

THE LINE SHOWN ABOVE THE SHEET DRAWS PAGE SIZE

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LODI UNIFIED SCHOOL DISTRICT

LAKEWOOD ELEMENTARY SCHOOL SHADE STRUCTURE

1100 N HAM LN,
LODI, CA 95242

GENERAL NOTES

- CONSTRUCTION DOCUMENTS DESCRIBE THE PRODUCTS, SYSTEMS, QUALITIES, CONFIGURATION, AND PERFORMANCE SPECIFICATIONS THAT DELIVER THE OVERALL DESIGN INTENT OF THE PROJECT. THE CONSTRUCTION DOCUMENT DRAWINGS AND SPECIFICATIONS ARE COMPLEMENTARY, AND WHAT IS REQUIRED BY ONE SHALL BE AS BINDING AS IF REQUIRED BY BOTH.
- PERFORMANCE BY THE CONSTRUCTION TEAM SHALL BE CONSISTENT WITH THE CONSTRUCTION DRAWINGS AND SPECIFICATIONS AS NECESSARY TO DELIVER THE INDICATED RESULTS OF THE DESIGN INTENT.
- VERIFY ALL DIMENSIONS, LOCATIONS OF EXISTING UTILITIES, AND CONDITIONS ON THE JOB SITE PRIOR TO THE START OF WORK OR PORTIONS THEREOF. NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES BETWEEN THE ACTUAL FIELD CONDITIONS AND THE CONSTRUCTION DOCUMENTS. EXISTING CONDITIONS ARE INDICATED AS A RESULT OF FIELD OBSERVATION. DETAILS SHOWN ON AVAILABLE DOCUMENTS AND FIELD CONDITIONS AT THE TIME OF PREPARATION.
- ALL MATERIALS AND WORKMANSHIP SHALL COMPLY WITH ALL GOVERNING CODES, ORDINANCES, REGULATIONS AND LAWS. THE DESIGN ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS AND SCAFFOLDING IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR WHERE ANY CONFLICT OCCURS BETWEEN THE REQUIREMENTS OF LAWS, CODES, ORDINANCES, RULES AND REGULATIONS, THE MOST STRINGENT SHALL GOVERN. IN NO CASE SHALL WORKING DIMENSIONS BE SCALED FROM PLANS, SECTIONS OR DETAILS ON THE DRAWINGS. DETAILS MARKED "TYPICAL" SHALL APPLY IN ALL CASES UNLESS SPECIFICALLY NOTED OTHERWISE.
- ENACT ALL MEASURES TO PROTECT AND SAFEGUARD ALL EXISTING ELEMENTS TO REMAIN FROM BEING DAMAGED. REPLACE OR REPAIR EXISTING ELEMENTS DAMAGED BY THE EXECUTION OF THIS CONTRACT TO EQUAL OR BETTER CONDITION.
- PRIOR TO THE START OF WORK THE CONTRACTOR SHALL COORDINATE BETWEEN THE REQUIREMENTS OF ALL DISCIPLINES HEREIN AND BETWEEN THE REQUIREMENTS OF ALL DRAWINGS AND SPECIFICATIONS IN ORDER THAT ALL ITEMS SATISFACTORILY RELATE TO ONE ANOTHER. NOTIFY ARCHITECT IMMEDIATELY REGARDING ANY ITEMS THAT CANNOT BE COORDINATED.
- THE CONTRACTOR SHALL EXERCISE EXTREME CAUTION IN EXCAVATING AND TRENCHING ON THIS SITE TO AVOID EXISTING DUCTS, PIPING, CONDUIT, ETC. AND TO PREVENT HAZARD TO PERSONNEL AND/OR TO EXISTING UNDERGROUND UTILITIES OR STRUCTURES. THE CONTRACTOR SHALL BE DISCOVERED. THESE DRAWINGS AND SPECIFICATIONS DO NOT INCLUDE THE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.
- CUTTING, BORING, SAWCUTTING OR DRILLING THROUGH THE EXISTING OR NEW STRUCTURAL ELEMENTS SHALL NOT BE STARTED UNTIL THE DETAILS HAVE BEEN REVIEWED AND APPROVED BY THE ARCHITECT, AND STRUCTURAL ENGINEER OF RECORD.
- ALL WORK SHALL CONFORM TO 2022 EDITION TITLE 24, CALIFORNIA CODE OF REGULATION (CCR).
- THE LIMIT OF WORK LINE SHOWS THESE DRAWINGS IS AN APPROXIMATE LIMIT OF WORK ONLY. REFER TO CONSULTANT DRAWINGS FOR ADDITIONAL WORK, INCLUDING BUT NOT LIMITED TO INSTALLATION OF CONDUIT, MANHOLES, PULLBOXES, ETC WHICH ARE TO BE PART OF THIS WORK, ALTHOUGH OCCURRING OUTSIDE OF SHOWN LIMIT OF WORK LINES.
- IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES. ALL CONSTRUCTION AND DEMOLITION SHALL BE IN ACCORDANCE WITH CHAPTER 33 OF THE CBC AND CFC, AND THE WRITTEN SITE SAFETY PLAN.
- CONTRACTOR IS TO REVIEW AND COMPLY WITH ALL REQUIREMENTS AND MITIGATION MEASURES SET FORTH IN BOTH THE ENVIRONMENTAL IMPACT REPORT ADDENDUM TO THE ENVIRONMENTAL IMPACT REPORT (SCH NO. 2020271120) INCLUDING ATTACHED BIOLOGICAL RESOURCES TECHNICAL REPORT.
- NO DUMPING OR PLACING OF ANY DIRT OR DEBRIS SHALL BE ALLOWED OUTSIDE OF THE CONTRACTORS LIMIT OF WORK AREA TO THE FULLEST EXTENT PERMITTED BY LAW. THE GENERAL CONTRACTOR SHALL INDEMNIFY, DEFEND AND HOLD HARMLESS THE OWNER, THE ARCHITECT AND THE ARCHITECT'S CONSULTANTS, AGAINST ANY CLAIMS RELATED TO HAZARDOUS MATERIALS CAUSED, IN WHOLE OR IN PART, BY THE GENERAL CONTRACTOR OR ITS SUBCONTRACTORS. FURTHER, THE GENERAL CONTRACTOR AND ITS SUBCONTRACTORS SHALL OBTAIN CONTRACTUAL LIABILITY COVERAGE FOR THIS INDEMNIFICATION OBLIGATION AND SHALL NAME THE OWNER, THE ARCHITECT AND THE ARCHITECT'S CONSULTANTS AS ADDITIONAL INSURED UNDER ANY INSURANCE POLICIES PROVIDING SUCH PRIOR TO FABRICATION AND/OR DEFERRED SUBMITTAL ITEMS FOR THIS PROJECT.
- CHANGE TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24 CCR.
- A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR. INSPECTOR TO BE CLASS 2.

- A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT. THE REPORTS SHALL BE SUBMITTED TO ARCHITECT OF RECORD, STRUCTURAL ENGINEER OF RECORD, OWNER, INSPECTOR OR RECORD, AND THE DSA FIELD ENGINEER. THE REPORTS OF ANY FAILURES OF TESTS AND INSPECTIONS ARE TO BE SUBMITTED TO DSA DISTRICT STRUCTURAL ENGINEER. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS IS TO BE IN ACCORDANCE WITH TITLE 24, CCR, SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE DSA APPROVED CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY THE ARCHITECT BEFORE PROCEEDING WITH THE WORK. (SECTION 4-317(C), PART 1, TITLE 24, CCR).
- SUBSTITUTIONS AFFECTING ITEMS REGULATED BY THE DIVISION OF THE STATE ARCHITECT (DSA) REQUIRE DSA APPROVAL. AN ADDENDUM OR A CONSTRUCTION CHANGE DOCUMENT (CCD), DSA APPROVAL, SHALL BE OBTAINED PRIOR TO FABRICATION AND/OR INSTALLATION PER SECTION 4-338, PART 1, TITLE 24, CCR.

CODES

PARTIAL LIST OF APPLICABLE CODES

ALL WORK PERTAINING TO AND ALL MATERIALS SUPPLIED FOR EXECUTING AND COMPLETING THIS CONTRACT SHALL COMPLY WITH PROVISIONS SPECIFIED IN THE CONTRACT DOCUMENTS AND WITH ALL APPLICABLE LAWS, REGULATIONS AND ORDINANCES GOVERNING WORK INCLUDING, BUT NOT NECESSARILY LIMITED TO, THOSE OF:

- | | |
|------|--|
| 2025 | CALIFORNIA BUILDING STANDARDS, ADMINISTRATIVE CODE (CAC) |
| 2022 | PART 1, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) |
| 2022 | CALIFORNIA BUILDING CODE (CBC) |
| 2022 | PART 2, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR), VOL. 1&2 |
| 2022 | BASED ON THE 2021 INTERNATIONAL BUILDING CODE |
| 2022 | CALIFORNIA ELECTRICAL CODE (CEC) |
| 2022 | PART 3, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) |
| 2022 | BASED ON THE 2020 NATIONAL ELECTRICAL CODE (NEC) |
| 2022 | CALIFORNIA MECHANICAL CODE (CMC) |
| 2022 | PART 4, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) |
| 2022 | BASED ON THE 2021 UNIFORM MECHANICAL CODE (UMC) |
| 2022 | CALIFORNIA PLUMBING CODE (CPC) |
| 2022 | PART 5, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) |
| 2022 | BASED ON THE 2021 UNIFORM PLUMBING CODE (UPC) |
| 2022 | CALIFORNIA ENERGY CODE (CEC) |
| 2022 | PART 6, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) |
| 2022 | CALIFORNIA HISTORICAL BUILDING CODE (CHBC) |
| 2022 | PART 8, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) |
| 2022 | CALIFORNIA FIRE CODE (CFC) |
| 2022 | PART 9, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) |
| 2022 | BASED ON THE 2021 INTERNATIONAL FIRE CODE (IFC) |
| 2022 | CALIFORNIA EXISTING BUILDING CODE (CEBC) |
| 2022 | PART 10, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) |
| 2022 | BASED ON THE 2021 INTERNATIONAL EXISTING BUILDING CODE (IEBC) |
| 2022 | CALIFORNIA GREEN BUILDING STANDARDS CODE (CALGREEN) |
| 2022 | PART 11, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) |
| 2022 | CALIFORNIA REFERENCED STANDARDS CODE (CRSC) |
| 2022 | PART 12, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) |
| 2021 | NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) 101 |
| 2021 | LIFE SAFETY CODE |
| 2021 | NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) 99 |
| | HEALTH CARE FACILITIES CODE |

TITLE 19 CCR, PUBLIC SAFETY STATE FIRE MARSHAL REGULATIONS

FOR A COMPLETE LIST OF APPLICABLE NFPA STANDARDS REFER TO 2022 CBC (SFM) CHAPTER 35 AND CALIFORNIA FIRE CODE CHAPTER 80.

PARTIAL LIST OF APPLICABLE STANDARDS

FOR A LIST OF APPLICABLE STANDARDS, INCLUDING CALIFORNIA AMENDMENTS TO THE NFPA STANDARDS, REFER TO CBC CHAPTER 35 AND CFC CHAPTER 80.

ALL CONSTRUCTION AND DEMOLITION SHALL BE IN ACCORDANCE WITH CHAPTER 33 OF THE CBC AND CFC, AND THE WRITTEN SITE FIRE SAFETY PLAN.

STATEMENT OF GENERAL CONFORMANCE

APPLICATION NO.: 02-123929
FILE NO.: 39-50

THE DRAWINGS IDENTIFIED AS FOLLOWS:

- (X) ALL DRAWING SHEETS INCLUDED IN THIS SET NOT BEARING MY STAMP AND SIGNATURE
() DRAWING SHEETS DENOTED IN THE SHEET INDEX AS FOLLOWS:

() DRAWING SHEETS INCLUDED UNDER THE FOLLOWING PC APPROVAL(S):

HAVE BEEN PREPARED BY OTHER DESIGN PROFESSIONALS OR CONSULTANTS WHO ARE LICENSED AND/OR AUTHORIZED TO PREPARE SUCH DRAWINGS (PLANS) IN THIS STATE. IT HAS BEEN EXAMINED BY ME FOR:

- DESIGN INTENT AND APPEARS TO MEET THE APPROPRIATE REQUIREMENTS OF TITLE 24, CALIFORNIA CODE OF REGULATIONS AND THE PROJECT SPECIFICATIONS PREPARED BY ME, AND
- COORDINATION WITH MY DRAWINGS (PLANS) AND SPECIFICATIONS AND IS ACCEPTABLE FOR INCORPORATION INTO THE CONSTRUCTION OF THIS PROJECT.

PER TITLE 24, PART 1, SECTION 4-310(b), THIS STATEMENT OF GENERAL CONFORMANCE SHALL NOT BE CONSTRUED AS RELIEVING ME OF MY RIGHTS, DUTIES, AND RESPONSIBILITIES UNDER SECTION 17302 AND 81138 OF THE EDUCATION CODE AND SECTION 4-336, 4-341 AND 4-344 OF TITLE 24, PART 1.

I CERTIFY THAT:

THE PC APPROVED MANUFACTURER DRAWINGS PC# 04-122050 LISTED ON THE INDEX SHEET ARE IN GENERAL CONFORMANCE WITH THE PROJECT DESIGN INTENT, AND THEY HAVE BEEN COORDINATED WITH THE PROJECT PLANS AND SPECIFICATIONS.

SIGNATURE: *Jennifer Pearson Huang* 12/09/25
DATE: 12/09/25
ARCHITECT OR ENGINEER DESIGNATED TO BE IN GENERAL RESPONSIBLE CHARGE

JENNIFER PEARSON HUANG
PRINT NAME

C-35691
LICENSE NUMBER

05-31-27
EXPIRATION DATE

THIS PROJECT WILL NOT BE CERTIFIED UNTIL DSA #02-120455 IS CERTIFIED

PROJECT DESCRIPTION

GENERAL DESCRIPTION OF WORK

INSTALLATION OF (1) NEW SHADE STRUCTURE PER PC #04-122375

PROJECT DATA

PROJECT ADDRESS

1100 N HAM LN,
LODI, CA 95242

PROJECT INFORMATION

BUILDING NAME: LS

YEAR BUILT: THIS APPLICATION

TYPE OF CONSTRUCTION: TYPE II-B

OCCUPANCY: A-3

NUMBER OF OCCUPANTS: 128 OCC.

AUTOMATIC FIRE SPRINKLER SYSTEM: N/A

NUMBER OF STORIES: 1

BUILDING AREA: 1,920 SF

RISK CATEGORY: II

SITE CLASS: D (DEFAULT)

SEISMIC IMPORTANCE FACTOR: 1.00

FIRE HAZARD ZONE: NONE

DEFERRED SUBMITTAL

NONE

SHEET INDEX

NUMBER NAME

GENERAL SHEET
G0.10 COVER SHEET

1 PROJECT ANALYSIS
G1.11 CODE SITE PLAN & CODE INFORMATION
G1.51 LOCAL FIRE AUTHORITY SITE ACCESS PLAN
2

ARCHITECTURE
A1.21 PROJECT SITE PLAN
1

PC DRAWINGS
LS1.0 GENERAL INFO
LS1.1 GENERAL INFO
LS1.2 DSA 103
LS1.3 DSA 103
LS3.0 30' WIDE RECTANGULAR HIP FOUNDATION PLAN
LS3.1 30' WIDE RECTANGULAR HIP FRAMING & CONNECTION DETAILS
LS3.4 30' WIDE RECTANGULAR HIP STANDING SEAM ROOFING PLAN
7
Grand total: 11

SYMBOL LEGEND

NORTH ARROW

TICK INDICATES PLAN NORTH
ARROW INDICATES TRUE NORTH

ELEVATION CALLOUT
(TYPICAL FOR EXTERIOR)

LOCATION ON SHEET
SHEET WHERE ELEVATION IS DRAWN

ELEVATION CALLOUT
(TYPICAL FOR INTERIOR)

LOCATION ON SHEET
SHEET WHERE ELEVATION IS DRAWN

ELEVATION CALLOUT - ALT.
(TYPICAL FOR INTERIOR)

LOCATION & SHEET WHERE ELEVATION IS DRAWN

SECTION CALLOUT

INDICATES A SIMILAR CONDITION
LOCATION ON SHEET
SHEET WHERE SECTION IS DRAWN

DETAIL CALLOUT

INDICATES A SIMILAR CONDITION
LOCATION ON SHEET
SHEET WHERE SECTION IS DRAWN

CONTROL OR DATUM POINT

NAME OF ELEVATION (IF APPLICABLE)
ELEVATION ABOVE FINISHED FLOOR

GRID BUBBLE

EXISTING BUILDING GRID SYMBOL
GRID NUMBER
NEW BUILDING GRID SYMBOL

DOOR CALLOUT

DOOR NUMBER

INTERIOR FINISH CALLOUT

MATERIAL FINISH TYPE
(SEE FINISH SCHEDULE)

WINDOW CALLOUT

WINDOW NUMBER
(SEE WINDOW SCHEDULE)

WALL TYPE CALLOUT

WALL TYPE MARK - SEE A10.11
WALL STC RATING
WALL FIRE RATING TYPE

MATCHLINE REFERENCE

LOCATION ON SHEET
SHEET WHERE PLAN IS DRAWN

KEYNOTE

KEYNOTE NUMBER (SEE LEGEND ON SHEET)

ROOM EXITING INFORMATION

AREA (SQ FT)
OCCUPANT LOAD (AREA DIVIDED BY LOAD FACTOR)
OCCUPANT LOAD FACTOR (REFER TO TABLE 1004.5)
OCCUPANCY TYPE
NUMBER OF EXITS REQUIRED (REFER TO TABLE 1006.2.1)

WIC CASEWORK TAG

MANUFACTURER REFERENCE AND MODEL NUMBER

LOCK
CABINET DEPTH
CABINET HEIGHT
CABINET WIDTH

SHEET NUMBER SYSTEM

DISCIPLINE	SHEET TYPE	BUILDING LETTER, SEGMENT (USER DEFINED)
G GENERAL	0 CODE ANALYSIS, NOTES	
C CIVIL	1 SITE PLAN	
L LANDSCAPE	2 FLOOR PLAN	
A ARCHITECTURE	3 CEILING PLAN	
I INTERIORS	4 ROOF PLAN	
E EQUIPMENT	5 EXTERIOR ELEVATIONS	
S STRUCTURAL	6 SECTIONS	
P PLUMBING	7 ENLARGED PLANS	
M MECHANICAL	8 INTERIOR ELEVATIONS	
E ELECTRICAL	9 SCHEDULES	
FA FIRE ALARM	10 DETAILS	
T TELECOM		
AV AV EQUIPMENT		
K KITCHEN		
FP FIRE PROTECTION		

SERIES ORDER
SHEET TYPE
DISCIPLINE

BUILDING LETTER (IF APPLICABLE)
FLOOR LEVEL OR SEQUENTIAL ORDER

SEGMENT (IF APPLICABLE)
USER DEFINED (IF APPLICABLE)

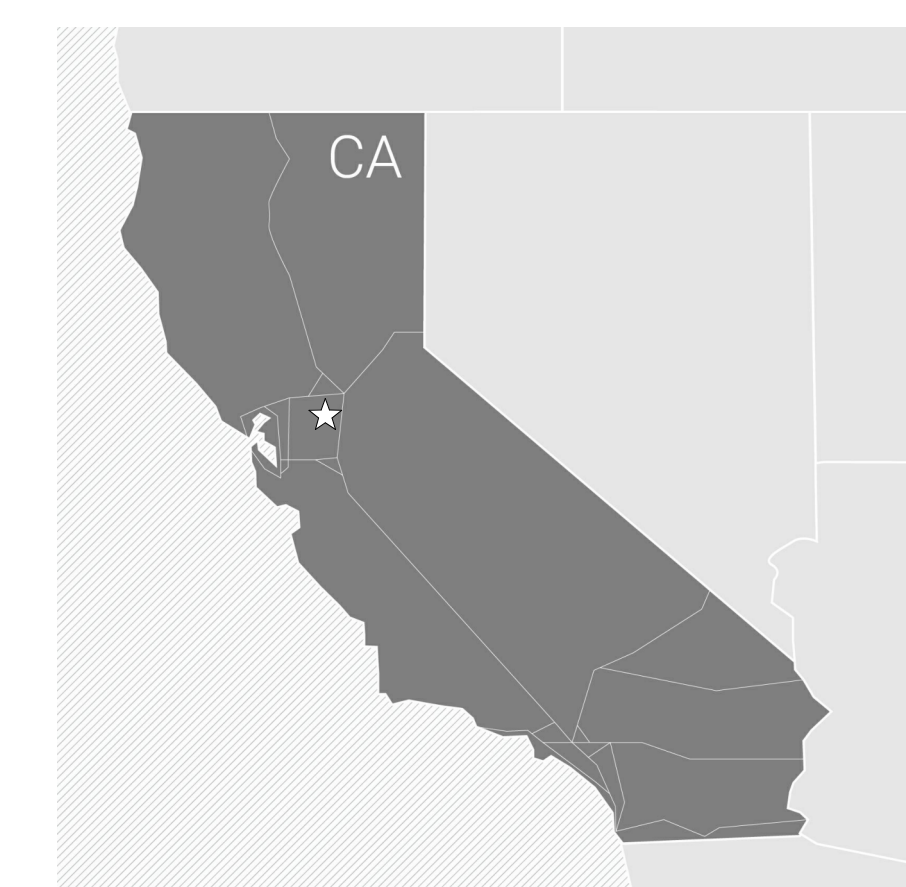
AA 1.1 A.A

ABBREVIATIONS

(E) EXISTING	FRP FIBERGLASS REINFORCED PLASTIC	PTC POST TENSIONED CONCRETE
AB ANCHOR BOLT	FRT FIRE RETARDANT TREATED	PTD PAPER TOWEL DISPENSER
AC PAVING	FS FINISH SURFACE	PTN PARTITION
ACC ACCESS/ACCESSIBLE	FTG FOOTING	PTS PNEUMATIC TUBE STATION / SYSTEM
ACU ACoustical CEILING PANEL	GB GRAB BAR	PVC POLYVINYL CHLORIDE
ACT ACoustical CEILING TILE	GFRG GLASS FIBER REINFORCED CONCRETE	PVMT PAVEMENT
ADJ ADJACENT/ADJUSTABLE	GL GLASS TYPE	QTY QUANTITY
ADVF ABOVE FINISH FLOOR	GLB GLUE LAMINATED BEAM	R RADIUS, RISER
AGG AGGREGATE	GYP BD GYPSUM BOARD	RB RESILIENT BASE
AHU AIR HANDLING UNIT	GYP PLAS GYPSUM PLASTER	RD ROOF DRAIN
ARCH ARCHITECTURAL	HB HOSE BIBB	RECEPT RECEPTACLE
ATT ATTENUATION	HD HEAVY DUTY	REF REFERENCE
AUTO AUTOMATIC	HDR HEADER	REFL REFLECTED, (IVE)
BLCK BLOCKING	HDWR HARDWARE	REFL REFLECTED, (IVE)
BLDG BUILDING	HGT HEIGHT	REFR REFRIGERATOR
BUR BUILT UP ROOFING	HLM HOLLOW METAL	REINFC REINFORCE/REINFORCED/ REINFORCEMENT
CABT CABINET	HP HIGH POINT	REINFC REINFORCEMENT
CFU CUBIC FEET	HSS HOLLOW STEEL SECTION	REM REMOVE
CFI CONTRACTOR FURNISHED, CONTRACTOR INSTALLED	INS INSIDE DIAMETER	RH ROUNDED HEAD
CFOI CONTRACTOR FURNISHED, OWNER INSTALLED	INT INTERIOR	RHS ROUND HEAD SCREW
CG CORNER GUARD	INV INVERT	RO ROUGH OPENING
CJ CONTROL JOINT	LANDS LANDSCAPE	ROW RIGHT OF WAY
CLF CHAIN LINK FENCE	LAV LAVATORY	SCH SCHEDULE (FOR PIPE)
CLR CLEAR	LLH LONG LEG HORIZONTAL	SCHED SCHEDULE / SCHEDULING
CMU CONCRETE MASONRY UNIT	LVV LONG LEG VERTICAL	SECT SECTION
CO CLEANOUT	LP LOW POINT	SG SAFETY GLASS
COL COLUMN	LT WT LIGHT WEIGHT	SHIT SHEET
COMP COMPOSITE	MACH MACHINE	SHTG SHEATHING
COORD COORDINATE	MB MACHINE BOLT	SMS SHEET METAL SCREW
CT CORRUGATED	MDF MEDIUM DENSITY FIBERBOARD	SND SANITARY WAPION DISPOSAL
CT CERAMIC TILE	MDO MEDIUM DENSITY OVERLAY	SOV SHUT OFF VALVE
CTSK COUNTER SKUNK	MECH MECHANICAL	SPEC SPECIFICATIONS
DEPR DEPRESSED / DEPRESSION	MED MEDIUM	SSS STAINLESS STEEL
DF DRINKING FOUNTAIN	MEMB MEMBRANE	STC SOUND TRANSMISSION CLASS
DISP DISPENSER	MFR MANUFACTURER	STL STEEL
DS DOWNSPOUT	MH MANHOLE	STSMS SELF TAPPING SHEET METAL
DW DISHWASHER	MO MOUNTED	SUSP SUSPENDED
DWG(S) DRAWING(S)	MTL METAL	SV SHEET VINYL
E/W EACH WAY	NIC NOT IN CONTRACT	SYM SYMMETRICAL
EIS EXTERIOR INSULATION FINISH	NR NON RATED	T&B TOP AND BOTTOM
SYSTEM	NTS NOT TO SCALE	TO TOP OF
EJ EXPANSION JOINT	O/ OVER	TOC TOP OF CURB / CONCRETE
ELEC ELECTRICAL	OIA ON CENTER	TOP TOP OF PARAPET
ELEV ELEVATION / ELEVATOR	OC ON CENTER	TOS TOP OF STEEL
ENCL ENCLOSE / ENCLOSURE	OD OUTSIDE DIAMETER	TOW TOP OF WALL
EOS EDGE OF SLAB	OFCI OWNER FURNISHED, CONTRACTOR INSTALLED	TPO TOILET PAPER DISPENSER
EP ELECTRICAL PANEL	OFVI OWNER FURNISHED, VENDOR INSTALLED	TS TACKABLE SURFACE
EQ EQUAL	OH OPERABLE	U/C UNDER CABINET / OR COUNTER
ESC EXCUTJOEN	OPNG OPENING	UNAL UNRUL
EWG ELECTRIC WATER COOLER	ORD OVERFLOW ROOF DRAIN	VAC VACUUM
EXP EXPOSED	P/L PROPERTY LINE	VB VAPOR BARRIER
FA FIRE ALARM	PA PUBLIC ADDRESS	VCT VINYL COMPOSITION TILE
FDC FIRE DEPARTMENT CONNECTION	PAP POWDER ACTUATED FASTENER	VIF VERIFY IN FIELD
FEC FIRE EXTINGUISHER	PCC PORTLAND CEMENT CONCRETE	VTR VENT THROUGH ROOF
FF FINISH FLOOR	PED PEDESTRIAN	VWC VINYL WALL COVERING
FG FINISH GRADE	PERF PERFORATED	W/ WITH
FH FIRE HOSE CABINET	PERIM PERIMETER	WO WOOD
FSH FLAT HEAD SCREW	PH PANIC HARDWARE	WB WOOD BASE
FIN FINISH	PIV POST INDICATOR VALVE	WC WATER CLOSET
FLR FLOOR	PL PLASTIC LAMINATE	WD WINDOW
FOC FACE OF CONCRETE	PLAS PLASTER	WGT WEIGHT
FOM FACE OF FINISH	PLUMB PLUMBING	WH WATER HEATER
FOS FACE OF STUD	PNT PAINT / PAINTED	WP WATERPROOFING/WALL PROTECTION
FP FIREPROOFING	POC POINT OF CONNECTION	WR WATER RESISTANT
FR FIRE RATED	PREP PREPARED	WRGB WATER RESISTANT GYPSUM BOARD
FRG FIRE RATED GLASS	POLY ISO POLYISOCYANURATE	WS BOARD
	PREP / PREPARATION	WSCOT WAINSCOT
		WWF WELDED WIRE FABRIC

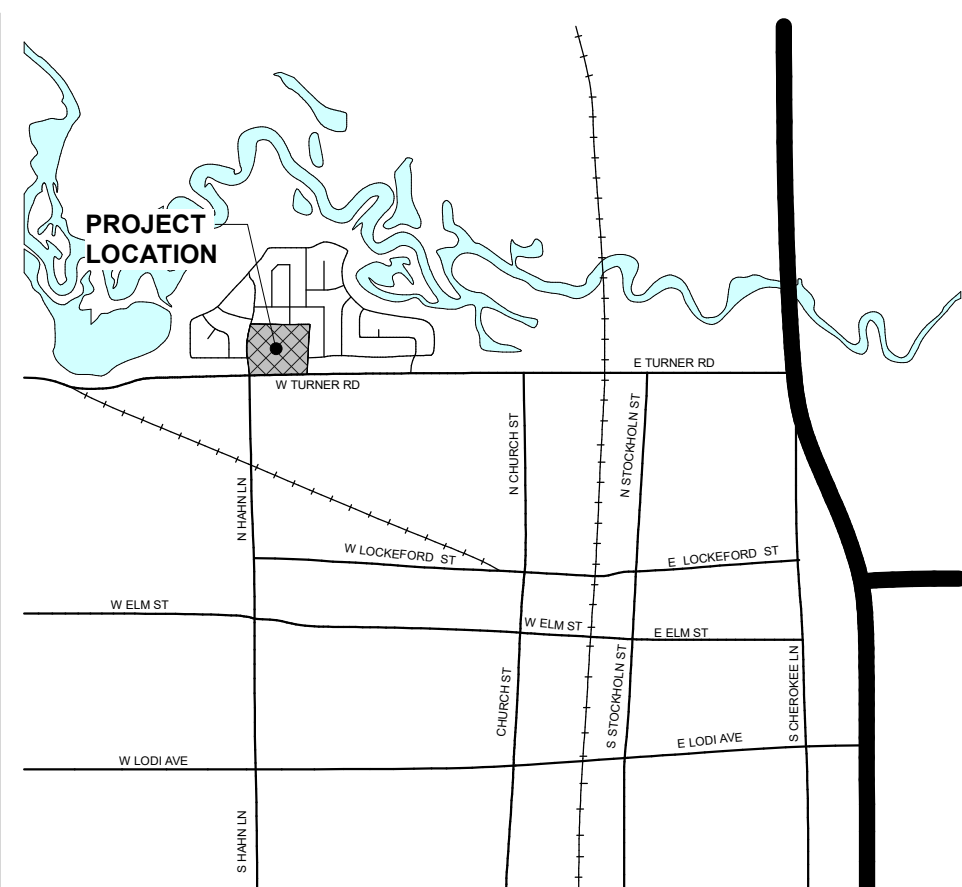
NOTE:
OTHER ABBREVIATIONS USED ON THESE DRAWINGS ARE CONSIDERED STANDARDS IN THE BUILDING INDUSTRY. CONTACT ARCHITECT FOR NECESSARY CLARIFICATION.

STATE MAP



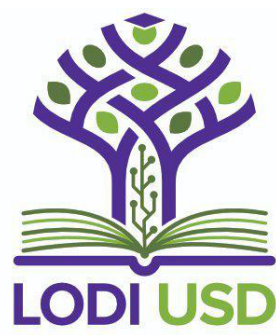
OVERALL SITE PLAN

VICINITY MAP



AGENCY
APPROVAL:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-123929 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 01/06/2026



DESIGN FOR GOOD

HMC Architects

3431-013-000

2101 CAPITOL AVENUE, SUITE 100,
SACRAMENTO, CA 95816
916 368 7990 / www.hmcarchitects.com © 2025 HMC GROUP

PROJECT TEAM

REGISTERED ARCHITECT
JENNIFER PEARSON HUANG
C-35691
STATE OF CALIFORNIA

OWNER
LODI UNIFIED SCHOOL DISTRICT
825 W LOCKEFORD ST, LODI, CA 95240
(209) 331-7000

ARCHITECTURAL
HMC ARCHITECTS
2101 CAPITOL #100, SACRAMENTO, CA 95816
(916) 368-7990

FACILITY:
LAKEWOOD ELEMENTARY SCHOOL
1100 N HAM LN,
LODI, CA 95242

PROJECT:
LAKEWOOD ELEMENTARY SCHOOL SHADE STRUCTURE

SHEET NAME:
COVER SHEET

CONSTRUCTION DOCUMENTS

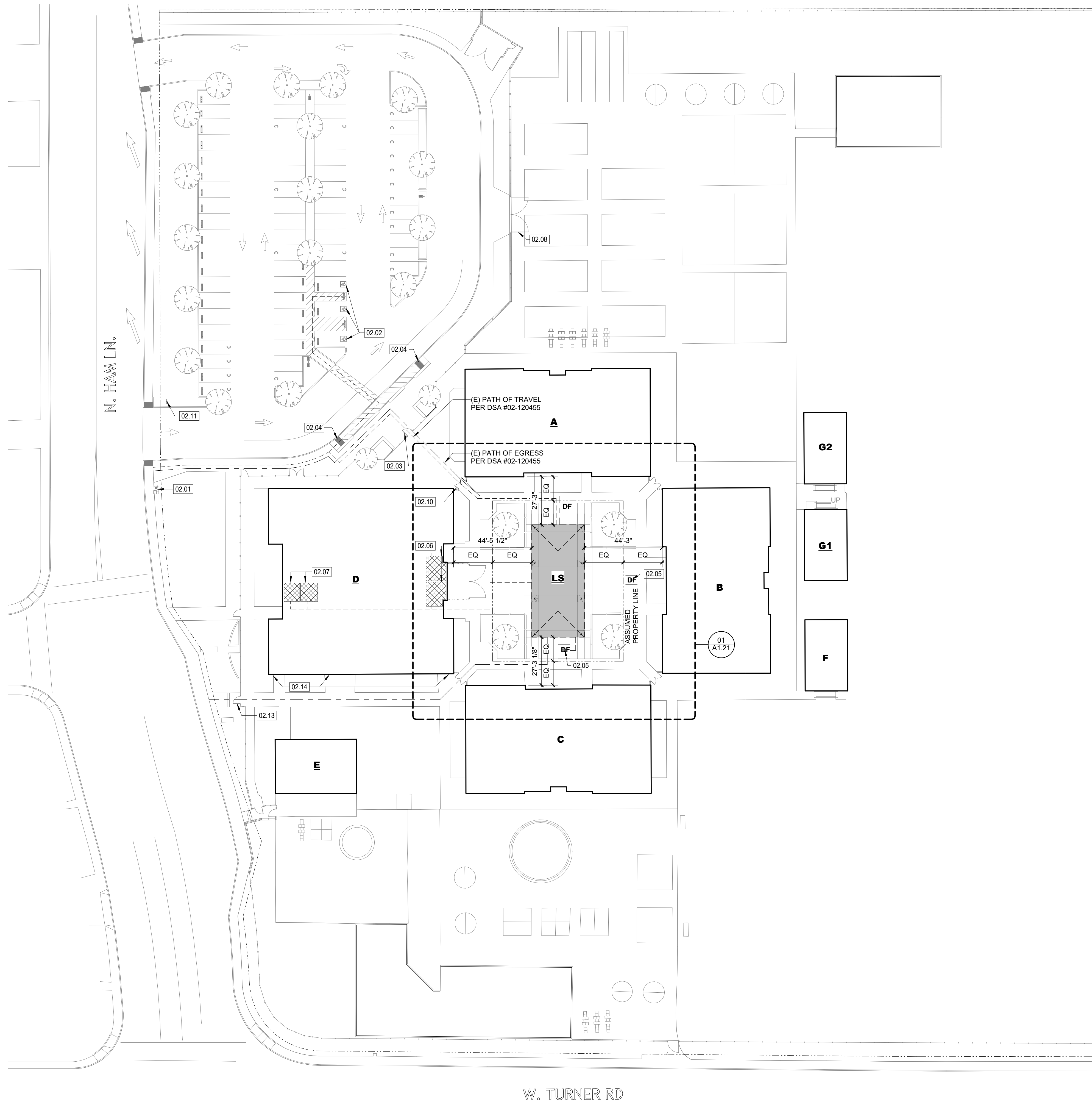
DATE: 2025.12.09

CLIENT PROJ NO:

SHEET:

G0.10

PLEASE RECYCLE



LEGEND

- X EXISTING BUILDING
- X SHADE STRUCTURE
- CONCRETE WALKPAVING | FOR REFERENCE ONLY
- (E) ACCESSIBLE RESTROOM
- ASSUMED PROPERTY LINE
- (E) CHAIN LINK FENCE
- (E) DECORATIVE METAL FENCE
- (E) FIRE HYDRANT (NTS)
- ACCESSIBLE PATH OF TRAVEL (SEE DEFINITION ON THIS SHEET)
- ACCESSIBLE PATH OF EGRESS

BUILDING DESIGNATIONS

BUILDING MARK	BUILDING NAME	DSA APPLICATION NUMBER
A	CLASSROOMS	23521, 02-106411
B	CLASSROOMS	23521, 02-106411
C	CLASSROOMS	23521, 02-106411
D	CLASSROOMS	25287, 02-106411, 02-112790
E	CLASSROOMS	#50143
F	CLASSROOMS	#50143
G1	CLASSROOMS	#50143
G2	CLASSROOMS	#50143
LS	LUNCH SHELTER	THIS APPLICATION

EXISTING ACCESSIBLE PARKING STALL CALCULATION

PARKING LOT A (EXISTING - NO SCOPE OF WORK)

TOTAL PARKING STALL COUNT: 66 STALLS

ACCESSIBLE PARKING STALLS PER TABLE 11B-208.2:

REQUIRED ACCESSIBLE STALLS: 3 (51-75 TOTAL STALLS)

REQUIRED VAN ACCESSIBLE STALLS: 1 (0-6 ACCESSIBLE STALLS)

ACCESSIBLE STALLS PROVIDED: 2 STANDARD & 1 VAN

EXISTING PATH OF TRAVEL (POT): ARCHITECT STATEMENT

DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE IN CHARGE STATEMENT: THE POT IDENTIFIED IN THESE CONSTRUCTION DOCUMENTS IS COMPLIANT WITH THE CURRENT APPLICABLE CALIFORNIA BUILDING CODE ACCESSIBILITY PROVISIONS FOR PATH OF TRAVEL REQUIREMENTS FOR ALTERATIONS, ADDITIONS AND STRUCTURAL REPAIRS. AS PART OF THE DESIGN OF THIS PROJECT, THE POT WAS EXAMINED AND ANY ELEMENTS, COMPONENTS OR PORTIONS OF THE POT THAT WERE DETERMINED TO BE NON-COMPLIANT

1) HAVE BEEN IDENTIFIED AND

2) THE CORRECTIVE WORK NECESSARY TO BRING THEM INTO COMPLIANCE HAS BEEN INCLUDED WITHIN THE SCOPE OF THIS PROJECT'S WORK THROUGH DETAILS, DRAWINGS, AND SPECIFICATIONS INCORPORATED INTO THESE CONSTRUCTION DOCUMENTS.

ANY NONCOMPLIANT ELEMENTS, COMPONENTS OR PORTIONS OF THE POT THAT WILL NOT BE CORRECTED BY THIS PROJECT BASED ON VALUATION THRESHOLD LIMITATIONS OR A FINDING OF UNREASONABLE HARDSHIP ARE SO INDICATED IN THESE CONSTRUCTION DOCUMENTS.

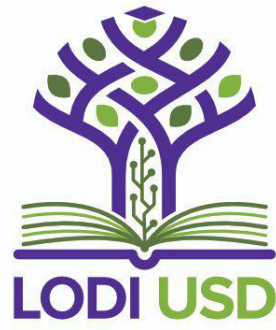
DURING CONSTRUCTION, IF THE POT ITEMS WITHIN THE SCOPE OF THE PROJECT REPRESENTED AS CBC COMPLIANT ARE FOUND TO BE NON-CONFORMING BEYOND REASONABLE CONSTRUCTION TOLERANCES, THEY SHALL BE BROUGHT TO COMPLIANCE WITH THE CBC AS PART OF THIS PROJECT BY MEANS OF A CONSTRUCTION CHANGE DOCUMENT.

PATH OF TRAVEL TECHNICAL REQUIREMENTS FOR ACCESSIBLE ROUTE:

ACCESSIBLE PATH OF TRAVEL AS INDICATED ON PLAN IS A BARRIER-FREE ACCESS ROUTE WITHOUT ABRUPT LEVEL CHANGES EXCEEDING 1/2" IF BEVELED AT 1:2 MAXIMUM SLOPE OR VERTICAL LEVEL CHANGES NOT EXCEEDING 1/4" MAXIMUM AND AT LEAST 48" IN WIDTH. SURFACE IS STABLE, FIRM, AND SLIP-RESISTANT. CROSS-SLOPE SHALL NOT BE STEEPER THAN 1:48 AND SLOPE IN THE DIRECTION OF TRAVEL SHALL NO BE STEEPER THAN 1:20. ACCESSIBLE PATH OF TRAVEL SHALL BE MAINTAINED FREE OF OVERHANGING OBSTRUCTIONS TO 80" MINIMUM AND FREE OF OBJECTS PROTRUDING MORE THAN 4" FROM THE WALL, ABOVE 27" AND LES THAN 80" ABOVE THE FLOOR. ARCHITECTS SHALL VERIFY THAT THERE ARE NO BARRIERS IN THE BATH OF TRAVEL.

AGENCY APPROVAL:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-123929 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 01/06/2026



DESIGN FOR GOOD
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3431-013-000
2101 CAPITOL AVENUE, SUITE 100,
SACRAMENTO, CA 95816
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ISSUE

DESCRIPTION	DATE
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KEYNOTES

- 02.01 (E) FIRE HYDRANT
- 02.02 (E) ACCESSIBLE PARKING PER PER DSA #02-120455
- 02.03 (E) PEDESTRIAN GATE PER PER DSA #02-120455
- 02.04 (E) TRUNCATED DOWNS PER DSA #02-120455
- 02.05 (E) DRINKING FOUNTAIN PER DSA #02-120455
- 02.06 (E) ACCESSIBLE BOYS AND GIRLS RESTROOMS PER DSA #02-111648
- 02.07 (E) ACCESSIBLE STAFF RESTROOM PER DSA #02-111648
- 02.08 (E) 20'-0" WIDE FIRE ACCESS GATE W/ KNOX BOX PER DSA #02-120455
- 02.10 6'-0" WIDE GATE WITH PANIC HARDWARE, PROVIDE KNOX BOX
- 02.11 (E) PARKING ENTRY SIGN
- 02.13 (E) 4' WIDE GATE WITH PANIC HARDWARE
- 02.14 (E) WALL MOUNTED EXTERIOR LIGHTING
- 02.15 (E) POLE MOUNTED SITE LIGHTING

NOTES

FACILITY:
LAKEWOOD ELEMENTARY SCHOOL
1100 N HAM LN,
LODI, CA 95242

PROJECT:
LAKEWOOD ELEMENTARY SCHOOL SHADE STRUCTURE

SHEET NAME:
CODE SITE PLAN & CODE INFORMATION

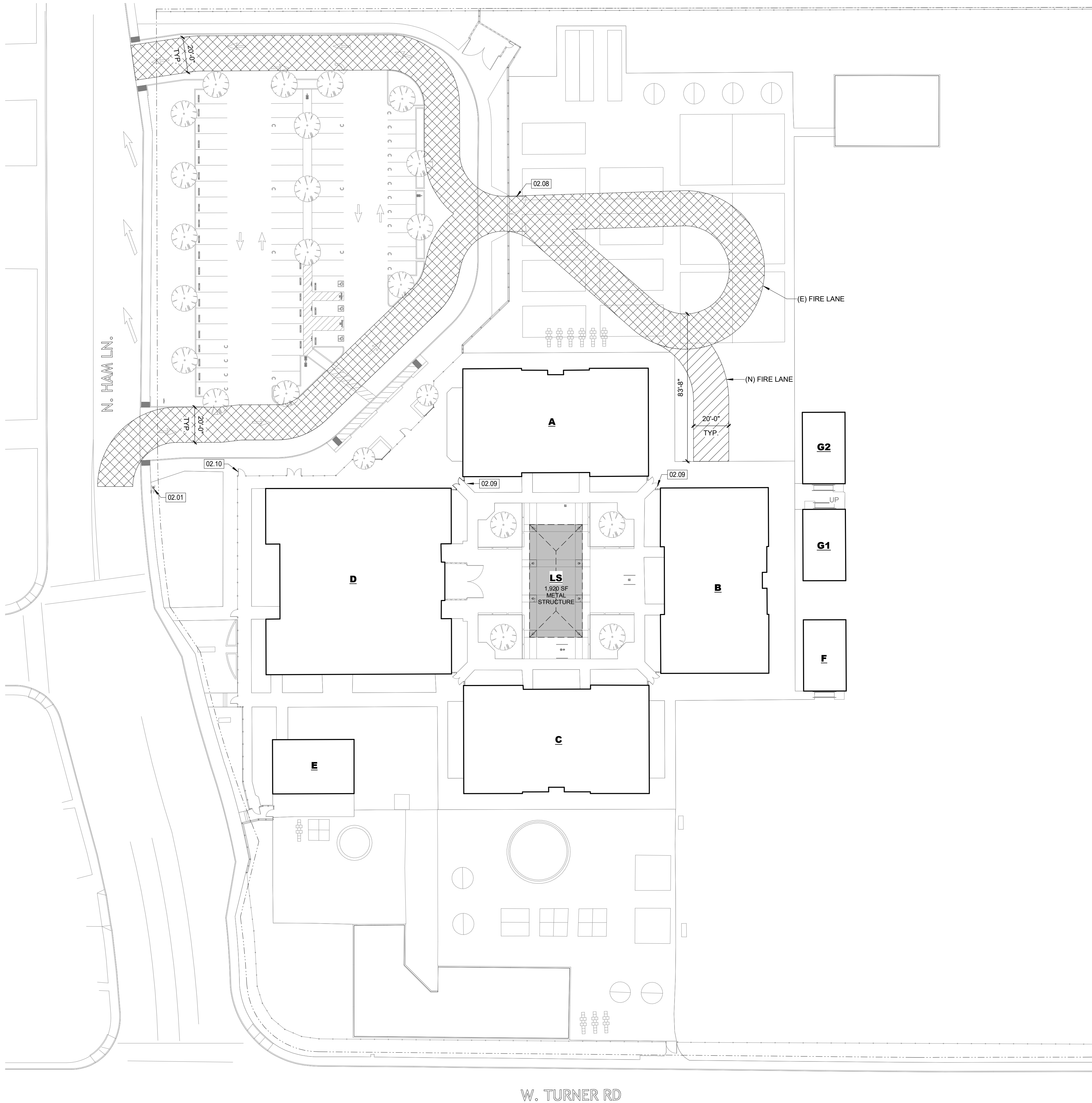
CONSTRUCTION DOCUMENTS

DATE: 2025.12.09
CLIENT PROJ NO:

SHEET:

CODE SITE PLAN & CODE INFORMATION

01
1" = 30'-0"



LEGEND

- APPROXIMATE LIMIT OF WORK, REFER TO ADDITIONAL DRAWINGS FOR WORK WHICH MAY EXTEND BEYOND THIS APPROXIMATE LIMIT OF WORK LINE.
- X EXISTING BUILDINGS
- (E) FIRE ACCESS LANE, 20'-0" WIDE MINIMUM
- (N) FIRE ACCESS LANE, 20'-0" WIDE MINIMUM
- PROPERTY LINE
- (E) CHAIN LINK FENCE
- (E) DECORATIVE METAL FENCE
- (E) FIRE HYDRANT (NTS)

LOCAL FIRE AUTHORITY REVIEW

DSAR 810
FIRE & LIFE SAFETY SITE CONDITIONS SUBMITTAL

Division of the State Architect (DSA) documents referenced within this publication are available on the DSA eForms at <https://www.dsa.ca.gov/Forms>.

To facilitate the Division of the State Architect's (DSA) fire and life safety plan review of project site conditions, DSA requires the design professional to provide the following information for review of the project site conditions, including but not limited to: (1) a site plan showing the location of all buildings, including existing and proposed buildings, and for the alternate design options for the department emergency vehicle access, and the emergency vehicle access; (2) information associated with compliance items 1 through 3 below is to be provided for all project types included above; information associated with items 4 through 7 is to be completed when an alternate design is utilized. Acknowledgement by the school district and signature from the Local Fire Authority (LFA) is only required when an alternate design is being requested.

The Project Information and Fire & Life Safety Information sections are to be completed for all projects and stamped with the fire access site plan. When the information is provided to the project, it is required to be stamped and signed by the fire access site plan. The information is required to be stamped and signed by the fire access site plan.

For additional information refer to the instructions at the end of this form and DSA Policy PL 68-01: Fire Flow for Buildings.

PROJECT INFORMATION	
School District/Owner:	Lodi Unified School District
Project Name/Location:	Lakewood Elementary School
Project Address:	1100 N. Ham Ln, Lodi, CA 95242

FIRE & LIFE SAFETY INFORMATION	
1. Has a fire hydrant been installed within the past 12 months? (If not, provide a copy of the last test.)	Yes ☒ No ☐
2. Has the fire hydrant been tested and performed as part of the LFA review?	Yes ☒ No ☐
3. Is the project located within a designated fire hazard severity zone (FHSZ) as indicated by Cal-Fire? (If yes, indicate FHSZ description below.)	Yes ☐ No ☒
Refer to the following website for FHSZ locations: https://www.fire.ca.gov/land-use/development/developmental-review	
4. Indicate whether the project is located in a fire hazard severity zone (FHSZ) as indicated by Cal-Fire? (If yes, indicate FHSZ description below.)	Yes ☐ No ☒
Refer to the following website for FHSZ locations: https://www.fire.ca.gov/land-use/development/developmental-review	

CONFORMANCE WITH FIRE FLOW REQUIREMENTS				
	ACCEPTANCE			
	Yes	No	Yes	No
1. Emergency vehicle access roadways do not meet CFC requirements.				
2. Accessible alternate emergency vehicle and personnel access as planned for the project is not acceptable for providing fire suppression and protection of life and property.				
3. Fire Hydrants: Number and spacing does not meet CFC requirements.				
4. Fire Hydrants: Number and spacing does not meet CFC requirements.				
5. Fire Hydrants: Water flow and pressure are less than CFC minimum.				
6. Accessible alternate: The available flow and pressure is acceptable for providing fire suppression and protection of life and property.				
7. Location of the department (contractor) testing the hydrant system or emergency system does not meet CFC requirements.				
8. Accessible alternate: The available flow and pressure is acceptable for providing fire suppression and protection of life and property.				

School District Acceptance of Acceptable Design Alternatives

By signing this form, the school district acknowledges and accepts the proposed design as an alternative to California Building Code (CBC) and California Fire Code (CFC) and fire flow requirements, as indicated by one or more of the conditions listed above, for providing fire suppression and protection of life and property.

Approved for: Edith A. Holbert Date: 12/14/2025 Title: Executive Director of Operations

Signature: [Signature] Date: 12/14/2025

LFA Agency Name: Lodi Fire Department

LFA Review Official (Sign): Chief

Fire Marshal: Mark Marshall Work Phone: 209-333-5553

Print Email: markmarshall@lodi.ca.gov

LFA Reviewer's Signature: Ryan Smart Date: 12/12/2025

FIRE FLOW TEST

CITY OF LODI FIRE DEPARTMENT
EST. 1907 Ken Johnson, Fire Chief

December 16, 2025

Lodi Unified School District
1100 E. Van Nuys
Lodi, CA 95240
Attn: Anthony Kaban

RE: HYDRANT FLOW TEST - LAKEWOOD ELEMENTARY SCHOOL

Dear Authority,

On Monday, December 16, 2025, Lodi Fire Dept. Engine 2011, conducted a hydrant flow test on Hydrant #11579 and #11603. The test was completed at the request of Anthony Kaban, Planning Analyst.

Results are as follows (Officer site record 2.17):

Hydrant #	Location	Static Pressure - PSI	Residual Pressure - PSI	CFC
#11579	Han Ln & Kirkwood Dr	40	30	1440
#11603	Turner Ln / Southside of school	40	30	1440

Thank you,
Ryan Smart
Fire Marshal
Lodi Fire Dept.
211 W. Van Nuys
Lodi, CA 95240

Lodi Unified School District
1100 E. Van Nuys
Lodi, CA 95240

SEE OTHER SHEETS FOR CONSTRUCTION

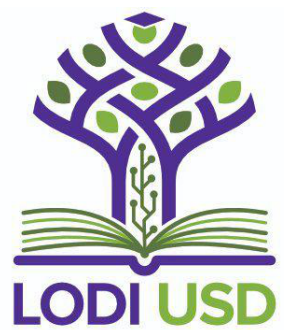
THIS PLAN INCLUDES INFORMATION FOR LOCAL FIRE AUTHORITY APPROVAL ONLY. REFER TO OTHER SHEETS FOR SITE CONSTRUCTION DETAILS.

EMERGENCY RESPONDER RADIO COVERAGE

NEW BUILDINGS SHALL BE PROVIDED WITH EMERGENCY RESPONDER RADIO COVERAGE IN ACCORDANCE WITH CALIFORNIA FIRE CODE SECTION 510. THE PROJECT ARCHITECT (AOR) SHALL CONTACT THE LOCAL FIRE DEPARTMENT AND/OR EMERGENCY COMMUNICATIONS AUTHORITY TO OBTAIN DESIGN, EQUIPMENT SPECIFICATIONS, TESTING AND ACCEPTANCE CRITERIA. PLANS AND REQUEST DOCUMENTATION SHALL BE SUBMITTED TO THE LOCAL AUTHORITY HAVING JURISDICTION FOR REVIEW AND APPROVAL UPON COMPLETION. COPIES OF THE APPROVED PLANS, EQUIPMENT DATA SHEETS, TESTING AND ACCEPTANCE DOCUMENTATION SHALL BE PROVIDED TO THE SCHOOL DISTRICT.

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SACRAMENTO, CA 95816
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DESIGN FOR GOOD
JANISSE PEARSON
C-35691
REN. 05/31/27
STATE OF CALIFORNIA

ISSUE

DESCRIPTION	DATE
02.01 (E) FIRE HYDRANT	
02.08 (E) 20'-0" WIDE FIRE ACCESS GATE W/ KNOX BOX PER DSA #02-120455	
02.09 (E) 6' WIDE GATE, PROVIDE KNOX BOX	
02.10 6'-0" WIDE GATE WITH PANIC HARDWARE, PROVIDE KNOX BOX	

KEYNOTES

- 02.01 (E) FIRE HYDRANT
- 02.08 (E) 20'-0" WIDE FIRE ACCESS GATE W/ KNOX BOX PER DSA #02-120455
- 02.09 (E) 6' WIDE GATE, PROVIDE KNOX BOX
- 02.10 6'-0" WIDE GATE WITH PANIC HARDWARE, PROVIDE KNOX BOX

NOTES

1. EMERGENCY ACCESS LANES SHOWN ARE EXISTING TO REMAIN PER DSA #02-111648
2. HYDRANTS INCLUDED IN FIRE FLOW ANALYSIS ARE OFF SITE.

FACILITY:

LAKEWOOD ELEMENTARY SCHOOL
1100 N HAM LN,
LODI, CA 95242

PROJECT:

LAKEWOOD ELEMENTARY SCHOOL SHADE STRUCTURE

SHEET NAME:

LOCAL FIRE AUTHORITY SITE ACCESS PLAN

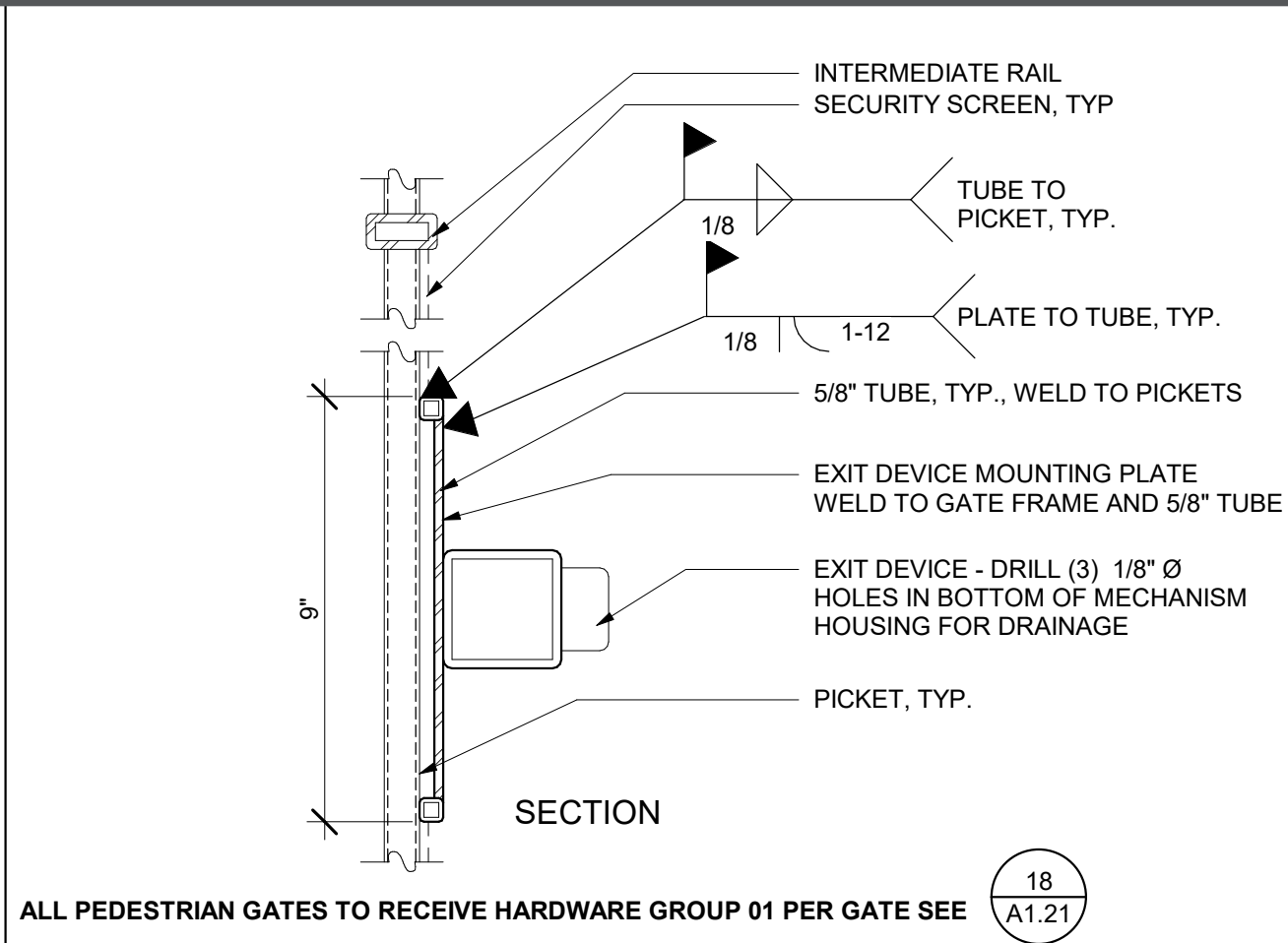
CONSTRUCTION DOCUMENTS

DATE: 2025.12.09

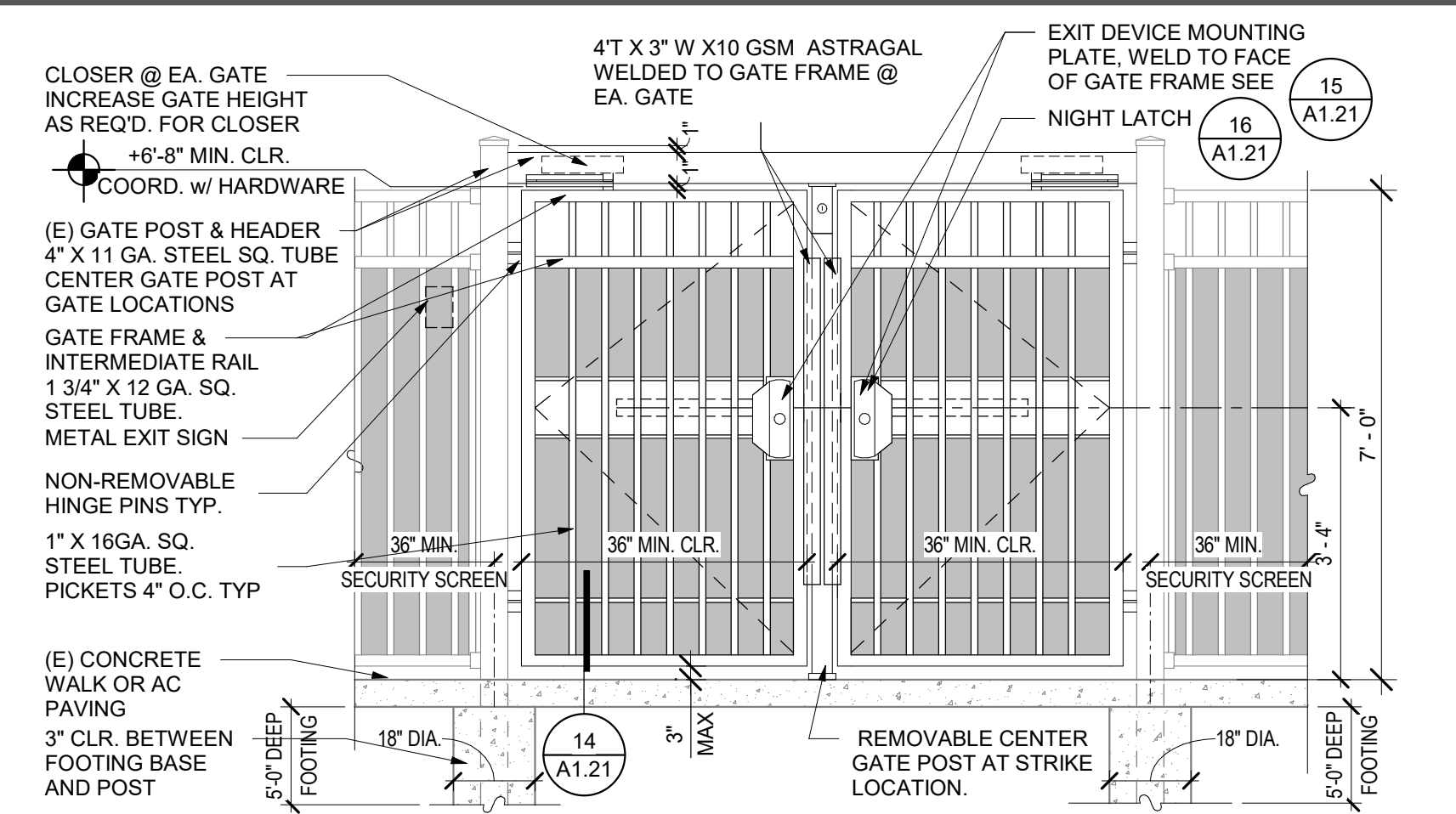
CLIENT PROJ NO:

SHEET:

Autodesk Docs: 02421013000 Lodi USD LAKWOOD ES SHADE STRUCTURE 02421013000-A-LUSD-LAKWOOD ES SHADE STRUCTURE 12/12/2025 11:55:02 AM



NOTES:
1. ALL CAVITIES BETWEEN KICK PLATE AND FENCING MATERIAL MUST BE CAPPED ON ALL SIDES
2. ALL MATERIALS ARE GALVANIZED TO BE POWDER COATED BLACK 2-3 MILS MIN.
3. SECURITY SCREEN (SHOWN SHADED) CONTINUOUS BACK SIDE OF PICKETS
4. NO MANUALLY OPERATED CANE BOLTS ARE ALLOWED ON GATES WITH EXIT DEVICES
5. ALL PEDESTRIAN DECORATIVE METAL FENCE GATES TO RECEIVE HARDWARE GROUP 01 PER GATE SEE 18 A1.21



LEGEND		
X	EXISTING BUILDINGS	
—	(E) CHAIN LINK FENCE	
—	(E) DECORATIVE METAL FENCE	
---	PROPERTY LINE	

EXTERIOR COLOR SCHEDULE		
	MATERIAL	COLOR
M1	METAL	METAL FINISH COLOR 1
M2	METAL	METAL FINISH COLOR 2
M3	METAL	METAL PREFINISH COLOR 3
M4	METAL	METAL PREFINISH COLOR 4

- CUSTOM COLORS TO BE SUBMITTED PER SPECIFICATIONS
- CLEAN, PREP AND PAINT ALL SURFACES NOT PREFINISHED

DECORATIVE METAL GATE HARDWARE

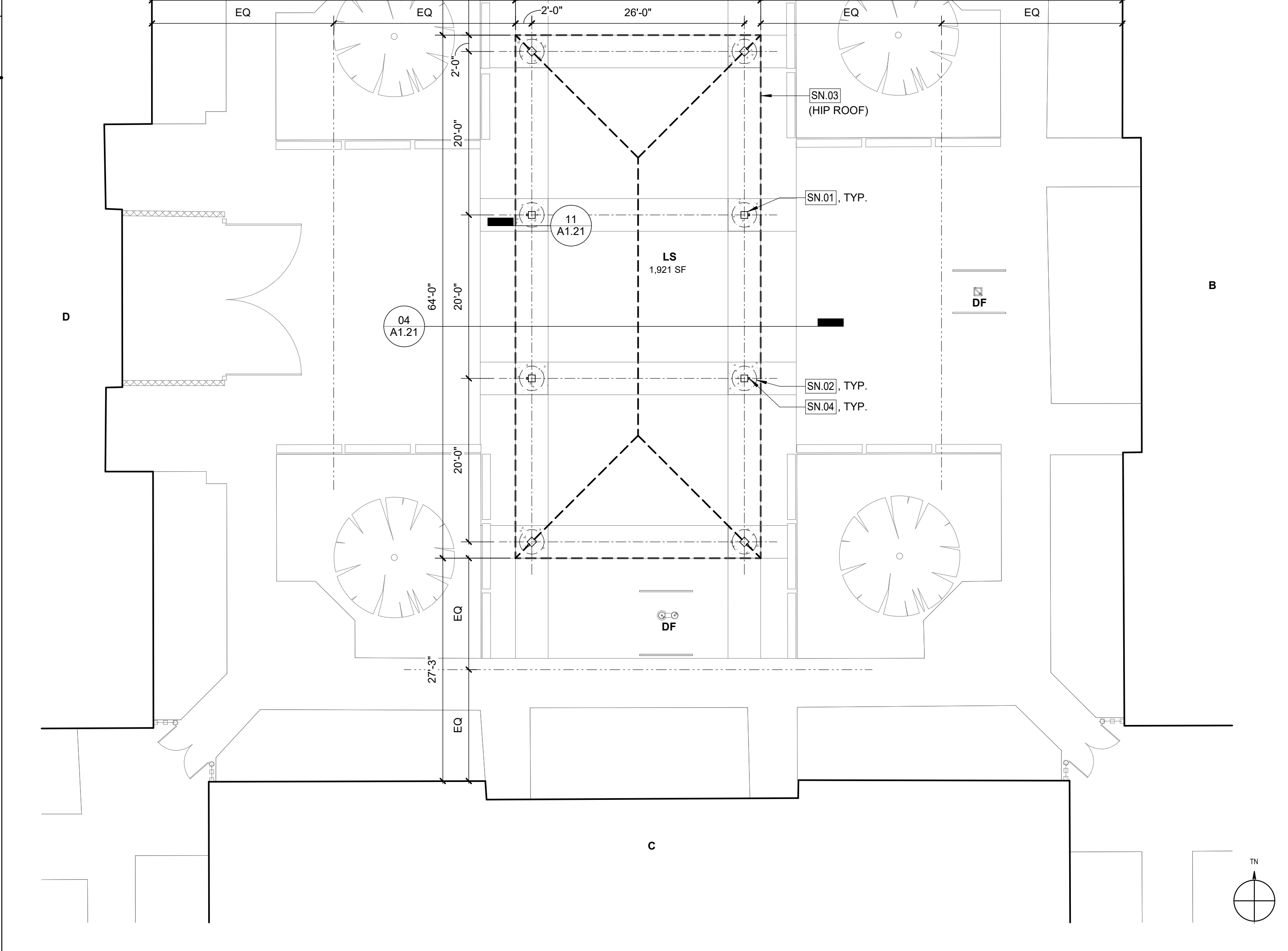
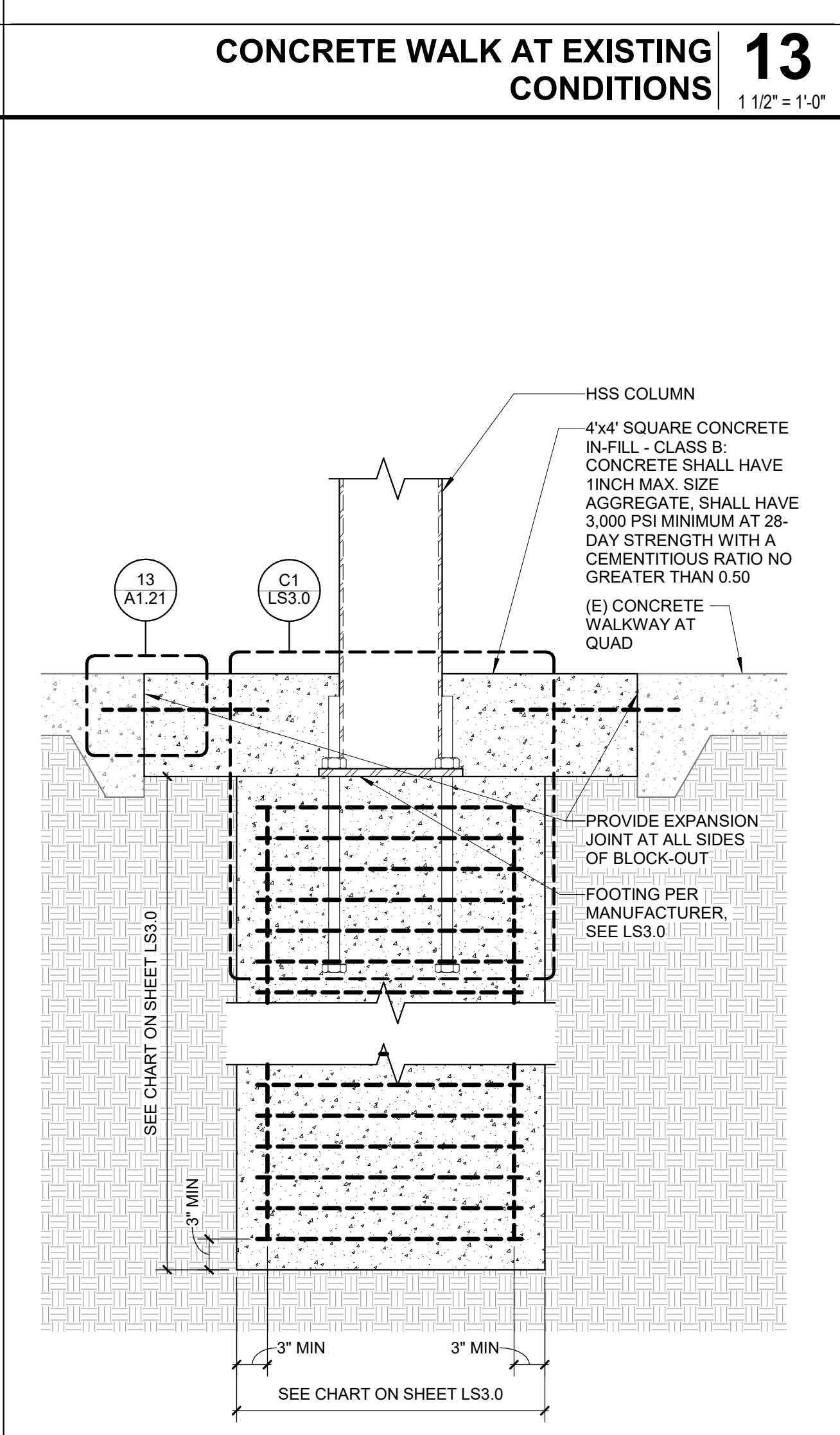
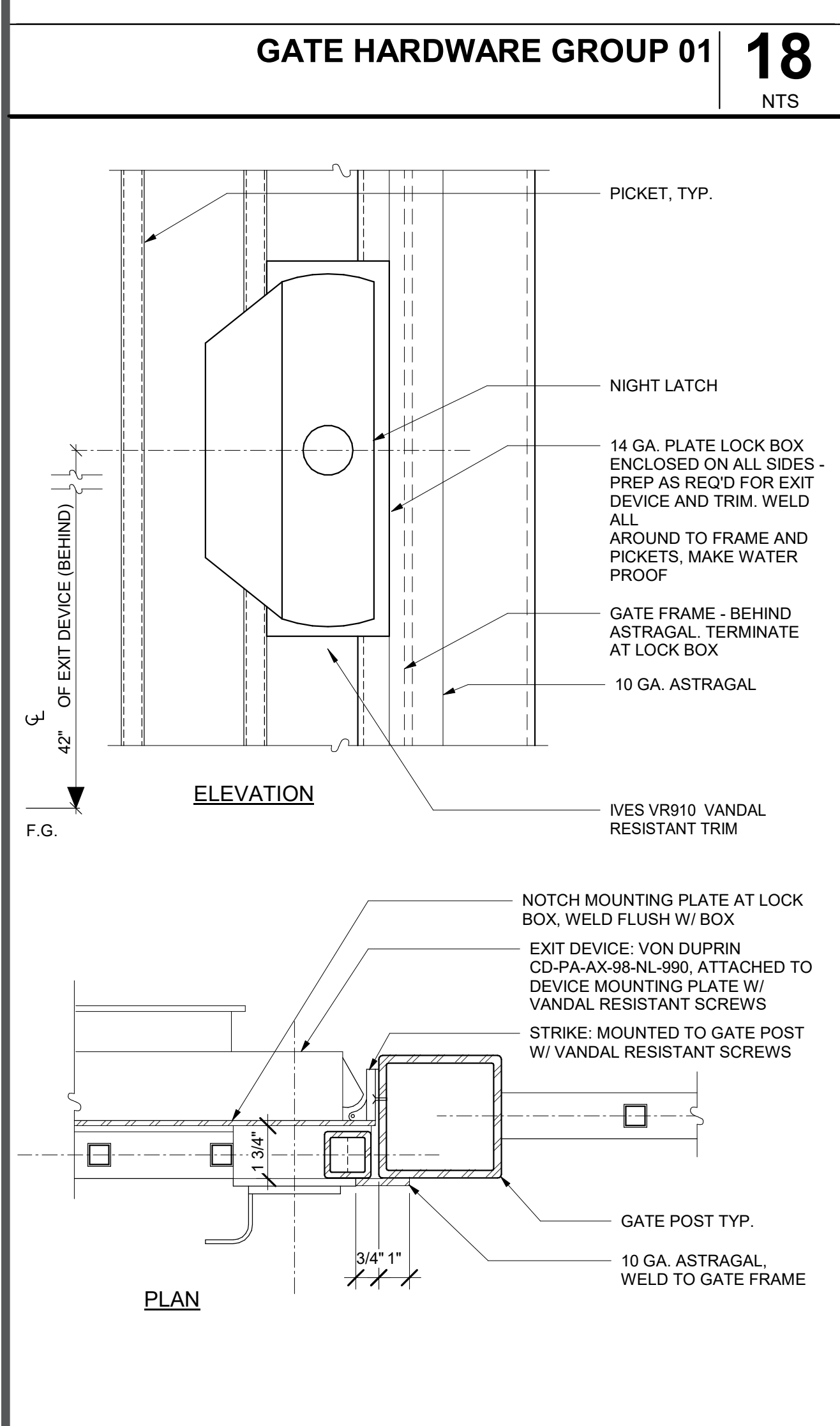
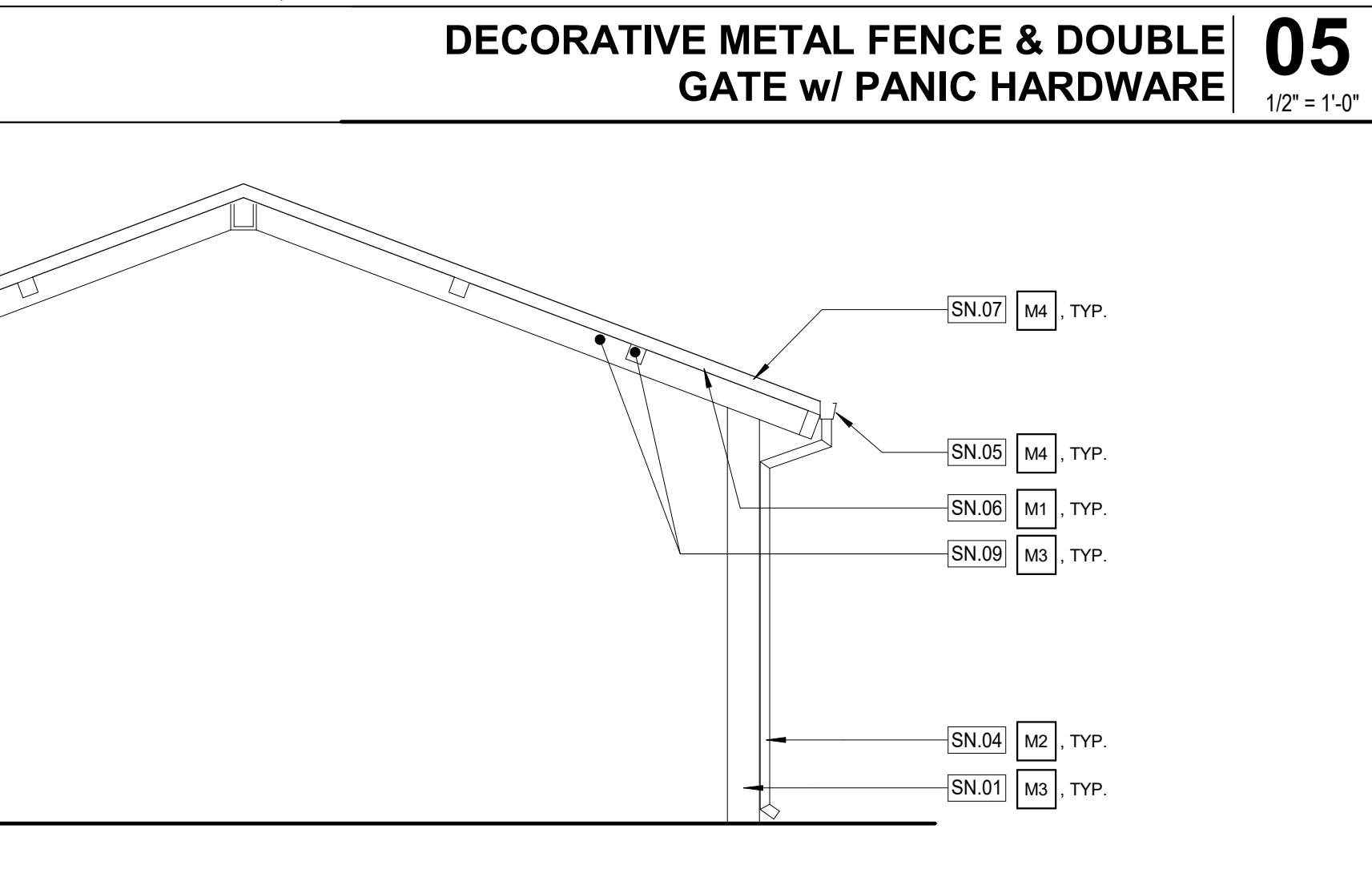
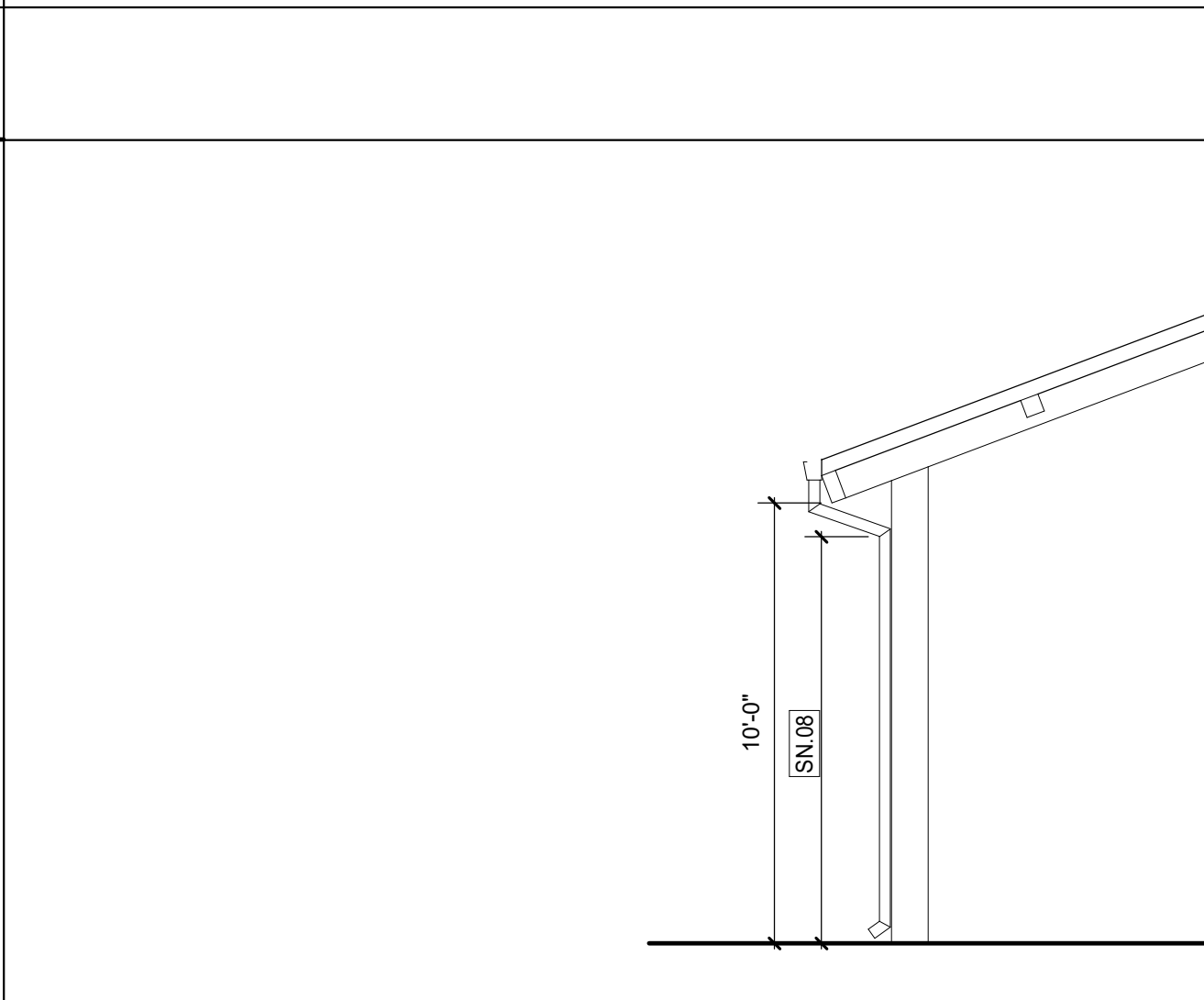
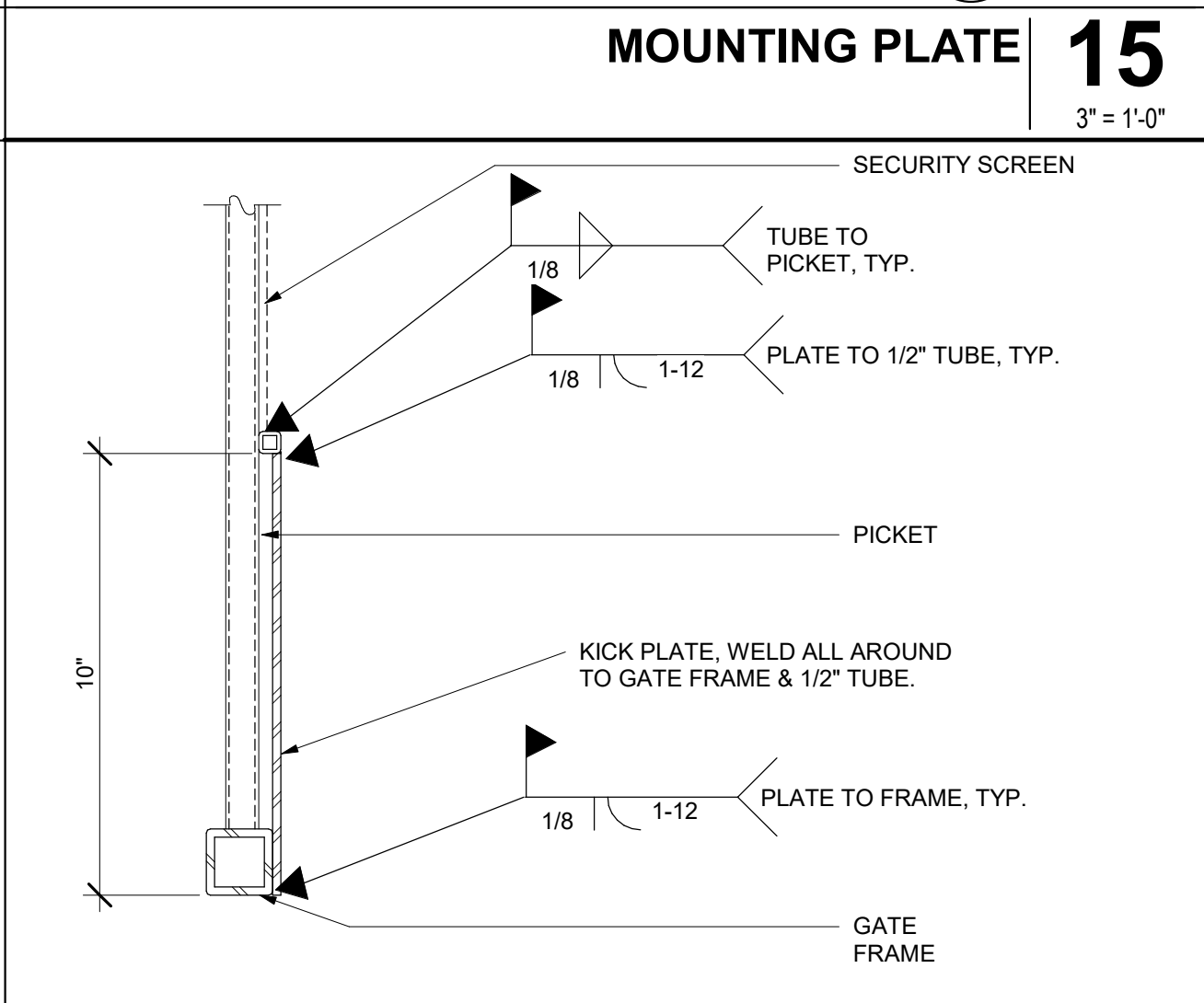
- ALL HARDWARE SHALL MEET CBC AND ADA REQUIREMENTS FOR ACCESSIBILITY WHEN IN CONFLICT, COMPLIED WITH THE MOST STRINGENT.
REFER TO HARDWARE GROUP FOR PEDESTRAIN GATES:
- HINGES:
A. GATE LEAF 6'-0" WIDE OR LESS SHALL RECEIVE THREE NO. 1-3030 HEAVY DUTY BUTT HINGES BY KING ARCHITECTURAL METAL OR EQUAL PER LEAF
B. GATE LEAF 6'-0" WIDE AND LESS THAN 12'-0" WIDE SHALL RECIEVE THREE NO. 1-4040 EXTRA HEAVY DUTY BUTT HINGES BY KING ARCHITECTURAL METAL OR EQUAL PER LEAF
- LOACKABLE CANE BOLT NO. 30-6020 EXTRA HEAVY DUTY CANE BOLT BY KING ARCHITECTURAL METALS OR EQUAL PER LEAF
A. PROVIDE AT ALL GATES EXCEPT PEDESTRIAN GATES
- N-GROUND CANE BOLT SLEEVE: 12" LONG STEEL PIPE SIZED TO MATCH CANE BOLT PROVIDE FOR EACH LEAF AT BOTH THE OPEN AND CLOSED POSITIONS. WHERE INSTALLED IN LOCATIONS OTHER THAN CONCRETE, PROVIDE CENTERED IN A 12"x12"x16" CONCRETE PAD
- MISCELLANEOUS HARDWARE: ITEMS OF HARDWARE, INCLUDING PADLOCK HASPS AND EYES SHALL BE FABRICATED BY THE FENCE MANUFACTURER
A. KICK PLATES - 12 GAUGE GALV. STEEL SHEET, 10" HIGH x GATE WIDTH
B. EXIT DEVICE MOUNTING PLATE - 12 GAUGE GALV. STEEL SHEET x GATE WIDTH ASTRAGAL - 10 GAUGE GALV. STEEL SHEET
- PEDESTRIAN GATE HARDWARE - GROUP G01
A. HINGES - THREE NO. 1-3030 HEAVY DUTY BUTT HINGES BY KING ARCHITECTURAL
B. PANIC HARDWARE - VON DUPRIN CD-AX-PA-98-EO
C. GATE PULL - IVES VR910 NL
D. RIM CYLINDER - SCHLAGE 20-740
E. MORTISE CYLINDER (DOGGING) - SCHLAGE 26-091 ICX XQ11-948
F. FINAL CORKS - SCHLAGE PRIMUS 20-740
G. SURFACE CLOSER - LCN 4111 AVB EDA SRI
H. GATE STOPHOLDER - CON DUPRIN FS43 (MOUNTED TO 12"x12"x6" MIN. CONCRETE PAD)

SECURITY SCREEN: 16-GAUGE, GALVANIZED AND PERORATED STEEL SHEET WITH 3/16-INCH HOLES STAGGERED AT 14-INCHES ON CENTER, WELDED TO THE FABRICATED GATE OR FENCE PANEL ASSEMBLY. HOT-DIP GALVANIZED, BASED ON MCNICHOLS ITEM #1431141648. SECURITY FABRIC U-EDGING: 14 GA. GALVANIZED HOT ROLLED U SHAPED EDGING, 1" TALL FACE x 3/8" OPENING WIDTH. BASIS OF DESIGN IS MCNICHOLS QUALITY U-EDGING, ITEM NUMBER #4003801410 (WWW.MCNICHOLS.COM)

FABRICATION

- PICKETS, RAILS, AND POSTS SHALL BE PRECUT TO SPECIFIED LENGTHS. RAILS SHALL BE PRE-PUNCHED TO ACCEPT PICKETS.
- PICKETS SHALL BE INSERTED INTO THE PRE-PUNCHED HOLES IN THE RAILS AND SHALL BE ALIGNED TO STANDARD SPACING USING A SPECIALLY CALIBRATED ALIGNMENT FIXTURE. EACH PICKET AND RAIL SHALL BE JOINED USING THE FUSION WELDING PROCESS THUS COMPLETING THE RIGID PANEL ASSEMBLY. COMPLETED PANELS SHALL BE CAPABLE OF SUPPORTING A 600-POUND LOAD APPLIED AT MIDSPAN WITHOUT PERMANENT DEFORMATION. PANELS SHALL BE BIASABLE TO A 25 PERCENT SLOPE IN GRADE.
- GATES:
1. FABRICATE USING MANUFACTURER'S STANDARD MATERIALS AND COMPONENTS EXCEPT UPRIGHTS SHALL BE 11 GAUGE, 2-INCH SQUARE TUBING
2. RAIL AND UPRIGHT INTERSECTIONS SHALL BE JOINED BY WELDING
3. PICKET AND RAIL INTERSECTIONS SHALL BE JOINED BY THE SAME WELDING PROCESS USED FOR PANEL ASSEMBLY

SELF-CLOSING GATES TO COMPLY WITH CBC AND ADA GUIDELINES FOR ACCESSIBLE DESIGN, SECTION 404.2.8.2.



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ISSUE

DESCRIPTION	DATE
-------------	------

KEYNOTES

SN.01	HSS COLUMN PER PC #04-122375
SN.02	DRILLED PIER FOOTING PER PC #04-122375
SN.03	ROOF OVERHANG ABOVE
SN.04	3" DIAMETER SCHEDULE 40 STEEL DOWNSPOUT PER PC #04-122375
SN.05	PREFINISHED GUTTER AND FLASHING PER PC #04-122375
SN.06	PAINT DECK UNDERSIDE
SN.07	PREFINISHED STANDING SEAM ROOF PER PC #04-122375
SN.08	HOLD TIGHT TO UNDERSIDE OF FRAMING: MAINTAIN 9'-0" CLEARANCE MIN.
SN.09	HSS BEAM PER PC #04-122375

NOTES

- REFER TO SHEET G0.10 FOR TYPICAL SYMBOLS AND ABBREVIATIONS
- CONTRACTOR IS RESPONSIBLE FOR REPAIR/REPLACEMENT OF ALL HARDSCAPE OUTSIDE OF LIMIT OF WORK

FACILITY:
LAKEWOOD ELEMENTARY SCHOOL
1100 N HAM LN,
LODI, CA 95242

PROJECT:
LAKEWOOD ELEMENTARY SCHOOL SHADE STRUCTURE

SHEET NAME:
PROJECT SITE PLAN

CONSTRUCTION DOCUMENTS

DATE: 2025.12.09	CLIENT PROJ NO:
------------------	-----------------

SHEET:

PLEASE RECYCLE

A1.21

DESIGN CRITERIA

BASE LOCATION LOCATED AT BOTTOM OF BASEPLATE/TOP OF FOOTING

DESCRIPTION

DESIGN VALUES

DEAD AND LIVE LOADS

ROOF LIVE LOAD

20 PSF

ROOF DEAD LOAD (SUPERIMPOSED ON FRAME)

5 PSF MAX

ROOF PANEL DEAD LOAD

M=1.1 PSF, G = 1.2 PSF, S = 1.3 PSF

COLLATERAL DEAD LOAD

M = 3.9 PSF, G = 3.8 PSF, S = 3.7 PSF

ROOF LIVE LOAD

ROOF LIVE LOAD, L_r

20 PSF

ROOF SNOW LOAD

GROUND SNOW LOAD, P_g

20 PSF

RISK CATEGORY

II

ROOF SNOW LOAD: SLOPED, P_s

20 PSF

FOR SNOW LOAD CONDITIONS ONLY - SITE APPLICATION REVIEWER SHALL VERIFY THE STTUCTURE SHALL BE LOCATED AT LEAST 20 FEET FROM ANY ADJACENT STRUCTURE FOR SNOW DRIFT.

SNOW LOAD SLOPE FACTOR, C_s

1.0

SNOW LOAD EXPOSURE FACTOR, C_e

1.0

SNOW LOAD IMPORTANCE FACTOR, I_s

1.0

THERMAL FACTOR, C_t

1.2

LOWEST ANTICIPATED SERVICE TEMPERATURE

30°

WIND DESIGN

BASIC WIND SPEED (3 SECOND GUST), V_{ult}, V_{bsd}

100 MPH, 78 MPH

RISK CATEGORY

II

EXPOSURE CATEGORY

C

FACTORS: K_z, K_{zt}, K_d

0.85, 1.0, 0.85

q_h = 0.00256 K_z K_{zt} K_d V²

18.50 PSF

C_{mh} PER ASCE FIGURE 27.3-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED

CASE A (1.1 / -1.2) CASE B (0.01 / -0.69)

C_{ml} PER ASCE FIGURE 27.3-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED

CASE A (-0.17 / -1.09) CASE B (-0.96 / -1.65)

C_u PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (< h)

CASE A (-0.8 / -1.2) CASE B (0.8 / 0.5)

C_u PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (> h, < 2h)

CASE A (-0.6 / -0.9) CASE B (0.5 / 0.5)

C_u PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (>2h)

CASE A (-0.3 / -0.6) CASE B (0.3 / 0.3)

COMPONENTS & CLADDING - C_u (PRESSURE/SUCTION) CLEAR / OBSTRUCTED

ZONE 3 - (2.29 / -2.11) / (1.0 / -3.0)

ZONE 2 - (1.77 / -1.63) / (0.8 / -2.3)

ZONE 1 - (1.15 / -1.05) / (0.5 / -1.5)

SEISMIC DESIGN

LATERAL FORCE RESISTING SYSTEM

STEEL - ORDINARY CANTILEVER COLUMN

ANALYSIS PROCEDURE

EQUIVALENT LATERAL FORCE

SEISMIC IMPORTANCE FACTOR, I_s

1.0

SEISMIC SITE CLASS

D

MCE_R SPECTRAL RESPONSE ACOCELERATION @ 0.2 s, S_{0.2}

2.60

MCE_T SPECTRAL RESPONSE ACOCELERATION @ 0.2 s, S_T

0.90

SHORT PERIOD SITE COEFFICIENT, F_a

1.20

LONG PERIOD COEFFICIENT, F_v

1.70

FUNDAMENTAL PERIOD OF THE STRUCTURE, T (WORST CASE FOR ALL STRUCTURES)

0.152 s

DESIGN SPECTRAL RESPONSE ACOCELERATION AT SHORT PERIOD, S_{0.2}

2.08 □

DESIGN SPECTRAL RESPONSE ACOCELERATION AT SHORT PERIOD, S_{0.2} - USED TO DETERMINE C_s (WITH CAP PER ASCE 7.12.8.1.3) SOIL PROPERTIES MAY NOT BE CLASSIFIED AS SITE CLASS E.

DESIGN SPECTRAL RESPONSE ACOCELERATION AT 1-s PERIODS, S_{0.1}

2.08 * 0.70 = 1.456 □

SEISMIC DESIGN CATEGORY

E

SITE SPECIFIC RESPONSE ANALYSIS NOT REQUIRED PER ASCE 7.11.4.8 EXCEPTION 2

T_s = 0.49 s T < 1.5 * T_s

RESPONSE MODIFICATION FACTOR, R

1.25

OVERSTRENGTH FACTOR, Q

1.25

REDUNDANCY FACTOR, ρ

1.3

HORIZONTAL OR VERTICAL IRREGULARITIES

NONE

SEISMIC RESPONSE COEFFICIENT, C_s (20' WIDE, 30' WIDE, 40' WIDE)

1.16 1.00 1.00

DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)

12.73 PSF [] 13.41 PSF [X] 14.65 PSF []

ALLOWABLE SOIL BEARING FOR FOUNDATIONS

VARIES - SEE FOUNDATION CHARTS

FLOOD DESIGN - DESIGN IS ASSUMED TO NOT BE IN FLOOD HAZARD AREA

IF PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED & SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED.

STRUCTURAL SEPARATION

ALL DEFLECTIONS SHOWN ALSO INCLUDE THE P-DELTA ROTATION PER IBC-7

DEFLECTIONS ARE FOR (1) STRUCTURE

SOIL CLASSES PER CBC TABLE 1806A.2

MAXIMUM DRIFT δ_{max} SIDE COLUMNS

20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

MINIMUM SEPARATION (δ_m = C_d δ_{mmax}) C_d = 1.25

20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

MAXIMUM DRIFT δ_{max} END COLUMNS

20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

MINIMUM SEPARATION (δ_m = C_d δ_{mmax}) C_d = 1.25

20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) (INCHES)

Soil Class 5 Soil Class 4 Soil Class 3

[] 2.40 [] 2.55 [] 2.65

[X] 2.15 [] 2.30 [] 2.40

[] 2.20 [] 2.20 [] 2.30

[] 3.00 [] 3.19 [] 3.31

[X] 2.69 [] 2.88 [] 3.00

[] 2.75 [] 2.75 [] 2.88

Soil Class 5 Soil Class 4 Soil Class 3

[] 2.40 [] 2.55 [] 2.65

[X] 2.15 [] 2.30 [] 2.40

[] 2.20 [] 2.20 [] 2.30

[] 3.00 [] 3.19 [] 3.31

[X] 2.69 [] 2.88 [] 3.00

[] 2.75 [] 2.75 [] 2.88

INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWINGS TO DSA:

BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:

STEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT
-HIP STRUCTURES UP TO 20' WIDE USE THE "RH 20" BASE FRAME
-HIP STRUCTURES UP TO 30' WIDE USE THE "RH 30" BASE FRAME
-HIP STRUCTURES UP TO 40' WIDE USE THE "RH 40" BASE FRAME
-MAXIMUM WIDTH IS 40' (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE)
-THE 24', 44', 64', 84' AND 104' LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON (20' BAYS ARE THE MOST ECONOMIC)
-FRAME LENGTHS ASSUME 2' OVERHANGS (UNO BY ARCHITECT - 2' MAX DIMENSION)

STEP 1	FRAME DIMENSIONS					
	SUGGESTED			OTHER		
	FRAME WIDTH	[] 20'	[X] 30'	[] 40'	[] (40' MAX)	
FRAME LENGTH	[] 44'	[X] 64'	[] 84'	[] 104'	[] (NO MAX)	

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT
-“M” REPRESENTS MCELROY METAL “MULTI-RIB” ROOF PANEL
-“G” REPRESENTS MCELROY METAL “MEGA-RIB” ROOF PANEL
-“S” REPRESENTS MCELROY METAL “MEDALLION-LOK” 16” STANDING SEAM ROOF PANEL

STEP 2	ROOF PANEL		
	ROOF PANEL TYPE	[] M	[] G

STEP 3: IDENTIFY THE S_{ss} ACCELERATION (g) FOR YOUR PROJECT
-S_{ss} VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES
-S_{ss} VALUE DEPENDS ON THE PROJECTS GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73)
-FIND S_{ss} VALUES FOR YOUR PROJECT ON THE USGS WEBSITE (SEARCH INTERNET FOR "USGS SEISMIC DESIGN MAPS")

STEP 3	PROJECT SITE - S _{ss} ACCELERATION (g)
	0.614

STEP 4: IDENTIFY THE S_{ss} REGION FOR YOUR PROJECT
-THE REGIONS ARE DEPENDANT ON THE S_{ss} VALUE DETERMINED IN STEP 3
-THE S_{ss} REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME

STEP 4	S _{ss} REGION			
	DESCRIPTION	S _{ss} REGIONS	MAX DEAD LOAD	
		X	0 < S _{ss} <= 2.14	5 PSF
			2.14 < S _{ss} <= 2.50	5 PSF
			2.50 < S _{ss} <= 2.60	5 PSF

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT
- THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED
- THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME
- BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4 FOR YOUR S_{ss} VALUE
- S_{ss} VALUE USED IN CALCULATION IS THE CAPPED S_{ss} (SEE DESIGN CRITERIA)

STEP 5	TOTAL ROOF DEAD LOAD	
	DEAD LOAD	EXAMPLES
	ROOF DECK	1.3 PSF M=1.1PSF; G=1.2PSF ;S=1.3PSF (SEE STEP 2)
	COLLATERAL	PSF LIGHTNING,FIRE SUPPRESSION,SOLAR PANELS,ETC
TOTAL	1.3 PSF	ADD ROOF DECK AND COLLATERAL LOADS (MAX 5 PSF)

STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT
-IDENTIFY SOIL CLASS FOR PROJECT SITE PER SITE SPECIFIC SOIL CONDITIONS
-USE THIS TO SELECT CORRECT FOUNDATION SIZE ON FOUNDATION SHEET

STEP 6	FOUNDATION REQUIREMENTS		
	[X] GEOTECHNICAL REPORT NOT REQUIRED	[] GEOTECHNICAL REPORT REQUIRED	
	SOIL CLASS 5 (BEARING) 1500 PSF [X]	SOIL CLASS 4 (BEARING) 2000 PSF []	SOIL CLASS 3 (BEARING) 3000 PSF []
	SOIL CLASS 5 (LATERAL BEARING) 200 PSF/FT	SOIL CLASS 5 (LATERAL BEARING) 300 PSF/FT	SOIL CLASS 5 (LATERAL BEARING) 400 PSF/FT
COHESION 130 PSF	FRICITION COEFFICIENT 0.25	FRICITION COEFFICIENT 0.30	

- SELECT AND VERIFY MINIMUM SEPARATION DISTANCE BETWEEN STRUCTURES

STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT
-MAXIMUM CLEAR HEIGHT IS 12'-0"; (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE)
-MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

STEP 7	MISCELLANEOUS		
	DESIGN OPTIONS		
	CLEAR HEIGHT	[] 8' [X] 10' [] 12' MAX	
	ELECTRICAL CUTOUTS	[] YES	[X] NO
GUTTERS	[X] YES	[] NO	

STEP 8: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT
-REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF PANEL TYPE (STEP 2)
-IDENTIFY THE APPLICABLE SHEET INDEX

STEP 8	SHEET INDEX											
	BASE FRAME			RH 20			RH 30			RH 40		
	ROOF PANEL TYPE	M	G	S	M	G	S	M	G	S		
	SELECT ONE	[X]	[]	[]	[]	[]	[X]	[]	[]	[]		
	GENERAL NOTES	LS1.0	LS1.0	LS1.0	LS1.0	LS1.0	LS1.0	LS1.0	LS1.0	LS1.0		
	FOUNDATION PLAN	LS2.0	LS2.0	LS2.0	LS3.0	LS3.0	LS3.0	LS4.0	LS4.0	LS4.0		
	FRAMING PLAN	LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.1	LS4.1	LS4.1		
	FRAME CONNECTION DETAILS	LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.2	LS4.2	LS4.2		
	ROOFING LAYOUT & DETAILS	LS2.2	LS2.2	LS2.4	LS3.2	LS3.3	LS3.4	LS4.3	LS4.3	LS4.5		
	DSA 103 EXAMPLE	LS1.2	LS1.2	LS1.3	LS1.2	LS1.2	LS1.2	LS1.2	LS1.2	LS1.2		
	MISC DESIGN OPTIONS	LS5.0	LS5.0	LS5.0	LS5.0	LS5.0	LS5.0	LS5.0	LS5.0	LS5.0		
	-											
	-											

STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL
-INCLUDE "MISC DESIGN OPTIONS" SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS

STEP 10: IDENTIFY PROJECT NAME AND LOCATION

PROJECT NAME:	SCHOOL DISTRICT:
LUNCH SHELTER	Lodi Unified School District

STEP 11: CROSS OUT EXAMPLE 103 FORMS & INCORPORATE REQUIRED SPECIAL INSPECTIONS 103 FORMS THAT ARE PROJECT SPECIFIC

SITE SPECIFIC PARAMETERS

INSTRUCTIONS: DESIGN PROFESSIONAL SHALL CHECK THE APPROPRIATE SELECTION BOXES BELOW AND ENTER THE DESIGN PARAMETERS APPLICABLE TO THE SPECIFIC PROJECT SITE

SNOW

p_g = 0 psf

P_f = psf

C_e = psf

WIND

V = 94 mph < XX mph

k_{zt} = 1 < 1

EXPOSURE: C [X] D []

SEISMIC

[X] DESIGN BASED ON SITE CLASS D

NO GEOTECHNICAL INVESTIGATION REQUIRED

S_{ss} = 0.614 F_o = 1.2

[] DESIGN BASED ON SITE CLASS DETERMINED PER CHAPTER 20 OF ASCE 7-16

GEOTECHNICAL INVESTIGATION PROVIDED

SITE CLASS: C [] D [] E []

S_{ss} = F_o = PER ASCE 7-16 SUPPL 3, TABLE 11.4-1

[] DESIGN BASED ON SITE SPECIFIC GROUND MOTION HAZARD ANALYSIS

PER CHAPTER 21 OF ASCE 7-16

SHORT-PERIOD DESIGN SPECTRAL RESPONSE PARAMETER, S_{ds}, SHALL BE AS SPECIFIED IN GEOTECHNICAL INVESTIGATION

COS APPROVAL REQUIRED

NOT ELIGIBLE FOR OTC REVIEW

SITE CLASS: C [] D [] E []

S_{ds} = F_o S_{ss} = 0.536

[X] SITE CLASS C or D: 0.7 x S_{ds}* = 0.7 x 0.536 = 0.375 < XXX

[] SITE CLASS E: S_{ds} = < XXX

C_s = XXX USED IN DESIGN

SEISMIC DESIGN CATEGORY D [X] E []

*SITE SPECIFIC S_{ds} VALUE BEFORE APPLYING REDUCTION

ALLOWED BY ASCE 7 SECTION 12.8.1.3

ABBREVIATIONS:			
ACI	AMERICAN CONCRETE INSTITUTE	MPH	MILES PER HOUR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	M	MULTI-RIB ROOF PANEL (MCELROY)
ASM	ASSEMBLY (INTERNAL REFERENCE)	NTS	NOT TO SCALE
ASTM	AMERICAN SOCIETY FOR TESTING AND MATLS	NO	NUMBER
AWS	AMERICAN WELDING SOCIETY	OC	ON CENTER
CBC	CALIFORNIA BUILDING CODE	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADMIN
CJP	COMPLETE JOINT PENETRATION	PCF	POUNDS PER CUBIC FOOT
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENCE
GA	GAGE	SQ	SQUARE
IN	INCHES	SS	STANDING SEAM ROOF PANEL (MCELROY)
KSI	KIPS PER SQUARE INCH	TYP	TYPICAL
MAX	MAXIMUM	UNO	UNLESS NOTED OTHERWISE
MIN	MINIMUM	USGS	U.S. GEOLOGICAL SURVEY
MISC	MISCELLANEOUS	W/	WITH

ARCHITECTURAL REQUIREMENTS

DESCRIPTION	DESIGN VAULES
TYPE OF CONSTRUCTION	II-B
OCCUPANCY CLASSIFICATION	A-3
NUMBER OF STORIES	1
FIRE SPRINKLER SYSTEM	NOT BY ICON/WEIGHT NOT INCLUDED IN DESIGN
MOST COMMON RH20 MIN/MAX SQ.FT (SEE STEP 1)	480/2,080
MOST COMMON RH30 MIN/MAX SQ.FT (SEE STEP 1)	720/3,120
MOST COMMON RH40 MIN/MAX SQ.FT (SEE STEP 1)	960/4,160

AREA OVER 4000 SQ.FT REQUIRES GEOHAZARD REPORT
ALLOWABLE ARE FOR II-B / A-3 IS 9500 SQ.FT

RELATED BUILDING CODES AND STANDARDS

TITLE 24 CODES:

2022 CALIFORNIA ADMINISTRATIVE CODE (CAC).....(PART 1, TITLE 24, CCR)
2022 CALIFORNIA BUILDING CODE (CBC),PART 2, TITLE 24 CCR
2022 CALIFORNIA ELECTRICAL CODE.....(PART 3, TITLE 24, CCR)
2022 CALIFORNIA MECHANICAL CODE (CMC).....(PART 4, TITLE 24, CCR)
2022 CALIFORNIA PLUMBING CODE (CPC).....(PART 5, TITLE 24, CCR)
2022 CALIFORNIA ENERGY CODE.....(PART 6, TITLE 24, CCR)
2022 CALIFORNIA FIRE CODE (CFC)(PART 9, TITLE 24, CCR)
2022 CALIFORNIA GREEN BUILDING STANDARDS CODE.....(PART 11, TITLE 24, CCR)
2022 CALIFORNIA REFERENCE STANDARDS CODE.....(PART 12, TITLE 24, CCR)
TITLE 19 CCR, PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS:
2022 CBC, CHAPTER 35
2022 CFC, CHAPTER 80

SCOPE OF WORK NARRATIVE

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRICATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF HOLLOW STRUCTURAL STEEL MEMBERS SUPPORTED BY CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN ALLOWS THE STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

PRE-CHECK (PC) DOCUMENT
Code: 2022 CBC
A separate project application for construction is required.

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-123929 INC:
REVIEWED FOR
SS [X] FLS [X] ACS [X]
DATE: 01/06/2026

ICON STD RH/DSA-PC

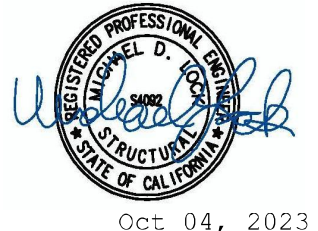
DRAWN BY: JD

DATE: 7/25/2023

REV

REV DATE

JRMA
ARCHITECTS ENGINEERS
2700 SATURN ST #BREA, CA 92821
T. 714.524.1870 F. 714.524.1875
WWW.JRMA.COM



Oct. 04, 2023

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122375 PC
REVIEWED FOR
SS [X] FLS [X] ACS [X] CG [X]
DATE: 10/10/2023

GENERAL INFO

ICON Shelter Systems Inc
DISTINCTIVE STEEL SHELTERS
WWW.ICONSHelters.COM
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616.396.0919
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LS1.0

GENERAL:

1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C., TITLE 24, AND ALL STATE AND FEDERAL REGULATIONS.
3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING WITH ANY WORK INVOLVED.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS, ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISIT TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION.
6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS, EXCEPT AS AMENDED BY CBC CHAPTER 35.
7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK.
10. THE SCHOOL DISTRICT INSPECTOR ON RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF INSTALLATION.
11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
13. MEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.

STRUCTURAL AND MISCELLANEOUS STEEL:

1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFICATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 KSI, GRADE B OR A501 UNLESS NOTED OTHERWISE.
3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A-500, GRADE B (OR C), Fy = 46 KSI MIN.
4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESS CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
5. ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A-36, Fy = 36 KSI.
6. ALL PLATE STEEL SHALL CONFORM TO ASTM A-572, Fy= 50 KSI.
7. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A-653, CS = TYPE B, Fy = 50 KSI Fu = 65 KSI
8. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
9. ALL ROOF DECKS SHALL HAVE KYNAR 500 METAL COATING.
10. ALL ROOF DECKS SHALL CONFORM TO ASTM A-792, Fy = 50 KSI.
11. ALL BASE CONNECTIONS ARE A PART OF THE LATERAL FORCE RESISTING SYSTEM

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-316(e) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS.
2. FOR THE SITE SPECIFIC PROJECT, J. R. MILLER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.
3. FOR THE SITE SPECIFIC PROJECT, J.R. MILLER & ASSOCIATES' RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC ONLY.
4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM J.R. MILLER & ASSOCIATES' RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT.
5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE BUT ARE NOT LIMITED TO, STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF OF THE VERIFIED REPORT FOR COMPLETED WORK.
6. J.R. MILLER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

CONSTRUCTION NOTES

1. A DSA-CERTIFIED CLASS 3 (MINIMUM) PROJECT INSPECTOR IS REQUIRED FOR THIS PROJECT.
2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.
3. A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.
5. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS ARE THAT ALL THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK, (SECTION 4-317(c), PART 1, TITLE 24, CCR)
6. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES

WELDING:

1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-lb @ (0° F).
3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE PROPER MATERIAL ID AND WELDING.
4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFICATIONS.

BOLTING:

1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE HOT DIPPED GALVANIZED ASTM F3125 GRADE A325 HIGH STRENGTH BOLTS (UNO), WITH THE NUTS CONFORMING TO HOT DIPPED GALVANIZED ASTM A-563 GRADE DH.
2. HIGH STRENGTH BOLTS SHALL BE VERIFIED AND INSPECTED PER CBC 1705A2.1.
3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS -- INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
4. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F-436.
5. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME.
BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6.
AP)RETENSIONED JOINTS MUST BE INSTALLED AND INSPECTED TO MEET ONE OF THE FOLLOWING REQUIREMENTS:
 1. TURN-OF-NUT PRETENSIONING: PER SECTION 8.2.1 OF THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, WASHERS ARE NOT REQUIRED FOR THIS METHOD, THE NUT OR HEAD SHALL BE ROTATED AS SPECIFIED IN TABLE 8.2. THE PART NOT TURNED SHALL BE PREVENTED FROM ROTATING.
 2. CALIBRATED WRENCH: PER THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, WASHERS ARE REQUIRED (NOT SUPPLIED BY ICON) THESE SHALL BE INSTALLED PER THE INSTALLATION TORQUE DETERMINED IN THE PRE-INSTALLATION VERIFICATION OF THE FASTENER ASSEMBLY PER SECTION 7. THE PART NOT TURNED SHALL BE PREVENTED FROM ROTATING.
 3. IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH "PT REQUIRED"
- B) ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF THE SNUG-TIGHTENED JOINTS. SNUG TIGHT CONDITION EXISTS WHEN ALL PILES IN A CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT BY THE BOLTS IN THE JOINT AND ALL OF THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

FOUNDATIONS:

1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER CBC TABLE 1806A, UNLESS NOTED OTHERWISE. PASSIVE PRESSURE IS ASSUMED TO START 12" BELOW TOP OF FOOTING.
2. PER CBC SECTION 1803A.2, GEOTECHNICAL REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS. ALLOWABLE FOUNDATION AND LATERAL SOIL PRESSURE VALUES MAY BE DETERMINED FROM TABLE 1806A.2.
3. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D-1557 OR AS RECOMMENDED BY THE GEO-TECH ENGINEER. FLOODING NOT PERMITTED.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORINGS, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
5. MINIMUM SETBACK FROM TOE OF SLOPE ON AN ASCENDING SLOPE SHALL BE 15 FEET AND MINIMUM SETBACK FROM TOE OF SLOPE ON A DESCENDING SLOPE SHALL BE 40 FEET
6. PER CBC SECTION 1803A.6, GEOHAZARD REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS.
7. GEOHAZRD REPORTS ARE TO COMPLY WITH DSA IR A-4 PER IR-7 SECTION 1.8
8. SITE SPECIFIC GEOTECHNICAL REPORT IS REQUIRED AT THE TIME OF SITE APPLICATION IF USING OTHER THAN CLASS 5 SOIL, PER DSA IR PC-7
9. LATERAL BEARING HAS BEEN INCREASED PER CBC 1806A.3.4 FOR THE 1/2" DEFLECTION & HAS BEEN DESIGNED FOR P-DELTA EFFECTS. NO 1/3 INCREASE HAS BEEN APPLIED.
10. MINIMUM CLEARANCE BETWEEN PIERS SHALL BE 8'-0".

CONCRETE:

1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

STRENGTH P _c (28 DAYS)	W/C RATIO (NON-AIR ENTRAINED)	W/C RATIO (AIR ENTRAINED)	SUMP (±")	UNIT WEIGHT (NORMAL WEIGHT)
5000 PSI	0.44	0.35	3"	150 PCF

2. CONCRETE MIX DESIGN PARAMETERS ARE GOOD FOR EXPOSURE CATEGORIES F0, F1 & F2. THE AIR ENTRAINMENT FOR THESE CATEGORIES SHALL BE AS FOLLOWS: F0-0, F1-4.5, F2-6
3. CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA.
4. AGGREGATES SHALL CONFORM TO THE ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.005. MAX AGGREGATE SIZE = 1".
5. CEMENT SHALL CONFORM TO ASTM C-150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS.
6. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT. ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED.
7. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
8. CONCRETE DURABILITY SHALL BE PER CBC 1904A.1 ACI 318-19, CHAPTER 19.
9. CONCRETE SHALL BE TESTED PER CBC 1903A, TABLE 1705A.3. AND ACI 318-19, SECTION 26.12.
10. NO ADMXTURE SHALL CONTAIN CALCIUM CHLORIDE.

REINFORCING STEEL:

1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A-615, AS FOLLOWS:
GR 60: (#4 BARS AND LARGER)
GR 40: (#3 BARS)
2. DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
A. CAST AGAINST EARTH 3"
B. CAST AGAINST FORM BELOW GRADE 2"
C. FORMED SLABS (#11 BAR & SMALLER).....3/4"
D. SLABS ON GRADE (FROM TOP OF SLAB) 1"
4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD.
5. REINFORCING SHALL BE LAP SPLICED PER ACI 318-19, SECTION 25.5.
6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
7. WELDING OF REINFORCING IS NOT ALLOWED.
8. REINFORCING STEEL SHALL BE INSPECTED PER CBC 1705A.3.

POWDER-COAT FINISH SYSTEM:

ALL BUILDINGS THAT HAVE A POWDER-COATED FINISH SHALL MEET THE FOLLOWING SPECIFICATIONS:

1. THE STEEL FRAME (HSS SECTIONS, COLD FORMED & PLATE STEEL) SHALL BE SHOT-BLASTED TO A NEAR WHITE CONDITION PER SSPC-10 SPECIFICATIONS.
2. THE STEEL SHALL BE WASHED IN A ZINC PHOSPHATE IN AN MINIMUM THREE STAGE ELECTRO DEPOSITION PRE-TREATEMENT PROCESS.
3. IMMEDIATELY FOLLOWING PRE-TREATMENT THE STEEL SHALL BE TOTALLY COATED IN AN EPOXY PRIMER TO A UNIFORM THICKNESS OF A MINIMUM OF 0.7 TO 0.9 MILS. THE E-COATING SHALL PROVIDE A MINIMUM OF 1000 HOURS OF SALT SPRAY CORROSION PROTECTION TO THE STEEL.
4. THE STEEL SHALL THEN HAVE A TGC POLYESTER COLOR COAT APPLIED OVER THE E-COATED SURFACE.
5. THE FINISH THICKNESS OF THESE APPLICATIONS SHALL BE A MINIMUM OF 8 TO 12 MILS.
6. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, & COLD FORMED STEEL ETC.) NOT POWDER-COATED SHALL BE PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3"(UNLESS NOTED OTHERWISE).

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DATE: 01/06/2026

ICON STD RH/DSA-PC

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DATE 7/25/2023

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REV DATE

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REGISTERED PROFESSIONAL
J. R. MILLER
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DIV. OF THE STATE ARCHITECT
Oct. 04, 2023

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122375 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 10/10/2023

GENERAL INFO

ICON Shelter Systems Inc

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LS1.1

PRE-CHECK (PC) DOCUMENT
Code: 2022 CBC
A separate project application for construction is required.

Application Number: 04-122188	School Name: PC Update	School District: PC Update
DSA File Number:	Increment Number:	Date Created: 2023-04-19 08:36:32

IMPORTANT: This form is only a summary list of structural tests and some of the special inspections required for the project. Generally, the structural tests and special inspections noted on this form are those that will be performed by the Geotechnical Engineer of Record, Laboratory of Record, or Special Inspector. The actual complete test and inspection program must be performed as detailed on the DSA approved documents. The appendix at the bottom of this form identifies work NOT subject to DSA requirements for special inspection or structural testing. The project inspector is responsible for providing inspection of all facets of construction, including but not limited to, special inspections not listed on this form such as structural wood framing, high-load wood diaphragms, cold-formed steel framing, anchorage of non-structural components, etc., per Title 24, Part 2, Chapter 17A (2022 CBC).

KEY TO COLUMNS	
1. TYPE	2. PERFORMED BY
Continuous – Indicates that a continuous special inspection is required	GE (Geotechnical Engineer) – Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative. LOR (Laboratory of Record) – Indicates that the test or special inspection shall be performed by a testing laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program (see CAC Section 4-335). PI (Project Inspector) – Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA. SI (Special Inspection) – Indicates that the special inspection shall be performed by an appropriately qualified/approved special inspector.
Periodic – Indicates that a periodic special inspection is required	
Test – Indicates that a test is required	

Table 1705A.6, Table 1705A.7, Table 1705A.8		
Application Number:	School Name:	School District:
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S5. RETAINING WALLS:			
Test or Special Inspection		Type	Performed By
☐	a. Placement, compaction and inspection of backfill.	Continuous	GE* 1705A.6.1. * By geotechnical engineer or his or her qualified representative. (See section S2 above).
☐	b. Placement of soil reinforcement and/or drainage devices.	Continuous	* By geotechnical engineer or his or her qualified representative.
☐	c. Segmental retaining walls; inspect placement of units, dowels, connectors, etc.	Continuous	* By geotechnical engineer or his or her qualified representative. See DSA IR 18-2.
☐	d. Concrete retaining walls.	Provide tests and inspections per CONCRETE section below.	
☐	e. Masonry retaining walls.	Provide tests and inspections per MASONRY section below.	

Table 1705A.3; ACI 318-19 Sections 26.12 & 26.13		
Application Number:	School Name:	School District:
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DSA File Number:	Increment Number:	Date Created:

C3. PRECAST CONCRETE (IN ADDITION TO SECTION C1):				
Test or Special Inspection		Type	Performed By	Code References and Notes
☐	a. Inspect fabrication of precast concrete members.	Continuous	SI	ACI 318-19 Section 26.13.
☐	b. Inspect erection of precast concrete members.	Periodic	SI	Table 1705A.3 Item 10. * May be performed by PI when specifically approved by DSA.
☐	c. For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high deformability elements (MDE or HDE) in structures assigned to Seismic Design Category D, E or F, inspect such connections and reinforcement in the field for: 1. Installation of the embedded parts 2. Completion of the continuity of reinforcement across joints. 3. Completion of connections in the field.	Continuous	SI	Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5
☐	d. Inspect installation tolerances of precast concrete diaphragm connections for compliance with ACI 550.5.	Periodic	SI	Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5

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06-1511-Durham	Independent Number:	Date Created:

S1. GENERAL:			
	Test or Special Inspection	Type	Performed By
☒	a. Verify that: • Site has been prepared properly prior to placement of controlled fill and/or excavations for foundations. • Foundation excavations are extended to proper depth and have reached proper material. • Materials below footings are adequate to achieve the design bearing capacity.	Periodic	GE*
		Code References and Notes	
		* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) form for exemptions.)	

Table 1705A.6, Table 1705A.7, Table 1705A.8		
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Application Number:	School Name:	School District:
4-122188	PC Update	PC Update
OSA File Number:	Increment Number:	Date Created:

C5. POST-INSTALLED ANCHORS:			
Test or Special Inspection	Type	Performed By	Code References and Notes
☐ a. Inspect installation of post-installed anchors	See Notes	SI*	1617A.1.19, Table 1705A.3 Item 4a (Continuous) & 4b (Periodic), 1705A.3.8 (See Appendix (end of this form) for exemptions). ACI 318-14 Sections 17.8 & 26.13. * May be performed by the project inspector when specifically approved by DSA.
☐ b. Test post-installed anchors.	Test	LOR	1910A.5. (See Appendix (end of this form) for exemptions.)

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SEA File Number:	Increment Number:	Date Created:

Application Number:	School Name:	School District:
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DSA File Number:	Increment Number:	Date Created:

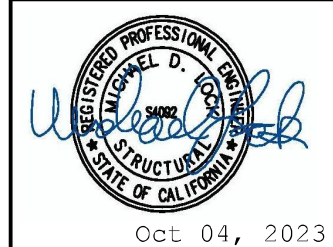
1705A.2.1, Table 1705A.2.1; AISC 303-16; AISC 341-16; AISC 358-16; AISC 360-16; AISI 100-20; PCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

Application Number: 04-122188	School Name: PC Update	School District: PC Update
PC Update	PC Update	PC Update

S/A2. HIGH-STRENGTH BOLTS:				
Test or Special Inspection		Type	Performed By	Code References and Notes
☒	a. Verify identification markings and manufacturer's certifications of compliance conform to ASTM standards specified in the DSA-approved documents.	Periodic	SI	Table 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3.2, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8; DSA IR 17-9.
☒	b. Test high-strength bolts, nuts and washers.	Test	LOR	Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR 17-8.
☒	c. Bearing-type "snug tight" connections.	Periodic	SI	Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N6; RCSC 2014 Section 9.1; DSA IR 17-9.
☒	d. Pretensioned and slip-critical connections.	*	SI	Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. "Continuous" or "Periodic" depends on the tightening method used.

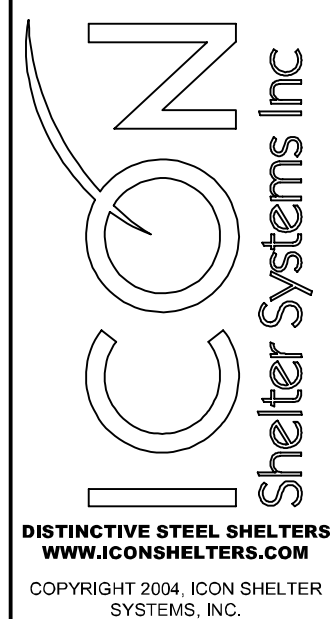
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DATE: 01/06/2026

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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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2023-04-19 08:36:32

S/A3. WELDING:				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☒ a. Verify weld filler material identification markings per AWS designation listed on the DSA-approved documents and the WPS.	Periodic	SI	1705A.2.5, Table 1705A.2.1 Items 4 & 5; AWS D1.1 and AWS D1.8 for structural steel; AWS D1.2 for Aluminum; AWS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-3.	
☒ b. Verify weld filler material manufacturer's certificate of compliance.	Periodic	SI	DSA IR 17-3.	
☒ c. Verify WPS, welder qualifications and equipment.	Periodic	SI	DSA IR 17-3.	

S/A4. SHOP WELDING (IN ADDITION TO SECTION S/A3):				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☒ a. Inspect groove welds, multi-pass fillet welds, single pass fillet welds > 5/16", plug and slot welds.	Continuous	SI	Table 1705A.2.1 Items 5a.1-4; AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.	
☒ b. Inspect single-pass fillet welds ≤ 5/16", floor and roof deck welds.	Periodic	SI	1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6; AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.	
☐ c. Inspect welding of stairs and railing systems.	Periodic	SI	1705A.2.1; AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3.	
☐ d. Verification of reinforcing steel weldability other than ASTM A706.	Periodic	SI	1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.	
☐ e. Inspect welding of reinforcing steel.	Continuous	SI	Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3.	

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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Test or Special Inspection		Type	Performed By	Code References and Notes
S/A8. SPRAYED FIRE-RESISTANT MATERIALS:				
Test or Special Inspection		Type	Performed By	Code References and Notes
☐	a. Examine structural steel surface conditions, inspect application, take samples, measure thickness and verify compliance of all aspects of application with DSA-approved documents.	Periodic	SI	1705A.15, 1705A.1, 1705A.2, 1705A.3, 1705A.4.
☐	b. Test density.	Test	LOR	1705A.15.1, 1705A.15.5, ASTM E736
☐	c. Bond strength adhesion/cohesion.	Test	LOR	1705A.15.1, 1705A.15.4, ASTM E605

S/A9. ANCHOR BOLTS AND ANCHOR RODS:				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☒ a. Anchor Bolts and Anchor Rods	Test	LOR	Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-11.	
☐ b. Threaded rod not used for foundation anchorage.	Test	LOR	Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11.	

S/A10. STORAGE RACK SYSTEMS:				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☐ a. Materials used, to verify compliance with one or more of the material test reports in accordance with the approved construction documents.	Periodic	SI	Table 1705A.13.7	
☐ b. Fabricated storage rack elements.	Periodic	SI	1704A.2.5; Table 1705A.13.7	

Appendix: Work Exempt from DSA Requirements for Structural Tests / Special Inspections

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Exempt items given in DSA IR A-22 or the 2019 CBC (including DSA amendments) and those items identified below with a check mark by the design professional are NOT subject to DSA requirements for the structural tests / special inspections noted. Items marked as **exempt** shall be identified on the approved construction documents. The project inspector shall verify all construction complies with the approved construction documents.

SOILS:	
☐	1. Deep foundations acting as a cantilever footing with a design based on minimum allowable pressures per CBC Table 1806A.2 and without a geotechnical report for the following cases: A) free standing sign or scoreboard, B) cell or antenna towers and poles less than 35'-0" tall (e.g., lighting poles, flag poles, poles supporting open mesh fences, etc.), C) single-story structure with dead load less than 5 psf (e.g., open fabric shade structure), or D) covered walkway structure with an apex height less than 10'-0" above adjacent grade.
☐	2. Shallow foundations, etc. are exempt from special inspections and testing by a Geotechnical Engineer for the following cases: A) buildings without a geotechnical report and meeting the exception item #1 criteria in CBC Section 1803A.2 supported by native soil (any excavation depth) or fill soil (not exceeding 12" depth per CBC Section 1804A.6), B) soil scarification/compaction not exceeding 12" depth, C) native or fill soil supporting exterior non-structural flatwork (e.g., sidewalks, site concrete ramps, site stairs, parking lots, driveways, etc.), D) unpaved landscaping and playground areas, or E) utility trench backfill.

CONCRETE/MASONRY:	
☐	1. Post-installed anchors for the following: A) exempt non-structural components (e.g., mechanical, electrical, plumbing equipment - see item 7 for "Welding" in the Appendix below) given in CBC Section 1617A.1.18 (which replaces ASCE 7-16, Section 13.1.4) or B) interior nonstructural wall partitions meeting criteria listed in exempt item 3 for "Welding" in the Appendix below
☐	2. Concrete batch plant inspection is not required for items given in CBC Section 1705A.3.3.2 subject to the requirements and limitations in that section.
☐	3. Non-bearing non-shear masonry walls may be exempt from certain DSA masonry testing and special inspection items as allowed per DSA IR 21-1. Refer to construction documents for specific exemptions accordingly for each applicable wall condition.
☐	4. Epoxy shear joints in site flatwork and/or other non-structural concrete.

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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PC Update
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2023-04-19 08:36:32

Test or Special Inspection	Type	Performed By	Code References and Notes
S/A5. FIELD WELDING (IN ADDITION TO SECTION S/A3):			
Test or Special Inspection	Type	Performed By	Code References and Notes
☐ a. Inspect groove welds, multi-pass fillet welds, single pass fillet welds > 5/16", plug and slot welds.	Continuous	SI	Table 1705A.2.1 Items 5a.1–4; AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.
☐ b. Inspect single-pass fillet welds ≤ 5/16".	Periodic	SI	Table 1705A.2.1 Item 5a.5; AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.
☐ c. Inspect end-welded studs (ASTM A-108) installation (including bend test).	Periodic	SI	2213A.2; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1; DSA IR 17-3.
☐ d. Inspect floor and roof deck welds.	Periodic	SI	1705A.2.2, Table 1705A.2.1 Item 5a.6; AISC 360-16 (AISC 341-16 as applicable); AWS D1.3; DSA IR 17-3.
☐ e. Inspect welding of structural cold-formed steel.	Periodic	SI*	1705A.3.5; AWS D1.3; DSA IR 17-3. The quality control provisions of AISI S240-20 Chapter D shall also apply. * May be performed by the project inspector when specifically approved by DSA.
☐ f. Inspect welding of stairs and railing systems.	Periodic	SI*	1705A.2.1; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3. * May be performed by the project inspector when specifically approved by DSA.
☐ g. Verification of reinforcing steel weldability.	Periodic	SI	1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.
☐ h. Inspect welding of reinforcing steel.	Continuous	SI	Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3.

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

Test or Special Inspection	Type	Performed By	Code References and Notes	
☐ c. Storage rack anchorage installation.	Periodic	SI	ANSI/MHI 6.1 Section 7.3.2; Table 1705A.13.7	
☐ d. Completed storage rack system to indicate compliance with the approved construction documents.	Periodic	SI*	Table 1705A.13.7; * May be performed by the project inspector when specifically approved by DSA.	

S/A11. Other Steel				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☐ a.				

Appendix: Work Exempt from DSA Requirements for Structural Tests / Special Inspections

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

CONCRETE/MASONRY:	
☐	5. Testing of reinforcing bars is not required for items given in CBC Section 1910A.2 subject to the requirements and limitations in that section.
WELDING:	
☐	1. Solid-clad and open-mesh fences, gates with maximum leaf span of 10', and gates with a maximum rolling section of 10' all having an apex height less than 8'-0" above lowest adjacent grade. When located above circulation or occupied space below, these gates/fences are not located within 1.5x gate/fence height (max 8'-0") to the edge of floor or roof.
☐	2. Handrails, guardrails, and modular or relocatable ramps associated with walking surfaces less than 30" above adjacent grade (excluding post base connections per the "Exception" language in Section 1705A.2.1); fillet welds shall not be ground flush.
☐	3. Non-structural interior cold-formed steel framing spanning less than 15'-0", such as in interior partitions, interior soffits, etc. supporting only self weight and light-weight finishes or adhered tile, masonry, stone, or terra cotta veneer no more than 5/8" thickness and apex less than 20'-0" in height and not over an exit way. Maximum tributary load to a member shall not exceed the equivalent of that occurring from a 10'x10' opening in a 15' tall wall for a header or king stud.
☐	4. Manufactured support frames and curbs using hot rolled or cold-formed steel (i.e., light gauge) for mechanical, electrical, or plumbing equipment weighing less than 2000# (equipment only) (connections of such frames to superstructure elements using welding will require special inspection as noted in selected item(s) for Sections S/A3, S/A4 and/or S/A5 of listing above).
☐	5. Manufactured components (e.g., Tolco, B-Line, Afcon, etc.) for mechanical, electrical, or plumbing hanger support and bracing (connections of such components to superstructure elements using welding will require special inspection as noted in selected item(s) for Sections S/A3, S/A4 and/or S/A5 of listing above).
☐	6. TV Brackets, projector mounts with a valid listing (see DSA IR A-5) and recreational equipment (e.g., playground structures, basketball backstops, etc.) (connections of such elements to superstructure elements using welding will require special inspection as noted in selected item(s) for sections S/A3, S/A4 and/or S/A5 located in the Steel/Aluminum category of listing above).
☐	7. Any support for exempt non-structural components given in CBC Section 1617A.1.18 (which replaces ASCE 7-16, Section 13.1.4) meeting the following: A) when supported on a floor/roof, <400# and resulting composite center of mass (including component's center of mass) ≤4' above supporting floor/roof, B) when hung from a wall or roof/floor, <20# for discrete units or <5 plf for distributed systems.

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

Test or Special Inspection		Type	Performed By	Code References and Notes
S/A6. NONDESTRUCTIVE TESTING:				
Test or Special Inspection		Type	Performed By	Code References and Notes
☐	a. Ultrasonic	Test	LOR	1705A.2.1, 1705A.2.5; AISC 341-16 J6.2, AISC 360-16 N5.5; AWS D1.1, AWS D1.8; DSA IR 17-2.
☐	b. Magnetic Particle	Test	LOR	1705A.2.1, 1705A.2.5; AISC 341-16 J6.2, AISC 360-16 N5.5; AWS D1.1, AWS D1.8; DSA IR 17-2.
☐	c.	Test	LOR	

S/A7. STEEL JOISTS AND TRUSSES:				
Test or Special Inspection	Type	Performed By	Code References and Notes	
☐ a. Verify size, type and grade for all chord and web members as well as connectors and weld filler material; verify joist profile, dimensions and camber (if applicable); verify all weld locations, lengths and profiles; mark or tag each joist.	Continuous	SI	1705A.2.3, Table 1705A.2.3; AWS D1.1; DSA IR 22-3 for steel joists only. 1705A.2.4; AWS D1.3 for cold-formed steel trusses.	

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (OTHER), 2022 CBC

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

X1. OTHER:			
Test or Special Inspection	Type	Performed By	Code References and Notes
☐ a. Load test for identified product(s).	Test	LOR	1709A.2, 1709A.3. Testing is not required for: 1) a product with a valid evaluation service report per DSA IR A-5, or 2) a product that can be justified by structural calculation.
☐ b. Installation torque for non-HS bolts	Continuous	SI*	Applicable to communication towers identified as Essential Service Facility Projects (ESFP). Calibrated wrench use required, verified by SI during installation. DSA Policy PL 18-01: Communication Towers, Poles and Buildings Utilized by State Agencies for Essential Services Communications.*EXCEPTION: Non-ESFP may use PI without need for notification to DSA.
☐ c.			

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS(SIGNATURE), 2022 CBC

Application Number:
04-122188
DSA File Number:

School Name:
PC Update
Increment Number:

School District:
PC Update
Date Created:
2023-04-19 08:36:32

Name of Architect or Engineer in general responsible charge:

Name of Structural Engineer (When structural design has been delegated):

Signature of Architect or Structural Engineer: _____ Date: _____

Note: To facilitate DSA electronic mark-ups and identification stamp application, DSA recommends against using secured electronic or digital signatures.

DSA STAMP

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-123929 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 01/06/2026

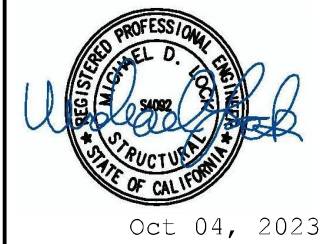
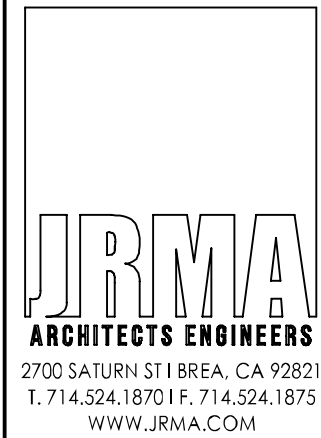
ICON STD RH/DSA-PC

DRAWN BY: JD

DATE: 7/25/2023

REV

REV DATE



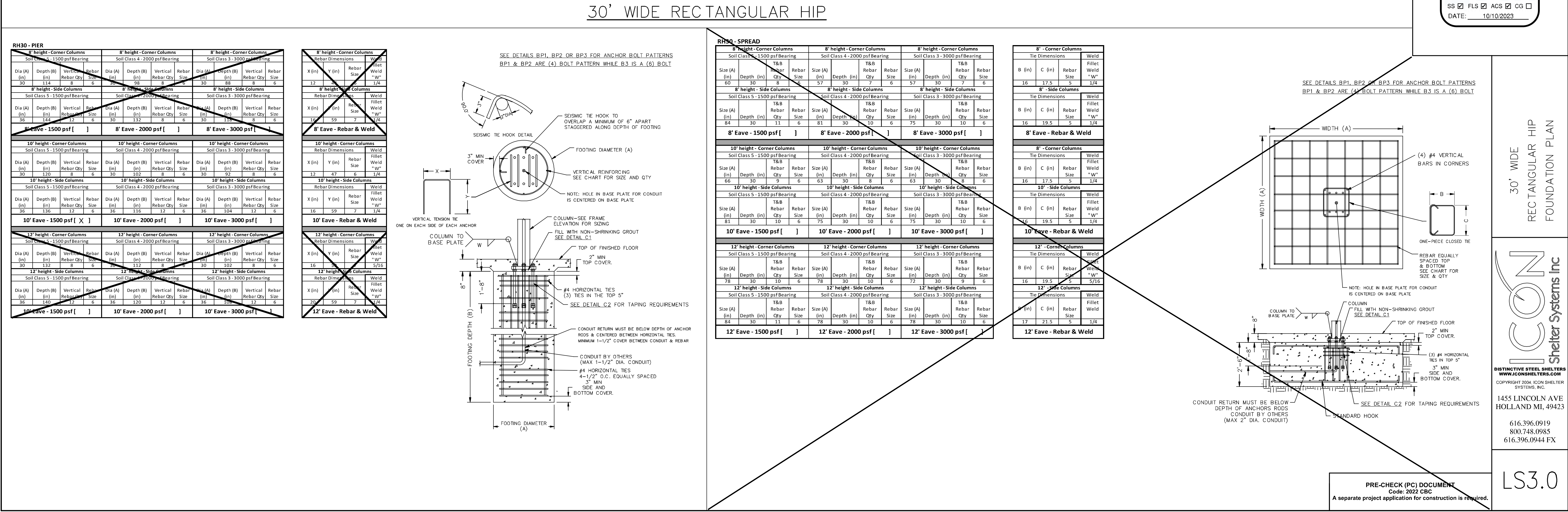
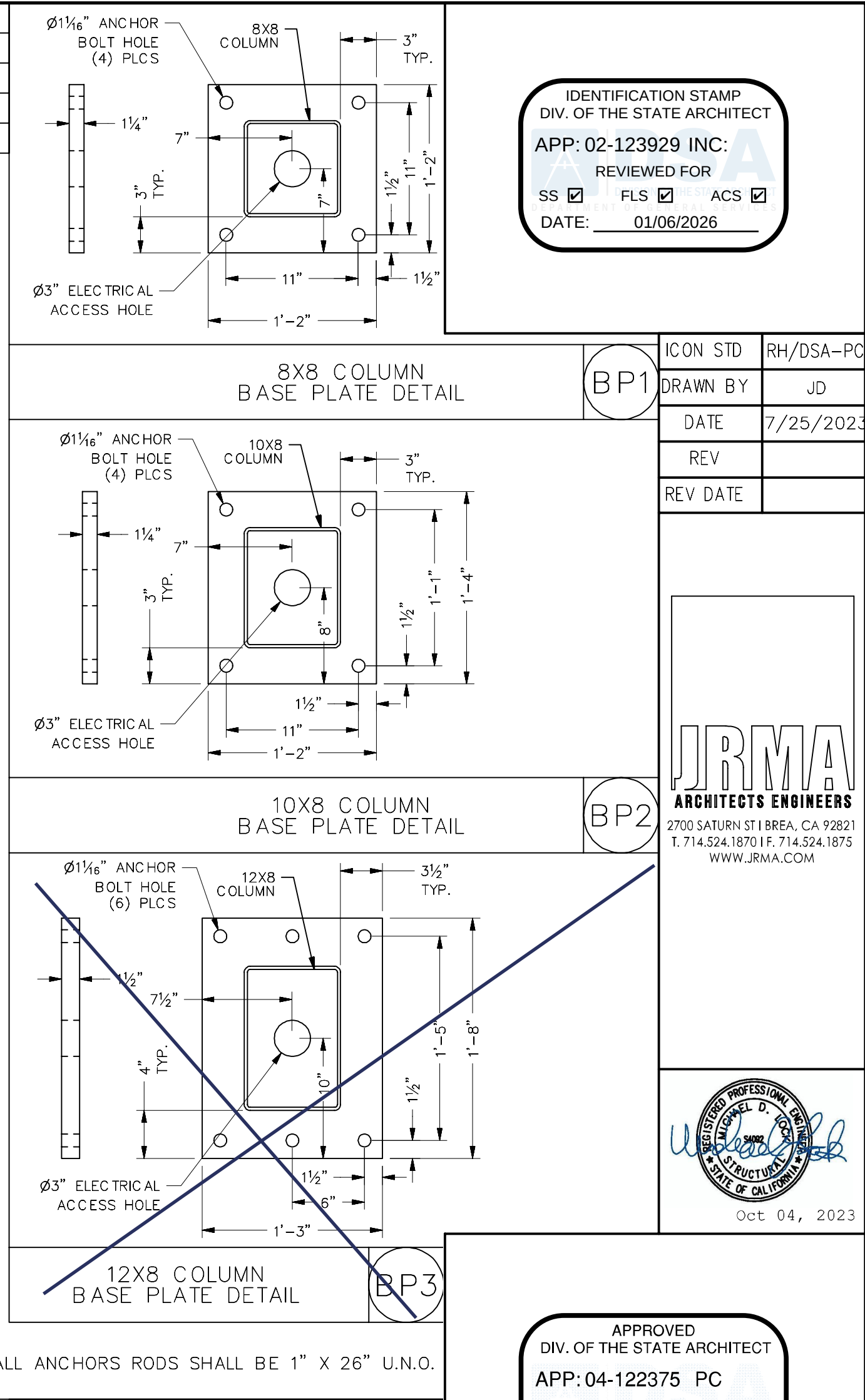
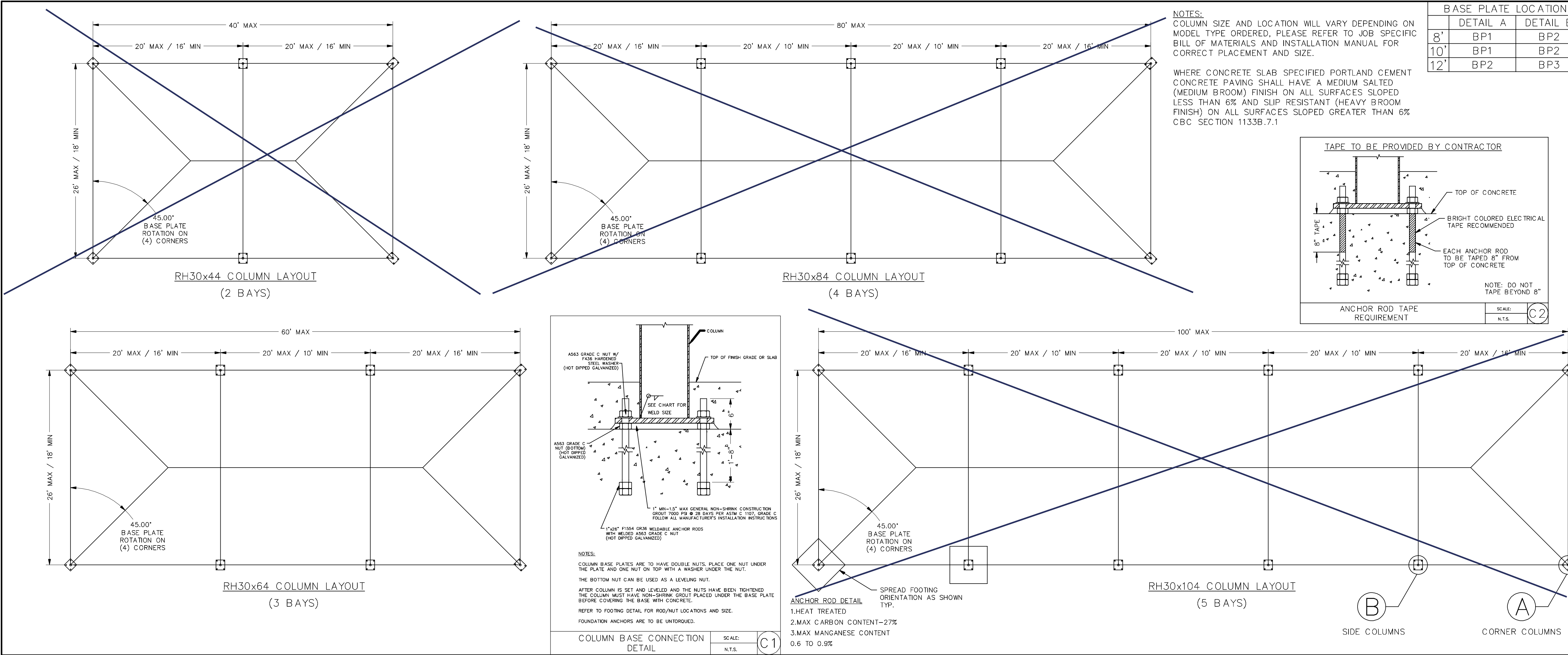
APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122375 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 10/10/2023

DSA 103

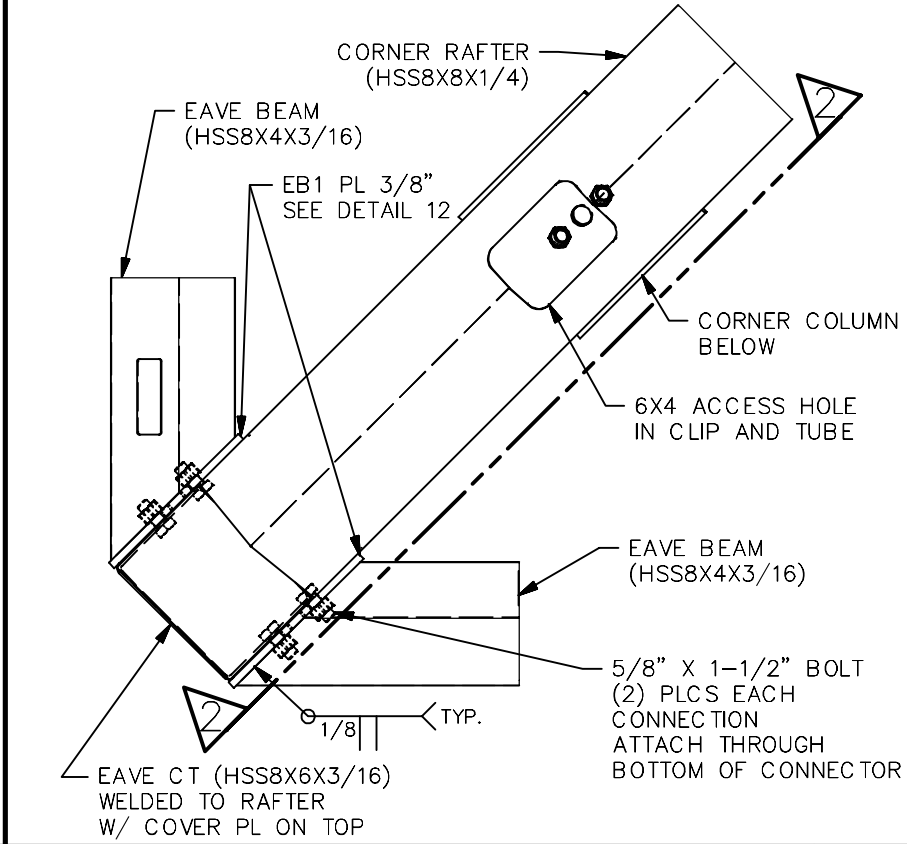


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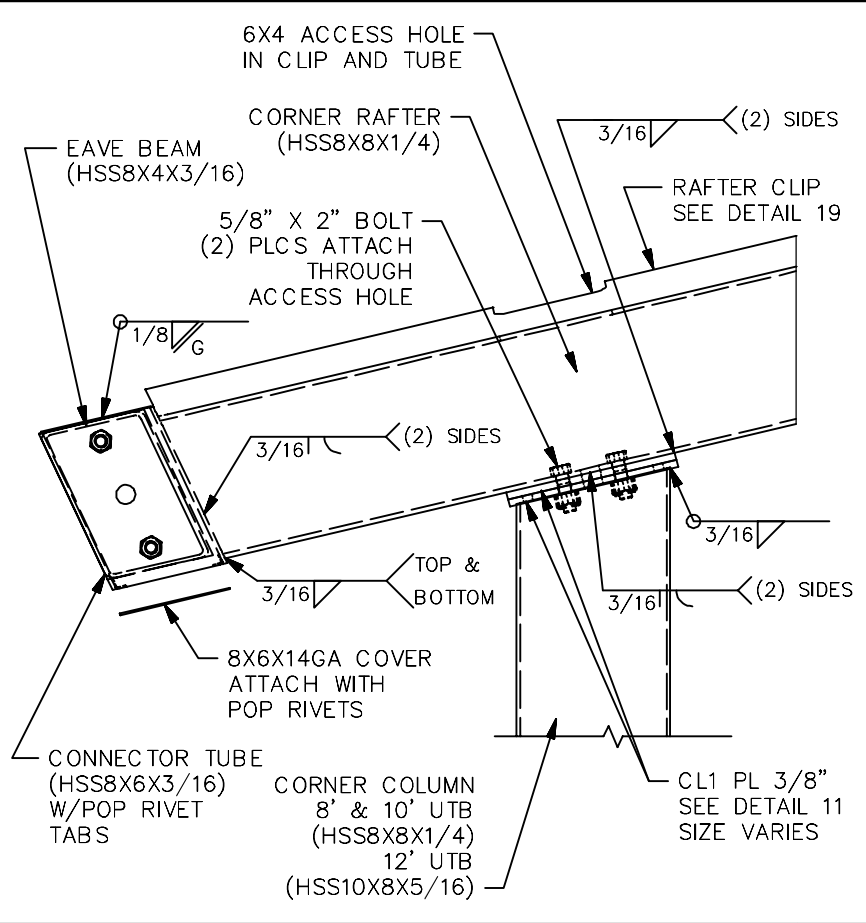
PRE-CHECK (PC) DOCUMENT
Code: 2022 CBC
A separate project application for construction is required.



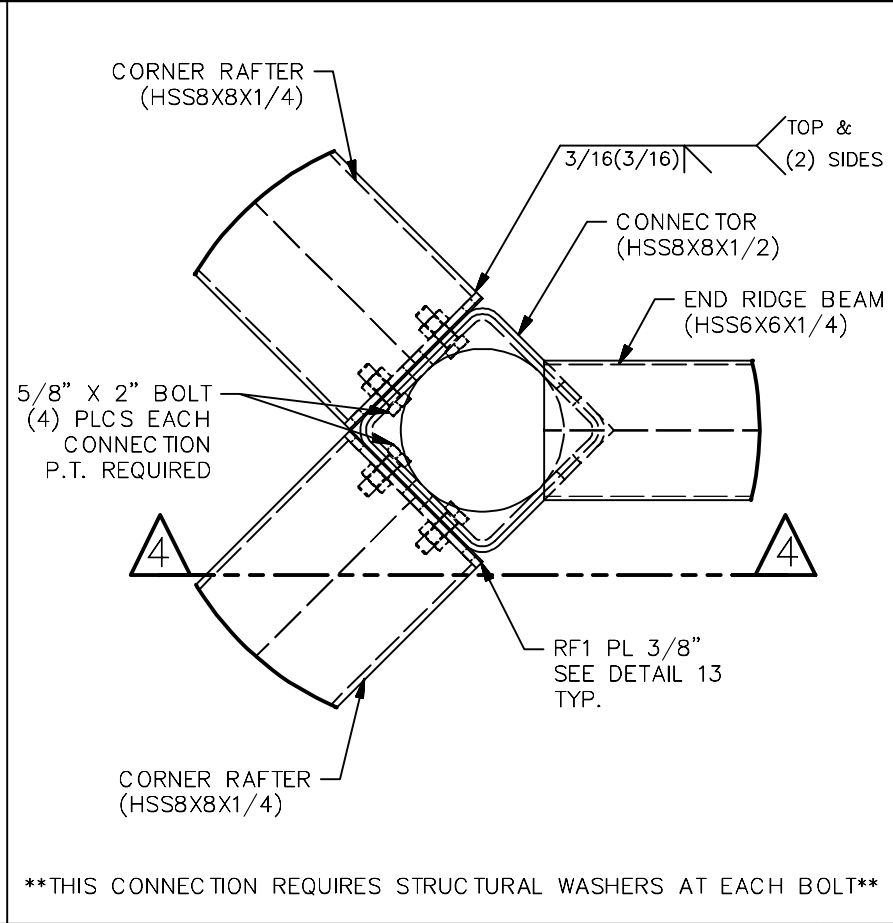
ALL STRUCTURAL FASTENERS (NUTS, BOLTS, & WASHERS) ON THIS SHEET TO BE HOT DIPPED GALVANIZED



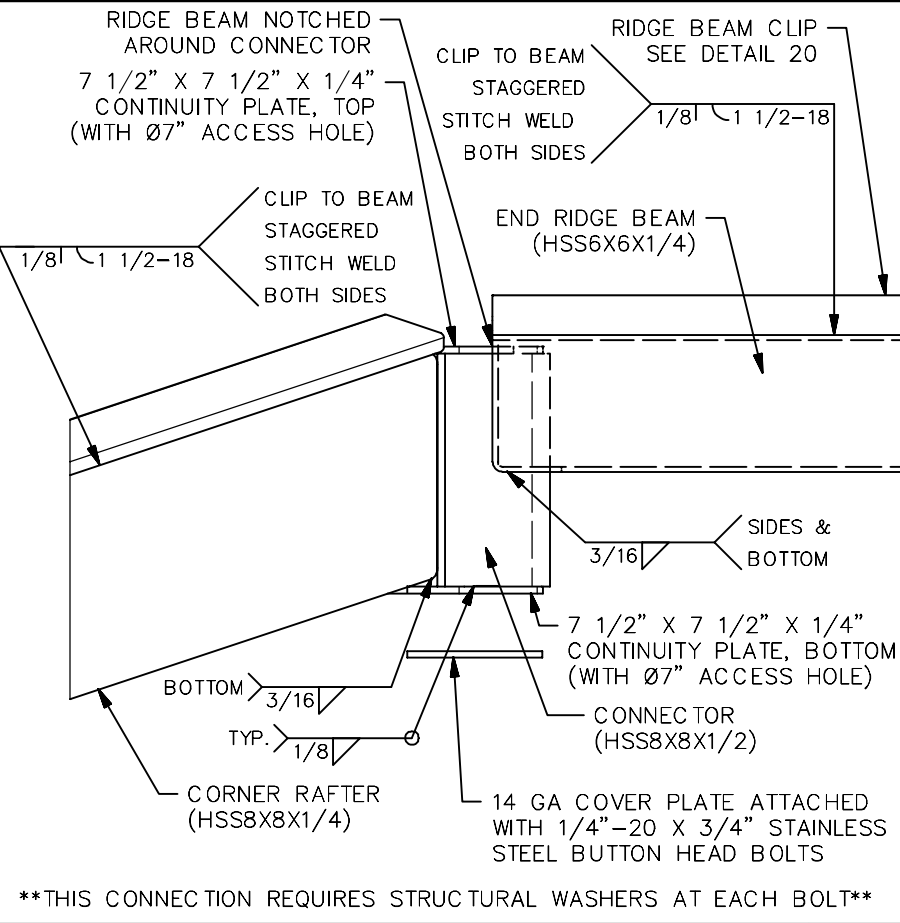
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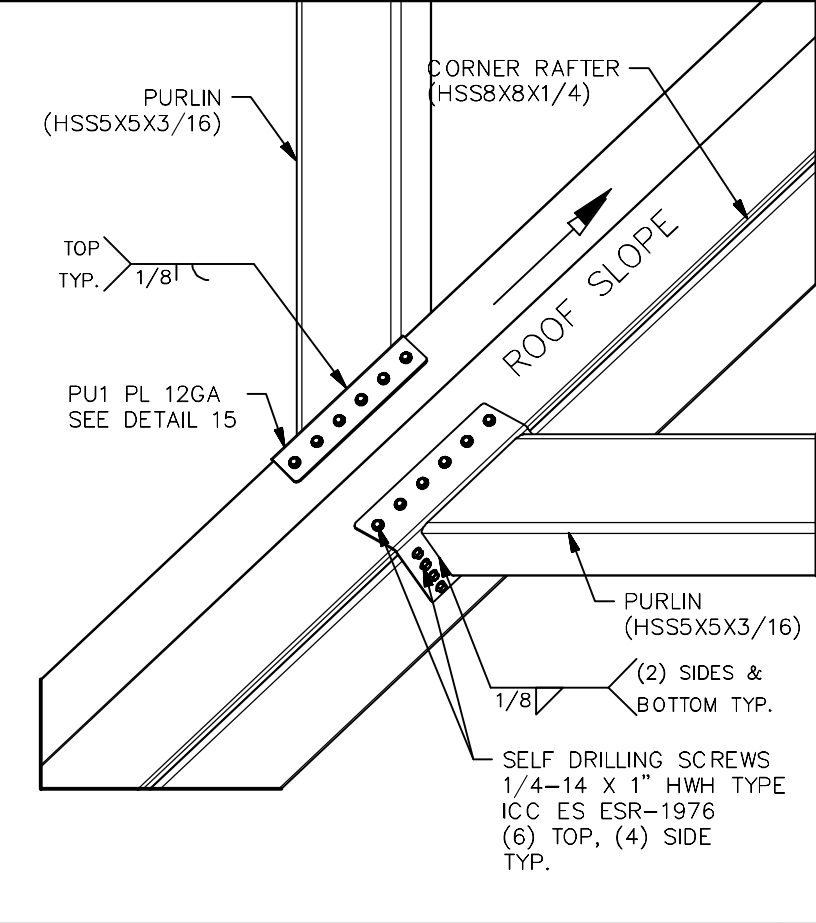
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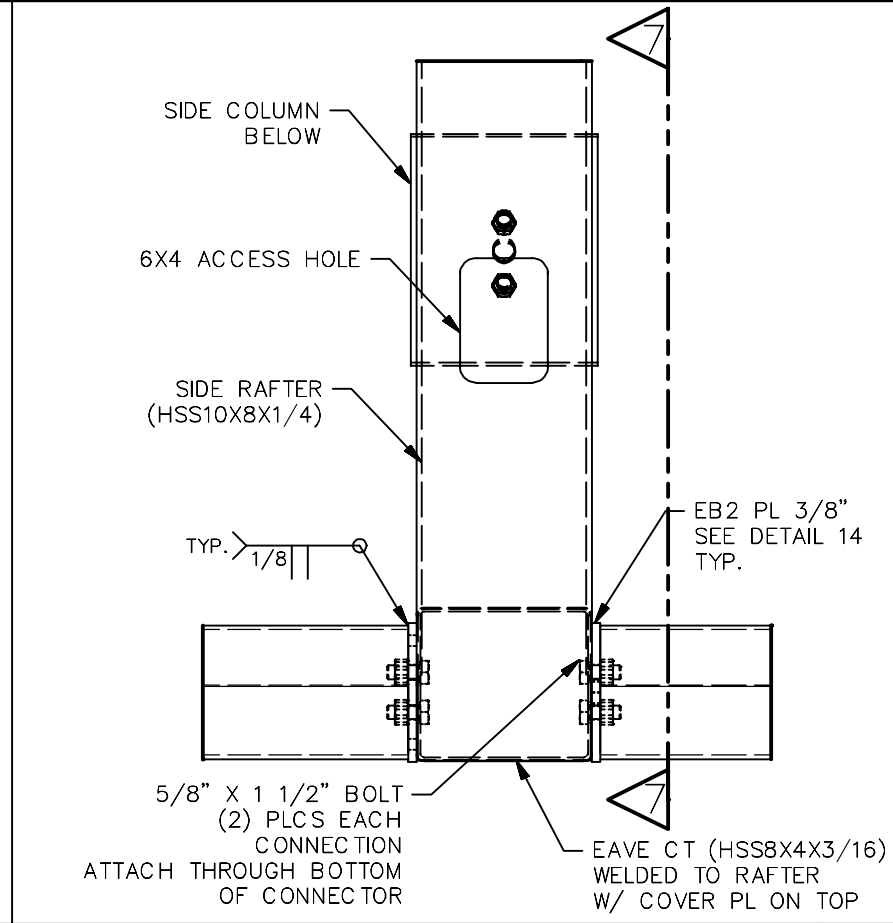
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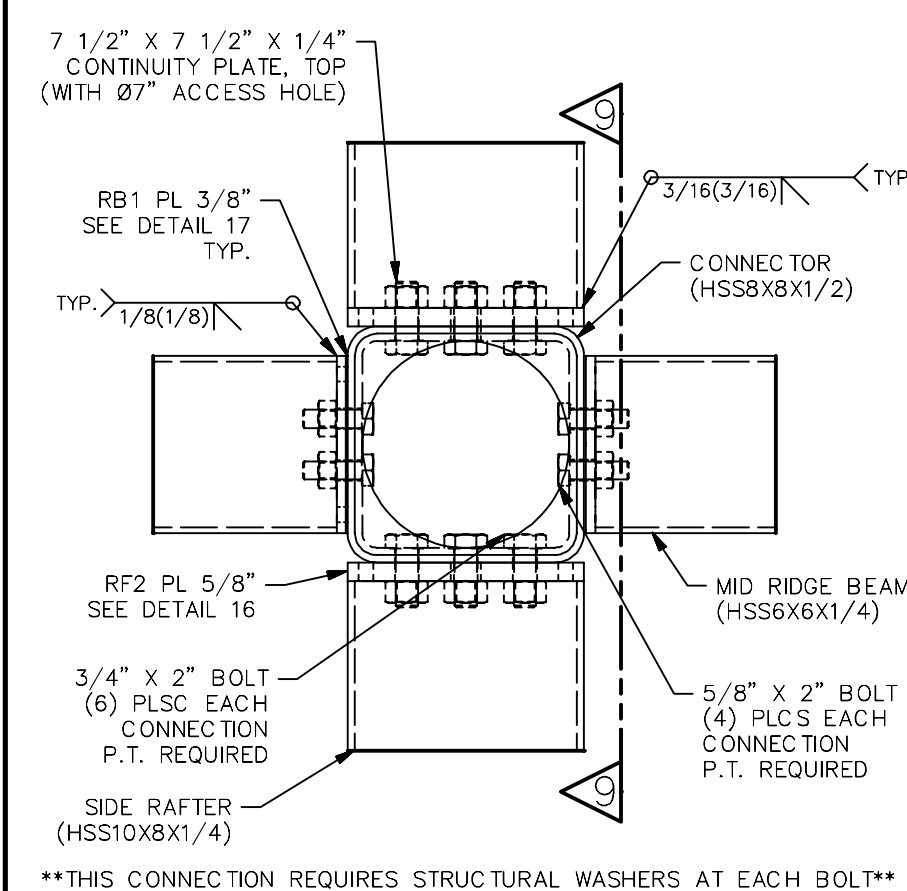
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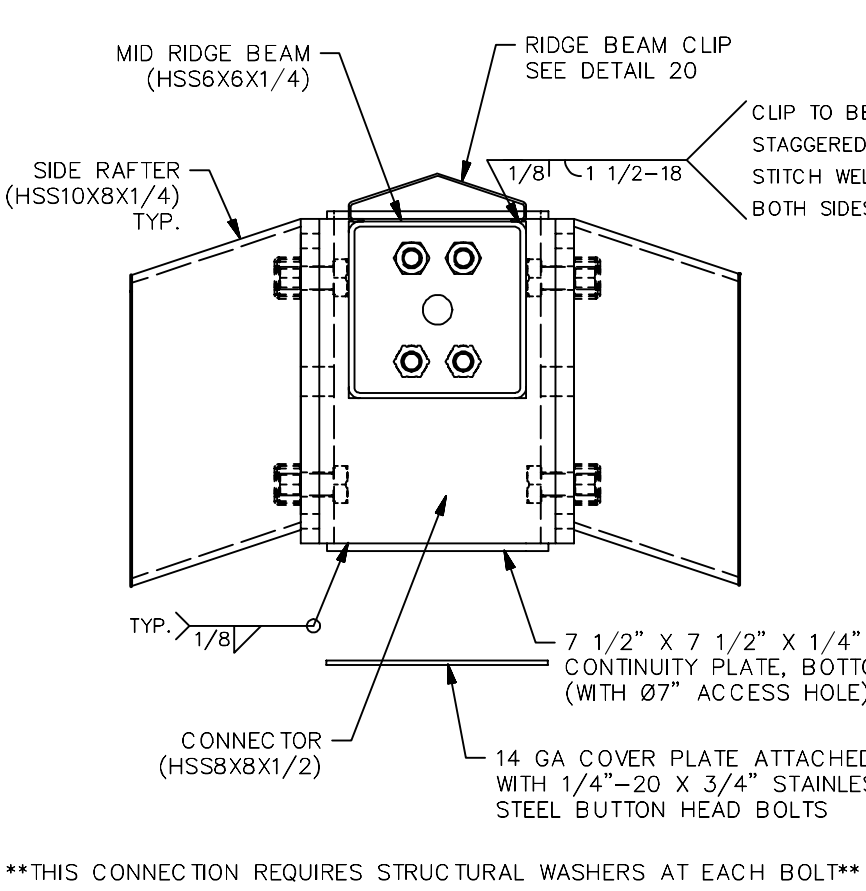
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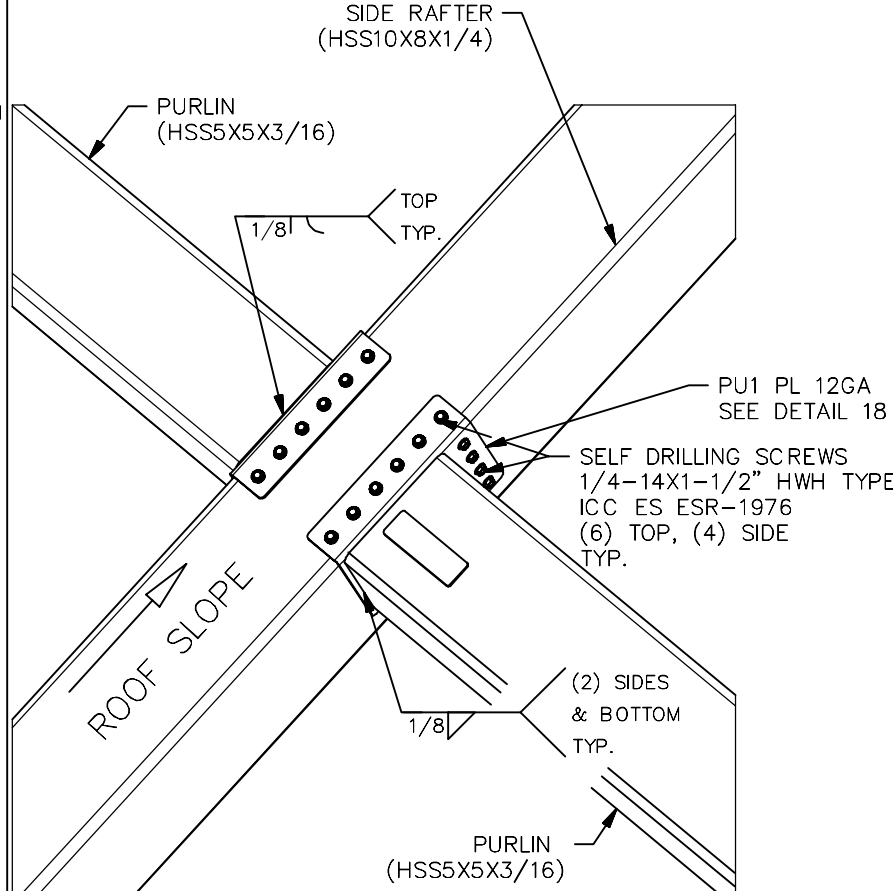
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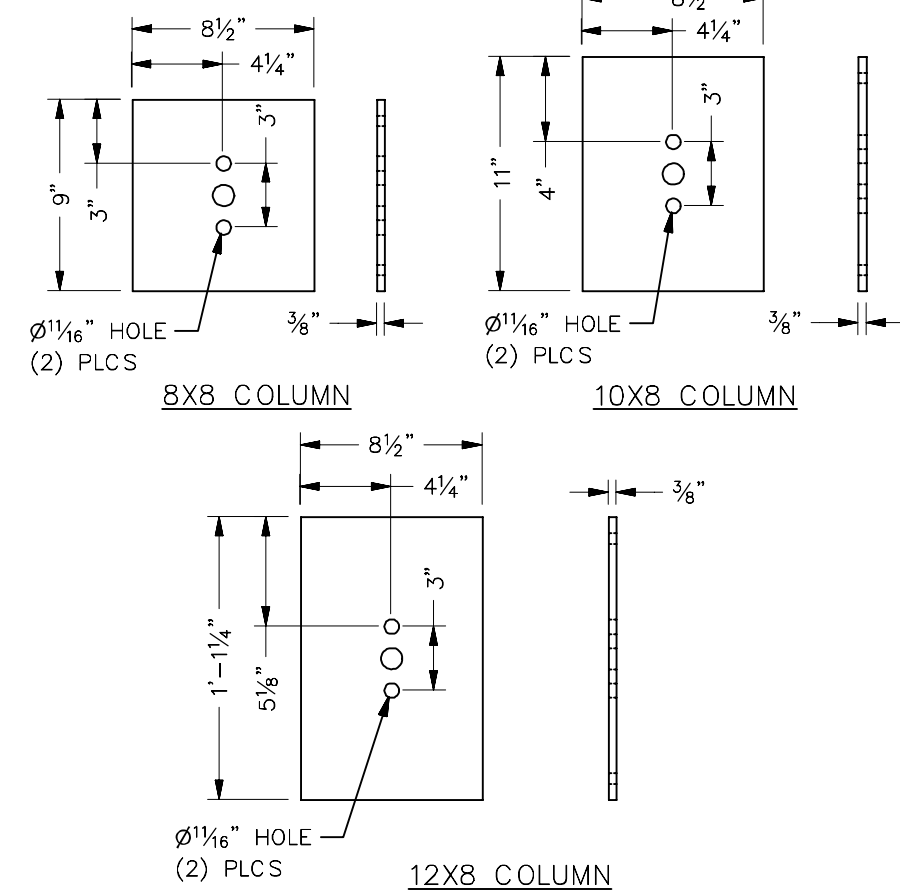
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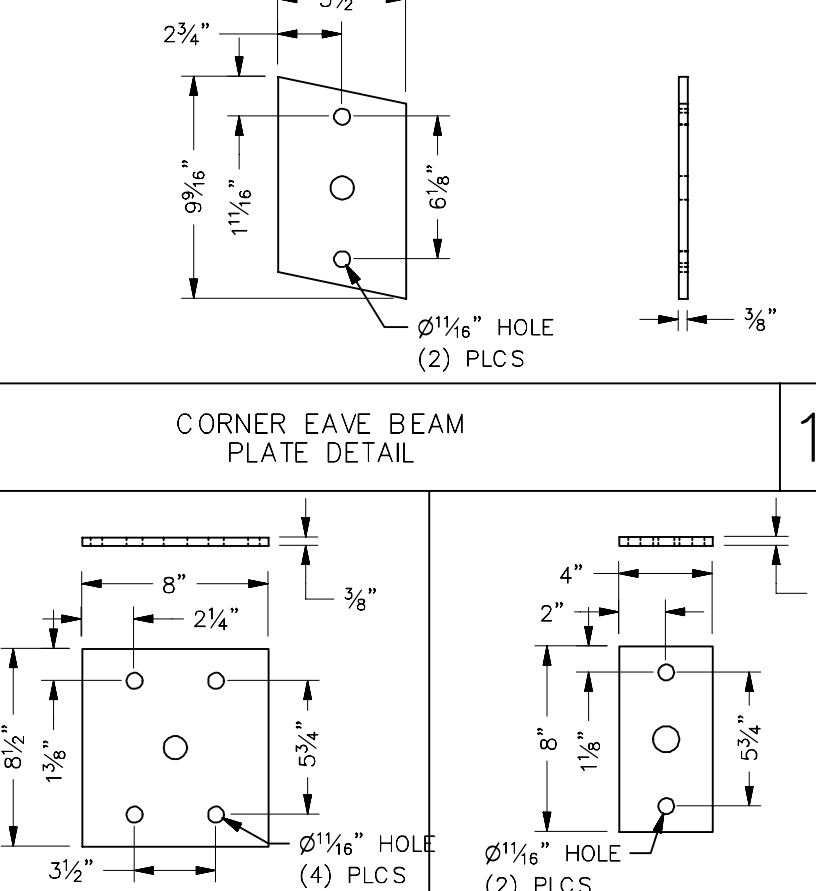
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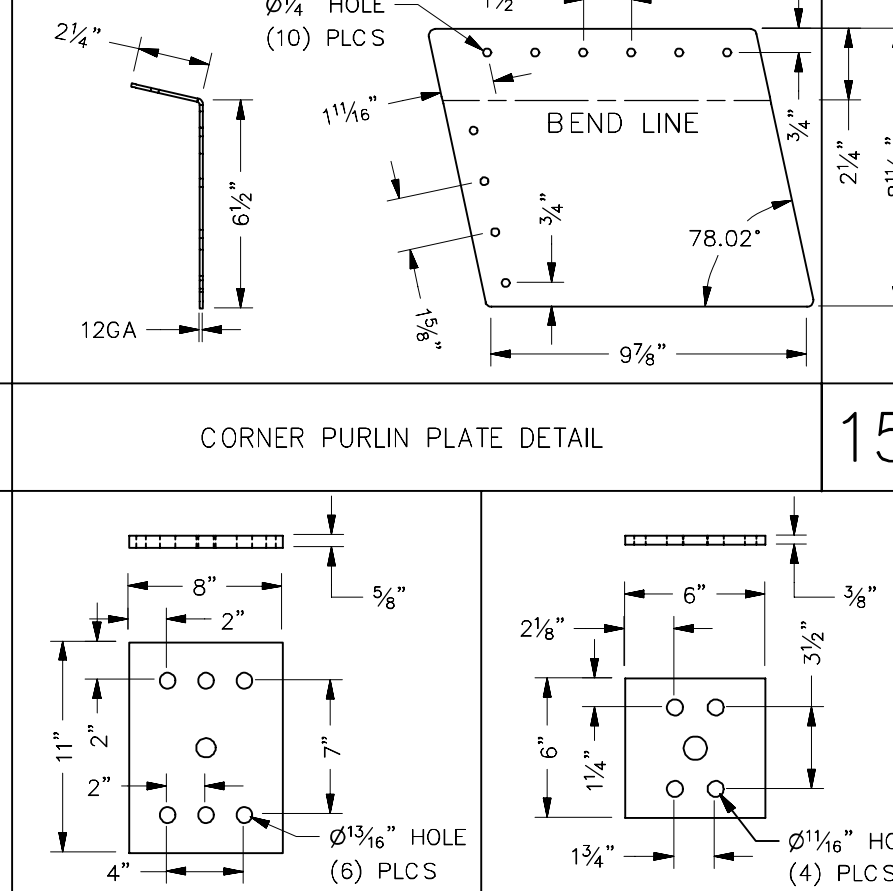
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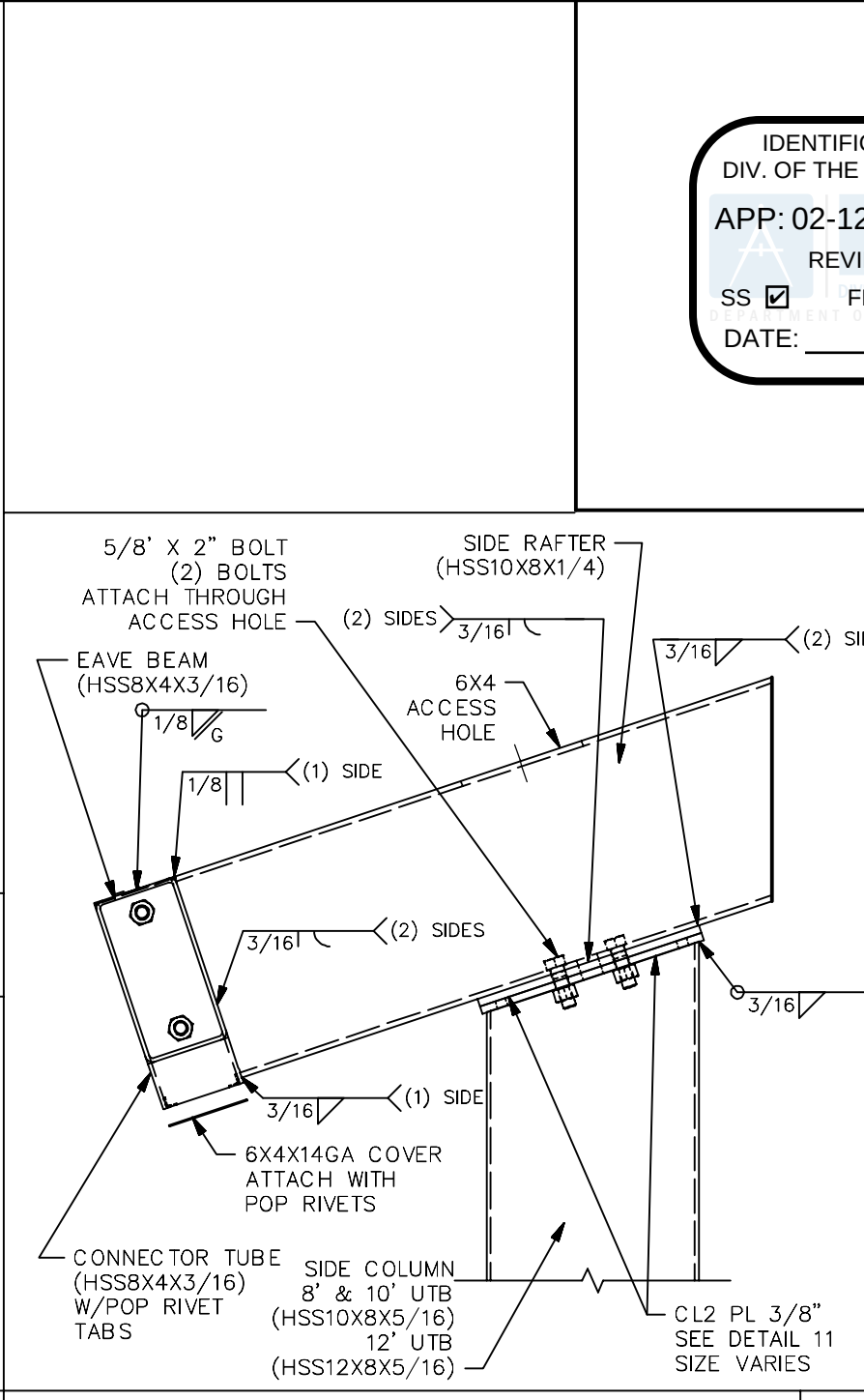
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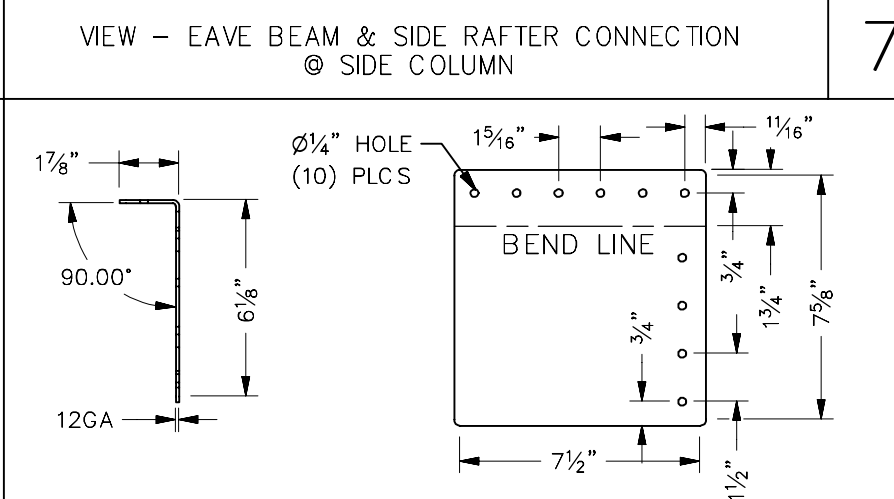
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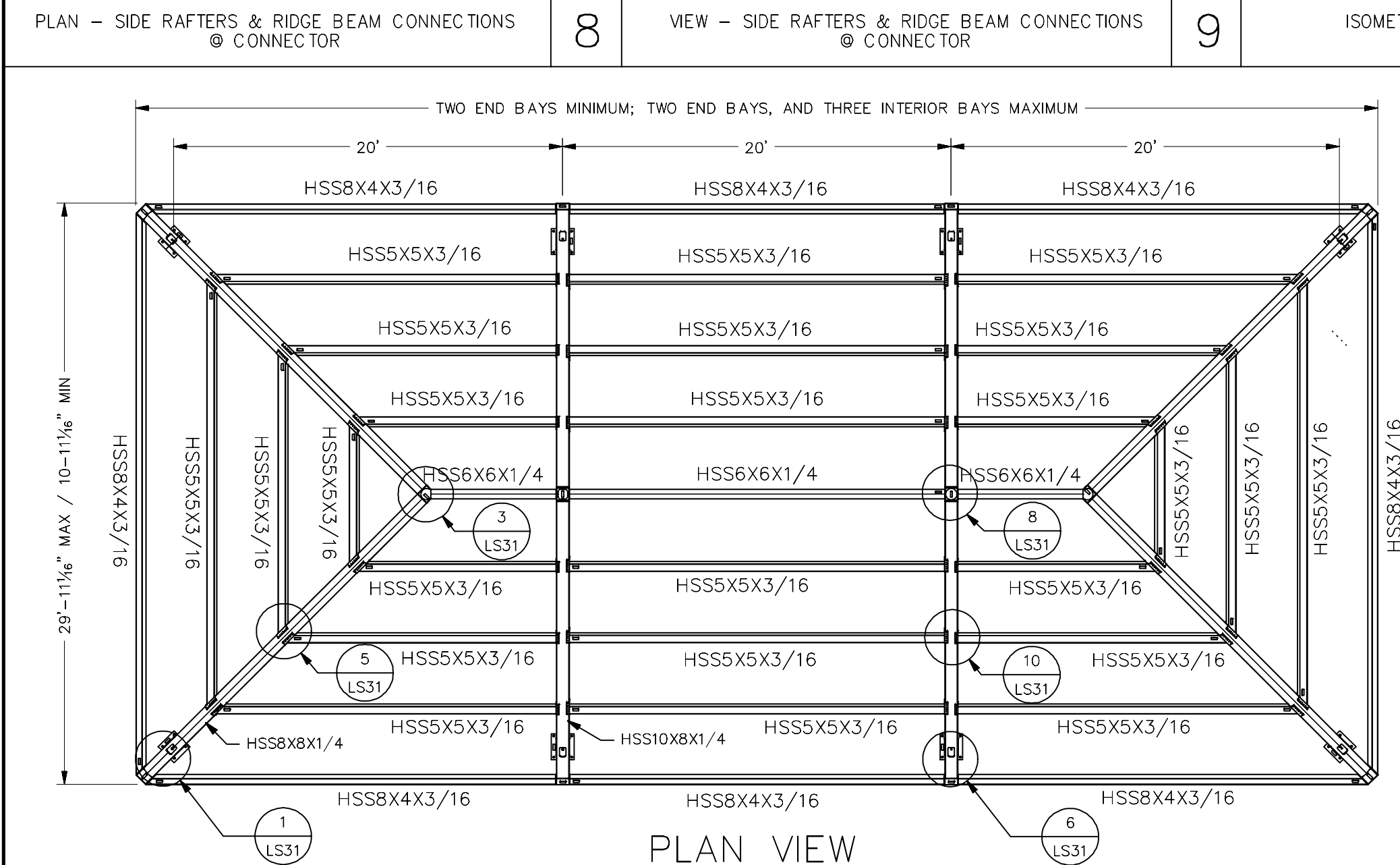
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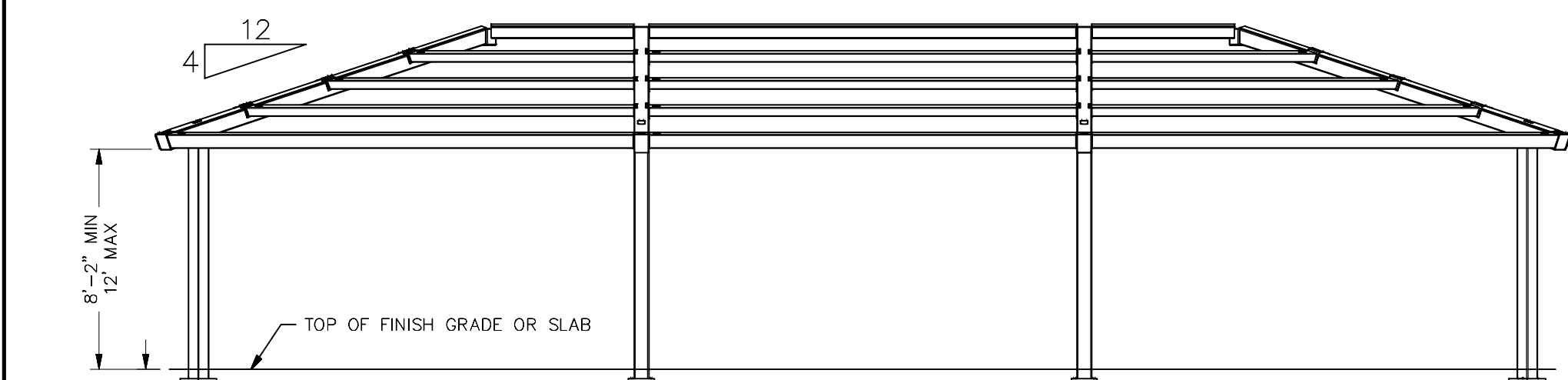


14



PLAN VIEW

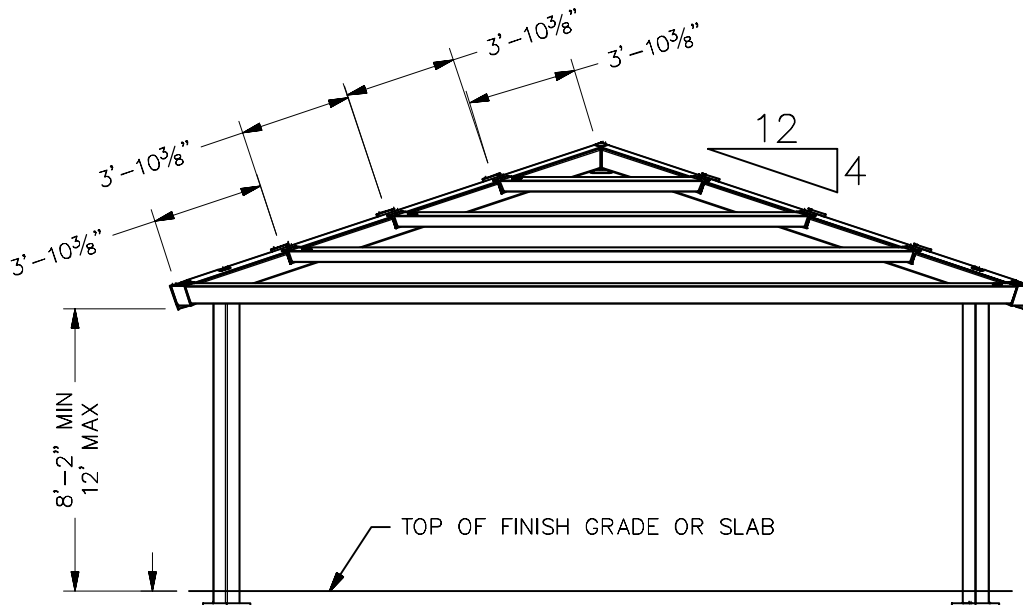
MODEL DESIGNATION	
RH30X44	2 BAY
RH30X64	3 BAY
RH30X84	4 BAY
RH30X104	5 BAY



FRONT VIEW

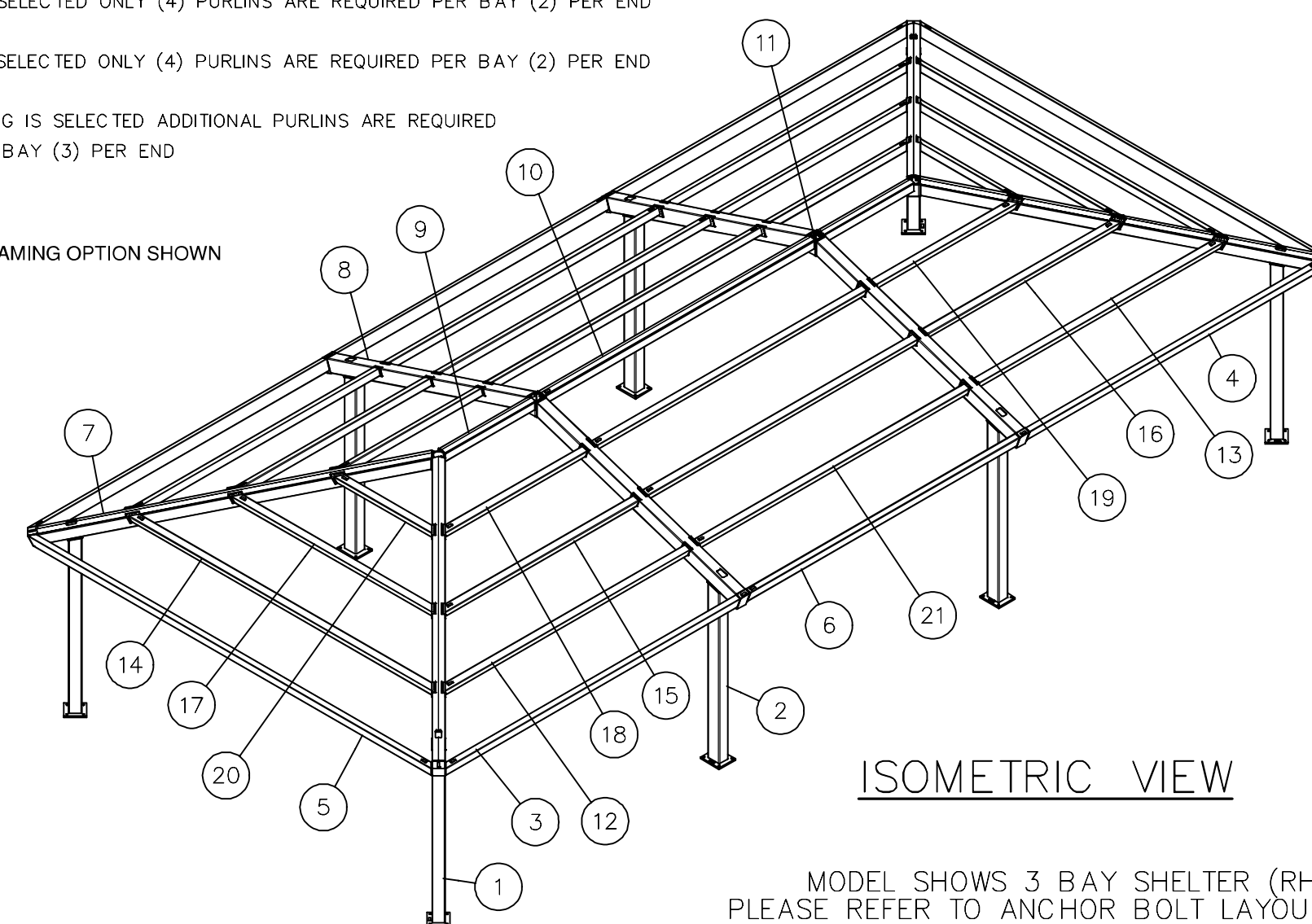
- CORNER COLUMN 8" UTB - (HSS8X8X1/4)
SIDE COLUMN 8" UTB - (HSS10X8X5/16)
CORNER COLUMN 10" UTB - (HSS8X8X1/4)
SIDE COLUMN 10" UTB - (HSS10X8X5/16)
CORNER COLUMN 12" UTB - (HSS10X8X5/16)
SIDE COLUMN 12" UTB - (HSS12X8X5/16)

98" MIN IF USED OVER ACCESSIBLE PARKING OR ACCESS AISLES
114" MIN IF LOCATED OVER ACCESSIBLE PASSENGER LOADING ZONES



SIDE VIEW

STANDING SEAM FRAMING OPTION SHOWN



ISOMETRIC VIEW

MODEL SHOWS 3 BAY SHELTER (RH30X64)
PLEASE REFER TO ANCHOR BOLT LAYOUT SHEET FOR CORRECT COLUMN PLACEMENT BASED ON SIZE ORDERED

30' WIDE RECTANGULAR HIP

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-123929 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 01/06/2026

ICON STD RH/DSA-PC
DRAWN BY JD
DATE 7/25/2023
REV
REV DATE

JRMA
ARCHITECTS ENGINEERS
2700 SATURN ST BREA, CA 92821
1.714.524.1870 IF, 714.524.1875
WWW.JRMA.COM

REGISTERED PROFESSIONAL
DIV. OF THE STATE ARCHITECT
OCT. 04, 2023

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122375 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒ CG ☒
DATE: 10/10/2023

30' WIDE
RECTANGULAR HIP
FRAMING &
CONNECTION DETAILS

Shelter Systems Inc
DISTINCTIVE STEEL SHELTERS
WWW.ICONSHELTERS.COM
COPYRIGHT 2004, ICON SHELTER SYSTEMS, INC.
1455 LINCOLN AVE
HOLLAND MI, 49423
616.396.0919
800.748.0985
616.396.0944 FX

LS3.1

PRE-CHECK (PC) DOCUMENT
Code: 2022 CBC
A separate project application for construction is required.

