with ACI 301 (ACI 301M) for placing concrete.
- E. Consolidate concrete with mechanical vibrating equipment.
- F. Finish: Provide smooth, as-cast concrete finish imparted by form-facing material, arranged in an orderly and symmetrical manner with a minimum of seams. Repair and patch holes and defective areas. Remove fins and other projections exceeding 1/8 inch (3 mm).

3.04 ERECTION OF STRUCTURAL FRAMING

- A. Erect fabricated engineered structures according to manufacturer's written instructions and drawings.
- B. Do not field cut, drill, or alter structural members without written approval from system manufacturer's professional engineer.

RG+
22-1545

FABRICATED ENGINEERED STRUCTURES
13 34 00 - 8

HENDERSON SCHOOL LUNCH SHELTER
LODI, CALIFORNIA
ADDENDUM 1
21 JANUARY 26

- C. Set structural framing accurately in locations and to elevations indicated, according to AISC specifications referenced in this Section. Maintain structural stability of frame during erection.
- D. Align and adjust structural framing before permanently fastening. Before assembly, clean bearing surfaces and other surfaces that will be in permanent contact with framing. Perform necessary adjustments to compensate for discrepancies in elevations and alignment.
 - 1. Level and plumb individual members of structure.
- E. Secondary Framing: Erect framing level, plumb, rigid, secure, and true to line.
- F. Bracing: Install bracing where indicated on erection drawings.
- G. Erection Tolerances: Maintain erection tolerances of structural framing within AISC 303.

3.05 METAL ROOF PANEL INSTALLATION

- A. Standing Seam Metal Panels: Fasten metal panels to supports with fasteners at each lapped joint at location and spacing recommended by manufacturer.
 - 1. Lap ribbed or fluted sheets one full rib. Apply panels and associated items true to line for neat and weathertight enclosure.
 - 2. Provide metal-backed washers under heads of exposed fasteners bearing on weather side of metal panels.
 - 3. Locate and space exposed fasteners in uniform vertical and horizontal alignment. Use proper tools to obtain controlled uniform compression for positive seal without rupture of washer.
 - 4. Install screw fasteners with power tools having controlled torque adjusted to compress washer tightly without damage to washer, screw threads, or panels. Install screws in predrilled holes.
 - 5. Flash and seal panels with weather closures at perimeter of all openings.

3.06 SHEET METAL FLASHING INSTALLATION

- A. General: Anchor flashing and sheet metal and other components of the Work securely in place, with provisions for thermal and structural movement so that completed flashing and sheet metal shall not rattle, leak, or loosen, and shall remain watertight. Use fasteners, solder, welding rods, protective coatings, separators, sealants, and other miscellaneous items as required to complete flashing and sheet metal system.
 - 1. Install flashing and sheet metal true to line and levels indicated. Provide uniform, neat seams with minimum exposure of solder, welds, and sealant.
 - 2. Install flashing and sheet metal to fit substrates and to result in watertight performance. Verify shapes and dimensions of surfaces to be covered before fabricating sheet metal.

HENDERSON SCHOOL LUNCH SHELTER
LODI, CALIFORNIA
ADDENDUM 1
21 JANUARY 26

3. Install exposed flashing and sheet metal without excessive oil canning, buckling, and tool marks.
 4. Torch cutting of flashing and sheet metal is not permitted.
- B. Hanging Gutters: Join sections with riveted and soldered joints or with lapped joints sealed with sealant. Provide for thermal expansion. Attach gutters at eave or fascia to firmly anchored gutter brackets spaced not more than 36 inches apart. Provide end closures and seal watertight with sealant. Slope to downspouts.
1. Install gutter with expansion joints at locations indicated, but not exceeding, 50 feet apart. Install expansion-joint caps.
- C. Downspouts: Anchor pipe downspouts securely in place, with provisions for thermal and structural movement. Use fasteners, anchors, protective coatings, sealants, and other miscellaneous items as required to complete installation.
1. Install pipe downspouts level, plumb, true to line and elevation and without jogs in alignment.
 2. Install pipe downspouts to fit substrates and to result in watertight performance.

3.07 ACCESSORY INSTALLATION

- A. General: Install accessories with positive anchorage to building and provide for thermal expansion. Coordinate installation with other components.
1. Where dissimilar metals contact each other or corrosive substrates, protect against galvanic action by painting contact surfaces with corrosion-resistant coating, by applying rubberized-asphalt underlayment to each contact surface, or by other permanent separation as recommended by manufacturer.

3.08 FIELD QUALITY CONTROL

- A. Special Inspections: Owner will engage a qualified special inspector to perform field quality control special inspections and to submit reports.
- B. Concrete Tests: Perform according to ACI 301 (ACI 301M).
- C. Installation will be considered defective if it does not pass tests and inspections.
- D. Prepare test and inspection reports.

END OF SECTION

	MATERIAL	COLOR
M1	METAL	METAL FINISH COLOR 1 (1)
M2	METAL	METAL FINISH COLOR 2 (1)
M3	METAL	METAL PREFINISH COLOR 1(1)(3)
M4	METAL	METAL PREFINISH COLOR 2(1)(4)

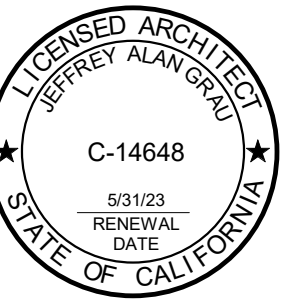
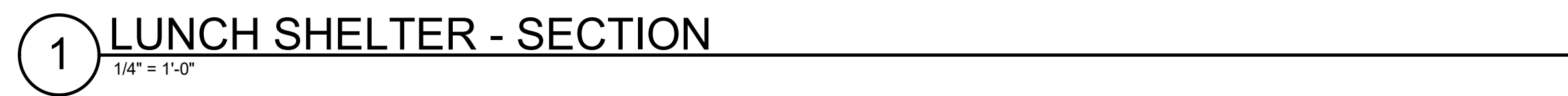
(1) CUSTOM COLORS TO BE SUBMITTED PER SPECIFICATIONS.

(2) CLEAN, PREP AND PAINT ALL SURFACES NOT PREFINISHED.

(3) MATCH METAL PAINT "COLOR 1" WITH COLOR DESIRED IN PREFINISHED "COLOR 1"

(4) MATCH METAL PAINT "COLOR 2" WITH COLOR DESIRED IN PREFINISHED "COLOR 2"

SN.01	HSS COLUMN
SN.02	DRILLED PIER FOOTING
SN.03	ROOF OVERHANG ABOVE
SN.04	HOLD TIGHT TO UNDERSIDE OF FRAMING; MAINTAIN 9'-0" CLEARANCE MIN.
SN.05	3" DIAMETER SCHEDULE 40 STEEL DOWNSPOUT, PAINTED
SN.06	PREFINISHED GUTTER AND FLASHING
SN.07	PAINT DECK UNDERSIDE
SN.08	PREFINISHED STANDING SEAM ROOF



LODI UNIFIED SCHOOL DISTRICT
LODI, CA

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PROJECT NO.	22-1545
DATE:	1/21/26
SHEET	

A1.1.2

STRUCTURAL SEPARATION		DEFLECTIONS ARE FOR (1) STRUCTURE			
ALL DEFLECTIONS SHOWN ALSO INCLUDE THE P-DELTA ROTATION PER IRC-7		SOIL CLASSES PER CBC TABLE 1806A.2			
MAXIMUM DRIFT	δ_{max}	SIDE COLUMNS	<u>Soil Class 5</u>	<u>Soil Class 4</u>	<u>Soil Class 3</u>
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[] 2.40	[] 2.55	[] 2.65
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[x] 2.15	[] 2.30	[] 2.40
40' WIDE (8' EAVE + T, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[] 2.20	[] 2.20	[] 2.30
MINIMUM SEPARATION	$(\delta_m = C_d \delta_{max})$	$C_d = 1.25$			
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[] 3.00	[] 3.19	[] 3.31
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[x] 2.69	[] 2.88	[] 3.00
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[] 2.75	[] 2.75	[] 2.88
MAXIMUM DRIFT	δ_{max}	END COLUMNS	<u>Soil Class 5</u>	<u>Soil Class 4</u>	<u>Soil Class 3</u>
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[] 2.40	[] 2.55	[] 2.65
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[x] 2.15	[] 2.30	[] 2.40
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[] 2.20	[] 2.20	[] 2.30
MINIMUM SEPARATION	$(\delta_m = C_d \delta_{max})$	$C_d = 1.25$			
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[] 3.00	[] 3.19	[] 3.31
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[x] 2.69	[] 2.88	[] 3.00
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)		[] 2.75	[] 2.75	[] 2.88

GENERAL:

1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL STATE AND FEDERAL REGULATIONS.
3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING WITH ANY WORK INVOLVED.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS, ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISIT TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION.
6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS, EXCEPT AS AMENDED BY CBC CHAPTER 35.
7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK.
10. THE SCHOOL DISTRICT INSPECTOR ON RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF INSTALLATION.
11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
13. MEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.

STRUCTURAL AND MISCELLANEOUS STEEL:

1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFICATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 KSI, GRADE B OR A501 UNLESS NOTED OTHERWISE.
3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A-500, GRADE B (OR C), Fy = 46 KSI MIN.
4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESS CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
5. ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A-36, Fy = 36 KSI.
6. ALL PLATE STEEL SHALL CONFORM TO ASTM A-572, Fy= 50 KSI.
7. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A-653, CS = TYPE B, Fy = 50 KSI Fu = 65 KSI
8. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
9. ALL ROOF DECKS SHALL HAVE KYNAR 500 METAL COATING.
10. ALL ROOF DECKS SHALL CONFORM TO ASTM A-792, Fy = 50 KSI.
11. ALL BASE CONNECTIONS ARE A PART OF THE LATERAL FORCE RESISTING SYSTEM

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-316(e) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS.
2. FOR THE SITE SPECIFIC PROJECT, J. R. MILLER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.
3. FOR THE SITE SPECIFIC PROJECT, J.R. MILLER & ASSOCIATES' RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC ONLY.
4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM J.R. MILLER & ASSOCIATES' RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT.
5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE BUT ARE NOT LIMITED TO, STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF OF THE VERIFIED REPORT FOR COMPLETED WORK.
6. J.R. MILLER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

CONSTRUCTION NOTES

1. A DSA-CERTIFIED CLASS 3 (MINIMUM) PROJECT INSPECTOR IS REQUIRED FOR THIS PROJECT.
2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.
3. A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.
5. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS ARE THAT ALL THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK, (SECTION 4-317(c), PART 1, TITLE 24, CCR)
6. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES

WELDING:

1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 FT-LB @ (0° F).
3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE PROPER MATERIAL ID AND WELDING.
4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFICATIONS.

BOLTING:

1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE HOT DIPPED GALVANIZED ASTM F3125 GRADE A325 HIGH STRENGTH BOLTS (UNO), WITH THE NUTS CONFORMING TO HOT DIPPED GALVANIZED ASTM A-563 GRADE DH.
2. HIGH STRENGTH BOLTS SHALL BE VERIFIED AND INSPECTED PER CBC 1705A2.1.
3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS -- INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
4. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F-436.
5. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6.
a) PRETENSIONED JOINTS MUST BE INSTALLED AND INSPECTED TO MEET ONE OF THE FOLLOWING REQUIREMENTS:
 1. TURN-OF-NUT PRETENSIONING: PER SECTION 8.2.1 OF THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, WASHERS ARE NOT REQUIRED FOR THIS METHOD, THE NUT OR HEAD SHALL BE ROTATED AS SPECIFIED IN TABLE 8.2. THE PART NOT TURNED SHALL BE PREVENTED FROM ROTATING.
 2. CALIBRATED WRENCH: PER THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, WASHERS ARE REQUIRED (NOT SUPPLIED BY ICON) THESE SHALL BE INSTALLED PER THE INSTALLATION TORQUE DETERMINED IN THE PRE-INSTALLATION VERIFICATION OF THE FASTENER ASSEMBLY PER SECTION 7. THE PART NOT TURNED SHALL BE PREVENTED FROM ROTATING.
 3. IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH "PT REQUIRED"
- b) ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF THE SNUG-TIGHTENED JOINTS. SNUG TIGHT CONDITION EXISTS WHEN ALL PILES IN A CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT BY THE BOLTS IN THE JOINT AND ALL OF THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

FOUNDATIONS:

1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER CBC TABLE 1806A, UNLESS NOTED OTHERWISE. PASSIVE PRESSURE IS ASSUMED TO START 12" BELOW TOP OF FOOTING.
2. PER CBC SECTION 1803A.2, GEOTECHNICAL REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONES OR SIEISMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS. ALLOWABLE FOUNDATION AND LATERAL SOIL PRESSURE VALUES MAY BE DETERMINED FROM TABLE 1806A.2.
3. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D-1557 OR AS RECOMMENDED BY THE GEO-TECH ENGINEER. FLOODING NOT PERMITTED.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORINGS, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
5. MINIMUM SETBACK FROM TOE OF SLOPE ON AN ASCENDING SLOPE SHALL BE 15 FEET AND MINIMUM SETBACK FROM TOE OF SLOPE ON A DESCENDING SLOPE SHALL BE 40 FEET
6. PER CBC SECTION 1803A.6, GEOHAZARD REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONES OR SIEISMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS.
7. GEOHAZRD REPORTS ARE TO COMPLY WITH DSA IR A-4 PER IR-7 SECTION 1.8
8. SITE SPECIFIC GEOTECHNICAL REPORT IS REQUIRED AT THE TIME OF SITE APPLICATION IF USING OTHER THAN CLASS 5 SOIL, PER DSA IR PC-7
9. LATERAL BEARING HAS BEEN INCREASED PER CBC 1806A.3.4 FOR THE 1/2" DEFLECTION & HAS BEEN DESIGNED FOR P-DELTA EFFECTS. NO 1/3 INCREASE HAS BEEN APPLIED.
10. MINIMUM CLEARANCE BETWEEN PIERS SHALL BE 8'-0".

CONCRETE:

1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

STRENGTH P _c (28 DAYS)	W/C RATIO (NON-AIR ENTRAINMENT)	W/C RATIO (AIR ENTRAINMENT)	SUMP (±")	UNIT WEIGHT (NORMAL WEIGHT)
5000 PSI	0.44	0.35	3"	150 PCF

2. CONCRETE MIX DESIGN PARAMETERS ARE GOOD FOR EXPOSURE CATEGORIES F0, F1 & F2. THE AIR ENTRAINMENT FOR THESE CATEGORIES SHALL BE AS FOLLOWS: F0-0, F1-4.5, F2-6
3. CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA.
4. AGGREGATES SHALL CONFORM TO THE ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.005. MAX AGGREGATE SIZE = 1".
5. CEMENT SHALL CONFORM TO ASTM C-150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS.
6. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT. ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED.
7. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
8. CONCRETE DURABILITY SHALL BE PER CBC 1904A.1 ACI 318-19, CHAPTER 19.
9. CONCRETE SHALL BE TESTED PER CBC 1903A, TABLE 1705A.3. AND ACI 318-19, SECTION 26.12.
10. NO ADMXTURE SHALL CONTAIN CALCIUM CHLORIDE.

REINFORCING STEEL:

1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A-615, AS FOLLOWS:
GR 60: (#4 BARS AND LARGER)
GR 40: (#3 BARS)
2. DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
A. CAST AGAINST EARTH 3"
B. CAST AGAINST FORM BELOW GRADE 2"
C. FORMED SLABS (#11 BAR & SMALLER).....3/4"
D. SLABS ON GRADE (FROM TOP OF SLAB) 1"
4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD.
5. REINFORCING SHALL BE LAP SPliced PER ACI 318-19, SECTION 25.5.
6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
7. WELDING OF REINFORCING IS NOT ALLOWED.
8. REINFORCING STEEL SHALL BE INSPECTED PER CBC 1705A.3.

POWDER-COAT FINISH SYSTEM:

ALL BUILDINGS THAT HAVE A POWDER-COATED FINISH SHALL MEET THE FOLLOWING SPECIFICATIONS:

1. THE STEEL FRAME (HSS SECTIONS, COLD FORMED & PLATE STEEL) SHALL BE SHOT-BLASTED TO A NEAR WHITE CONDITION PER SSPC-10 SPECIFICATIONS.
2. THE STEEL SHALL BE WASHED IN A ZINC PHOSPHATE IN AN MINIMUM THREE STAGE ELECTRO DEPOSITION PRE-TREATEMENT PROCESS.
3. IMMEDIATELY FOLLOWING PRE-TREATMENT THE STEEL SHALL BE TOTALLY COATED IN AN EPOXY PRIMER TO A UNIFORM THICKNESS OF A MINIMUM OF 0.7 TO 0.9 MILS. THE E-COATING SHALL PROVIDE A MINIMUM OF 1000 HOURS OF SALT SPRAY CORROSION PROTECTION TO THE STEEL.
4. THE STEEL SHALL THEN HAVE A TGC POLYESTER COLOR COAT APPLIED OVER THE E-COATED SURFACE.
5. THE FINISH THICKNESS OF THESE APPLICATIONS SHALL BE A MINIMUM OF 8 TO 12 MILS.
6. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, & COLD FORMED STEEL ETC.) NOT POWDER-COATED SHALL BE PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3{UNLESS NOTED OTHERWISE).

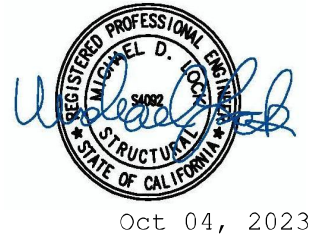
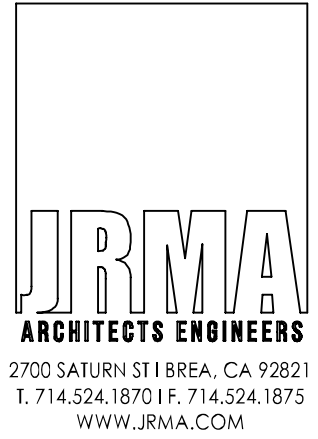
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DRAWN BY JD

DATE 7/25/2023

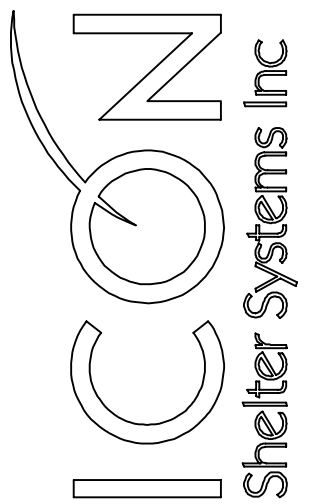
REV

REV DATE



APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122375-PC
REVIEWED FOR
SS ☒ FL ☒ ACS ☒ CG ☐
DATE 10/10/2023

GENERAL INFO



DISTINCTIVE STEEL SHELTERS
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1455 LINCOLN AVE
HOLLAND MI, 49423

616.396.0919
800.748.0985
616.396.0944 FX

PRE-CHECK (PC) DOCUMENT
Code: 2022 CBC
A separate project application for construction is required.

LS1.1
AD 1.03

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS, 2022 CBC

Application Number: 04-122188 DSA File Number:	School Name: PC Update Increment Number:	School District: PC Update Date Created: 2023-04-19 08:36:32
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2022 CBC

IMPORTANT: This form is only a summary list of structural tests and some of the special inspections required for the project. Generally, the structural tests and special inspections noted on this form are those that will be performed by the Geotechnical Engineer of Record, Laboratory of Record, or Special Inspector. The actual complete test and inspection program must be performed as detailed on the DSA approved documents. The appendix at the bottom of this form identifies work NOT subject to DSA requirements for special inspection or structural testing. The project inspector is responsible for providing inspection of all facets of construction, including but not limited to, special inspections not listed on this form such as structural wood framing, high-load wood diaphragms, cold-formed steel framing, anchorage of non-structural components, etc., per Title 24, Part 2, Chapter 17A (2022 CBC).

****NOTE:** Undefined section and table references found in this document are from the CBC, or California Building Code.

KEY TO COLUMNS

1. TYPE	2. PERFORMED BY
Continuous – Indicates that a continuous special inspection is required	GE (Geotechnical Engineer) – Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative. LOR (Laboratory of Record) – Indicates that the test or special inspection shall be performed by a testing laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program. See CAC Section 4-335.
Periodic – Indicates that a periodic special inspection is required	PI (Project Inspector) – Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA.
Test – Indicates that a test is required	SI (Special Inspection) – Indicates that the special inspection shall be performed by an appropriately qualified/approved special inspector.

DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022)	DEPARTMENT OF GENERAL SERVICES Page 1 of 19	STATE OF CALIFORNIA
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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2022 CBC

Application Number: 04-122188 DSA File Number:	School Name: PC Update Increment Number:	School District: PC Update Date Created: 2023-04-19 08:36:32
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Test or Special Inspection	Type	Performed By	Code References and Note
☐ b. Verify pier locations, diameters, plumbness, bell diameters (if applicable), lengths and embedment into bedrock (if applicable); record concrete or grout volumes.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) for exemptions.)
☐ c. Confirm adequate end strata bearing capacity.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) for exemptions.)
☐ d. Concrete piers.	Provide tests and inspections per CONCRETE section below.		

S5. RETAINING WALLS:				
	Test or Special Inspection	Type	Performed By	Code References and Notes
☐	a. Placement, compaction and inspection of backfill.	Continuous	GE*	1705A.6.1. * By geotechnical engineer or his or her qualified representative. (See section S2 above).
☐	b. Placement of soil reinforcement and/or drainage devices.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative.
☐	c. Segmental retaining walls; inspect placement of units, dowels, connectors, etc.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. See DSA IR 18-2.
☐	d. Concrete retaining walls.	Provide tests and inspections per CONCRETE section below.		
☐	e. Masonry retaining walls.	Provide tests and inspections per MASONRY section below.		

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (CONCRETE), 2022 CBC

Application Number: 04-122188 DSA File Number:	School Name: PC Update Increment Number:	School District: PC Update Date Created: 2023-04-19 08:36:32
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Test or Special Inspection	Type	Performed By	Code References and Notes
☐ c. Verify in-situ concrete strength prior to stressing of post-tensioning tendons.	Periodic	SI	Table 1705A.3 Item 13. Special inspector to verify specified concrete strength test prior to stressing.
☐ d. Inspect application of post-tensioning or prestressing forces and grouting of bonded prestressing tendons.	Continuous	SI	1705A.3.4, Table 1705A.3 Item 9; ACI 318-14 Section 26.13

C3. PRECAST CONCRETE (IN ADDITION TO SECTION C1):				
Test or Special Inspection		Type	Performed By	Code References and Notes
☐	a. Inspect fabrication of precast concrete members.	Continuous	SI	ACI 318-19 Section 26.13.
☐	b. Inspect erection of precast concrete members.	Periodic	SI*	Table 1705A.3 Item 10. * May be performed by PI when specifically approved by DSA.
☐	c. For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high deformability elements (MDE or HDE) in structures assigned to Seismic Design Category D, E or F, inspect such connections and reinforcement in the field for: 1. Installation of the embedded parts 2. Completion of the continuity of reinforcement across joints. 3. Completion of connections in the field.	Continuous	SI	Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5
☐	d. Inspect installation tolerances of precast concrete diaphragm connections for compliance with ACI 550.5.	Periodic	SI	Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5

DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022)	DEPARTMENT OF GENERAL SERVICES Page 7 of 19	STATE OF CALIFORNIA
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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2022 CBC

Application Number: 04-122188 DSA File Number:	School Name: PC Update Increment Number:	School District: PC Update Date Created: 2023-04-19 08:36:32
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Geotechnical Reports: Project has a geotechnical report, or CDs indicate soils special inspection is required by GE

S1. GENERAL:			
Test or Special Inspection	Type	Performed By	Code References and Notes
☒ a. Verify that: • Site has been prepared properly prior to placement of controlled fill and/or excavations for foundations. • Foundation excavations are extended to proper depth and have reached proper material. • Materials below footings are adequate to achieve the design bearing capacity.	Periodic	GE*	* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) form for exemptions.)

S2. SOIL COMPACTION AND FILL:				
Test or Special Inspection		Type	Performed By	Code References and Notes
☐	a. Perform classification and testing of fill materials.	Test	LOR*	* Under the supervision of the geotechnical engineer.
☒	b. Verify use of proper materials, densities and inspect lift thicknesses, placement and compaction during placement of fill.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. (Refer to specific items identified in the Appendix (end of this form) form for exemptions where soils SI and testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form DSA 291 shall satisfy the soil SI and test reporting requirements for the exempt items.)
☒	c. Compaction testing.	Test	LOR*	* Under the supervision of the geotechnical engineer. (Refer to specific items identified in the Appendix (end of this form) for exemptions where soils testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form DSA 291 shall satisfy the soil test reporting requirements for the exempt items.)

DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022)	DEPARTMENT OF GENERAL SERVICES Page 2 of 19	STATE OF CALIFORNIA
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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2022 CBC

Application Number: 04-122188 DSA File Number:	School Name: PC Update Increment Number:	School District: PC Update Date Created: 2023-04-19 08:36:32
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S6. OTHER SOILS:				
	Test or Special Inspection	Type	Performed By	Code References and Notes
☐	a. Soil Improvements	Test	GE*	Submit a comprehensive report documenting final soil improvements constructed, construction observation and the results of the confirmation testing and analysis to CGS (California Geological Survey) for final acceptance. * By geotechnical engineer or his or her qualified representative.
☐	b. Inspection of Soil Improvements	Continuous	GE*	* By geotechnical engineer or his or her qualified representative.
☐	c.			

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (CONCRETE), 2022 CBC

Application Number: 04-122188 DSA File Number:	School Name: PC Update Increment Number:	School District: PC Update Date Created: 2023-04-19 08:36:32
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C4. SHOTCRETE (IN ADDITION TO SECTION C1):			
Test or Special Inspection	Type	Performed By	Code References and Notes
☐ a. Inspect shotcrete placement for proper application techniques.	Continuous	SI	1705A.3.9, Table 1705A.3 Item 7, 1908A.1, 1908A.2, 1908A.3. See ACI 506.2-13 Section 3.4, ACI 506R-16.
☐ b. Sample and test shotcrete (f _c).	Test	LOR	1908A.2, 1705A.3.9

C5. POST-INSTALLED ANCHORS:			
Test or Special Inspection	Type	Performed By	Code References and Notes
☐ a. Inspect installation of post-installed anchors	See Notes	SI*	1617A.1.19, Table 1705A.3 Item 4a (Continuous) & 4b (Periodic), 1705A.3.8 (See Appendix (end of this form) for exemptions). ACI 318-14 Sections 17.8 & 26.13. * May be performed by the project inspector when specifically approved by DSA.
☐ b. Test post-installed anchors.	Test	LOR	1910A.5. (See Appendix (end of this form) for exemptions.)

C6. OTHER CONCRETE:			
Test or Special Inspection	Type	Performed By	Code References and Notes
☐ a.			

DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022)	DEPARTMENT OF GENERAL SERVICES Page 8 of 19	STATE OF CALIFORNIA
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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2022 CBC

Application Number: 04-122188 DSA File Number:	School Name: PC Update Increment Number:	School District: PC Update Date Created: 2023-04-19 08:36:32
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S3. DRIVEN DEEP FOUNDATIONS (PILES):			
Test or Special Inspection	Type	Performed By	Code References and Notes
☐ a. Verify pile materials, sizes and lengths comply with the requirements.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative.
☐ b. Determine capacities of test piles and conduct additional load tests as required.	Test	LOR*	* Under the supervision of the geotechnical engineer.
☐ c. Inspect driving operations and maintain complete and accurate records for each pile.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative.
☐ d. Verify locations of piles and their plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and record any pile damage.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative.
☐ e. Steel piles.	Provide tests and inspections per STEEL section below.		
☐ f. Concrete piles and concrete filled piles.	Provide tests and inspections per CONCRETE section below.		
☐ g. For specialty piles, perform additional inspections as determined by the registered design professional in responsible charge.	*	*	* As defined on drawings or specifications.

S4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS):			
Test or Special Inspection	Type	Performed By	Code References and Note
☐ a. Inspect drilling operations and maintain complete and accurate records for each pier.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) for exemptions.)

DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022)	DEPARTMENT OF GENERAL SERVICES Page 3 of 19	STATE OF CALIFORNIA
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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (CONCRETE), 2022 CBC

Application Number: 04-122188 DSA File Number:	School Name: PC Update Increment Number:	School District: PC Update Date Created: 2023-04-19 08:36:32
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C1. CAST-IN-PLACE CONCRETE				
Test or Special Inspection		Type	Performed By	Code References and Notes
☒	a. Verify use of required design mix.	Periodic	SI	Table 1705A.3 Item 5, 1910A.1.
☒	b. Identify, sample, and test reinforcing steel.	Test	LOR	1910A.2; ACI 318-19 Ch.20 and Section 26.6.1.2; DSA IR 17-10. (See Appendix (end of this form) for exemptions.)
☒	c. During concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	Test	LOR	Table 1705A.3 Item 6; ACI 318-19 Sections 26.5 & 26.12.
☒	d. Test concrete (f _c).	Test	LOR	1905A.1.17; ACI 318-19 Section 26.12.
☐	e. Batch plant inspection:	See Notes	SI	Default of 'Continuous' per 1705A.3.3. If approved by DSA, batch plant inspection may be reduced to 'Periodic' subject to requirements in Section 1705A.3.3.1, or eliminated per 1705A.3.3.2. See IR 17-13. (See Appendix (end of this form) for exemptions.)
☐	f. Welding of reinforcing steel.	Provide special inspection per STEEL, Category S/A4(d) & (e) and/or S/A5(g) & (h) below.		

C2. PRESTRESSED / POST-TENSIONED CONCRETE (IN ADDITION TO SECTION C1):				
	Test or Special Inspection	Type	Performed By	Code References and Notes
☐	a. Sample and test prestressing tendons and anchorages.	Test	LOR	1705A.3.4, 1910A.3
☐	b. Inspect placement of prestressing tendons.	Periodic	SI	1705A.3.4, Table 1705.3 Items 1 & 9.

DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022)	DEPARTMENT OF GENERAL SERVICES Page 6 of 19	STATE OF CALIFORNIA
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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMINUM), 2022 CBC

Application Number: 04-122188 DSA File Number:	School Name: PC Update Increment Number:	School District: PC Update Date Created: 2023-04-19 08:36:32
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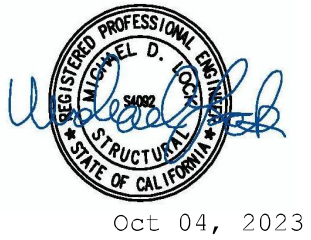
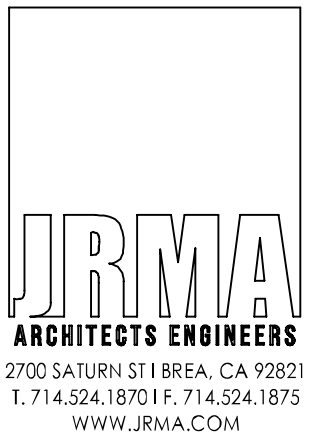
S/A1. STRUCTURAL STEEL, COLD-FORMED STEEL AND ALUMINUM USED FOR STRUCTURAL PURPOSES			
Test or Special Inspection	Type	Performed By	Code References and Notes
☒ a. Verify identification of all materials and: • Mill certificates indicate material properties that comply with requirements. • Material sizes, types and grades comply with requirements.	Periodic	SI	Table 1705A.2.1 Items 3a–3c, 2202A.1; AISI S100-20 Section A3.1 & A3.2; AISI S240-20 Section A3 & A5; AISI S220-20 Sections A4 & A6. * By special inspector or qualified technician when performed off-site.
☒ b. Test unidentified materials	Test	LOR	2202A.1.
☒ c. Examine seam welds of HSS shapes	Periodic	SI	DSA IR 17-3.
☒ d. Verify and document steel fabrication per DSA-approved construction documents.	Periodic	SI	Not applicable to cold-formed steel light-frame construction, except for trusses (1705A.2.4).
☐ e. Buckling restrained braces.	Test	LOR	Testing and special inspections in accordance with IR 22-4.

S/A2. HIGH-STRENGTH BOLTS:				
	Test or Special Inspection	Type	Performed By	Code References and Notes
☒	a. Verify identification markings and manufacturer's certificates of compliance conform to ASTM standards specified in the DSA-approved documents.	Periodic	SI	Table 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.
☒	b. Test high-strength bolts, nuts and washers.	Test	LOR	Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR 17-8.
☒	c. Bearing-type ("snug tight") connections.	Periodic	SI	Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Section 9.1; DSA IR 17-9.
☒	d. Pretensioned and slip-critical connections.	*	SI	Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. * "Continuous" or "Periodic" depends on the tightening method used.

DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022)	DEPARTMENT OF GENERAL SERVICES Page 9 of 19	STATE OF CALIFORNIA
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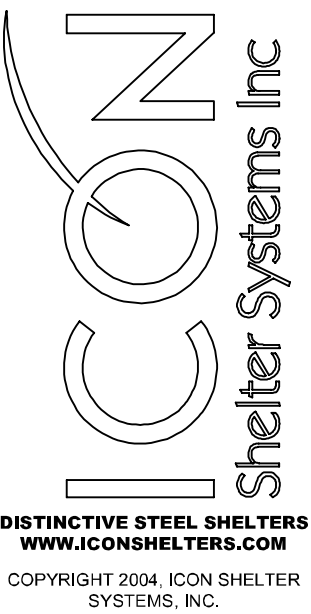
PRE-CHECK (PC) DOCUMENT
Code: 2022 CBC
A separate project application for construction is required.

ICON STD	RH/DSA-PC
DRAWN BY:	JD
DATE	7/25/2023
REV	
REV DATE	



APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122188 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 10/10/2023

DSA 103



616.396.0919
800.748.0985
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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

S/A3. WELDING:				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☒ a. Verify weld filler material identification markings per AWS designation listed on the DSA-approved documents and the WPS.	Periodic	SI	1705A.2.5, Table 1705A.2.1 Items 4 & 5; AWS D1.1 and AWS D1.8 for structural steel; AWS D1.2 for Aluminum; AWS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-3.	
☒ b. Verify weld filler material manufacturer's certificate of compliance.	Periodic	SI	DSA IR 17-3.	
☒ c. Verify WPS, welder qualifications and equipment.	Periodic	SI	DSA IR 17-3.	

S/A4. SHOP WELDING (IN ADDITION TO SECTION S/A3):				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☒ a. Inspect groove welds, multi-pass fillet welds, single pass fillet welds > 5/16", plug and slot welds.	Continuous	SI	Table 1705A.2.1 Items 5a.1-4; AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.	
☒ b. Inspect single-pass fillet welds ≤ 5/16", floor and roof deck welds.	Periodic	SI	1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6; AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.	
☐ c. Inspect welding of stairs and railing systems.	Periodic	SI	1705A.2.1; AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3.	
☐ d. Verification of reinforcing steel weldability other than ASTM A706.	Periodic	SI	1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.	
☐ e. Inspect welding of reinforcing steel.	Continuous	SI	Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3.	

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

Test or Special Inspection		Type	Performed By	Code References and Notes
S/A8. SPRAYED FIRE-RESISTANT MATERIALS:				
Test or Special Inspection		Type	Performed By	Code References and Notes
☐	a. Examine structural steel surface conditions, inspect application, take samples, measure thickness and verify compliance of all aspects of application with DSA-approved documents.	Periodic	SI	1705A.15, 1705A.1, 1705A.2, 1705A.3, 1705A.4.
☐	b. Test density.	Test	LOR	1705A.15.1, 1705A.15.5, ASTM E736
☐	c. Bond strength adhesion/cohesion.	Test	LOR	1705A.15.1, 1705A.15.4, ASTM E605

S/A9. ANCHOR BOLTS AND ANCHOR RODS:				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☒ a. Anchor Bolts and Anchor Rods	Test	LOR	Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-11.	
☐ b. Threaded rod not used for foundation anchorage.	Test	LOR	Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11.	

S/A10. STORAGE RACK SYSTEMS:				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☐ a. Materials used, to verify compliance with one or more of the material test reports in accordance with the approved construction documents.	Periodic	SI	Table 1705A.13.7	
☐ b. Fabricated storage rack elements.	Periodic	SI	1704A.2.5; Table 1705A.13.7	

Appendix: Work Exempt from DSA Requirements for Structural Tests / Special Inspections

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

Exempt items given in DSA IR A-22 or the 2019 CBC (including DSA amendments) and those items identified below with a check mark by the design professional are NOT subject to DSA requirements for the structural tests / special inspections noted. Items marked as exempt shall be identified on the approved construction documents. The project inspector shall verify all construction complies with the approved construction documents.

SOILS:	
☐	1. Deep foundations acting as a cantilever footing with a design based on minimum allowable pressures per CBC Table 1806A.2 and without a geotechnical report for the following cases: A) free standing sign or scoreboard, B) cell or antenna towers and poles less than 35'-0" tall (e.g., lighting poles, flag poles, poles supporting open mesh fences, etc.), C) single-story structure with dead load less than 5 psf (e.g., open fabric shade structure), or D) covered walkway structure with an apex height less than 10'-0" above adjacent grade.
☐	2. Shallow foundations, etc. are exempt from special inspections and testing by a Geotechnical Engineer for the following cases: A) buildings without a geotechnical report and meeting the exception item #1 criteria in CBC Section 1803A.2 supported by native soil (any excavation depth) or fill soil (not exceeding 12" depth per CBC Section 1804A.6), B) soil scarification/compaction not exceeding 12" depth, C) native or fill soil supporting exterior non-structural flatwork (e.g., sidewalks, site concrete ramps, site stairs, parking lots, driveways, etc.), D) unpaved landscaping and playground areas, or E) utility trench backfill.

CONCRETE/MASONRY:	
☐	1. Post-installed anchors for the following: A) exempt non-structural components (e.g., mechanical, electrical, plumbing equipment - see item 7 for "Welding" in the Appendix below given in CBC Section 1617A.1.18 (which replaces ASCE 7-16, Section 13.1.4) or B) interior nonstructural wall partitions meeting criteria listed in exempt item 3 for "Welding" in the Appendix below
☐	2. Concrete batch plant inspection is not required for items given in CBC Section 1705A.3.3.2 subject to the requirements and limitations in that section.
☐	3. Non-bearing non-shear masonry walls may be exempt from certain DSA masonry testing and special inspection items as allowed per DSA IR 21-1. Refer to construction documents for specific exemptions accordingly for each applicable wall condition.
☐	4. Epoxy shear dowels in site flatwork and/or other non-structural concrete.

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

Test or Special Inspection	Type	Performed By	Code References and Notes
S/A5. FIELD WELDING (IN ADDITION TO SECTION S/A3):			
Test or Special Inspection	Type	Performed By	Code References and Notes
☐ a. Inspect groove welds, multi-pass fillet welds, single pass fillet welds > 5/16", plug and slot welds.	Continuous	SI	Table 1705A.2.1 Items 5a.1–4; AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.
☐ b. Inspect single-pass fillet welds ≤ 5/16".	Periodic	SI	Table 1705A.2.1 Item 5a.5; AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.
☐ c. Inspect end-welded studs (ASTM A-108) installation (including bend test).	Periodic	SI	2213A.2; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1; DSA IR 17-3.
☐ d. Inspect floor and roof deck welds.	Periodic	SI	1705A.2.2, Table 1705A.2.1 Item 5a.6; AISC 360-16 (AISC 341-16 as applicable); AWS D1.3; DSA IR 17-3.
☐ e. Inspect welding of structural cold-formed steel.	Periodic	SI*	1705A.3.5; AWS D1.3; DSA IR 17-3. The quality control provisions of AISI S240-20 Chapter D shall also apply. * May be performed by the project inspector when specifically approved by DSA.
☐ f. Inspect welding of stairs and railing systems.	Periodic	SI*	1705A.2.1; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3. * May be performed by the project inspector when specifically approved by DSA.
☐ g. Verification of reinforcing steel weldability.	Periodic	SI	1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.
☐ h. Inspect welding of reinforcing steel.	Continuous	SI	Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3.

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

Test or Special Inspection	Type	Performed By	Code References and Notes	
☐ c. Storage rack anchorage installation.	Periodic	SI	ANSI/MHI16.1 Section 7.3.2; Table 1705A.13.7	
☐ d. Completed storage rack system to indicate compliance with the approved construction documents.	Periodic	SI*	Table 1705A.13.7; * May be performed by the project inspector when specifically approved by DSA.	

S/A11. Other Steel				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☐ a.				

Appendix: Work Exempt from DSA Requirements for Structural Tests / Special Inspections

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

CONCRETE/MASONRY:	
☐	5. Testing of reinforcing bars is not required for items given in CBC Section 1910A.2 subject to the requirements and limitations in that section.
WELDING:	
☐	1. Solid-clad and open-mesh fences, gates with maximum leaf span of 10', and gates with a maximum rolling section of 10' all having an apex height less than 8'-0" above lowest adjacent grade. When located above circulation or occupied space below, these gates/fences are not located within 1.5x gate/fence height (max 8'-0") to the edge of floor or roof.
☐	2. Handrails, guardrails, and modular or relocatable ramps associated with walking surfaces less than 30" above adjacent grade (excluding post base connections per the "Exception" language in Section 1705A.2.1); fillet welds shall not be ground flush.
☐	3. Non-structural interior cold-formed steel framing spanning less than 15'-0", such as in interior partitions, interior soffits, etc. supporting only self weight and light-weight finishes or adhered tile, masonry, stone, or terra cotta veneer no more than 5/8" thickness and apex less than 20'-0" in height and not over an exit way. Maximum tributary load to a member shall not exceed the equivalent of that occurring from a 10'x10' opening in a 15' tall wall for a header or king stud.
☐	4. Manufactured support frames and curbs using hot rolled or cold-formed steel (i.e., light gauge) for mechanical, electrical, or plumbing equipment weighing less than 2000# (equipment only) (connections of such frames to superstructure elements using welding will require special inspection as noted in selected item(s) for Sections S/A3, S/A4 and/or S/A5 of listing above).
☐	5. Manufactured components (e.g., Tolco, B-Line, Afcon, etc.) for mechanical, electrical, or plumbing hanger support and bracing (connections of such components to superstructure elements using welding will require special inspection as noted in selected item(s) for Sections S/A3, S/A4 and/or S/A5 of listing above).
☐	6. TV Brackets, projector mounts with a valid listing (see DSA IR A-5) and recreational equipment (e.g., playground structures, basketball backstops, etc.) (connections of such elements to superstructure elements using welding will require special inspection as noted in selected item(s) for sections S/A3, S/A4 and/or S/A5 located in the Steel/Aluminum category of listing above).
☐	7. Any support for exempt non-structural components given in CBC Section 1617A.1.18 (which replaces ASCE 7-16, Section 13.1.4) meeting the following: A) when supported on a floor/roof, <400# and resulting composite center of mass (including component's center of mass) ≤4' above supporting floor/roof, B) when hung from a wall or roof/floor, <20# for discrete units or <5 plf for distributed systems.

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

Test or Special Inspection		Type	Performed By	Code References and Notes
S/A6. NONDESTRUCTIVE TESTING:				
Test or Special Inspection		Type	Performed By	Code References and Notes
☐	a. Ultrasonic	Test	LOR	1705A.2.1, 1705A.2.5; AISC 341-16 J6.2, AISC 360-16 N5.5; AWS D1.1, AWS D1.8; DSA IR 17-2.
☐	b. Magnetic Particle	Test	LOR	1705A.2.1, 1705A.2.5; AISC 341-16 J6.2, AISC 360-16 N5.5; AWS D1.1, AWS D1.8; DSA IR 17-2.
☐	c.	Test	LOR	

S/A7. STEEL JOISTS AND TRUSSES:				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☐ a. Verify size, type and grade for all chord and web members as well as connectors and weld filler material, verify joist profile, dimensions and camber (if applicable); verify all weld locations, lengths and profiles; mark or tag each joist.	Continuous	SI	1705A.2.3, Table 1705A.2.3; AWS D1.1; DSA IR 22-3 for steel joists only. 1705A.2.4; AWS D1.3 for cold-formed steel trusses.	

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (OTHER), 2022 CBC

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

X1. OTHER:			
Test or Special Inspection	Type	Performed By	Code References and Notes
☐ a. Load test for identified product(s).	Test	LOR	1709A.2, 1709A.3. Testing is not required for: 1) a product with a valid evaluation service report per DSA IR A-5, or 2) a product that can be justified by structural calculation.
☐ b. Installation torque for non-HS bolts	Continuous	SI*	Applicable to communication towers identified as Essential Service Facility Projects (ESFP). Calibrated wrench use required, verified by SI during installation. DSA Policy PL 18-01: Communication Towers, Poles and Buildings Utilized by State Agencies for Essential Services Communications.*EXCEPTION: Non-ESFP may use PI without need for notification to DSA.
☐ c.			

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS(SIGNATURE), 2022 CBC

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

Name of Architect or Engineer in general responsible charge:

Name of Structural Engineer (When structural design has been delegated):

Signature of Architect or Structural Engineer: _____ Date: _____

Note: To facilitate DSA electronic mark-ups and identification stamp application, DSA recommends against using secured electronic or digital signatures.

DSA STAMP

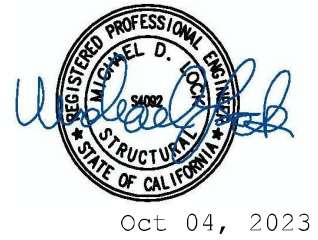
ICON STD
RH/DSA-PC

DRAWN BY: JD

DATE: 7/25/2023

REV

REV DATE

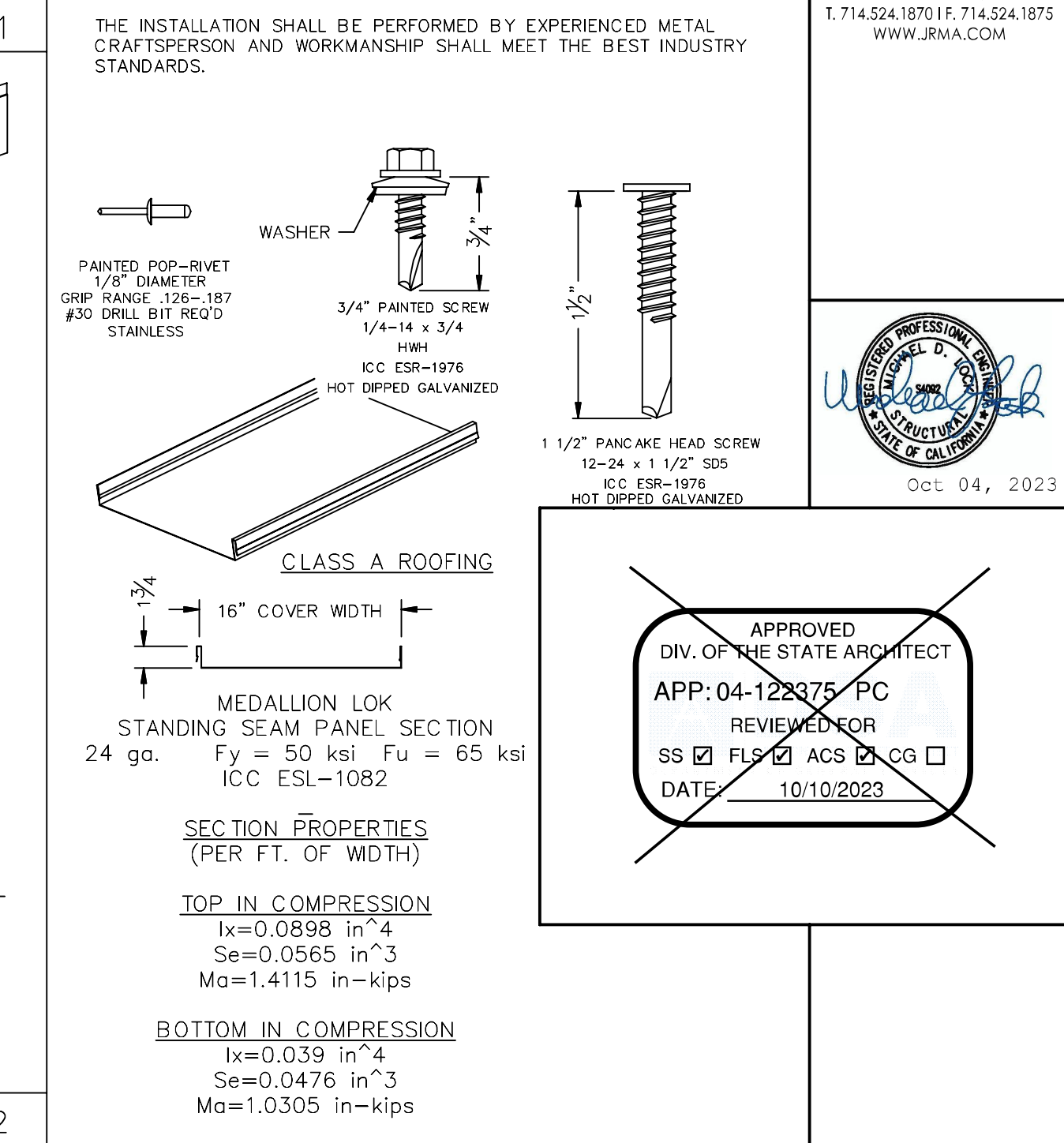
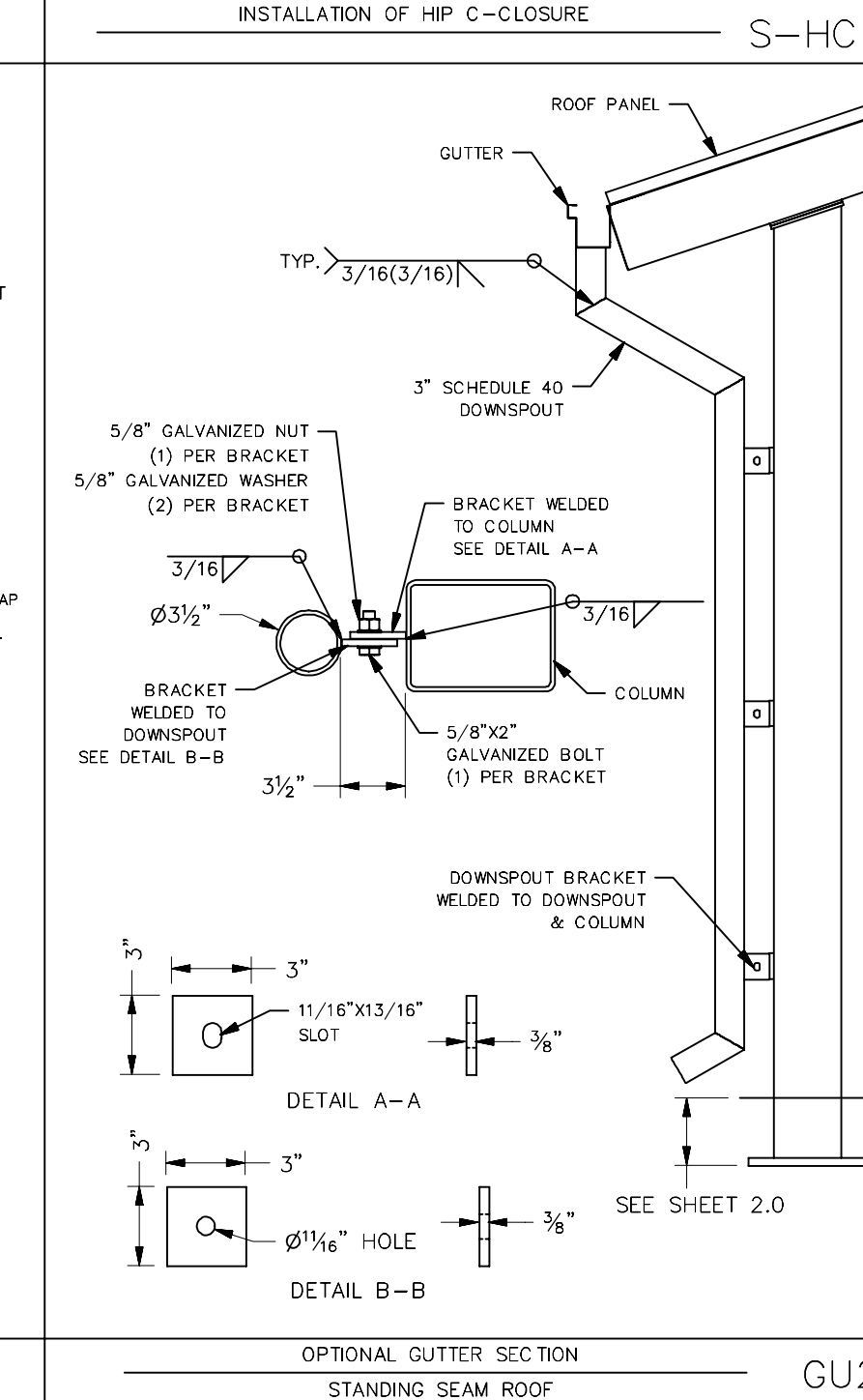
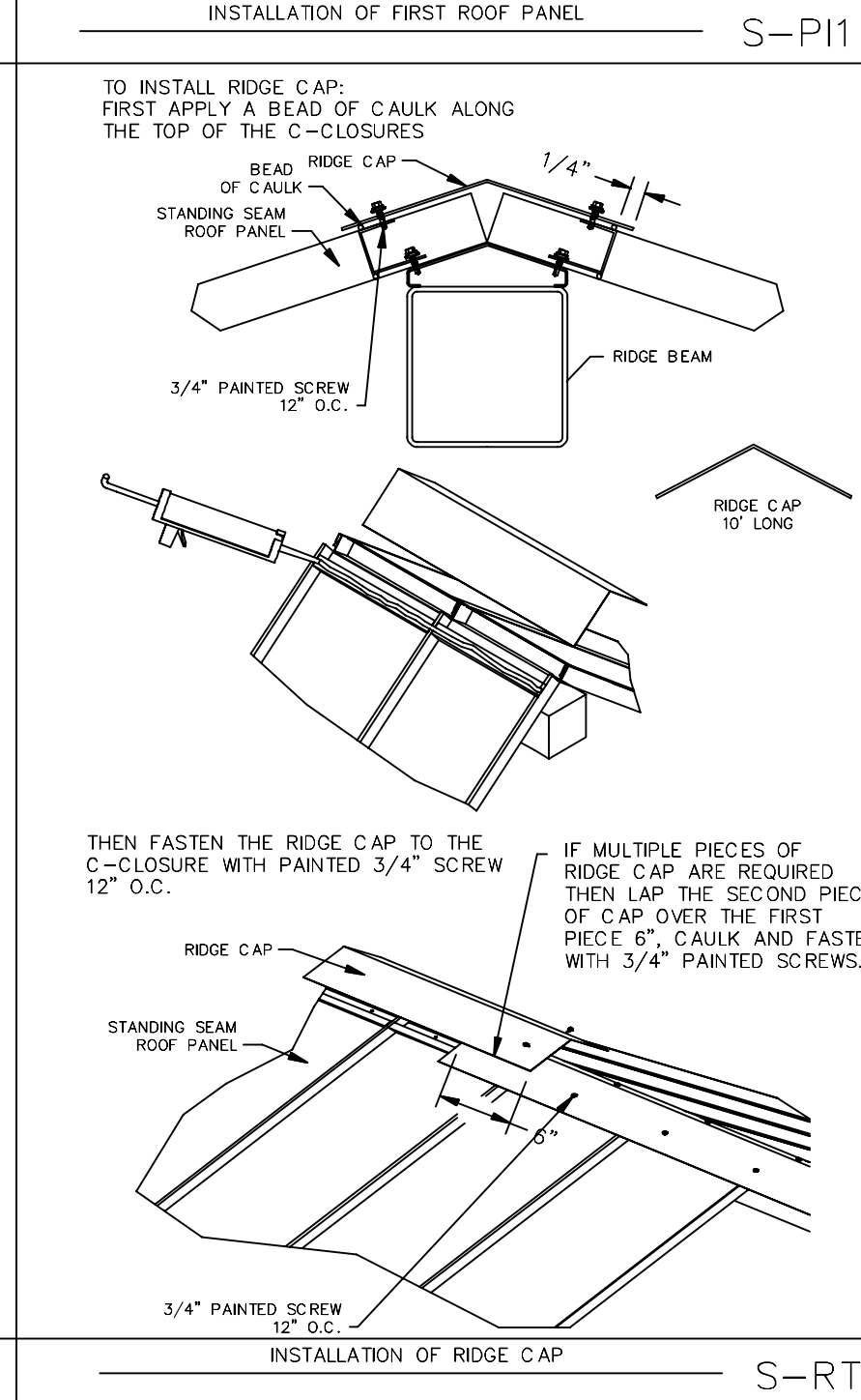
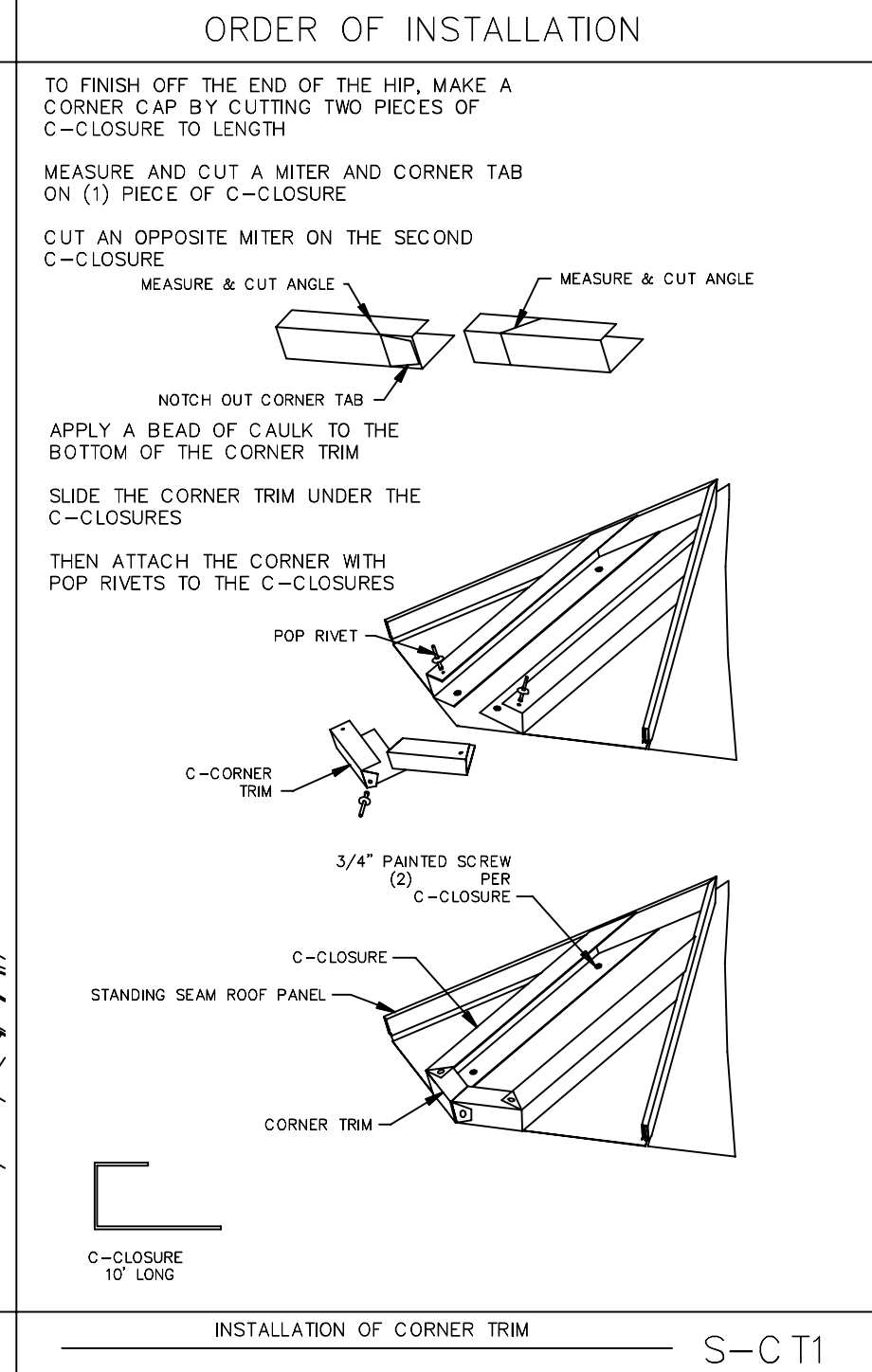
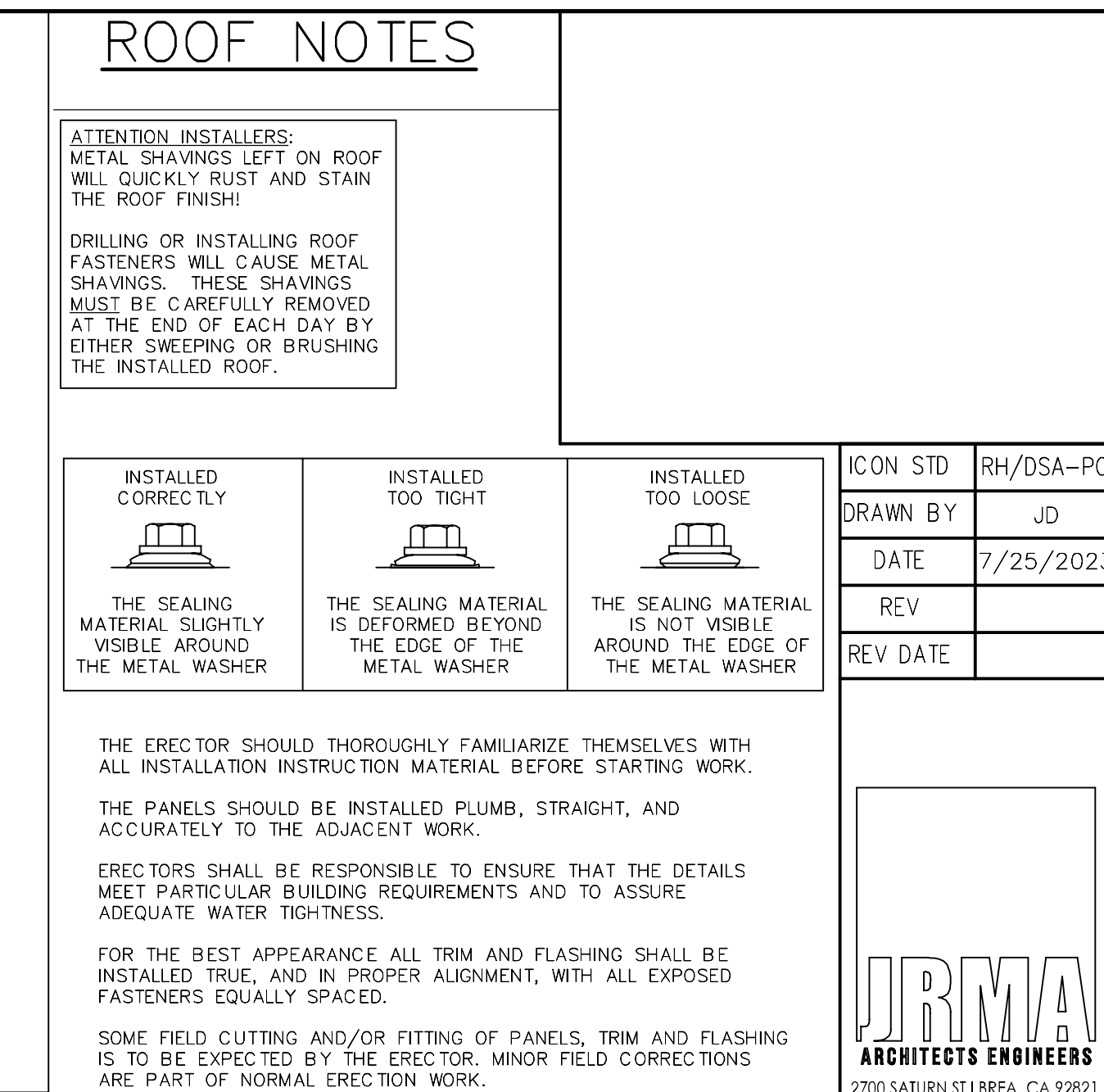
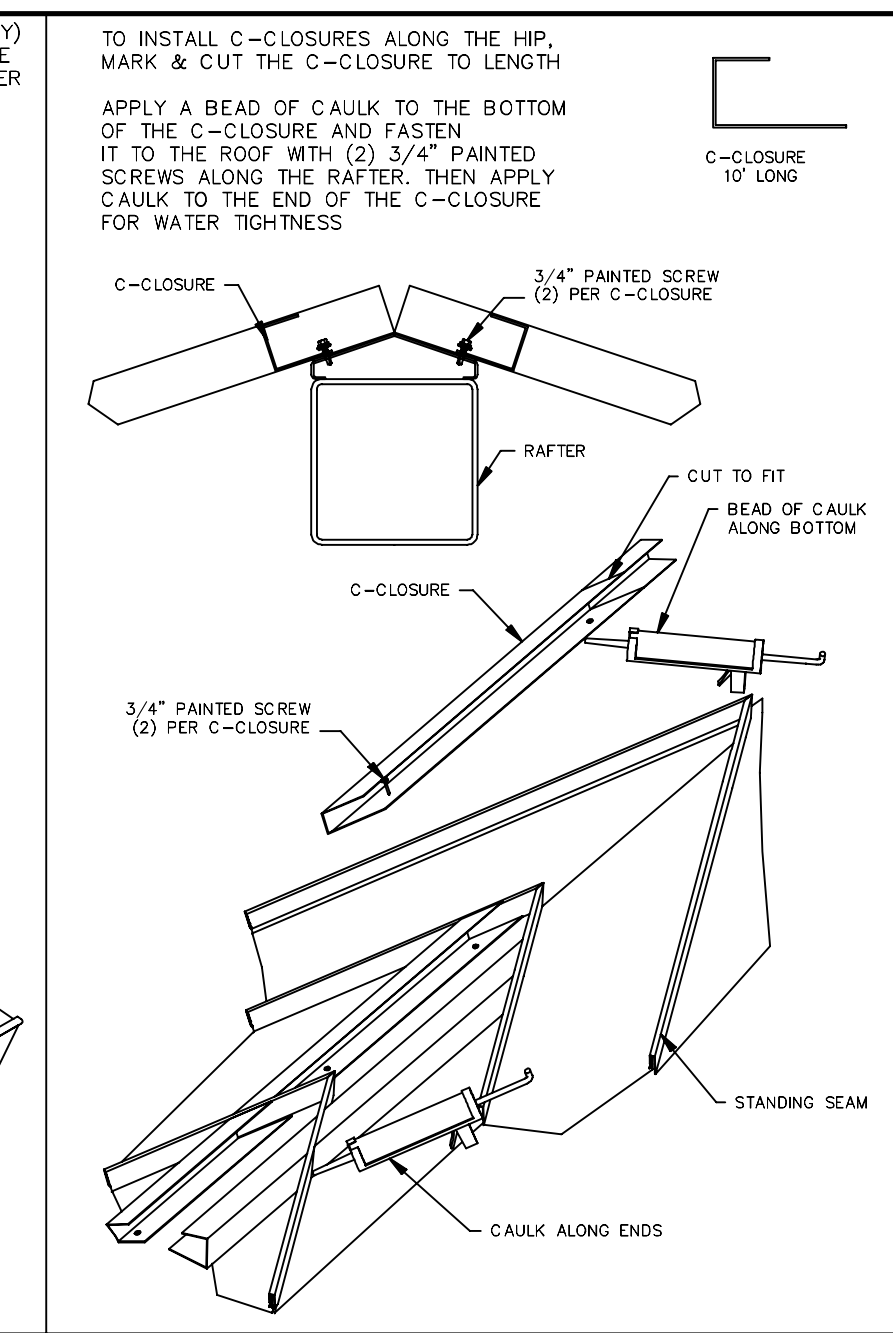
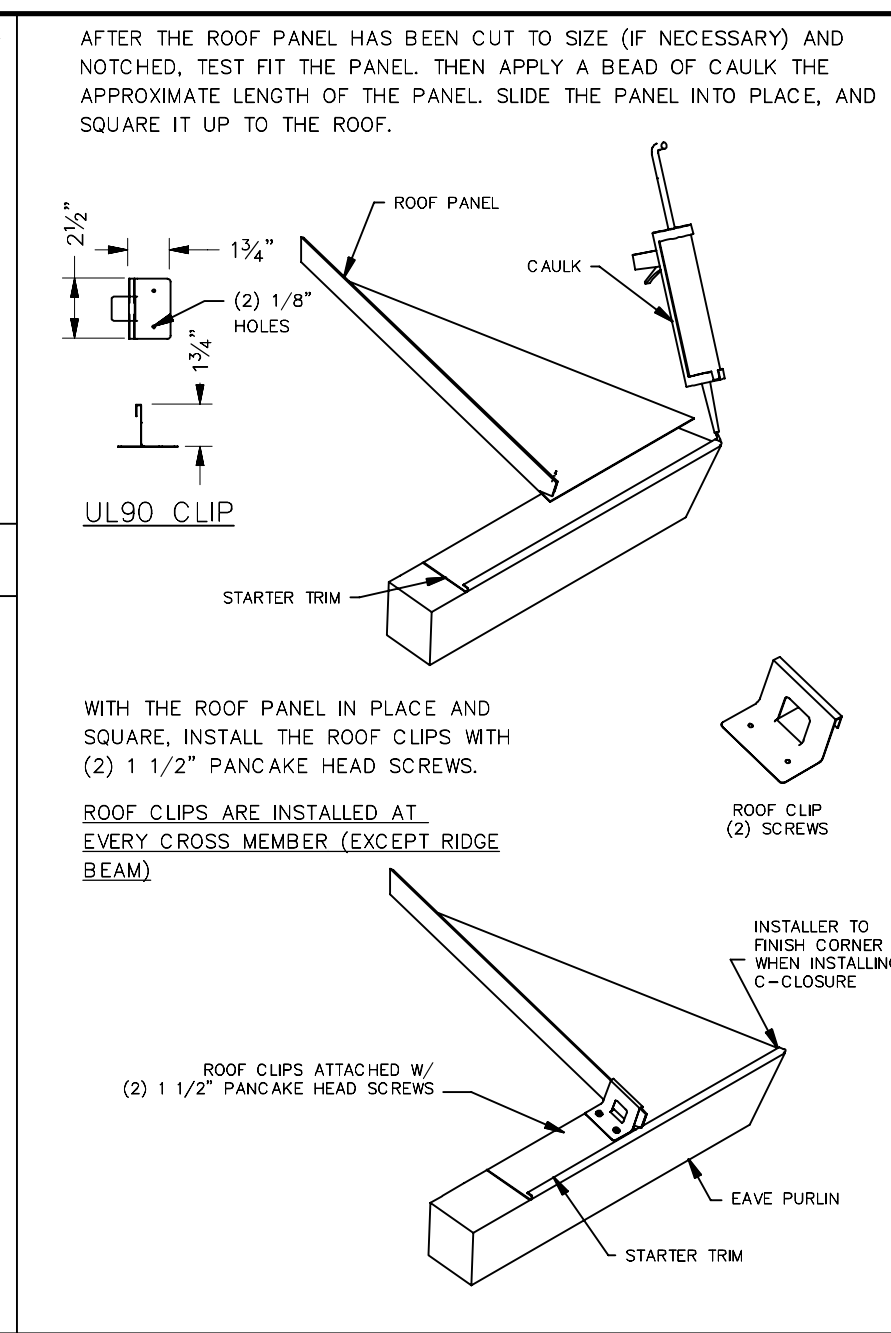
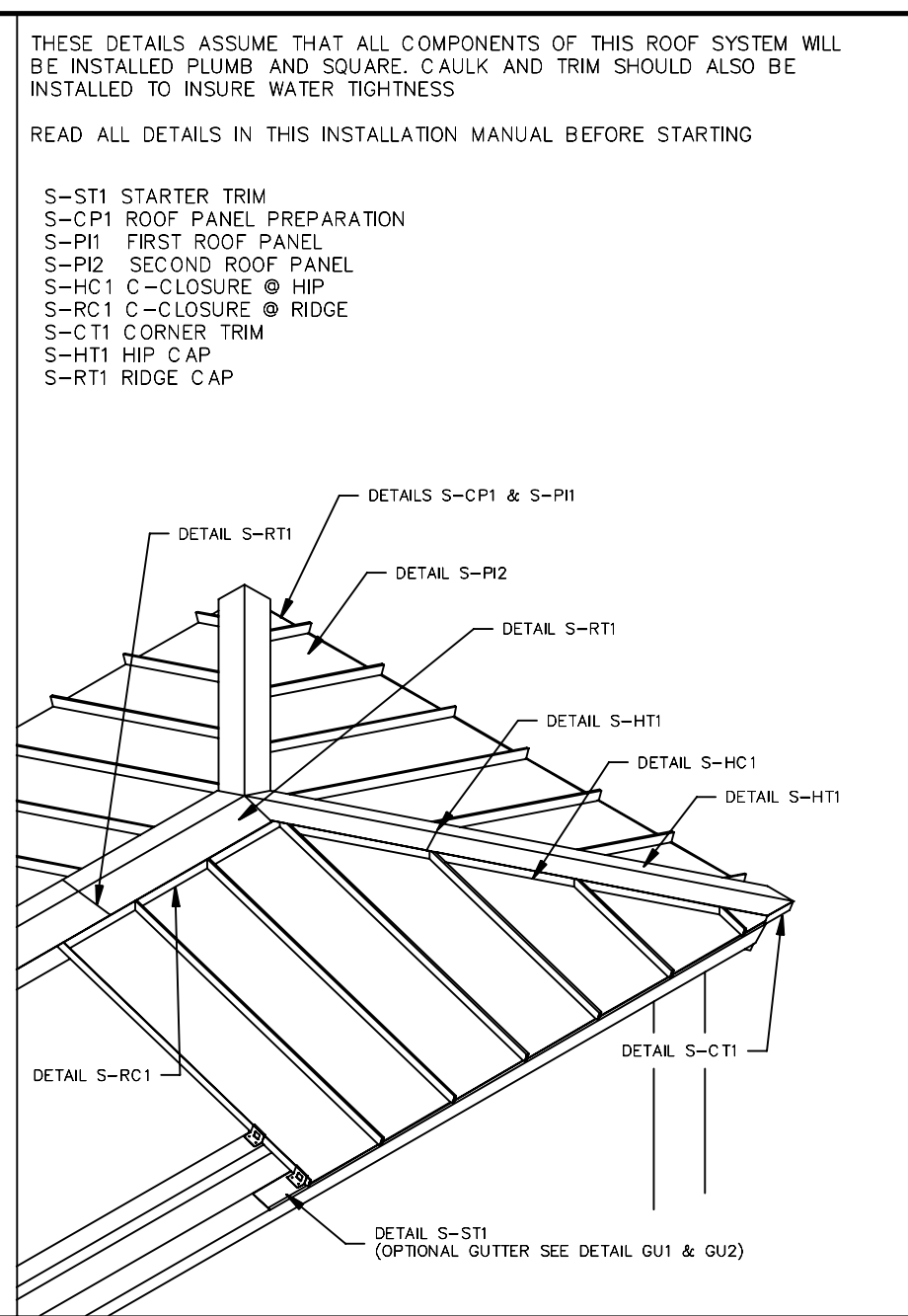


APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122188 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 10/10/2023

DSA 103



LS1.3
AD 1.05

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