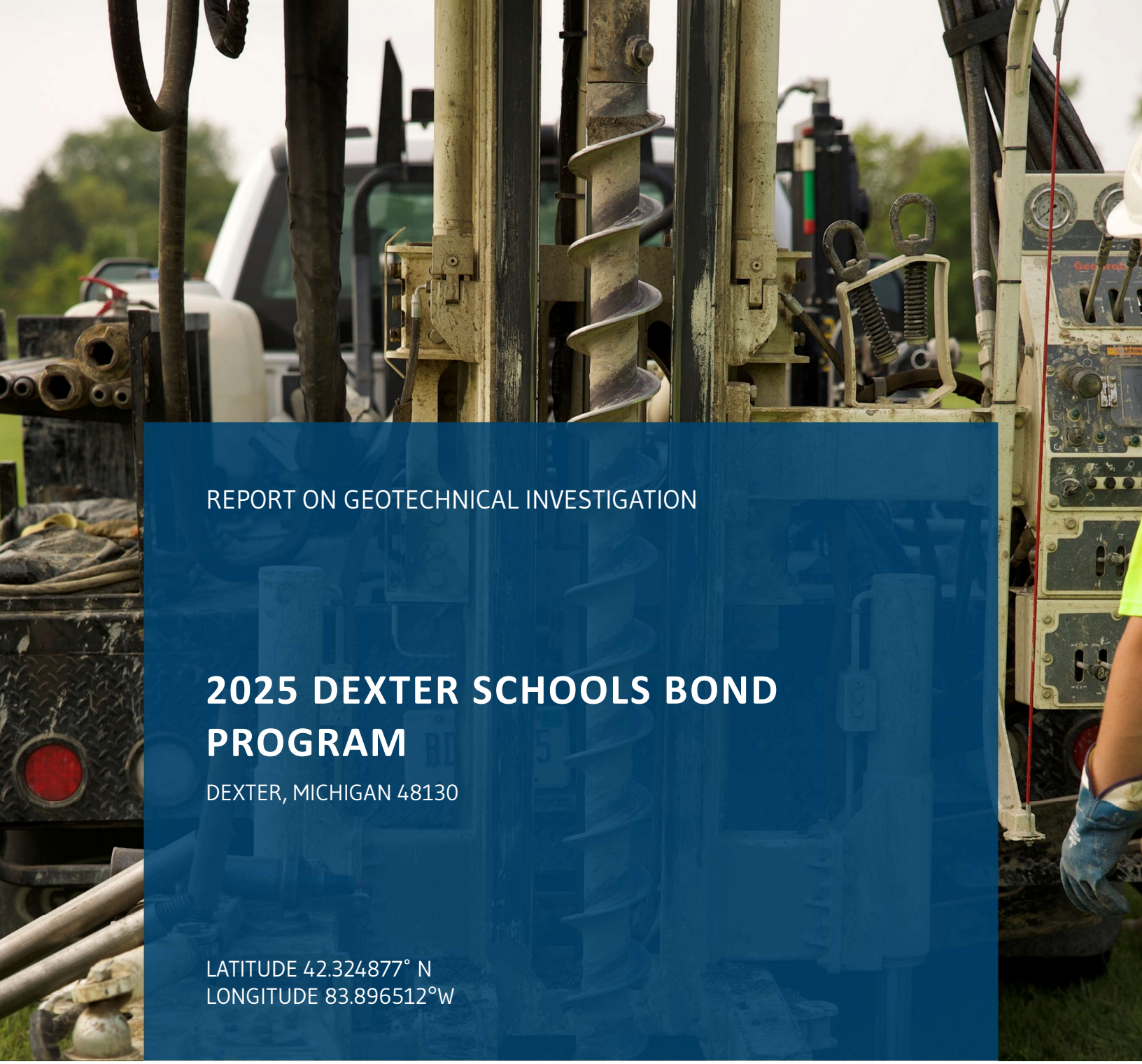


REPORT ON GEOTECHNICAL INVESTIGATION

# 2025 DEXTER SCHOOLS BOND PROGRAM

DEXTER, MICHIGAN 48130

LATITUDE 42.324877° N
LONGITUDE 83.896512°W

CONSULTING GROUP

Prepared for:

Granger Construction
6267 Aurelius Road
Lansing, Michigan 48911

G2 Project No. 253566
October 13, 2025

CONSULTING GROUP
Smart.Results.Fast.

October 13, 2025

Mr. Greg Brand, P.E.
Senior Project Manager
Granger Construction
6267 Aurelius Road
Lansing, Michigan 48911

**RE: Report on Geotechnical Investigation**
2025 Dexter Schools Bond Program
Dexter, Michigan 48130
G2 Project No. 253566

Dear Mr. Brand:

We have completed the geotechnical investigation associated with the 2025 Dexter Schools Bond Program, which includes the construction of building additions at Dexter High School and Wylie Elementary School, as well as athletic improvements at Dexter High School and Creekside Intermediate School. This report presents the results of our observations and analyses, as well as recommendations related to earthwork, subgrade preparation, foundation design, and below-grade wall design, as well as related construction considerations.

We appreciate the opportunity to be of service to you and look forward to discussing the recommendations presented. In the meantime, if you have any questions regarding the report or any other matter pertaining to the project, please call us.

Sincerely,

G2 Consulting Group, LLC

Tyler S. Hesse, P.E.
Project Manager

Jason B. Stoops, P.E.
Associate / Project Consultant

TSH/JBS/tsh

## EXECUTIVE SUMMARY

We understand the project consists of the construction of two (2) building additions at Dexter High School and one (1) building addition at Wylie Elementary School. In addition, improvements will be made to the athletic facilities at the Dexter High School and Creekside Intermediate School campuses. The athletic improvements will consist of constructing bleachers, PCC stairs, entry structures, and pole mounted batting cages.

Approximately 4 inches of Portland cement concrete (PCC) or 2 to 5 inches of sand/clay topsoil are present at the ground surface at the soil boring locations. Very loose to medium compact granular and stiff to hard cohesive fill, with intermixed wood debris and organic matter content, underlie the PCC and/or topsoil within borings B-02, B-08, B-12, B-15 through B-17, and B-19, and extend to approximate depths ranging from 4 to 8-1/2 feet below existing grade. In general, alternating strata of native medium compact to very compact granular soil and stiff to hard cohesive soil underlie the PCC, fill, and topsoil extending to the explored depths ranging from 10 to 45 feet below existing grade. However, native very loose to loose granular soil and medium cohesive soil are present within borings B-02 through B-04, B-09, B-11, B-18, and B-20 at approximate depths ranging from 3 to 9 feet below existing grade. During drilling operations, groundwater was encountered at depths ranging from 6 to 24-1/2 feet below existing grade within soil borings B-01, B-03 through B-06, B-12, B-13, and B-17 through B-20. No measurable groundwater was observed within the remaining borings during drilling operations. Upon the completion of drilling operations, groundwater was measured at a depth of 22 feet below existing grade within boring B-01. No measurable groundwater was observed within the remaining borings upon the completion of drilling operations.

The granular and cohesive fill soils do not appear to have been placed in an engineered manner and are not suitable for support of foundations; however, the fill is marginally suitable for support of floor slabs following successful completion of a proof-roll/proof-compaction evaluation and provide differential floor slab settlement can be tolerated. Based on the reported depth of the lower-level floor slab for the Dexter High School entry addition, approximately 14 feet below grade, we anticipate that the fill will be removed during construction.

We recommend that the two (2) Dexter High School additions, the Wylie Elementary School addition, bleachers, and entry structures be supported by conventional shallow foundations. The proposed foundations must extend through any existing fill and bear within the underlying native stiff to hard cohesive soils or native medium compact to compact granular soils. Also, native medium consistency sandy clay is present within the footprint of the proposed Wylie Elementary School addition near soil boring B-20 between the approximate depths of 3 and 5-1/2 feet below the ground surface. We recommend the foundations extend through this layer of medium consistency sandy clay and bear within the underlying very stiff silty clay. Alternatively, the existing fill and medium consistency sandy clay can be removed from the influence of the proposed foundations and be replaced with engineered fill overlying native soils. We recommend that foundations bearing within the native stiff to hard cohesive soils, native medium compact to compact granular soils, or engineered fill be designed based on net allowable bearing capacity of 3,000 psf. We understand that the pole mounted batting cage will be supported by drilled pier foundations. We recommend that following soil parameter be used for the design of the drilled pier foundations:

| Boring ID | Depth | Soil | Cohesion (psf) | Angle of Friction (°) | Unit Weight (pcf) | Soil Modulus (pci) |
|---|---|---|---|---|---|---|
| B-10 | 0 to 3-1/2 Feet | - | - | - | - | - |
| | 3-1/2 to 4-1/2 Feet | Hard Silty Clay | 4,500 | 0 | 130 | 700 |
| | 4-1/2 to 10 Feet | Medium Compact Sand | 0 | 32 | 115 | 75 |

Do not consider this summary separate from the entire text of this report, with all the conclusions and qualifications mentioned herein. Details of our analysis and recommendations are discussed in the following sections and in the Appendix of this report.

## PROJECT DESCRIPTION

We understand the project consists of the construction of two (2) building additions at Dexter High School and one (1) building addition at Wylie Elementary School. In addition, we understand that improvements will be made throughout the athletic facilities at the Dexter High School and Creekside Intermediate School campuses.

We understand that two (2) additions will be constructed at the northwest and south sides of the existing Dexter High School building. The northwest addition will be utilized as an entryway and a fitness area, will be a single-story structure, and will have an approximate footprint of 12,000 square feet. Furthermore, we understand the northwest addition will include a below-grade level approximately 14 feet below the main level finished floor elevation (FFE). The southern addition will be utilized as Career and Technical Education (CTE) space, will be a single-story, slab-on-grade structure, with an approximate footprint of 5,200 square feet.

The proposed Wylie Elementary School addition will be constructed on the south side of the existing structure. The addition will be a single-story, slab-on-grade structure with an approximate footprint of 9,400 square feet.

Finally, several athletic improvements will be made throughout the Dexter High School and Creekside Intermediate School campuses. The athletic improvements at the Dexter High School campus will include construction of entryways, aluminum bleachers, and pole mounted batting cages. The athletic improvements at the Creekside Intermediate campus will include the construction of an entryway and staircase at the north side of the existing football stadium.

The original scope of the investigation included an athletic fieldhouse to be constructed within the Dexter High School campus. At the time of our investigation, the final location of the fieldhouse was undecided, and we were informed that a supplemental investigation would be required once the location was finalized. As such, recommendations related to construction of the fieldhouse are not included within this report.

If information related to existing/final site grades, structural loading conditions, and/or finished floor elevations change or become available, G2 Consulting Group, LLC (G2) should be notified to re-evaluate the recommendations provided herein, if necessary. The purpose of our exploration is to determine and evaluate the general subsurface conditions at the above-mentioned locations and to develop recommendations for earthwork, subgrade preparation, foundation design, and below-grade wall design, as well as related construction considerations as they relate to the geotechnical conditions on site.

## SCOPE OF SERVICES

The field operations, laboratory testing, and engineering report preparation were performed under direction and supervision of a licensed professional engineer. Our services were performed according to generally accepted standards and procedures in the practice of geotechnical engineering in this area. Our scope of services for this project is as follows:

1. We performed a total of twenty (20) soil borings throughout the aforementioned areas. Soil borings B-01 through B-04 were performed within the influence of the northwestern addition at Dexter High School extending to a depth of 45 feet each. Soil borings B-05 and B-06 were performed within the influence of the southern addition at Dexter High School extending to a depth of 25 feet each. Soil borings B-07 through B-13 and B-14 through B-16 were performed within the vicinity of the athletic improvements at Dexter High School and Creekside Elementary School, respectively, extending to a depth of 10 feet each. Soil borings B-17 through B-20 were performed within the influence of the addition at the Wylie Elementary School extending to a depth of 25 feet each.

2. We performed laboratory testing on representative samples obtained from the soil borings. Laboratory testing included visual engineering classification, natural moisture content, organic matter content, and unconfined compressive strength determinations.

3. We prepared this engineering report. The report includes recommendations regarding earthwork, subgrade preparation, foundation design, and below-grade wall design, as well as construction considerations.

## FIELD OPERATIONS

Granger Construction selected the number, depth, and location of the soil borings. The boring locations were located in the field by a G2 representative using GPS assisted mobile technology and by measuring from known surface features using conventional taping methods. The approximate soil boring locations are shown on the Soil Boring Location Plans, Plate Nos. 1 through 4. A topographic survey detailing existing surface grades at the soil boring locations was unavailable at the time of this report and as such, we were unable to interpolate ground surface elevations at the boring locations. We highly recommend the ground surface elevations at the boring locations be determined so that the encountered soil stratigraphy can be tied to elevation.

The soil borings were performed using a truck mounted drilling rig. Continuous 2-1/4-inch, inside-diameter, hollow-stem augers were used to extend the boreholes to the explored depths. Soil samples were collected at 2-1/2-foot intervals within the upper 10-feet and at 5-foot intervals thereafter. The samples were obtained using the Standard Penetration Test (SPT) method (ASTM D1586), which involves driving a 2-inch diameter split-spoon sampler into the soil with a 140-pound weight falling 30-inches. The sampler is generally driven in three (3) successive 6-inch increments with the number of blows for each increment recorded. The number of blows required to advance the sampler the last 12-inches is termed the Standard Penetration Resistance (N). The blow counts for each 6-inch increment and the resulting N-value are presented on the soil boring logs.

The soil samples were placed in sealed containers and brought to our laboratory for testing and classification. During field operations, the driller maintained logs of the subsurface conditions, including changes in stratigraphy and observed groundwater levels. The final boring logs are based on the field boring logs supplemented by laboratory soil classification and test results. The boreholes were backfilled with auger cuttings and topped with cold-patch asphalt, where necessary, upon completion of drilling operations.

## LABORATORY TESTING

Representative soil samples were subjected to laboratory testing to determine soil parameters pertinent to subgrade preparation and foundation design. An experienced geotechnical engineer classified the samples in general conformance with the Unified Soil Classification System. Laboratory testing was conducted in conformance with the following ASTM Test Methods:

- "Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass" (ASTM D2216);
- "Standard Test Methods for Determining the Water (Moisture) Content, Ash Content, and Organic Material of Peat and Other Organic Soils" (ASTM D2974); and
- "Standard Test Method for Unconfined Compressive Strength of Cohesive Soil" (ASTM D2166).

The unconfined compressive strengths were determined by ASTM Test Method D 2166 and using a spring-loaded hand penetrometer. The hand penetrometer estimates the unconfined compressive strength to a maximum of 4-1/2 tons per square foot by measuring the resistance of the soil sample to the penetration of a spring-loaded cylinder.

The results of the moisture contents, organic matter contents, and unconfined compressive strengths are indicated on the Soil Boring Logs, Figure Nos. 1 through 20, at the depths the samples were obtained. The results of the unconfined compressive strengths determined in accordance with ASTM D2166 are presented graphically in the appendix as Figure No. 21.

We will hold the soil samples for 60 days from the date of this report, after which time they will be discarded. If you would like to have the samples, please coordinate their pickup prior to the discard date.

## SITE DESCRIPTION

### Dexter High School

The Dexter High School campus is located at 2200 N. Parker Road in Dexter, Michigan. The existing school can be accessed via two access drives leading eastward into the parcel and via an access drive extending southward from Shield Road. The existing site grades appear to be the highest near the intersection of Shield and N. Parker Road and appear to gently slope downward as you approach the existing school. The site grades immediately surrounding the building appear to be at higher elevations at the northwest end of the building near the entrance of the school and lower toward to the southeast end of the building. We understand the existing building has a common finished slab elevation across its footprint that roughly corresponds to the grades surrounding the southern end of the building. The changes in elevation at the site result in a building configuration with a walk-out basement with the below grade walls located at the north end of the building.

The existing school building is a single to four-story structure with one-story partially below grade at the north end. The campus also includes several soccer field, two (2) baseball diamonds, ten (10) tennis courts, and a detention pond, as well as associated bituminous concrete service drives and parking areas, and Portland cement concrete (PCC) sidewalks. The remainder of the campus is covered in manicured lawn, includes several mature trees, and isolated areas of dense vegetation.

A topographic survey detailing existing surface grades throughout the campus was unavailable at the time of this report; however, based on our review of topographical information available in Google Earth Pro, existing surface grades throughout the campus range from elevation 856 to 893 feet. Site grades surrounding the existing highschool range from 863 feet at the western face, 866 feet on the southern face, 865 to 866 feet on the eastern face, and 880 feet at the northern face of the building.

Based on historical aerial imagery, the parcel appears to have been used for agricultural purposes prior to construction of the Dexter High School Campus leading up to 1998. Sometime between 1998 and 2005, the Dexter High School building and northern athletic facilities were constructed. We observe the southwestern wooded area was cleared and twin turf fields were constructed sometime around 2020. We observed crack sealing has been performed throughout the bituminous concrete areas since the original construction of the campus.



Historical Aerial Imagery (1998)



Historical Aerial Imagery (2005)

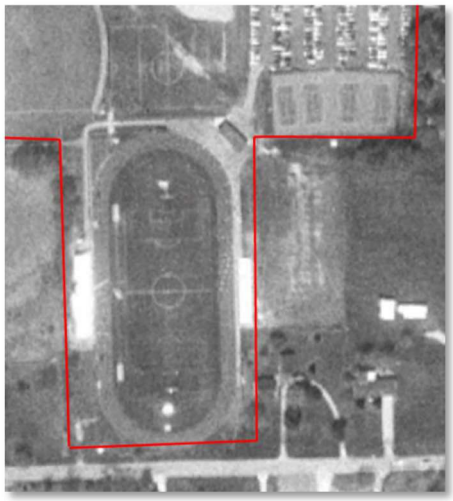


Aerial Imagery (2025)

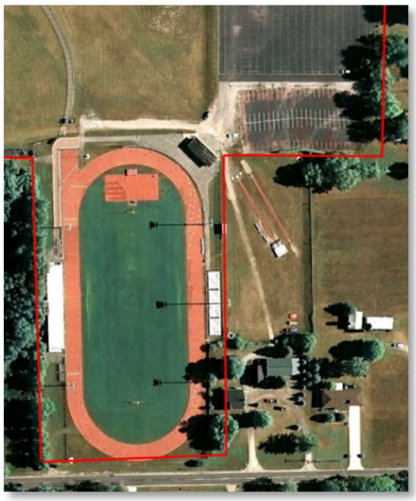
## Creekside Intermediate School

The Creekside Intermediate School campus is located at 2615 Baker Road in Dexter, Michigan. The site can be accessed via several access drives leading westward from Baker Road into the parcel. The existing site grades appear to be their highest along Baker Road ranging from 870 feet to the north and 878 feet to the south. The existing buildings and nearby parking lots appear to be constructed at elevations near 870 feet. The site gently slopes downward to the west from Baker Road. Baseball fields at the northwest corner of the parcel are located at elevations near 865 feet. Another baseball field located at the southwest corner of the parcel appear to be at an elevation near 863 feet.

The southern football field, running track, bleacher structures, and maintenance building appear to have been constructed sometime prior to 1993. Then, sometime around 2007, the athletic fields north of the football field were demolished and replaced with bituminous concrete parking areas. It appears that the northern restroom/concessions building was constructed sometime between 2007 and 2013.



Historical Aerial Imagery (1993)



Historical Aerial Imagery (2007)




Historical Aerial Imagery (2013)




Aerial Imagery (2025)

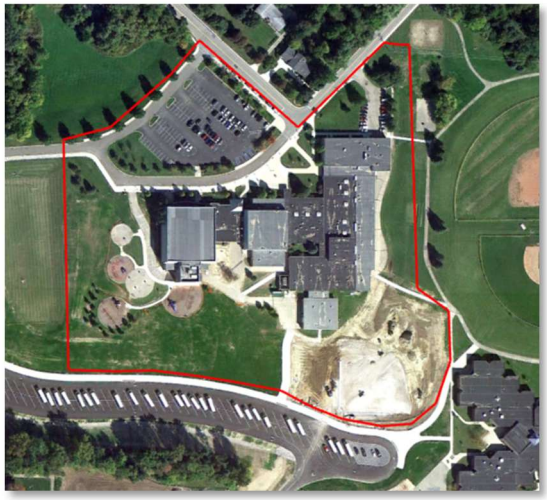
## Wylie Elementary School

The Wylie Elementary School campus is located at 3060 Kensington Street in Dexter, Michigan where Grand Street terminates into Kensington Street. The existing site grades appear to be relatively flat immediately surrounding the building at an elevation of 615 feet. Site grades south and east of the existing building range from 926 feet at the east to an elevation of around 908 feet in the southwest. The existing baseball field located to the southeast of the building appears to be constructed at an elevation around 917 feet.

The campus consists of a single-story, slab-on-grade school building, with associated playground structures, a baseball diamond, bituminous concrete pavements, and PCC sidewalks. The remainder of the campus is grass covered and includes several mature trees.

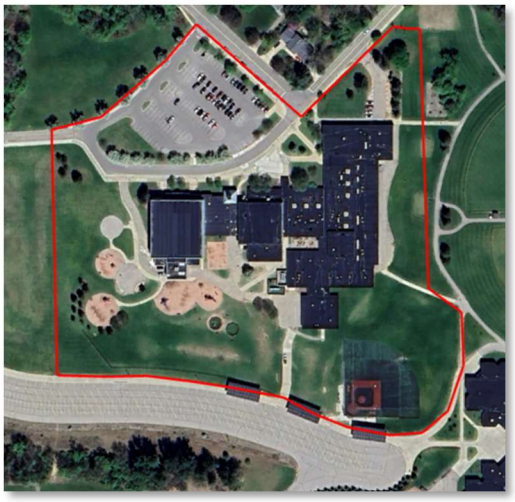


Historical Aerial Imagery (1993)



Historical Aerial Imagery (2011)

Based on historical aerial imagery, the Wylie Elementary School building was constructed sometime prior to 1993. Then, sometime around 2011, the southern bituminous concrete bus loop and baseball diamond were constructed. It appears that crack sealing has been performed throughout the pavement areas since the original construction of the campus.



Aerial Imagery (2025)

# SOIL AND GROUNDWATER CONDITIONS

## Soil Conditions

### Dexter High School Additions (B-01 through B-06)

Approximately 4 inches of PCC and 2 to 5 inches of sand and/or clay topsoil is present at the ground surface at soil borings B-01 and B-02, and B-03 through B-06, respectively. Sand and sandy clay fill, with intermixed wood debris, underlie the PCC within boring B-02 and extends to an approximate depth of 6 feet below grade. Alternating strata of native granular and cohesive soils underlie the PCC within borings B-01, the fill soils within B-02, and the topsoil within borings B-03 through B-06, and extend to the explored depths ranging from 25 to 45 feet below existing grade.

The sandy clay fill soils are very stiff in consistency, with moisture contents ranging from 10 to 13 percent, and unconfined compressive strengths ranging from 4,500 to 6,500 psf. The native granular soils are medium compact to very compact in relative density with SPT N-values ranging from 11 to 80 blows per foot (bpf). However, loose granular soils are present within borings B-02 through B-04, with SPT N-values ranging from 1 to 5 bpf, at depths ranging from 6 to 8 feet. The native cohesive soils are stiff to hard in consistency, with natural moisture contents ranging from 5 to 20 percent, and unconfined compressive strengths ranging from 3,000 psf to more than 9,000 psf.

### Dexter High School Athletic Improvements (B-07 through B-13)

Approximately 2 to 5 inches of sand/clay topsoil is present at the ground surface at soil borings B-07 through B-10, B-12, and B-13. Clayey sand and sandy clay fill underlie the topsoil within borings B-08 and B-12 and extend to an approximate depth of 4 to 8-1/2 feet. The sandy clay fill soils within boring B-12 have an organic matter content of 10.1 percent. Alternating strata of native granular and cohesive soils underlie the topsoil within borings B-07, B-09, B-10, and B-13, underlie the fill soils within borings B-08 and B-12, and are present at the ground surface at boring B-11, and extend to the explored depth of 10 feet below existing grade.

The clayey sand fill soils are very loose to medium compact in relative density, with SPT N-values ranging from 2 to 13 bpf. The sandy clay fill soils are very stiff in consistency, with a moisture content of 10 percent, and an unconfined compressive strength of 5,000 psf. The native granular soils are generally medium compact to very compact in relative density with SPT N-values ranging from 15 to 69 bpf. However, loose native granular soils, with SPT N-values ranging from 9 to 10 bpf, are present within borings B-09 and B-11, at approximate depths ranging from 3 to 9 feet. The native cohesive soils are very stiff to hard in consistency, with natural moisture contents ranging from 7 to 22 percent, and unconfined compressive strengths ranging from 5,000 psf to more than 9,000 psf.

### Creekside Intermediate School Athletic Improvements (B-14 through B-16)

Approximately 2 inches of sand topsoil is present at the ground surface at soil borings B-14 through B-16. Sand and clayey sand fill underlies the topsoil within borings B-15 and B-16 and extend to an approximate depth of 6 feet below existing grade. Alternating strata of native granular and cohesive soils underlie the topsoil within boring B-14 and the fill soils within borings B-15 and B-16, and extend to the explored depth of 10 feet below existing grade.

The granular fill soils are loose in relative density with SPT N-values ranging from 6 to 10 bpf. The native granular soils are medium compact in relative density with SPT N-values ranging from 19 to 28 bpf. The native cohesive soils are very stiff to hard in consistency, with natural moisture contents ranging from 8 to 11 percent, and unconfined compressive strengths ranging from 4,500 psf to more than 9,000 psf.

### Wylie Elementary School Addition (B-17 through B-20)

Approximately 2 to 5 inches of sand/clay topsoil is present at the ground surface at soil borings B-17 through B-20. Granular and cohesive fill soils consisting of clayey sand, silty clay, and sandy clay, with an organic matter content of 3.7 percent, underlie the topsoil within borings B-17 and B-19 and extend to an approximate depth of 7 feet below existing grade. Alternating strata of native granular and cohesive underlie the topsoil within borings B-18 and B-20, and the fill soils within borings B-17 and B-19 and extend to the explored depth of 25 feet below existing grade.

The granular fill soils are loose to medium compact in relative density, with SPT N-values ranging from 8 to 11 bpf. The cohesive fill soils are stiff to hard in consistency, with natural moisture contents ranging from 11 to 16 percent, and unconfined compressive strengths ranging from 2,500 psf to in excess of 9,000 psf. In general, the native granular soils are medium compact to very compact in relative density, with SPT N-values ranging from 14 to 66 bpf. However, very loose granular soils are present within borings B-18 and B-20, with an SPT N-value of 4 bpf, 7 to 9 feet and 1 to 3 feet, respectively. The native cohesive soils are generally stiff to hard in consistency, with natural moisture contents ranging from 9 to 17 percent, and unconfined compressive strengths ranging from 3,000 to in excess of 9,000 psf. However, medium consistency sandy clay is present within boring B-20, with a natural moisture content of 11 percent, a dry density of 116 pounds per cubic foot (pcf), and an unconfined compressive strength of 1,210 psf.

### General

The stratification depths shown on the soil boring logs represent the soil conditions at the boring locations. Variations may occur between and away from the boring locations. Additionally, the stratigraphic lines represent the approximate boundaries between soil types. The transition may be more gradual than indicated. We have prepared the boring logs on the basis of the field log of the soil conditions encountered supplemented by laboratory classification and testing.

The Soil Boring Location Plans, Plate No. 1 through 4, and Soil Boring Logs, Figure No. 1 through 20, are presented in the Appendix. The soil profiles described above are generalized descriptions of the conditions encountered at the boring locations. General Notes Terminology defining the nomenclature used on the soil boring logs and elsewhere in this report are presented on Figure No. 22.

## Groundwater Conditions

### Dexter High School Additions (B-01 through B-06)

During drilling operations, groundwater was encountered at approximate depths ranging from 12 to 20 feet below existing grade within soil borings B-01, and B-03 through B-06. No measurable groundwater was not present during drilling operations within boring B-02. Upon completion of drilling operations, we observed groundwater at a depth of 22 feet below existing grade within boring B-01. No measurable groundwater was observed within the remaining soil borings upon completion of drilling operations.

### Dexter High School Athletic Improvements (B-07 through B-13)

During drilling operations, groundwater was encountered at approximate depths ranging from 6 to 7 feet below existing grade within borings B-12 and B-13. No measureable groundwater was observed during drilling operations within boring B-07 through B-11. No measurable groundwater was observed within borings B-07 through B-13 upon the completion of drilling operations.

### Creekside Intermediate School Athletic Improvements (B-14 through B-16)

No measurable groundwater was observed during or upon the completion of drilling operations within borings B-14 through B-16.

### Wylie Elementary School Addition (B-17 through B-20)

During drilling operations, groundwater was encountered at approximate depths ranging from 18 to 24-1/2 feet below existing grade within borings B-17 through B-20. No measurable groundwater was observed upon the completion of drilling operations within borings B-17 through B-20.

### General

Fluctuations in perched and long-term groundwater levels should be anticipated due to seasonal variations and following periods of prolonged precipitation. It should also be noted that groundwater observations made during drilling operations in predominantly cohesive soils are not necessarily indicative of the static groundwater level. This is due to the low permeability of such soils and the tendency of drilling operations to seal off the natural paths of groundwater flow.

## SITE PREPARATION RECOMMENDATIONS

Granular and cohesive fill soils are present within soil borings B-02, B-08, B-12, B-15 through B-17, and B-19 and extend to approximate depths ranging from 4 to 8-1/2 feet below existing grade. The fill soils contain intermixed wood debris, organic matter contents ranging from 3.7 to 10.1 percent, and do not appear to have been placed in an engineered manner. The fill soils are not suitable for support of foundations, and as such, we recommend that foundations either extend through the fill to bear within native soils, or the fill soils be completely removed and replaced with engineered fill.

Based on the reported depth of the lower-level floor slab for the northwest addition at Dexter High School, we anticipate that the fill soils will be removed during construction. The fill soils within the influence of the Wylie Elementary School addition are marginally suitable for support of the floor slab, following successful completion of a proof-roll evaluation and provided floor slab settlement can be tolerated. If floor slab settlement cannot be tolerated, we recommend the fill be completely removed and replaced with engineered fill.

Topographic surveys detailing existing surface grades throughout the three (3) campuses were unavailable at the time of this report. Similarly, grading plans detailing finished site grades and FFEs were unavailable at the time of this report. It was reported to us that the lower-level FFE for the northwest addition at Dexter High School will match the existing lower-level elevation, approximately 14 feet below the exterior sidewalk elevation. Similarly, the FFE for the southeast addition at Dexter High School will match the existing lower-level elevation, which is within one (1) foot of the adjacent exterior grade. The FFE for the Wylie Elementary School addition will match the existing building FFE, which is within approximately one (1) to two (2) feet of existing grade. If information related to existing/final site grades becomes available, G2 should be notified to re-evaluate the recommendations provided herein, if necessary. Furthermore, we highly recommend elevation be assigned to the soil borings so that the encountered soil stratigraphy can be tied to elevation.

We anticipate earthwork operations will consist of demolition of the existing PCC sidewalks and bituminous pavements within the influence of the building additions and site improvements, complete removal of topsoil, trees, and vegetation within the influence of the development areas, removal of any existing utilities within the influence of the building additions, removal of unsuitable fill soils within the influence of the building additions, evaluating the base of the resulting excavations and improving the conditions, as necessary, placing engineered fill to achieve design subgrade elevation, proof-rolling/proof-compacting the exposed subgrade, excavating for utilities and foundations, and preparing subgrade for floor slab and pavement support. We recommend all earthwork operations be performed in accordance with comprehensive specifications and be properly monitored in the field by qualified personnel under the direction of a licensed professional engineer.

At the start of earthwork operations, any existing topsoil, trees, or vegetation should be entirely removed within the proposed development areas. Any existing utilities and associated backfill within the structure footprint(s) should be completely removed and replaced with engineered fill. Where utilities lie outside the structures zone of influence, utilities to be abandoned can be filled with cementitious grout and left-in-place.

After site stripping, the exposed subgrade is expected to consist of granular/cohesive fill soils and/or native granular/cohesive soils. Where cohesive soils are encountered, soils should be proof-rolled with a fully loaded tandem-axle dump truck and be evaluated for stability. Unsuitable soils exhibiting excessive instability, such as severe rutting, should be improved with additional compaction or undercut to expose stable soils. Where granular soils are encountered, soils should be proof-compacted with a heavy smooth-drum vibratory roller and should be visually evaluated for instability and/or unsuitable soil conditions by a qualified geotechnical engineer or technician. We recommend 10 passes in two (2) perpendicular directions during the proof-compaction operations. We recommend that the vibratory setting be turned off when within 25 feet of any existing structures. Any resulting undercut excavations should be evaluated for stability, improved as necessary, and should be backfilled with engineered fill.

Engineered fill should be free of organic matter, frozen soil, clods, or harmful material. The fill should be placed in uniform horizontal layers not more than 9-inches in loose thickness. The engineered fill should be compacted to achieve a dry density corresponding with 95 percent of the maximum dry density as determined by the Modified Proctor Compaction test (ASTM D1557).

To economically conduct earthwork operations at the site, imported fill, adhering to the aforementioned requirements, should consist of low plasticity clays or well-graded aggregates. Low plasticity clays, having a plasticity index less than 20 percent, should be placed within +3 or -1 percent of the optimum moisture content as determined by the Modified Proctor Test. For well-graded aggregates, such as MDOT Class II sand, we recommend the engineered fill be placed at ±2 percent of the optimum moisture content as determined by ASTM D1557.

We recommend the use of free-draining granular soils, such as MDOT Class II sand, within utility trenches and to backfill around below-grade walls. We recommend the use of engineered fill with a sufficient amount of fines (material passing the No. 200 sieve) in order to facilitate trenching and excavation techniques for strip and spread footing foundations.

## FOUNDATION RECOMMENDATIONS

Information regarding structural loading conditions associated with the building additions and athletic improvements were unavailable for our review at the time of this report. In the absence of such information, and for the purposes of this report, we have assumed that the structures will be lightly to moderately loaded. When information regarding structural loading conditions becomes available, G2 should be notified so that we may have an opportunity to review the loading conditions and revise the recommendations found in this report, if necessary.

### Dexter High School Additions

Based on the reported depth of the lower-level floor slab being at depth of approximately 14 feet relative to the existing site grades on the north end of the building, we anticipate foundations for the northwest addition at Dexter High School will be supported by native very stiff to hard cohesive soils and/or medium compact granular soils. We recommend the addition be supported by conventional shallow spread and/or strip footings designed to bear within the native very stiff to hard cohesive and/or medium compact granular soils. We recommend a net allowable bearing capacity of 3,000 psf be used for foundations bearing within the above-mentioned soils.

We recommend the southeast addition at Dexter High be supported by conventional shallow spread and/or strip footings designed to bear at a conventional 3-1/2 foot depth within the native medium compact to compact granular soils. We recommend a net allowable bearing capacity of 3,500 psf for foundations bearing within the native medium compact to compact granular soils.

### Dexter High School Athletic Improvements

We understand preliminary designs for the bleachers and entry structure indicate the use of shallow spread/strip footings and the pole mounted batting cage indicate the use of drilled pier foundations.

Clayey sand and sandy clay fill are present within borings B-08 and B-12 and extend to approximate depths ranging from 4 to 8-1/2 feet. The sandy clay fill within boring B-12 has an organic matter content of 10.1 percent. The fill soils are not suitable for support of foundations, and as such, we recommend foundations extend through the fill and bear within the underlying native granular/cohesive soils.

We recommend that the entry-way structure and bleachers be supported by conventional shallow spread and/or strip footing foundations designed to bear within the native very stiff to hard cohesive and/or medium compact to compact granular soils at approximate depths ranging from 3-1/2 to 8-1/2 feet below finished grade. We recommend a net allowable bearing capacity of 3,000 psf be used for foundations bearing within the above-mentioned native soils.

The design of drilled shaft requires an understanding of the axial, shear, and moment forces acting at the head of the drilled shaft. Once this information becomes available, please share it with us so that we may review and evaluate the suitability of the drilled shaft recommendations we present in this report and to evaluate the need for deeper borings.

We recommend drilled shafts be designed using the following soil parameters:

| Boring ID | Depth | Soil | Cohesion (psf) | Angle of Friction (°) | Unit Weight (pcf) | Soil Modulus (pci) |
|---|---|---|---|---|---|---|
| B-10 | 0 to 3-1/2 Feet | - | - | - | - | - |
| | 3-1/2 to 4-1/2 Feet | Hard Silty Clay | 4,500 | 0 | 130 | 700 |
| | 4-1/2 to 10 Feet | Medium Compact Sand | 0 | 32 | 115 | 75 |

Based on the anticipated embedment depth, we anticipate that foundations will bear within native medium compact granular soils. We recommend a net allowable bearing capacity of 3,000 psf be used for design of drilled pier bearing with the native medium compact granular soils.

Center-to-center pile spacing should not be less than three (3) pile diameters to avoid reduction in downward pile capacity due to group action. If the required drilled shaft depth extends below 10 feet, additional soils information may be required to finalize the foundation design.

## Creekside Intermediate School Athletic Improvements

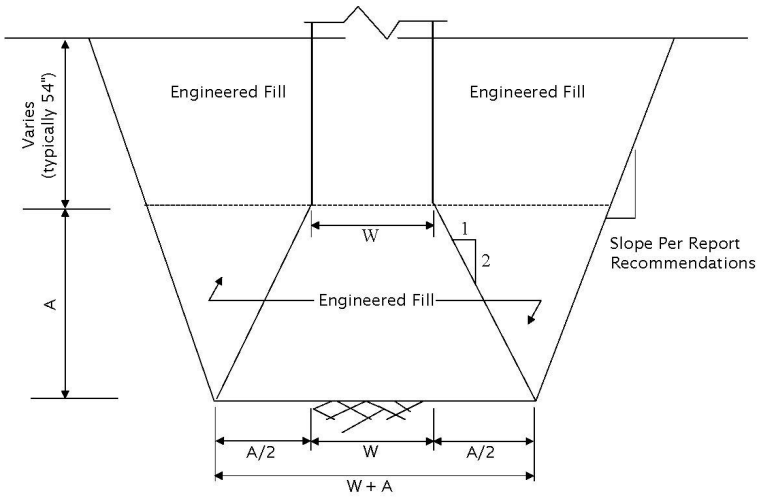
Granular fill soils are present within the vicinity of the proposed improvements at Creekside Intermediate School and extend to an approximate depth of 6 feet below existing grade. The fill soils are not suitable for the support of entryway or staircase foundations, and as such, we recommend that foundations extend through the fill and bear within the underlying native granular/cohesive soils found at depths ranging from 3-1/2 to 6 feet depending on the location.

We recommend that the entry-way structure and stairwell be supported by conventional shallow spread and/or strip footings designed to bear within the native granular/cohesive soils at depths ranging from 3-1/2 to 6 feet finished grade. We recommend a net allowable bearing capacity of 3,000 psf for foundations bearing within the native medium compact granular and/or very stiff to hard cohesive soils.

## Wylie Elementary School Addition

Granular and cohesive fill soils are present within the influence of the Wylie Elementary School addition and extend to an approximate depth of 7 feet below existing grade. The fill soils have an organic matter content of 3.7 percent. These soils are unsuitable for the support of foundations and marginally suitable for the support of floor slabs. In addition, native medium consistency sandy clay is present at the bearing depth associated with conventional foundations near the southwest corner of the addition. The building foundations could bear within the medium sandy clay layer; however, this would result in a reduced net allowable bearing capacity. As such, we recommend the granular/cohesive fill soils and the native medium sandy clay be completely removed to expose the native granular/cohesive soils and be replaced with engineered fill within the influence of the building addition.

The influence beneath a foundation can be described as the zone of soil encompassed by a trapezoid projected outward from the base of the foundation at a ratio of 1 horizontal unit to 2 vertical units extending to the top of the native. At a minimum all of the existing fill soils and the native medium cohesive soils in this zone must be removed and replaced with engineered fill.



We recommend the addition be supported by conventional shallow spread and/or strip footings designed to bear at a conventional 3-1/2 foot depth within the native stiff to very stiff cohesive soils, medium compact granular soils, and/or engineered fill overlying native granular/cohesive soils. Foundations bearing within these soils may be designed using a net allowable bearing capacity of 3,000 psf.

### General

Continuous wall or strip footings should be at least 18-inches in width and isolated spread footings should be at least 36-inches in their least dimension. Exterior foundations must bear at a minimum depth of 3-1/2 feet below finished grade for protection against frost heave. Interior footings may bear at shallower depths provided they extend through fill soils, suitable bearing materials are available for support, and foundations are continuously protected from frost penetration during construction.

Foundations installed immediately adjacent to the existing building foundations must bear at the same depth as the existing foundations. Under no circumstances shall excavations extend below adjacent foundations without proper underpinning. We do not know the bearing depth of the existing foundations; however, consideration could be given to excavating test pits adjacent to the building for the purpose of identifying the existing bearing soils and whether underpinning will be required or not.

To achieve a change in the level of a strip footing, the footing should be gradually stepped at a grade no steeper than two horizontal units to one vertical unit (2H:1V). If required to construct foundations at different levels, adjacent spread foundation should be designed and constructed so the least lateral distance between the foundations is equivalent to or more than the difference in their bearing levels. We recommend the new and existing building sections be structurally separated to allow for independent movements.

If the recommendations outlined in this report are adhered to, total and differential settlements for the completed structure should be within 1 inch and ½ inch, respectively. We expect settlements of these magnitudes are within tolerable limits for the type of structure proposed. We recommend all foundations be suitably reinforced to minimize the effects of differential settlements associated with local variations in subsoil conditions.

## FLOOR SLAB RECOMMENDATIONS

The FFEs for the two (2) additions at Dexter High School and the addition at Wylie Elementary School were unavailable at the time of this report. It was reported to us that the floor slabs for the two (2) additions at Dexter High School will match the existing lower-level floor slab elevation and the floor slab for the Wylie Elementary School addition will match the existing building FFE. Once the exact FFEs are determined, G2 should be notified so that we may review the recommendations presented in this report and make changes, if necessary.

Following satisfactory completion of the subgrade preparation recommendations as described within the *SITE PREPARATION RECOMMENDATIONS* section of this report, a subgrade modulus (k) of 125 pounds per cubic inch (pci) may be used for design of floor slabs supported by native very stiff to hard and/or medium compact to compact granular soils.

As previously stated, the fill soils within the influence of the Wylie Elementary School addition are marginally suitable for support of floor slabs. Provided differential floor slab settlement can be tolerated and following satisfactory completion of the subgrade preparation recommendations as described within the *SITE PREPARATION RECOMMENDATIONS* section of this report, a subgrade modulus (k) of 100 pounds per cubic inch (pci) may be used for design of the Wylie Elementary School floor slab supported by native medium compact granular soils and/or engineered fill overlying the native granular/cohesive soils. If the existing fill is completely removed and replaced with engineered fill, a subgrade modulus (k) of 150 pounds per cubic inch (pci) may be used for design of the floor slab.

We recommend at least 4 inches of clean free-draining material, such as 2NS sand or pea gravel, be placed between the subgrade and the bottom of the floor slab for use as a capillary break to reduce moisture transmission through the concrete floors and to reduce the potential for concrete curling. If moisture sensitive floor coverings are planned or if greater protection against vapor transmission is desired, a vapor barrier consisting of 10-mil plastic sheeting, or equivalent, may be placed on the sand layer beneath floor slabs. However, additional floor slab curing techniques will be required especially if floor slab placement occurs in the winter months to prevent floor slab curling. The floor slab should be isolated from the foundation system to allow for independent movement.

## BELOW-GRADE WALL RECOMMENDATIONS

We understand that the lower-level floor slab for the northwest addition at Dexter High School will be approximately 14 feet below the adjacent exterior grade. As such, we anticipate below-grade walls, up to 14 feet tall, will be constructed around the north, west, and eastern perimeter of the building addition.

Below-grade walls should be designed to withstand lateral earth pressures due to backfilled soils and adjacent surcharge loads. Below-grade walls considered to be fixed at the top should be designed on the basis of at-rest lateral earth pressures corresponding to an equivalent fluid pressure of 55 pounds per square foot per foot of depth for drained granular backfill soil.

Below-grade wall backfill should consist of MDOT Class II sand to improve the wall drainage. Lateral earth pressures are significantly influenced by the type and intensity of backfill compaction. We recommend thin lifts, approximately 6 inches in thickness, be placed and relatively small compaction equipment be used to compact backfill placed against below-grade walls.

We recommend a foundation drain system or weep holes be incorporated to maintain drained soil conditions. To prevent the development of hydrostatic pressures on below-grade foundation walls, a sub drain system should be installed at the foundation level. The perforated or slotted sub drains should be encased with at least 12-inches of clean gravel or pea gravel, and the gravel and pipe together should be wrapped with a suitable non-woven filter fabric, such as Mirafi 140N, to prevent the migration of surrounding soil fines into the gravel and drainpipe. The sub drain system should outlet water to the nearby catch basin.

## CONSTRUCTION CONSIDERATIONS

Information regarding proposed utility inverts was unavailable at the time of this report; however, it is assumed that utilities will extend approximately 5 to 6 feet below finished grade. As described within this report, we anticipate foundation excavations will extend 3-1/2 to 8-1/2 feet below finished grade.

We anticipate caving and/or sloughing of the surficial granular soils during shallow foundation excavation operations. As such, the foundation contractor should come prepared to over-excavate and form foundation excavations. If earth formed foundations are utilized, we recommend foundation excavation and concrete placement operations be conducted on the same day to minimize potential for cave-ins or stormwater run-off into the open excavations.

In addition, we anticipate caving and/or sloughing of the granular soils during drilled shaft excavation operations. Therefore, the contractor should come to the site prepared to extend temporary telescopic steel casing to the bottom of the drilled shaft excavation to maintain a stable excavation during construction operations. Once drilling is completed to the design depth, reinforcing steel should be set and concrete placed until a positive head of concrete has been established within the casing. This positive concrete head should be maintained while pulling the casing to prevent sloughing of the granular soils into the fresh concrete. After concrete has been placed to an appropriate grade, the casing may be removed and concrete placement operations completed. The telescoped casing must be removed in the opposite order in which it is installed (i.e. – from the inside out).

To reduce lateral movement of the drilled pier foundation, the contractor must place the concrete for the pier in intimate contact with undisturbed soil. Fill any voids or enlargements in the drilled pier shaft excavation with concrete at the time of drilled pier concrete placement. We recommend using a concrete mix design with a slump of 5 to 7 inches for free fall placement to reduce the potential for concrete arching and provide a workable material.

We recommend using a short temporary top form, such as a sono tube, to form the top portion of the drilled pier. The use of this top form is a very beneficial aid to the correct placement and orientation of the anchor bolts.

In general, we do not anticipate significant accumulations of groundwater within the construction excavations anticipated for this project. However, groundwater intrusion may be encountered during excavation operations for the lower-level portion of the northwest addition at Dexter High School. We anticipate that any groundwater seepage or surface runoff can be controlled with a properly constructed system of sumps and pumps. If groundwater is required to be lowered more than 2 feet, we recommend that a system of shallow well-points be utilized.

We recommend maximum slope inclinations of 2H:1V within the very loose to loose cohesive soils, 1-1/2H:1V within the medium compact granular and medium cohesive soils, 1H:1V within the compact granular and stiff cohesive soils, and 3/4H:1V within the very stiff to hard cohesive soils for excavations extending below a depth of 5-feet. Where seepage from excavation cuts is observed, the slopes will need to be flattened sufficiently to achieve stability, but in no case left steeper than 3H:1V at and below the seepage level.

All excavations should be safely sheeted, shored, sloped, or braced in accordance with MI-OSHA requirements. If material is stored or equipment is operations near an excavation, lower angle slopes or stronger shoring must be used to resist the extra pressure due to the superimposed loads.

## GENERAL COMMENTS

We have formulated the evaluations and recommendations presented in this report relative to site preparation and foundation construction on the basis of data provided to us relating to the project location and surface grade for the proposed site(s). Any significant change in this data should be brought to our attention for review and evaluation with respect to prevailing subsurface conditions. Furthermore, if changes occur in the design, location, or concept of the project, conclusions and recommendations contained in this report are not valid unless G2 reviews the changes. G2 will then confirm the recommendations presented herein or make changes in writing.

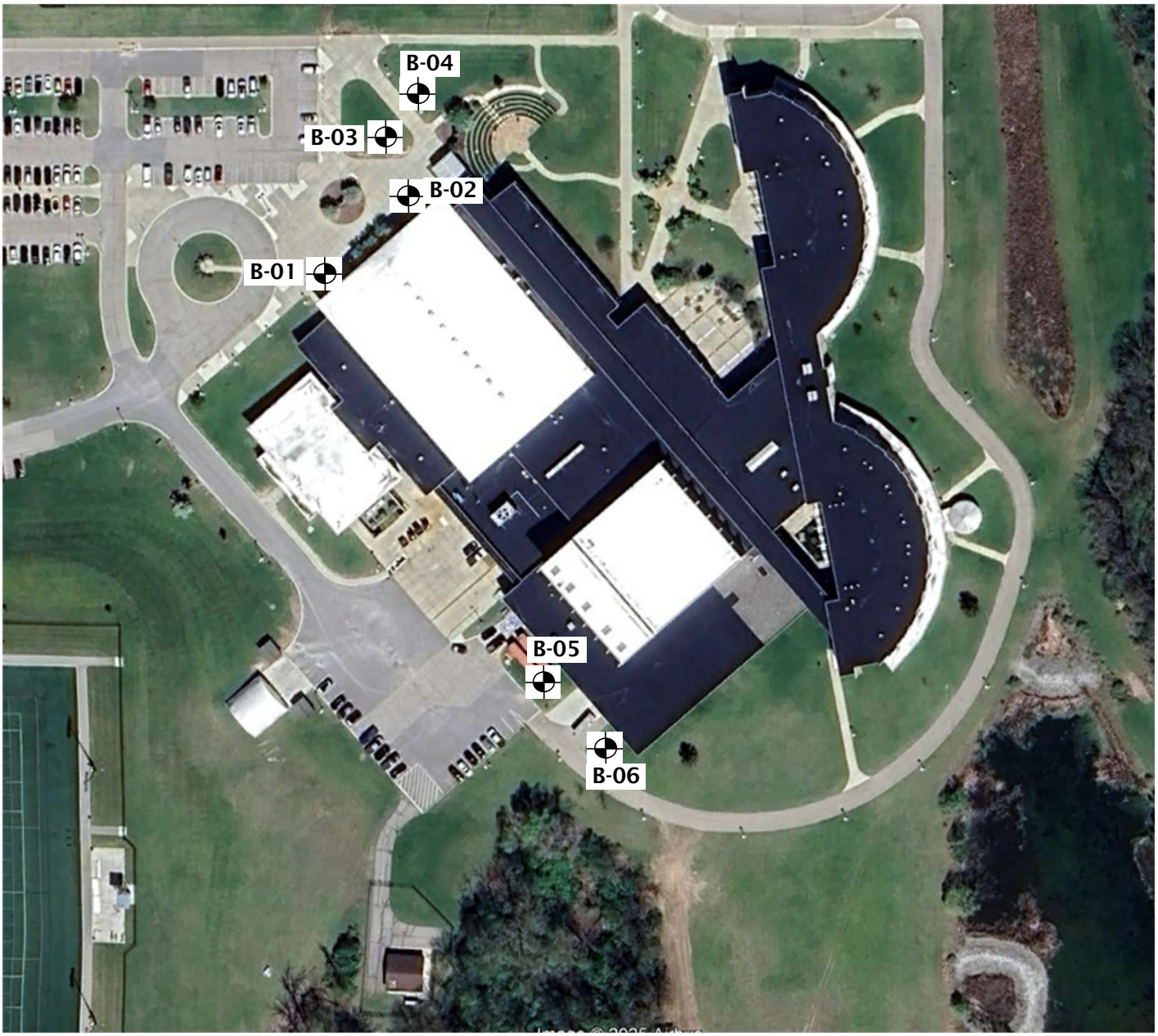
The scope of the present investigation was limited to evaluation of subsurface conditions for the support of proposed structures and other aspects of the proposed improvements. No chemical, environmental, or hydrogeological testing or analyses were included in the scope of this investigation.

We base the analyses and recommendations submitted in this report upon the data from the soil borings performed at the approximate locations shown on the Soil Boring Location Plans, Plate Nos. 1 through 4. This report does not reflect variations that may occur between the actual boring locations and the actual structure locations. The nature and extent of any such variations may not become clear until the time of construction. If significant variations then become evident, it may be necessary for us to re-evaluate our report recommendations.

We recommend G2 Consulting Group, LLC observe all geotechnical related work, including subgrade preparation, foundation construction, and engineered fill placement. G2 Consulting Group, LLC will perform the appropriate testing to confirm the geotechnical conditions given in the report are found during construction.

# APPENDIX

| | |
|---|---|
| SOIL BORING LOCATION PLANS | PLATE NOS. 1 THROUGH 4 |
| SOIL BORING LOGS | FIGURE NOS. 1 THROUGH 20 |
| UNCONFINED COMPRESSIVE STRENGTH TEST RESULTS | FIGURE NO. 21 |
| GENERAL NOTES TERMINOLOGY | FIGURE NO. 22 |




## Legend

Soil borings performed by 2G Drilling between August 26, 2025 and August 29, 2025

## Notes

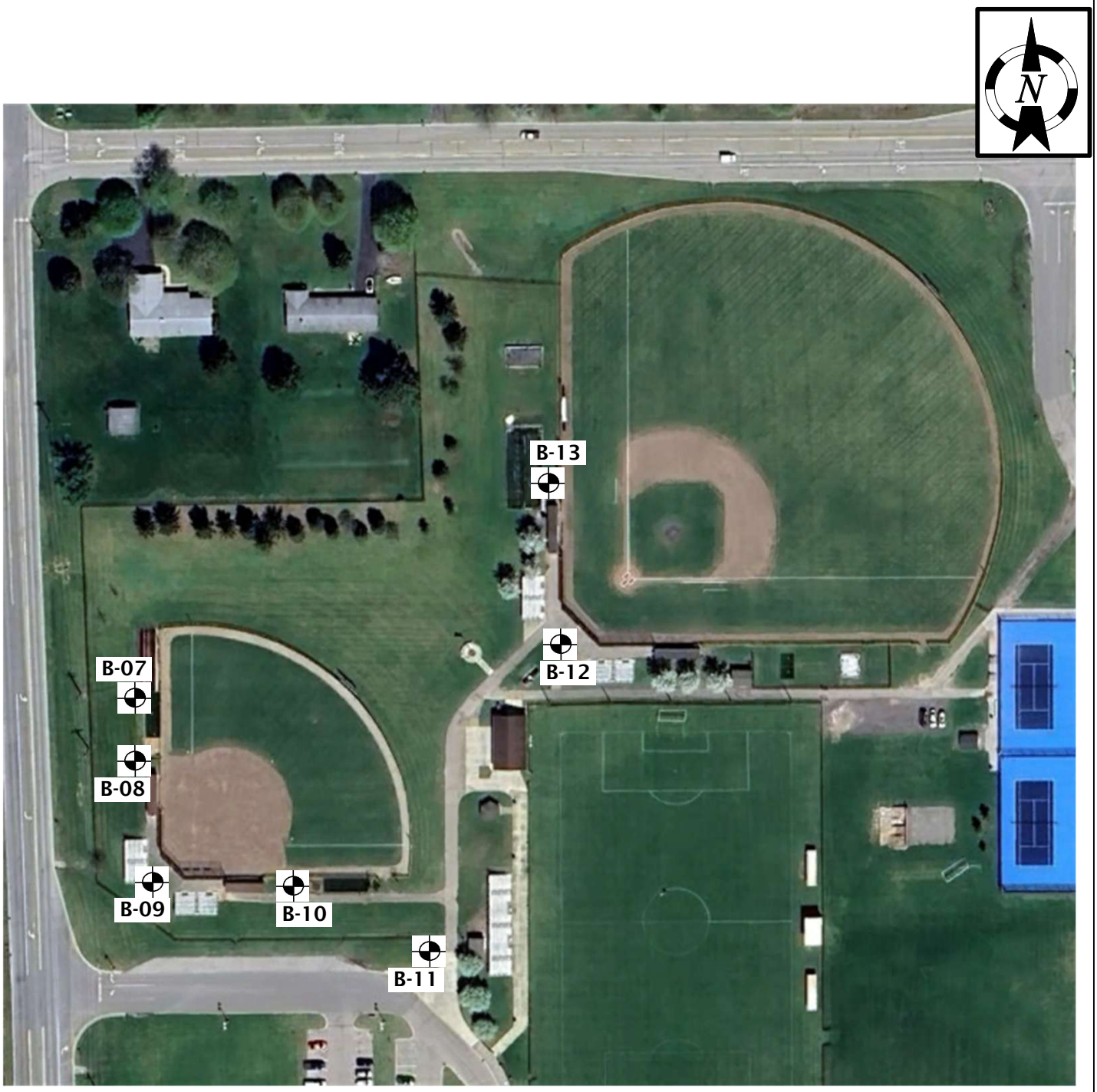
1. Soil borings B-01 through B-06 drilled to depths ranging from 25 to 45 feet.

# Soil Boring Location Plan

2025 Dexter Bond Program
Dexter High School Additions
Dexter, Michigan 48130

2G CONSULTING GROUP

| | |
|---|---|
| Project No. 253566 | |
| Drawn by: TSH | |
| Date: 9/26/2025 | Plate No. 1 |
| Scale: NTS | |




## Legend

Soil borings performed by 2G Drilling between August 29, 2025 and September 9, 2025

## Notes

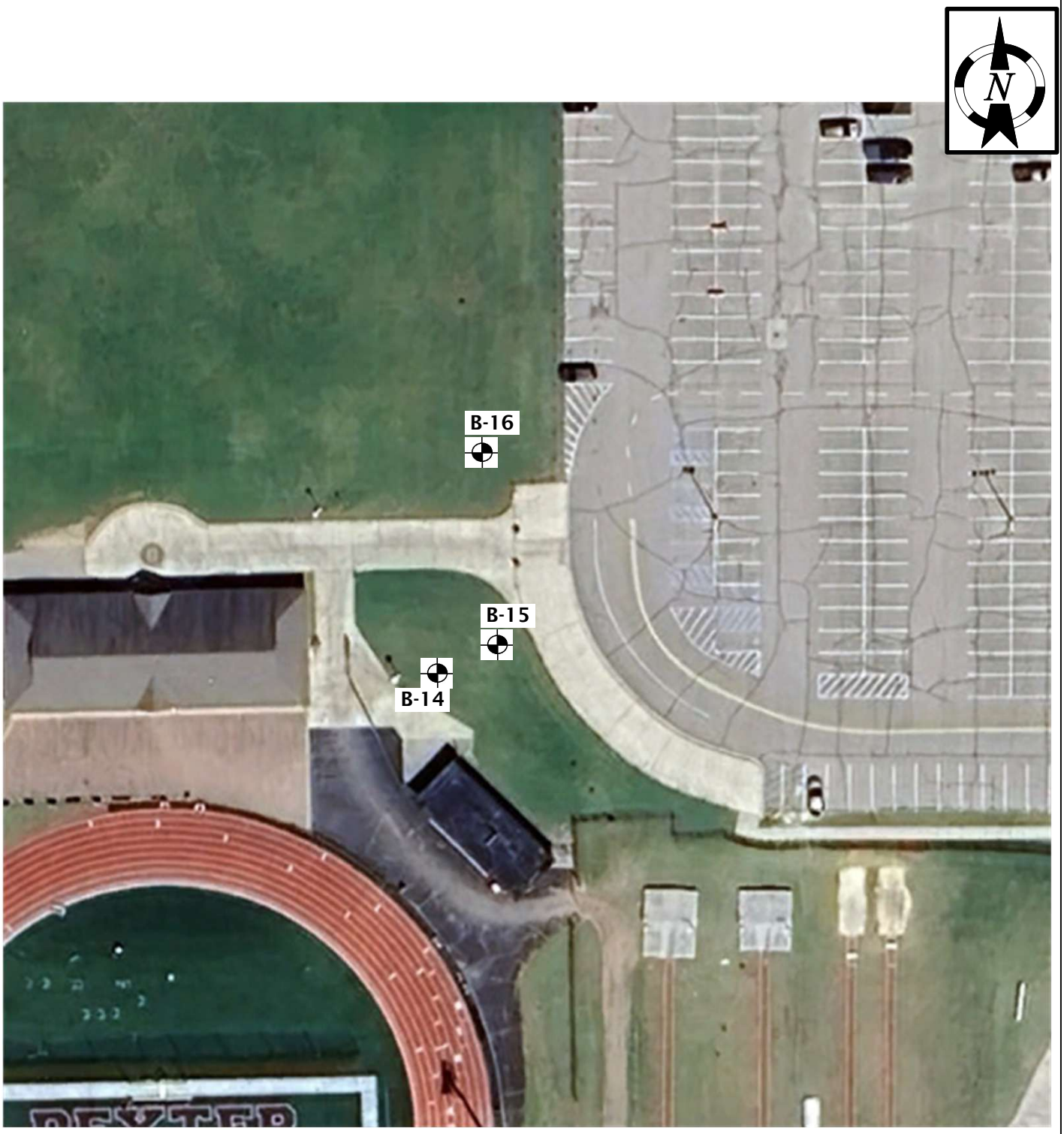
1. Soil borings B-07 through B-13 drilled to a depth of 10 feet each.

**Soil Boring Location Plan**

2025 Dexter Bond Program
Dexter High School Athletic Improvements
Dexter, Michigan 48130

2G CONSULTING GROUP

| | |
|---|---|
| Project No. 253566 | |
| Drawn by: TSH | |
| Date: 9/26/2025 | Plate No. 2 |
| Scale: NTS | |




## Legend

Soil borings performed by 2G Drilling on September 9, 2025

## Notes

1. Soil borings B-14 through B-16 drilled to a depth of 10 feet each.

**Soil Boring Location Plan**

2025 Dexter Bond Program
Creekside Intermediate School
Dexter, Michigan 48130

2G CONSULTING GROUP

| | |
|---|---|
| Project No. 253566 | |
| Drawn by: TSH | |
| Date: 9/26/2025 | Plate No. 3 |
| Scale: NTS | |




## Legend

Soil borings performed by 2G Drilling on September 8, 2025

## Notes

1. Soil borings B-17 through B-20 drilled to a depth of 25 feet each.

**Soil Boring Location Plan**

2025 Dexter Bond Program
Wylie Elementary School
Dexter, Michigan 48130

2G CONSULTING GROUP

| | |
|---|---|
| Project No. 253566 | |
| Drawn by: TSH | |
| Date: 9/26/2025 | Plate No. 4 |
| Scale: NTS | |

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.321468° Longitude: -83.895888°

# Soil Boring No. B-01

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH (ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Portland Cement Concrete (4 inches) 0.3 | | S-01 | 7 6 12 | 18 | | | |
| 5 | | Medium Compact Brownsih Gray Clayey Sand with trace silt and gravel | 5 | S-02 | 6 11 17 | 28 | | | |
| | | 7.5 | | S-03 | 4 7 7 | 14 | | | |
| 10 | | | 10 | S-04 | 8 8 6 | 14 | 7.8 | | 5500* |
| 15 | | Very Stiff Brown Sandy Clay with trace silt and gravel | 15 | S-05 | 5 5 8 | 13 | 9.6 | | 6500* |
| | | 19.0 | | S-06 | 6 8 16 | 24 | 12.2 | | 4500* |
| 20 | | Very Stiff Brownish Gray Silty Clay with trace sand and gravel | 20 | | | | | | |
| | | 22.0 | | S-07 | 6 8 16 | 24 | 19.3 | | 5000* |
| 25 | | Very Stiff to Hard Gray Silty Clay with trace gravel | 25 | | | | | | |
| | | 29.0 | | S-08A | 7 | | | | 9000* |
| 30 | | Medium Compact to Compact Gray Sandy Silt with trace clay and gravel | 30 | S-08B | 16 33 | 49 | | | |

Total Depth: 45 ft
Drilling Date: August 27, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
20 feet during drilling operations; 22 feet upon completion

Notes:
Borehole collapsed at 30 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings; topped with cold-patch asphalt

Figure No. 1a

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.321468° Longitude: -83.895888°

# Soil Boring No. B-01

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| 35 | | Medium Compact to Compact Gray Sandy Silt with trace clay and gravel *(continued)* | 35 | S-09 | 20<br>15<br>20 | 35 | | | |
| 40 | | | 40 | S-10 | 8<br>12<br>17 | 29 | | | |
| 45 | | 42.0<br>Medium Compact Gray Clayey Sand with trace silt and gravel<br>45.0 | 45 | S-11 | 8<br>13<br>16 | 29 | | | |
| 50 | | End of Boring @ 45 ft | 50 | | | | | | |
| 55 | | | 55 | | | | | | |
| 60 | | | 60 | | | | | | |

Total Depth: 45 ft
Drilling Date: August 27, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
20 feet during drilling operations; 22 feet upon completion

Notes:
Borehole collapsed at 30 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings; topped with cold-patch asphalt

Figure No. 1b

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.321651° Longitude: -83.895591°

# Soil Boring No. B-02

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

| DEPTH (ft) | PRO-FILE | | DEPTH (ft) |
|---|---|---|---|
| | | Portland Cement Concrete (4 inches) 0.3 | |
| | | Fill: Brown Sand with trace gravel 1.0 | |
| | | Fill: Very Stiff Brownish Gray Sandy Clay with trace gravel; occasional sand seams and wood fragments | |
| 5 | | | 5 |
| | | 6.0 | |
| | | Loose Brown Clayey Sand with trace gravel; occasional cobbles | |
| | | 8.0 | |
| 10 | | | 10 |
| | | Hard Mottled Brown Silty Clay with trace sand and gravel | |
| 15 | | | 15 |
| | | 19.0 | |
| 20 | | | 20 |
| | | Medium Compact Gray Silt with trace sand; occasional sand seams | |
| | | 23.0 | |
| 25 | | | 25 |
| | | Medium Compact Gray Silty Sand | |
| | | 29.0 | |
| 30 | | Compact to Very Compact Clayey Silt with trace sand and gravel | 30 |

## SOIL SAMPLE DATA

| SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|
| S-01 | 2 1 2 | 3 | 12.9 | | 4500* |
| S-02 | 3 3 4 | 7 | 9.7 | | 6500* |
| S-03 | 3 2 3 | 5 | | | |
| S-04 | 8 6 9 | 15 | 7.9 | | 9000* |
| S-05 | 4 4 5 | 9 | 13.5 | | 9000* |
| S-06 | 6 6 5 | 11 | | | |
| S-07 | 6 8 10 | 18 | | | |
| S-08 | 8 13 26 | 39 | | | |

Total Depth: 45 ft
Drilling Date: August 27, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during and upon completion of drilling operations

Notes:
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings; topped with cold-patch asphalt

Figure No. 2a

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.321651° Longitude: -83.895591°

# Soil Boring No. B-02

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Compact to Very Compact Clayey Silt with trace sand and gravel *(continued)* | | | | | | | |
| | | 32.0 | | | | | | | |
| 35 | | Hard Brownish Gray Sandy Clay with trace silt and gravel | 35 | S-09 | 18<br>27<br>36 | 63 | 8.8 | | 9000* |
| | | 37.0 | | | | | | | |
| 40 | | Hard Gray Silty Clay with trace sand and gravel | 40 | S-10 | 27<br>28<br>37 | 65 | 14.0 | | 9000* |
| | | 41.0 | | | | | | | |
| 45 | | Very Compact Gray Sand with trace silt | 45 | S-11 | 46<br>50/4" | --- | | | |
| | | 45.0 | | | | | | | |
| | | End of Boring @ 45 ft | | | | | | | |
| 50 | | | 50 | | | | | | |
| 55 | | | 55 | | | | | | |
| 60 | | | 60 | | | | | | |

Total Depth: 45 ft
Drilling Date: August 27, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during and upon completion of drilling operations

Notes:
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings; topped with cold-patch asphalt

Figure No. 2b

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.321775° Longitude: -83.895663°

# Soil Boring No. B-03

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

| DEPTH (ft) | PRO-FILE | | DEPTH (ft) |
|---|---|---|---|
| | | Topsoil: Dark Brown Sandy Clay (5 inches) 0.4 | |
| 5 | | Stiff to Very Stiff Brown Sandy Clay with trace silt and gravel; occasional cobbles 6.0 | 5 |
| | | Loose Brown Silty Sand with trace clay and gravel 8.0 | |
| 10 | | Very Compact Brownish Gray Clayey Sand with trace silt and gravel; occasional sand seams 14.0 | 10 |
| 15 | | Very Stiff Brown Silty Clay with trace sand 19.0 | 15 |
| 20 | | Compact Brown Silty Sand with trace clay and gravel 24.0 | 20 |
| 25 | | Compact Gray Sand with trace silt and gravel 29.0 | 25 |
| 30 | | Hard Gray Silty Clay with trace sand | 30 |

## SOIL SAMPLE DATA

| SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|
| S-01 | 11<br>10<br>9 | 19 | 13.0 | | 3000* |
| S-02 | 5<br>4<br>4 | 8 | 11.5 | | 4500* |
| S-03 | 3<br>2<br>3 | 5 | | | |
| S-04 | 12<br>46<br>21 | 67 | | | |
| S-05 | 2<br>5<br>6 | 11 | 20.1 | | 4000* |
| S-06 | 13<br>18<br>17 | 35 | | | |
| S-07 | 13<br>16<br>15 | 31 | | | |
| S-08 | 20<br>48<br>48 | 96 | 7.0 | | 9000* |

Total Depth: 45 ft
Drilling Date: August 27, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
19 feet during drilling operations; wet cave at 6 feet upon removal of drilling augers

Notes:
Borehole collapsed at 6 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 3a

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.321775° Longitude: -83.895663°

# Soil Boring No. B-03

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Hard Gray Silty Clay with trace sand *(continued)* | | | | | | | |
| | | 34.0 | | | | | | | |
| 35 | | | 35 | S-09 | 23<br>28<br>32 | 60 | | | |
| | | Very Compact Gray Silty Sand with trace clay and gravel | | | | | | | |
| | | 39.0 | | | | | | | |
| 40 | | | 40 | S-10 | 32<br>43<br>50/5" | --- | 5.2 | | 9000* |
| | | Hard Gray Sandy Clay with trace silt and gravel | | | | | | | |
| | | 42.0 | | | | | | | |
| | | Compact Gray Sand with trace silt and gravel | | | | | | | |
| 45 | | 45.0 | 45 | S-11 | 13<br>18<br>32 | 50 | | | |
| | | End of Boring @ 45 ft | | | | | | | |
| 50 | | | 50 | | | | | | |
| 55 | | | 55 | | | | | | |
| 60 | | | 60 | | | | | | |

Total Depth: 45 ft
Drilling Date: August 27, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
19 feet during drilling operations; wet cave at 6 feet upon removal of drilling augers

Notes:
Borehole collapsed at 6 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 3b

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.321892° Longitude: -83.895559°

# Soil Boring No. B-04

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH (ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Topsoil: Dark Brown Sandy Clay (5 inches) 0.4 | | S-01 | 11 7 4 | 11 | | | |
| 5 | | Loose to Medium Compact Brown Clayey Sand with trace silt and gravel; occasional cobbles | 5 | S-02 | 5 6 4 | 10 | | | |
| | | | | S-03 | 4 3 10 | 13 | | | |
| | | 8.0 | | | | | | | |
| 10 | | | 10 | S-04 | 20 19 11 | 30 | | | |
| | | Medium Compact Brown Clayey Sand with trace silt and gravel; occasional cobbles | | | | | | | |
| 15 | | | 15 | S-05 | 6 5 6 | 11 | | | |
| | | 16.0 | | | | | | | |
| 20 | | Medium Compact Brownish Gray Clayey Sand with trace silt and gravel | 20 | S-06 | 11 10 15 | 25 | | | |
| | | 22.0 | | | | | | | |
| 25 | | | 25 | S-07 | 15 37 39 | 76 | | | |
| | | Very Compact Brown Silty Sand with trace clay and gravel | | | | | | | |
| 30 | | 30.0 | 30 | S-08 | 20 40 40 | 80 | 9.7 | | 8000* |

Total Depth: 45 ft
Drilling Date: August 26, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
15 feet during drilling operations; wet cave at 30-1/2 feet upon removal of drilling augers

Notes:
Borehole collapsed at 30-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 4a

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.321892° Longitude: -83.895559°

# Soil Boring No. B-04

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Hard Brownish Gray Sandy Clay with trace gravel | | | | | | | |
| 35 | | 35.0 | 35 | S-09 | 12<br>16<br>18 | 34 | 8.1 | | 9000* |
| | | Medium Compact Gray Gravelly Sand with trace clay | | | | | | | |
| 40 | | | 40 | S-10 | 11<br>10<br>12 | 22 | | | |
| | | 41.0 | | | | | | | |
| | | Hard Gray Silty Clay with trace sand and gravel | | | | | | | |
| 45 | | 45.0 | 45 | S-11 | 21<br>31<br>40 | 71 | 6.9 | | 9000* |
| | | End of Boring @ 45 ft | | | | | | | |
| 50 | | | 50 | | | | | | |
| 55 | | | 55 | | | | | | |
| 60 | | | 60 | | | | | | |

Total Depth: 45 ft
Drilling Date: August 26, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
15 feet during drilling operations; wet cave at 30-1/2 feet upon removal of drilling augers

Notes:
Borehole collapsed at 30-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 4b

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.320458° Longitude: -83.895170°

# Soil Boring No. B-05

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH (ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Topsoil: Dark Brown Sand (3 inches) 0.3 | | | | | | | |
| | | Compact Gray Silty Sand with trace clay and gravel; occasional cobbles | | S-01 | 15 23 26 | 49 | | | |
| | | 3.0 | | S-02 | 18 23 24 | 47 | | | |
| 5 | | Compact Brownish Gray Sand with trace silt and gravel; occasional cobbles | 5 | S-03 | 17 22 22 | 44 | | | |
| | | 8.0 | | S-04 | 22 17 15 | 32 | | | |
| 10 | | Compact Reddish Brown Sand with trace silt and gravel; occasional cobbles | 10 | S-05 | 12 17 19 | 36 | | | |
| | | 12.0 ▽ | | | | | | | |
| | | Compact Reddish Brown Clayey Sand with trace gravel | | S-06 | 17 23 24 | 47 | | | |
| 15 | | 15.0 | 15 | S-07 | 12 19 21 | 40 | | | |
| | | Compact Brown Sandy Gravel with trace clay | | | | | | | |
| 20 | | | 20 | | | | | | |
| 25 | | 25.0 | 25 | | | | | | |
| | | End of Boring @ 25 ft | | | | | | | |
| 30 | | | 30 | | | | | | |

Total Depth: 25 ft
Drilling Date: August 28, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
12 feet during drilling operations; wet cave at 11-1/2 feet upon removal of drilling augers

Notes:
Borehole collapsed at 11-1/2 ft after auger removal

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 5

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.320283° Longitude: -83.894967°

# Soil Boring No. B-06

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Topsoil: Dark Brown Sand (2 inches) 0.2 | | S-01 | 16 17 18 | 35 | | | |
| 5 | | | 5 | S-02 | 16 13 11 | 24 | | | |
| | | Medium Compact to Compact Brown Gravelly Sand with trace silt | | S-03 | 7 7 7 | 14 | | | |
| 10 | | | 10 | S-04 | 8 6 5 | 11 | | | |
| | | 12.0 | | | | | | | |
| 15 | | Compact Reddish Brown Gravelly Sand with trace silt and clay | 15 | S-05 | 16 20 24 | 44 | | | |
| | | 17.0 | | | | | | | |
| 20 | | Compact Brown Gravelly Sand with trace silt and clay; occasional sand seams | 20 | S-06 | 6 18 23 | 41 | | | |
| | | 22.0 | | | | | | | |
| 25 | | Compact Gray Sandy Gravel with trace silt and clay 25.0 | 25 | S-07 | 16 21 27 | 48 | | | |
| | | End of Boring @ 25 ft | | | | | | | |
| 30 | | | 30 | | | | | | |

Total Depth: 25 ft
Drilling Date: August 29, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
12 feet during drilling operations; wet cave at 7-1/2 feet upon removal of drilling augers

Notes:
Borehole collapsed at 7-1/2 ft after auger removal

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 6

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.323710° Longitude: -83.899201°

# Soil Boring No. B-07

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Topsoil: Dark Brown Sandy Clay (2 inches) 0.2 | | | | | | | |
| | | Hard Brown Sandy Clay with trace silt and gravel | | S-01 | 3<br>3<br>4 | 7 | 9.5 | | 9000* |
| | | 3.0 | | | | | | | |
| 5 | | Hard Brown Silty Clay with trace sand and gravel; occasional cobbles | 5 | S-02 | 8<br>13<br>24 | 37 | 14.0 | | 9000* |
| | | 6.0 | | | | | | | |
| | | Very Compact Brown Silty Sand with trace gravel; occasional cobbles | | S-03 | 28<br>40<br>50/5" | --- | | | |
| 10 | | 10.0 | 10 | S-04 | 48<br>50/4" | --- | | | |
| | | End of Boring @ 10 ft | | | | | | | |
| 15 | | | 15 | | | | | | |
| 20 | | | 20 | | | | | | |
| 25 | | | 25 | | | | | | |
| 30 | | | 30 | | | | | | |

Total Depth: 10 ft
Drilling Date: September 2, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 7 feet upon removal of drilling augers

Notes:
Borehole collapsed at 7 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 7

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.323572° Longitude: -83.899196°

# Soil Boring No. B-08

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Topsoil: Dark Brown Clayey Sand (3 inches) 0.3 | | | | | | | |
| | | Fill: Very Loose Dark Brown Clayey Sand with trace gravel | | S-01 | 2<br>1<br>1 | 2 | | | |
| | | 4.0 | | | | | | | |
| 5 | | Hard Gray Silty Clay with trace gravel | 5 | S-02 | 4<br>4<br>7 | 11 | 10.4 | | 9000* |
| | | 7.0 | | S-03 | 12<br>18<br>18 | 36 | | | |
| | | Compact Gray Sandy Silt with trace clay and gravel 9.0 | | | | | | | |
| 10 | | Very Compact Brown Sand with trace silt and gravel 10.0 | 10 | S-04 | 24<br>33<br>36 | 69 | | | |
| | | End of Boring @ 10 ft | | | | | | | |
| 15 | | | 15 | S-05 | | | | | |
| 20 | | | 20 | S-06 | | | | | |
| 25 | | | 25 | S-07 | | | | | |
| 30 | | | 30 | | | | | | |

Total Depth: 10 ft
Drilling Date: September 2, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 6-3/4 feet upon removal of drilling augers

Notes:
Borehole collapsed at 7 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 8

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.323265° Longitude: -83.899153°

# Soil Boring No. B-09

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

Topsoil: Dark Brown Sandy Clay (2 inches) — 0.2

Hard Brown Sandy Clay with trace silt and gravel — 3.0

Very Stiff Mottled Brown and Gray Silty Clay with trace sand and gravel — 6.5

Loose Brown Silt with trace clay and sand — 9.0

Hard Gray Silty Clay with trace gravel — 10.0

End of Boring @ 10 ft

## SOIL SAMPLE DATA

| DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|
| 0 | S-01 | 4 2 6 | 8 | 13.6 | | 9000* |
| 5 | S-02 | 4 5 6 | 11 | 11.1 | | 7000* |
| | S-03 | 4 2 7 | 9 | | | |
| 10 | S-04 | 5 6 6 | 12 | 10.3 | | 9000* |

Total Depth: 10 ft
Drilling Date: September 2, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 8 feet upon removal of drilling augers

Notes:
Borehole collapsed at 8 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 9

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.323254° Longitude: -83.898674°

# Soil Boring No. B-10

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

| DEPTH (ft) | PRO-FILE | | DEPTH (ft) |
|---|---|---|---|
| | | Topsoil: Dark Brown Sandy Clay (5 inches) 0.4 | |
| | | Hard Brown Silty Clay with trace sand and gravel 4.5 | |
| 5 | | Medium Compact Brown Sand with trace clay and gravel 5.0 | 5 |
| | | Medium Compact Brown Clayey Sand with trace silt and gravel 7.5 | |
| 10 | | Medium Compact Brown Sand with trace clay and gravel 10.0 | 10 |
| | | End of Boring @ 10 ft | |
| 15 | | | 15 |
| 20 | | | 20 |
| 25 | | | 25 |
| 30 | | | 30 |

## SOIL SAMPLE DATA

| SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|
| S-01 | 3 4 6 | 10 | 14.5 | | 9000* |
| S-02A | 20 6 | | 21.7 | | 9000* |
| S-02B | 9 | 15 | | | |
| S-03 | 7 9 14 | 23 | | | |
| S-04 | 10 11 16 | 27 | | | |

Total Depth: 10 ft
Drilling Date: September 9, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during dirlling operations; dry cave at 8-1/2 feet upon removal of drilling augers

Notes:
Borehole collapsed at 8-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 10

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.323073° Longitude: -83.898231°

# Soil Boring No. B-11

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Medium Compact Brown Clayey Sand with trace silt and gravel | | S-01 | 10<br>14<br>12 | 26 | | | |
| | | 3.0 | | | | | | | |
| 5 | | Loose Brown Sand with trace silt and gravel | 5 | S-02 | 5<br>4<br>6 | 10 | | | |
| | | 5.0 | | | | | | | |
| | | Very Stiff Brownish Gray Sandy Clay with trace silt and gravel | | S-03 | 5<br>5<br>6 | 11 | 9.8 | | 6000* |
| 10 | | 10.0 | 10 | S-04 | 3<br>6<br>4 | 10 | 8.3 | | 7000* |
| | | End of Boring @ 10 ft | | | | | | | |
| 15 | | | 15 | | | | | | |
| 20 | | | 20 | | | | | | |
| 25 | | | 25 | | | | | | |
| 30 | | | 30 | | | | | | |

Total Depth: 10 ft
Drilling Date: September 2, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 7-1/4 feet upon removal of drilling augers

Notes:
Borehole collapsed at 7-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 11

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.323859° Longitude: -83.897777°

# Soil Boring No. B-12

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

- 0.3: Topsoil: Dark Brown Sand (4 inches)
- Fill: Loose to Medium Compact Brown Clayey Sand with trace silt and gravel
- 7.0: Fill: Very Stiff Gray Sandy Clay with trace silt and gravel; occasional roots (Organic Matter Content = 10.1%)
- 8.5: Hard Gray Silty Clay with trace sand and gravel
- 10.0: End of Boring @ 10 ft

## SOIL SAMPLE DATA

| DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|
| 5 | S-01 | 4 3 3 | 6 | | | |
| | S-02 | 6 5 8 | 13 | | | |
| | S-03 | 6 9 9 | 18 | 10.3 | | 5000* |
| 10 | S-04 | 6 10 12 | 22 | 7.2 | | 9000* |

Total Depth: 10 ft
Drilling Date: August 29, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
6 feet during drilling operations; wet cave at 8 feet upon removal of drilling augers

Notes:
Borehole collapsed at 8 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 12

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.324221° Longitude: -83.897820°

# Soil Boring No. B-13

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

- 0.3: Topsoil: Dark Brown Sandy Clay (3-1/2 inches)
- Hard Brown Silty Clay with trace sand and gravel
- 3.5: Medium Compact Brown Sand with trace silt and clay
- 7.0: Very Stiff Brownish Gray Silty Clay with trace sand and gravel
- 9.0: Hard Gray Silty Clay with trace gravel
- 10.0: End of Boring @ 10 ft

## SOIL SAMPLE DATA

| DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|
| 0 | S-01 | 2 2 5 | 7 | 12.9 | | 9000* |
| 5 | S-02 | 4 8 9 | 17 | | | |
| | S-03 | 5 7 8 | 15 | 11.6 | | 5000* |
| 10 | S-04 | 5 8 14 | 22 | 7.1 | | 9000* |

Total Depth: 10 ft
Drilling Date: August 29, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
7 feet during drilling operations; wet cave at 7-1/2 feet upon removal of drilling augers

Notes:
Borehole collapsed at 7-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 13

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.327094° Longitude: -83.889376°

# Soil Boring No. B-14

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

| DEPTH (ft) | PRO-FILE | | DEPTH (ft) |
|---|---|---|---|
| | | Topsoil: Dark Brown Sand (2 inches) | 0.2 |
| | | Very Loose Brown Clayey Sand with trace silt and gravel | |
| | | | 3.0 |
| | | Medium Compact Brown Silty Sand with trace gravel | |
| 5 | | | 4.5 5 |
| | | Hard Brown Silty Clay with trace sand and gravel | |
| 10 | | | 10.0 10 |
| | | End of Boring @ 10 ft | |
| 15 | | | 15 |
| 20 | | | 20 |
| 25 | | | 25 |
| 30 | | | 30 |

## SOIL SAMPLE DATA

| SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|
| S-01 | 3<br>2<br>2 | 4 | | | |
| S-02 | 9<br>13<br>15 | 28 | | | |
| S-03 | 4<br>5<br>6 | 11 | 9.5 | | 9000* |
| S-04 | 5<br>9<br>10 | 19 | 10.5 | | 8500* |

Total Depth: 10 ft
Drilling Date: September 9, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 7-1/2 feet upon removal of drilling augers

Notes:
Borehole collapsed at 7-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 14

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.327125° Longitude: -83.889265°

# Soil Boring No. B-15

CONSULTING GROUP

| SUBSURFACE PROFILE | | | | SOIL SAMPLE DATA | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| DEPTH ( ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH ( ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
| | | Topsoil: Dark Brown Sand (2 inches) 0.2 | | | | | | | |
| | | Fill: Loose Dark Brown Clayey Sand with trace silt and gravel | | S-01 | 3 4 2 | 6 | | | |
| | | 4.0 | | | | | | | |
| 5 | | Fill: Loose Brown Sand with trace silt and gravel; occasional clay clods | 5 | S-02 | 2 3 4 | 7 | | | |
| | | 6.0 | | | | | | | |
| | | Hard Gray Silty Clay with trace sand and gravel | | S-03 | 3 6 8 | 14 | 9.8 | | 9000* |
| | | 9.5 | | | | | | | |
| 10 | | Medium Compact Gray Sand with trace clay and gravel 10.0 | 10 | S-04 | 4 8 11 | 19 | | | |
| | | End of Boring @ 10 ft | | | | | | | |
| 15 | | | 15 | | | | | | |
| 20 | | | 20 | | | | | | |
| 25 | | | 25 | | | | | | |
| 30 | | | 30 | | | | | | |

Total Depth: 10 ft
Drilling Date: September 9, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; wet cave at 8 feet upon removal of drilling augers

Notes:
Borehole collapsed at 8 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 15

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.327343° Longitude: -83.889315°

# Soil Boring No. B-16

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

- 0.2: Topsoil: Dark Brown Sand (2 inches)
- Fill: Loose Dark Brown Clayey Sand with trace silt and gravel
- 6.0: Very Stiff Brown Silty Clay with trace gravel
- 9.0: Hard Gray Sandy Clay with trace silt and gravel
- 10.0: End of Boring @ 10 ft

## SOIL SAMPLE DATA

| DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|
| 0 | S-01 | 5<br>4<br>4 | 8 | | | |
| 5 | S-02 | 7<br>7<br>3 | 10 | | | |
| 6.0 | S-03 | 3<br>2<br>2 | 4 | 9.5 | | 4500* |
| 9.0 | S-04 | 6<br>9<br>9 | 18 | 8.0 | | 9000* |
| 10.0 | | | | | | |

Total Depth: 10 ft
Drilling Date: September 9, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
Dry during drilling operations; dry cave at 7-1/2 feet upon removal of drilling augers

Notes:
Borehole collapsed at 7-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 16

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.328801° Longitude: -83.882207°

# Soil Boring No. B-17

CONSULTING GROUP

## SUBSURFACE PROFILE

## SOIL SAMPLE DATA

| DEPTH (ft) | PRO-FILE | GROUND SURFACE ELEVATION: N/A | DEPTH (ft) | SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|---|---|---|---|
| | | Topsoil: Dark Brown Clayey Sand (2 inches) 0.2 | | | | | | | |
| | | Fill: Loose Brown Clayey Sand with trace silt | | S-01 | 5<br>4<br>4 | 8 | | | |
| | | 3.0 | | | | | | | |
| 5 | | Fill: Stiff Brown Silty Clay with trace sand and gravel; occasional roots and grass blades (Organic Matter Content = 3.7%) | 5 | S-02 | 1<br>2<br>2 | 4 | 15.8 | | 2500* |
| | | 7.0 | | S-03 | 5<br>6<br>8 | 14 | | | |
| | | Medium Compact Brown Silty Sand with trace clay 9.0 | | | | | | | |
| 10 | | | 10 | S-04 | 11<br>11<br>11 | 22 | | | |
| 15 | | Medium Compact to Compact Brown Sand with trace clay and gravel; occasional clay seams | 15 | S-05 | 8<br>16<br>26 | 42 | | | |
| | | 19.5 | | | | | | | |
| 20 | | | 20 | S-06 | 19<br>22<br>24 | 46 | | | |
| 25 | | Compact to Very Compact Brown Clayey Sand with trace silt and gravel 25.0 | 25 | S-07 | 43<br>50/5" | --- | | | |
| | | End of Boring @ 25 ft | | | | | | | |
| 30 | | | 30 | | | | | | |

Total Depth: 25 ft
Drilling Date: September 8, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
19-1/2 feet during drilling operations; wet cave at 18-1/2 feet upon removal of drilling augers

Notes:
Borehole collapsed at 18-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 17

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.328782° Longitude: -83.881990°

# Soil Boring No. B-18

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

| DEPTH (ft) | PROFILE | DESCRIPTION | DEPTH (ft) |
|---|---|---|---|
| | | Topsoil: Dark Brown Sandy Clay (2 inches) | 0.2 |
| | | Medium Compact Brown Clayey Sand with trace silt and gravel | 3.0 |
| 5 | | Stiff Brown Sandy Clay with trace silt | 5 |
| | | | 7.0 |
| | | Very Loose Brown Silty Sand with trace gravel; occasional clay clods | 9.0 |
| 10 | | Medium Compact Brown Clayey Sand with trace silt and gravel | 10 |
| | | | 12.0 |
| 15 | | Very Compact Brown Clayey Sand with trace silt and gravel | 15 |
| | | | 18.0 |
| 20 | | Very Compact Brown Sand with trace silt and gravel | 20 |
| | | | 23.0 |
| 25 | | Hard Brown Silty Clay with trace sand; occasional cobbles | 25.0 25 |
| | | End of Boring @ 25 ft | |
| 30 | | | 30 |

## SOIL SAMPLE DATA

| SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|
| S-01 | 12<br>11<br>9 | 20 | | | |
| S-02 | 1<br>1<br>2 | 3 | 15.0 | | 3500* |
| S-03 | 0<br>2<br>2 | 4 | | | |
| S-04 | 5<br>10<br>19 | 29 | | | |
| S-05 | 21<br>28<br>38 | 66 | | | |
| S-06 | 23<br>26<br>31 | 57 | | | |
| S-07 | 43<br>50/4" | --- | 10.4 | | 9000* |

Total Depth: 25 ft
Drilling Date: September 8, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
18 feet during drilling operations; wet cave at 19 feet upon removal of drilling augers

Notes:
Borehole collapsed at 19 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 18

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.328620° Longitude: -83.881992°

# Soil Boring No. B-19

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

| DEPTH (ft) | PRO-FILE | DESCRIPTION | DEPTH (ft) |
|---|---|---|---|
| | | Topsoil: Dark Brown Sand (5 inches) | 0.4 |
| | | Fill: Hard Dark Brown Sandy Clay with trace gravel | 2.5 |
| 5 | | Fill: Medium Compact Dark Brown Clayey Sand with trace silt | 5 |
| | | | 7.0 |
| 10 | | Stiff to Very Stiff Brown Silty Clay with trace sand | 10 |
| | | | 12.0 |
| 15 | | Compact Brown Clayey Sand with trace silt and gravel; occasional cobbles | 15 |
| 20 | | | 20 |
| | | | 22.0 |
| 25 | | Very Compact Brown Sand with trace silt and gravel; occasional cobbles and boulders | 25 |
| | | | 25.0 |
| 30 | | End of Boring @ 25 ft | 30 |

## SOIL SAMPLE DATA

| SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|
| S-01 | 14<br>12<br>5 | 17 | 11.0 | | 9000 |
| S-02 | 4<br>1<br>10 | 11 | | | |
| S-03 | 1<br>1<br>3 | 4 | 16.5 | | 3000 |
| S-04 | 0<br>1<br>2 | 3 | 14.9 | | 5000 |
| S-05 | 6<br>15<br>20 | 35 | | | |
| S-06 | 21<br>22<br>17 | 39 | | | |
| S-07 | 31<br>50/4" | --- | | | |

Total Depth: 25 ft
Drilling Date: September 8, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

Water Level Observation:
24-1/2 feet during drilling operations; wet cave at 22-1/2 feet upon removal of drilling augers

Notes:
Borehole collapsed at 24-1/2 ft after auger removal

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 19

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No. 253566

Latitude: 42.328626° Longitude: -83.882205°

# Soil Boring No. B-20

CONSULTING GROUP

## SUBSURFACE PROFILE

GROUND SURFACE ELEVATION: N/A

| DEPTH (ft) | PRO-FILE | | DEPTH (ft) |
|---|---|---|---|
| | | Topsoil: Dark Brown Sand (2 inches) — 0.2 | |
| | | Very Loose Brown Silty Sand — 3.0 | |
| 5 | | Medium Brown Sandy Clay with trace silt and gravel — 5.5 | 5 |
| | | Very Stiff Brown Silty Clay with trace sand and gravel — 8.0 | |
| 10 | | | 10 |
| 15 | | Compact Brown Silty Sand with trace gravel; occasional cobbles — 18.0 | 15 |
| 20 | | | 20 |
| | | Compact Brown Clayey Sand with trace gravel — 24.0 | |
| 25 | | Stiff Brownish Gray Silty Clay with trace sand and gravel — 25.0 | 25 |
| | | End of Boring @ 25 ft | |
| 30 | | | 30 |

## SOIL SAMPLE DATA

| SAMPLE TYPE-NO. | BLOWS/ 6-INCHES | STD. PEN. RESISTANCE (N) | MOISTURE CONTENT (%) | DRY DENSITY (PCF) | UNCONF. COMP. STR. (PSF) |
|---|---|---|---|---|---|
| S-01 | 1 2 2 | 4 | | | |
| S-02 | 2 2 3 | 5 | 11.1 | 116 | 1210 |
| S-03 | 13 16 15 | 31 | 11.1 | | 4000* |
| S-04 | 8 19 20 | 39 | | | |
| S-05 | 9 20 20 | 40 | | | |
| S-06 | 15 20 25 | 45 | | | |
| S-07 | 29 40 50/3" | --- | 9.2 | | 3500* |

Total Depth: 25 ft
Drilling Date: September 8, 2025
Inspector:
Contractor: 2G Drilling
Driller: Q. Tores

Drilling Method:
2-1/4-inch inside-diameter hollow-stem augers

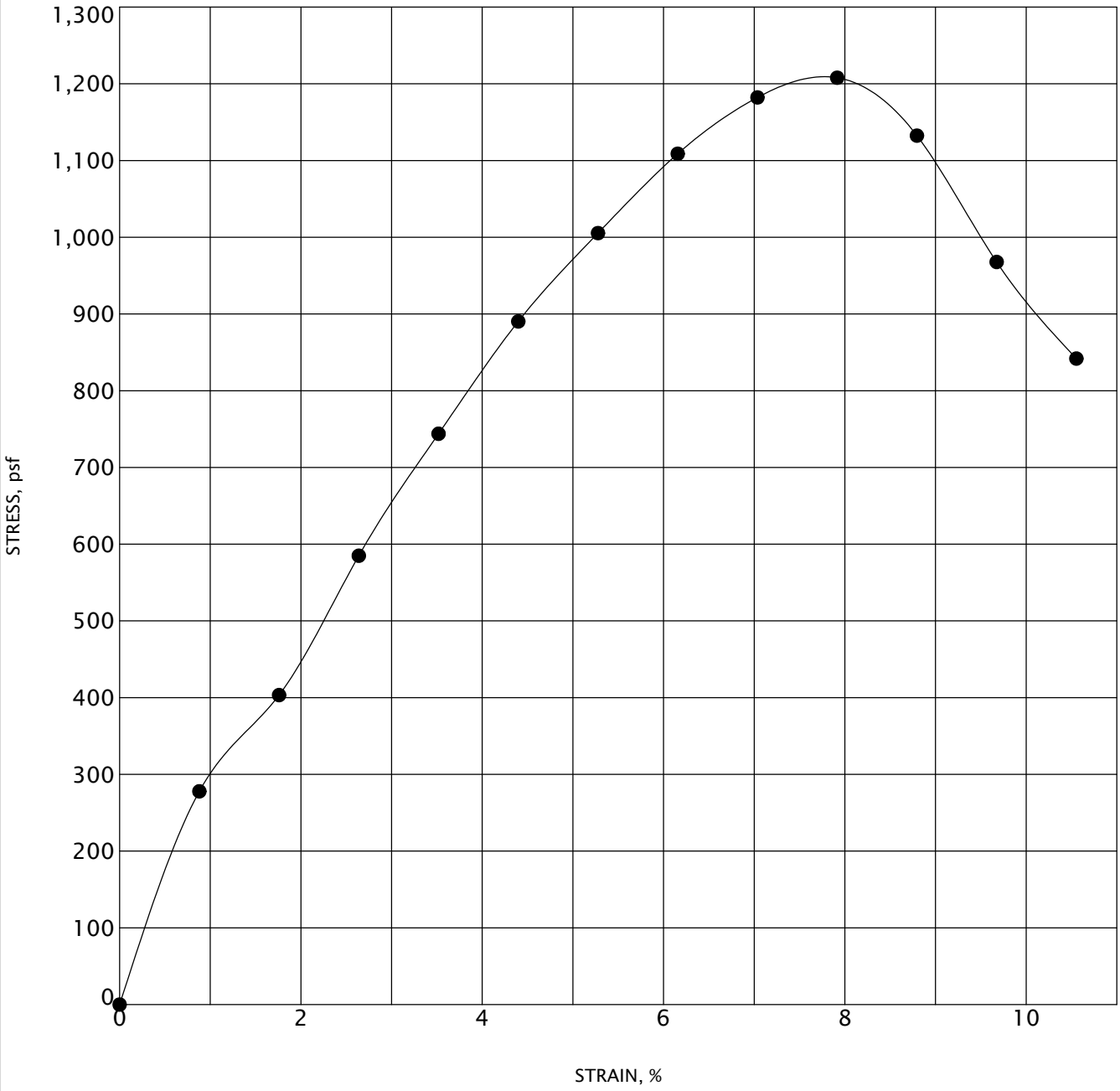
Water Level Observation:
23-1/2 feet during drilling operations; wet cave at 20 feet upon removal of drilling augers

Notes:
Borehole collapsed at 23-1/2 ft after auger removal
* Calibrated Hand Penetrometer

Excavation Backfilling Procedure:
Borehole backfilled with auger cuttings

Figure No. 20

SOIL / PAVEMENT BORING 253566.GPJ 20150116 G2 CONSULTING DATA TEMPLATE.GDT 10/13/25




| Specimen | | Classification | MC% | $\gamma_d$ | UC |
|---|---|---|---|---|---|
| ● B-20 | S-02 | Medium Brown Sandy Clay | 11 | 116 | 1210 |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |

CONSULTING GROUP

## UNCONFINED COMPRESSIVE STRENGTH TEST

Project Name: Dexter Schools 2025 Bond Program

Project Location: Dexter, Michigan

G2 Project No.: 253566

Figure No. 21

US_UNCONFINED 253566.GPJ 20140820 G2 CONSULTING DATA TEMPLATE.GDT 9/18/25

# GENERAL NOTES TERMINOLOGY

Unless otherwise noted, all terms herein refer to the Standard Definitions presented in ASTM D 653.

## PARTICLE SIZE

| | | |
|---|---|---|
| Boulders | | - greater than 12 inches |
| Cobbles | | - 3 inches to 12 inches |
| Gravel | - Coarse | - 3/4 inches to 3 inches |
| | - Fine | - No. 4 to 3/4 inches |
| Sand | - Coarse | - No. 10 to No. 4 |
| | - Medium | - No. 40 to No. 10 |
| | - Fine | - No. 200 to No. 40 |
| Silt | | - 0.005mm to 0.074mm |
| Clay | | - Less than 0.005mm |

## CLASSIFICATION

The major soil constituent is the principal noun, i.e. clay, silt, sand, gravel. The second major soil constituent and other minor constituents are reported as follows:

| Second Major Constituent (percent by weight) | Minor Constituent (percent by weight) |
|---|---|
| Trace - 1 to 12% | Trace - 1 to 12% |
| Adjective - 12 to 35% | Little - 12 to 23% |
| And - over 35% | Some - 23 to 33% |

## COHESIVE SOILS

If clay content is sufficient so that clay dominates soil properties, clay becomes the principal noun with the other major soil constituent as modifier, i.e. sandy clay. Other minor soil constituents may be included in accordance with the classification breakdown for cohesionless soils, i.e. silty clay, trace sand, little gravel.

| Consistency | Unconfined Compressive Strength (psf) | Approximate Range of (N) |
|---|---|---|
| Very Soft | Below 500 | 0 - 2 |
| Soft | 500 - 1,000 | 3 - 4 |
| Medium | 1,000 - 2,000 | 5 - 8 |
| Stiff | 2,000 - 4,000 | 9 - 15 |
| Very Stiff | 4,000 - 8,000 | 16 - 30 |
| Hard | 8,000 - 16,000 | 31 - 50 |
| Very Hard | Over 16,000 | Over 50 |

Consistency of cohesive soils is based upon an evaluation of the observed resistance to deformation under load and not upon the Standard Penetration Resistance (N).

## COHESIONLESS SOILS

| Density Classification | Relative Density % | Approximate Range of (N) |
|---|---|---|
| Very Loose | 0 - 15 | 0 - 4 |
| Loose | 16 - 35 | 5 - 10 |
| Medium Compact | 36 - 65 | 11 - 30 |
| Compact | 66 - 85 | 31 - 50 |
| Very Compact | 86 - 100 | Over 50 |

Relative Density of cohesionless soils is based upon the evaluation of the Standard Penetration Resistance (N), modified as required for depth effects, sampling effects, et

## SAMPLE DESIGNATIONS

AS - Auger Sample – Cuttings directly from auger flight
BS - Bottle or Bag Samples
S - Split Spoon Sample - ASTM D 1586
LS - Liner Sample with liner insert 3 inches in length
ST - Shelby Tube sample - 3 inch diameter unless otherwise noted
PS - Piston Sample - 3 inch diameter unless otherwise noted
RC - Rock Core - NX core unless otherwise noted

STANDARD PENETRATION TEST (ASTM D 1586) - A 2.0 inch outside-diameter, 1-3/8 inch inside-diameter split barrel sampler is driven into undisturbed soil by means of a 140-pound weight falling freely through a vertical distance of 30 inches. The sampler is normally driven three successive 6-inch increments. The total number of blows required for the final 12 inches of penetration is the Standard Penetration Resistance (N).