To: All Bidders

Subject: DSA Bulletin BU 24-05. This is a contract requirement of the project.

Per new DSA requirements, February 1, 2025, a completed Site Safety Plan (SSP) must be filed concurrently with the DSA 102-1C Construction Start Notice form. The plan must be created by the successfully awarded contractor and submitted to the Architect of Record within 7 days of District Notification of Board award of contract. See attached BU-24-05 for details on the required components of the SSP.

Enclosure: DSA BU 24-05



Pleasant Valley School District

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A DSA

BU 24-05

BULLETIN: FIRE SAFETY DURING CONSTRUCTION AND DEMOLITION

Division of the State Architect (DSA) documents referenced within this publication are available on the <u>DSA Forms</u> or <u>DSA Publications</u> webpages.

PURPOSE

A DSA bulletin is a notification to its stakeholders regarding any issue intended to be directed to a broad group of external stakeholders as well as DSA staff. This bulletin provides guidance on the development of, and requirement for, a site safety plan (SSP) for demolition projects and or the construction of new buildings, additions or alterations to existing buildings, reconstruction projects, repair, and nonconforming building rehabilitation projects submitted for approval under the 2022 CBC and later editions.

BACKGROUND

California Education Code (EDC) Sections 17280 and 81130 direct the Department of General Services (DGS) to supervise the design and construction any school building to ensure compliance with the standards published in Title 24, California Code of Regulations (CCR).

1. REQUIREMENT

1.1 Section 3302.3 of the California Building Code (CBC) directs compliance with Chapter 33 of the California Fire Code (CFC). Chapter 33 of the CFC outlines requirements for site safety during construction and demolition and directs that the property owner or their authorized agent is responsible for the development and implementation of a written site safety plan establishing a fire prevention program at the project site.

1.2 The school district, in collaboration with the project contractor is responsible for developing an SSP addressing the requirements of CFC Chapter 33. As first responders, DSA recommends that development of the SSP be in consultation and coordination with the local fire authority.

1.3 Beginning February 1, 2025, a completed site safety plan (SSP) is to be submitted to DSA in conjunction with the initial filing of form *DSA 102-IC: Construction Start Notice/Inspection Card Request*, inclusive of completing Section 3 of the form. Failure to include the SSP will result in rejection of the DSA form 102-IC.

1.3.1 Stockpile projects and similar factory-built construction projects do not require a site safety plan be submitted. For projects that include the construction and placement of these factory-built buildings on a site, the site safety plan needs not accompany the DSA 102 IC form when only seeking in-plant inspection cards.

1.4 The SSP is to be a single pdf (separate from the initial DSA 102-IC) with a file name formatted as Origin ID-App #_Site Safety Plan_submission date (i.e., YY-MM-DD).

1.5 DSA will receive and file the fire safety plan in the project file.

2. SITE SAFETY PLAN COMPONENTS

At a minimum, the SSP shall include the following information required in CFC Section 3303.1.1.

1. Name and contact information of site safety director.

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- 2. Documentation of training of the site safety director and fire watch personnel.
- 3. Procedures for reporting emergencies.
- 4. Fire department vehicle access routes.
- 5. Locations of fire protection equipment, including portable fire extinguishers, standpipes, fire department connections and fire hydrants.
- 6. Smoking and cooking policies, designated area to be used where approved, and signage locations in accordance with CFC Section 3305.8.
- 7. Location and safety considerations for temporary heating equipment.
- 8. Hot work (welding, roofing, etc.) plan.
- 9. Plans for control of combustible waste.
- 10. Locations and methods for storage and use of flammable and combustible liquids and other hazardous materials.
- 11. Provisions for site security.
- 12. Changes that affect this plan.
- 13. Other site- specific information requested by the local fire authority (LFA).

3. PROJECT PLAN SET COORDINATION

Because the SSP is submitted after project approval, project plans shall include the following statement in the Notes block, "All construction and demolition shall be in accordance with Chapter 33 of the CBC and CFC, and the written site safety plan."

REFERENCES:

California Code of Regulations (CCR) Title 24

- Part 1: California Administrative Code (CAC), Sections 4-331 and 4-406
- Part 2, California Building Code, Chapter 33

Part 9: California Fire Code (CFC), Chapter 33

A DSA Bulletin is a notification to its stakeholders regarding any issue intended to be directed to a broad group of external stakeholders as well as DSA staff.

PVSD - VALLE LINDO ELEMENTARY SHADE STRUCTURE

DEMOLITION NOTES

1. IDENTIFY ALL DAMAGED ELEMENTS DESIGNATED TO REMAIN OR BE RELOCATED. REQUEST CLARIFICATION FROM THE ARCHITECT BEFORE PROCEEDING WITH DEMOLITION WORK.

2. GENERAL CONTRACTOR IS RESPONSIBLE FOR REVIEWING AND VERIFYING DEMOLITION PLANS IN RELATION TO STRUCTURAL AND CONSTRUCTION DRAWINGS. CONTRACTOR SHALL VERIFY AND COORDINATE THE EXTENT OF DEMOLITION WORK WITH NEW WORK. THE ARCHITECT SHALL BE NOTIFIED OF ANY AND ALL CONFLICTS. DISCREPANCIES OR PROBLEMS

3. SEE MECHANICAL, PLUMBING, AND ELECTRICAL DEMOLITION PLANS FOR ADDITIONAL WORK.

4. THE CONTRACTOR SHALL VERIFY LOCATION OF ALL UTILITIES PRIOR TO DEMOLITION. THE CONTRACTOR SHALL BEWARE OF POTENTIAL HAZARDS FROM DEMOLITION WORK NEAR UTILITIES. PIPES AND CONDUIT ENCOUNTERED IN DEMOLISHED PARTITIONS AND AREAS WHICH ARE TO REMAIN IN USE SHALL BE RE-ROUTED AND CONCEALED. THOSE WHICH ARE TO BE ABANDONED SHALL BE CAPPED AND CONCEALED IN FLOOR, WALL OR CEILING.

5. BRACE AND SUPPORT EXISTING WORK PRIOR TO AND DURING DEMOLITION AND NEW WORK, AND UNTIL SAFE TO REMOVE SUCH BRACING AND SUPPORTS. THE CONTRACTOR IS RESPONSIBLE FOR ALL STRUCTURAL SHORING DESIGN AND CALCULATIONS

6. THE CONTRACTOR SHALL PERFORM ALL DEMOLITION WORK REQUIRED INCLUDING THE REMOVAL AND PROPER DISPOSAL OF ALL DEBRIS, BROKEN CONCRETE, ETC., FROM THE SITE. PROPER SHORING SHALL BE EXECUTED FOR THE SAFETY OF THE STRUCTURE AND WORKMEN.

7. THE OWNER SHALL HAVE FIRST RIGHTS OF REFUSAL FOR ALL DEMOLISHED MATERIALS.

8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DAMAGE RESULTING FROM DEMOLITION AT NO ADDITIONAL COST TO THE OWNER. THE GENERAL CONTRACTOR SHALL BE EXTREMELY CAREFUL TO PROTECT AND NOT TO DAMAGE ANY PORTION OF EXISTING INSTALLATION NOT BEING REMOVED. ANY EXISTING FACILITIES INDICATED TO REMAIN WHICH ARE SO DAMAGED SHALL BE REPLACED EQUAL TO ORIGINAL CONDITION AND TO THE SATISFACTION OF THE **OWNER**

CUT EXISTING PORTIONS OF WALLS, FLOORS, CEILINGS, ETC., WHERE INDICATED AND AS NECESSARY FOR NEW WORK. UNLESS SPECIFICALLY SHOWN ON THESE PLANS, NO STRUCTURAL MEMBER SHALL BE CUT, NEITHER DRILLED NOR NOTCHED, WITHOUT PRIOR WRITTEN AUTHORIZATION FROM THE STRUCTURAL ENGINEER, THE ARCHITECT AND THE AUTHORITY HAVING JURISDICTION

10. ALL TRADES CONCERNED SHALL COORDINATE EACH OTHER'S WORKS PRIOR TO AND DURING DEMOLITION.

11. ANY PROJECTING OR SURFACE-MOUNTED ITEMS BEING ABANDONED SHALL BE REMOVED, CAPPED AND CONCEALED BEHIND FINISHED SURFACES, UNLESS OTHERWISE NOTED. PATCH AND FINISH TO MATCH EXISTING ADJACENT SURFACE

12. SURFACES WHERE MATERIAL IS REMOVED TO INSTALL NEW WORK OR TO RECEIVE NEW FINISH SHALL BE REPAIRED AND PATCHED TO MATCH ORIGINAL CONDITIONS. RETEXTURE AND REPAINT WALL OR CEILING WHERE PATCHED TO MATCH EXISTING, WITH NO EVIDENCE THAT PATCH HAS OCCURRED.

13. ALL EXISTING AREAS TO REMAIN OR NEW CONSTRUCTION WORK THAT ARE DAMAGED SHALL BE PATCHED AS REQUIRED TO MATCH EXISTING ADJACENT AREA IN MATERIAL, FINISH AND COLOR, UNLESS OTHERWISE NOTED.

14. ALL EQUIPMENT AND MATERIAL WHICH ARE IN OPERATING CONDITION WHEN REMOVED SHALL BE MAINTAINED AS SUCH AND RETURNED TO THE OWNER OR TO BE REINSTALLED WHERE INDICATED. PROPERLY RECONNECT EQUIPMENT TO RESUME OPERATION.

15. DEMOLISH AND REMOVE WALLS, CEILINGS AND ALL OTHER ITEMS AND EQUIPMENT NOT REQUIRED TO REMAIN OR TO BE REUSED, SUCH AS, BUT NOT LIMITED TO, DOORS, BUCKS, MOLDINGS AND WALL COVERINGS, INCLUDING ITEMS WHICH MAY BE REASONABLY INFERRED AS NECESSARY TO PROPERLY PREPARE FOR THE EXECUTION AND INSTALLATION OF THE NEW WORK. REMOVE EXCESS DOORS, BUCKS, HARDWARE, LIGHTING FIXTURES, ELECTRICAL FITTINGS, CARPETS AND OTHER SALVAGEABLE MATERIAL TO BE STORED, RECYCLED, OR DISPOSED OF AS DIRECTED BY THE OWNER.

16. IN ALL AREAS WHERE DEMOLITION CAUSES UNEVENNESS OR VOIDS IN FLOOR, THE GENERAL CONTRACTOR SHALL PATCH TO LEVEL FLOOR WITH EXISTING SLAB AND/OR REQUIRED SURFACE TO RECEIVE NEW FLOOR FINISH. PATCH AND REPAIR SUBFLOOR AS REQUIRED TO RECEIVE NEW FINISH FLOORING IN A MANNER CONSISTENT WITH HIGH QUALITY WORKMANSHIP.

17. ALL TRENCHING OPERATIONS SHALL BE PERFORMED WITH A SPOTTER SPOTTER SHALL CONTINUOUSLY OBSERVE EXCAVATIONS TO LOCATE IRRIGATION LINES, CONDUITS AND ANY OTHER UTILITIES OR SUBTERRANEAN EQUIPMENT. ANY ITEMS ENCOUNTERED SHALL BE EXCAVATED BY HAND. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO REPAIR ANY IRRIGATION, UTILITIES, OR OTHER ITEMS DAMAGED AS A RESULT OF EXCAVATION.

18. WORK SHALL COMPLY WITH THE PROVISIONS OF CHAPTER 33 OF THE CBC AND CFC 2019 EDITION "FIRE SAFETY DURING CONSTRUCTION AND DEMOLITION"

GENERAL NOTES

THE PROJECT MANUAL ISSUED AS PART OF THESE CONSTRUCTION DOCUMENTS IS AN INTEGRAL PART OF THE CONTRACT DOCUMENTS.

2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THAT ALL WORKMANSHIP, MATERIALS AND CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE APPLICABLE CODES AND FEDERAL REQUIREMENTS AND REGULATIONS.

3. THE CONTRACTOR SHALL VISIT THE SITE AND VERIFY ALL DIMENSIONS PRIOR TO SUBMITTING A BID. THE CONTRACTOR IS ALSO RESPONSIBLE FOR OBTAINING ALL REQUIRED PERMITS

4. THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS, GRADES, ELEVATIONS AND DIMENSIONS BEFORE STARTING WORK. THE ARCHITECT SHALL BE NOTIFIED IMMEDIATELY OF ANY DISCREPANCY OR INCONSISTENCY

TOR SHALL DETERMINE THE LOCATION OF ALL EXISTING UTILITY SERVICES IN THE AREA TO BE EXCAVATED PRIOR TO THE BEGINNING OF EXCAVATION. THE CONTRACTOR SHALL PROTECT ALL UTILITY LINES AND SERVICE LINES TO REMAIN WHICH ARE ENCOUNTERED DURING CONSTRUCTION 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ENFORCEMENT OF FEDERAL AND STATE OF CALIFORNIA OCCUPATIONAL SAFETY AND HEALTH

ADMINISTRATION REQUIREMENTS AND REGULATIONS.

DO NOT SCALE ANY DRAWINGS IN THIS SET

8. ALL DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS AND DETAILS. SPECIFIC NOTES AND DETAILS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.

. MATERIAL NOTES AND DRAWINGS SHALL TAKE PRECEDENCE OVER THE SPECIFICATIONS. ISSUE RFI WHEN WHEN THERE IS A DISCREPANCY BETWEEN THE DRAWINGS AND THE SPECIFICATIONS.

10. ALL OMISSIONS AND CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT IMMEDIATELY BEFORE PROCEEDING WITH ANY WORK SO INVOLVED. NO CHANGES ARE TO BE MADE UNLESS THE ARCHITECT AND THE OWNER ARE NOTIFIED IN WRITING AND APPROVE SUCH A CHANGE ACCORDING TO THE CONTRACT.

11. THE ARCHITECT IS NOT RESPONSIBLE FOR ASBESTOS ABATEMENT

12. PROVIDE BACKING AT ALL INDICATED LOCATIONS OF FIXTURES, SIGNS, HANDRAILS, ETC.

13. TRADE NAMES AND MANUFACTURERS REFERRED TO ARE FOR QUALITY STANDARDS ONLY. SUBSTITUTIONS WILL BE PERMITTED AS APPROVED BY THE OWNER AND THE ARCHITECT ONLY UPON SUBMITTAL, AND FOR A LIMITED PERIOD.

14. ITEMS SHOWN AS N.I.C. ON PLANS MAY REQUIRE SEPARATE SUBMITTALS. APPROVALS AND PERMITS. INSTALLING CONTRACTOR IS RESPONSIBLE FOR OBTAINING PERMITS FOR SUCH ITEMS.

15. CONTRACTOR SHALL VERIFY MINIMUM 2% SITE DRAINAGE TO DRAINAGE INLETS. EXCEPTION: CROSS SLOPE OF ACCESSIBLE PATH OF TRAVEL SHALL BE 2% MAXIMUM.

16. CONTRACTOR SHALL ENSURE ALL FINISH MATERIALS WILL BE FLUSH WHERE NEW FINISH SURFACE MATERIALS JOIN EXISTING TO PROVIDE SMOOTH TRANSITION.

17. CONTRACTOR ACKNOWLEDGES THAT DRAWINGS ARE DIAGRAMMATIC IN NATURE AND ARE TO REPRESENT THE INTENT OF THE WORK TO BE ACCOMPLISHED. CONTRACTOR IS TO INSTALL MATERIALS AND SYSTEMS WITH EXPERIENCED SKILLED CRAFTSMEN WHO WILL BE RESPONSIBLE FOR THE INTEGRITY OF THEIR RESPECTIVE WORK. NOT EVERY DETAIL OF EACH CONDITION IS DRAWN. CONTRACTOR AND SUB-CONTRACTOR ARE RESPONSIBLE FOR COMPLETE WORKMANLIKE INSTALLATION OF ALL MATERIALS AND SYSTEMS AND WILL NEED TO PROVIDE ADDITIONAL DETAILS FOR INSTALLATION BASED ON GENERAL INFORMATION SHOWN. FOR INSTANCE, NOT EVERY WATERPROOFING OR FLASHING DETAIL FOR EVERY CONDITION IS SHOWN. CONTRACTOR IS TO PROVIDE DETAILS AND INSTALLATION FOR A COMPLETE WATERTIGHT INSTALLATION.

18. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS IS THAT THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CALIFORNIA CODE OF REGULATIONS. SHOULD ANY EXISTING CONDITION NOT COVERED BY THE CONTRACT DOCUMENTS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHERE IN THE FINISHED WORK WILL NOT COMPLY WITH THE T-24, CALIFORNIA CODE OF REGULATIONS, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILS AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. (SECTION 4-317(c) PART 1, TITLE 24, CCR)

19. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDUM OR A CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA.

20. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES.

21. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.

22. TITLE 24, PARTS 1-5 AND 9 MUST BE KEPT ON SITE DURING CONSTRUCTION. 23. THE SCOPE OF WORK/PROJECT DESCRIPTION: CLEARLY INDICATED THE SCOPE OF WORK ON THE COVER SHEET OR GENERAL NOTE SHEET OF THE DRAWINGS

24. WORK SHALL COMPLY WITH THE PROVISIONS OF CHAPTER 33 OF THE CBC AND CFC 2019 EDITION "FIRE SAFETY DURING CONSTRUCTION AND DEMOLITION".

PLEASANT VALLEY SCHOOL DISTRICT

SITE STRUCTURAL DESIGN DATA

SITE WIND DEISGN DATA (CBC 1603A.1.4) V = <u>93</u> MPH (CBC 1609A.3 FIGURE (1)) V_{ASD} = <u>78</u> MPH (CBC 1609A.3.1) RISK CATEGORY = II WIND EXPOSURE = C EARTHQUAKE DESIGN DATA (CBC 1603A.1.5) RISK CATEGORY = II SEISMIC IMPORANCE FACTOR $I_e = 1.00$ S_S = <u>1.737g</u> S₁ = <u>0.643g</u> SOIL SITE CLASS = <u>D (DEFAULT)</u>

S_{DS} = <u>1.16</u>

S_{D1} = 0.73 (GEOTECHNICAL REPORT)

 $PGA_M = 0.84$ (GEOTECHNICAL REPORT) SEISMIC DESIGN CATEGORY = D

GEOTECHNICAL INFORMATION (CBC 1603A.1.6)

GEOTECHNICAL REPORT REQUIRED AND PROVIDED PER DSA IR A-4 SOIL LOAD-BEARING VALUE = <u>3,000 PSF</u> (GEOTECHNICAL REPORT - IF NO GEOTECHNICAL REPORT THEN CBC TABLE 1806A.2)

APPLICABLE CODES

CONSTRUCTION SHALL COMPLY WITH THE FOLLOWING PARTS OF THE TITLE 24 OF THE CALIFORNIA CODE OF REGULATIONS (CCR), REFER TO SHADE STRUCTURE DRAWINGS FOR OTHER REQUIREMENTS

2022 CALIFORNIA BUILDING STANDARDS CODE C.C.R. TITLE 24: 2022 CALIFORNIA ADMINISTRATIVE CODE, C.C.R. TITLE 24 PART 1 2022 CALIFORNIA BUILDING CODE (CBC), C.C.R. TITLE 24 PART 2 2022 CALIFORNIA FIRE CODE, C.C.R. TITLE 24 PART 9 2022 CALIFORNIA EXISTING BUILDING CODE, C.C.R. TITLE 24 PART 10 2022 CALIFORNIA REFERENCED STANDARDS CODE, C.C.R. TITLE 24 PART 12

TITLE 19 C.C.R., PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS

APPLICABLE STANDARDS AS REFERENCED BY 2022 CBC CHAPTER 35: NATIONAL FIRE ALARM CODE (CALIF. AMENDED), NFPA 72 2022 EDITION (NOTE SEE UL STANDARD 1971 FOR "VISUAL DEVICES")

ARCHITECT'S STATEMENT OF GENERAL CONFORMANCE

- ☐ THE DRAWINGS OR SHEETS LISTED ON THE COVER SHEET OR INDEX SHEET
- THIS DRAWING, PAGE OR SPECIFICATION/CALCULATIONS

HAVE BEEN PREPARED BY OTHER DESIGN PROFESSIONALS OR CONSULTANTS WHO ARE LICENSED AND/OR AUTHORIZED TO PREPARE SUCH DRAWINGS IN THE STATE. IT HAS BEEN EXAMINED BY ME FOR:

1. DESIGN INTENT AND APPEARS TO MEET THE APPROPRIATE **REQUIREMENTS OF TITLE 24, CALIFORNIA CODE OR REGULATIONS** AND THE PROJECT SPECIFICATIONS PREPARED BY ME, AND 2. COORDINATION WITH MY PLANS AND SPECIFICATIONS AND IS ACCEPTABLE FOR INCORPORATION INTO THE CONSTRUCTION OF THE PROJECT.

I CERTIFY THAT:

ALL DRAWINGS OR SHEETS LISTED ON THE COVER OR INDEX SHEET THIS DRAWING OR PAGE

- ☐ IS/ARE IN GENERAL CONFORMANCE WITH THE PROJECT DESIGN INTENT.
- HAVE BEEN COORDINATED WITH THE PROJECT PLANS AND SPECIFICATIONS.

SIGNATURE

ARCHITECT IN RESPONSIBLE CHARGE: LICENSE NUMBER: EXPIRATION DATE:

DATE TYSON CLINE C-34436 1-31-2025

10/6/2023

PROJECT DESCRIPTION

THIS PROJECT CONSISTS OF INSTALLATION OF ONE (1) PRE-MANUFACTURED SHADE STRUCTURES AND ASSOCIATED SITE WORK.

SHADE STRUCTURE TYPE: SHADE STRUCTURE PC#: SHADE STRUCTURE DESCRIPTION .: SHADE STRUCTURE MODEL #:

30' WIDE RECTANGULAR GABLE 04-122188 30'X64'X10' CLR. HT RG30

SHADE AREA: OCCUPANCY TYPE: TYPE OF CONSTRUCTION

<u>1,920 SF</u> <u>A-3</u> <u>II-B</u>

OCCUPANT LOAD:

129 OCC. (15 SF/OCC.)

FLOOD HAZARD MAP

FLOOD ZONE DESIGNATION: ZONE X (0.2% CHANCE OF FLOODING) FIRM PANEL DESIGNATION: 06111C0931F EFFECTIVE DATE: 01/07/2015 BASE FLOOD ELEVATION: 0.0' APPLICABLE COMMUNITY ORDINANCE SECTION: VENTURA COUNTY, CA

GEOHAZARD REPORT

PROVIDED DUE TO PROJECT LOCATION IN LIQUEFACTION HAZARD ZONE. PROJECT NO. 1025.006. DECEMBER 2022 BY GEOTECHNIQUES

INSPECTOR OF RECORD

A DSA CERTIFIED PROJECT INSPECTOR EMPLOYED BY THE DISTRICT AND APPROVED BY THE DIVISION OF THE STATE ARCHITECT SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR. THE PROJECT INSPECTOR SHALL BE CERTIFIED BY DSA TO INSPECT CLASS 2.

WORK EXEMPT (DSA 103)

CONCRETE

- CONCRETE BATCH PLANT INSPECTION (REFER TO AS-102 KEYNOTES AND LEGEND)

GENERAL DSA NOTES

- CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY THE ADDENDA OR A CONSTRUCTION CHANGE DOCUMENT APPROVED BY THE DIVISION OF THE STATE ARCHITECT, AS REQUIRED BY SECTION 4-338, PART 1 TITTLE 24, CCR.
- A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE SCHOOL BOARD SHALL CONDUCT ALL REQUIRED TESTS & INSPECTIONS FOR THE PROJECT.
- ALL WORK SHALL CONFORM TO 2022 TITLE 24, CALIFORNIA CODE OF **REGULATIONS (CCR).**
- WHENEVER DSA FINDS ANY CONSTRUCTION WORK IS BEING PERFORMED IN A MANNER CONTRARY TO THE PROVISIONS OF CALIFORNIA BUILDING CODE AND THAT WOULD COMPROMISE THE STRUCTURAL INTEGRITY OF THE BUILDING, THE DEPARTMENT OF GENERAL SERVICES, STATE OF CALIFORNIA, IS AUTHORIZED TO ISSUE A STOP WORK ORDER PER SECTION 4-334.1 CALIFORNIA ADMINISTRATIVE CODE (PART 1, TITLE 24, CCR).
- GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES.
- TITLE 24, PARTS 1-5 AND 9 MUST BE KEPT ON SITE DURING 6. CONSTRUCTION.
- ALL STRUCTURAL, ARCHITECTURAL, AND MATERIALS INSTALLATION TO COMPLY WITH APPLICABLE CODES, STANDARDS, AND MANUFACTURER'S RECOMMENDATIONS.
- 8. IF ANY CONDITION IS DISCOVERED WHICH, IF LEFT UNCORRECTED, WOULD MAKE THE BUILDING NON-COMPLIANT WITH THE REQUIREMENTS OF THE EDITION OF THE CBC IN FORCE AT THE TIME OF ORIGINAL CONSTRUCTION, THE CONDITION MUST BE CORRECTED IN ACCORDANCE WITH CURRENT CODE REQUIREMENTS. A CONSTRUCTION CHANGE DOCUMENT, OR A SEPARATE SET OF PLANS AND SPECIFICATIONS DETAILING AND SPECIFYING THE REQUIRED REPAIR WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE REPAIR WORK.

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I.C.	NOT IN CONTRACT
). OR # ד פ	
1.0.	OVER
C.	ON CENTER
F.C.I.	OWNER FURNISHED CONTRACTO
EOI	
F.U.I.	OWNER FORNISHED
H.	OPPOSITE HAND
PN'G	OPENING
^э Р.	OPPOSITE
S RF	
	PLATE OR PLASTIC
AST.	PLASTER
YWD.	PLYWOOD
₹. 201	
(0). T.D.F.	PRESSURE TREATED DOUGLAS F
OR RAD	RADIUS
D.	ROOF DRAIN
EC.S	RECOMMENDATIONS
=F. =FI	
EFRIG.	REFRIGERATOR
EINF.	REINFORCED OR REINFORCEMEN
EQ.	REQUIRED OR REQUIREMENTS
= I . =	
M.	ROOM
DOF'G	ROOFING
C.	SOLID CORE
CHED.	SCHEDULE
IT G IT	SHEET
М.	SIMILAR
l	SAWCUT JOINT
M.S.	SHEET METAL SCREWS
לי לי	
о. Г.	STANDARD
TL.	STEEL
ĪN.	STAIN
OR.	STORAGE
JSP	SUSPENDED
A.	TOILET ACCESSORY
kВ	TOP & BOTTOM
EL.	TELEPHONE
IMP. G	
	TOOLED JOINT
0.	TOP OF
0.B.	TOP OF BEAM
0.M. 0 P	
0.r. 0.S.	TOP OF STEEL FRAMING
O.SHTG	TOP OF SHEATHING
PA	TRI-POLYMER ALLOY
5	
vv. ⁄P	TYPICAI
N.O.	UNLESS NOTED OTHERWISE
O.N.	UNLESS OTHERWISE NOTED
C.T.	VINYL COMPOSITION TILE
ERI. G D F	VERTICAL GRAIN
J.J.I .	DOUGLAS FIR
I.F.	VERIFY IN FIELD
	WITH
.U. D	WATERGLUSET
ت. .J.	WEAKENED-PLANE JOINT
.0.	WHERE OCCURS
.P.	WATERPROOF
.P.B.	
.r.ıvı. .R	WATER RESISTANT
 	
Ι.	WEIGHT
1. .W.F.	WEIGHT WELDED WIRE FABRIC



IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT APP: 03-123817 INC: REVIEWED FOR SS ☑ FLS ☑ ACS ☑ DATE: 03/07/2025
Roesling Nakamura Terada Architects 285 N Ventura Ave #102 Ventura, CA 93001 P805.626.5330 F805.626.5350 www.RNTarchitects.com
TYSON CLINE NOC-34436 THEREN. 1-31-25
PLEASANT VALLEY SCHOOL DISTRICTVALLE LINDO ELEMENTARY SHADE STRUCTURE777 AILEEN ST. CAMARILLO, CA 93010100% CDs 03-123817
No. Description Date Image: Section in the section is a section in the section is a section in the section is a sectio
RNT Project No. 19849.18 Date 10/27/2023 Drawn by CD Checked by CY Sheet Number T-1.1



		IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT
VENTURA COUNTY Fire Prevention Bureau	ACCESSIBLE P.O.T. NOTES	APP: 03-123817 INC: REVIEWED FOR
FIRE DEPARTMENT 165 Durley Avenue, Camarillo, CA 93010-8586 Office: (805) 389-9738 Fax: (805) 388-4356	ACCESSIBLE PATH OF TRAVEL AS INDICATED ON PLAN IS A BARRIER-FREE	SS 🗹 FLS 🗹 ACS 🗹 DATE: 03/07/2025
FIRE PREVENTION FOM 625	ROUTE WITHOUT ANY ABRUPT LEVEL CHANGES EXCEEDING 1/2" IF BEVELED AT 1:12 OR VERTICAL LEVEL CHANGES NOT EXCEEDING 1/4" MAX, AND AT LEAST 48"	
SECTION I – PROJECT INFORMATION (To Be Completed by Applicant)	IN WIDTH. SURFACE IS STABLE, FIRM, AND SLIP RESISTANT. CROSS SLOPE DOES NOT EXCEED 2% AND SLOPE IN THE DIRECTION OF TRAVEL IS LESS THAN 5%,	
Project Name: CAPE Charter School APN: Project Address: 777 Aileen City: Camarillo	UNLESS OTHERWISE INDICATED. ACCESSIBLE PATH OF TRAVEL SHALL BE MAINTAINED FREE OF OVERHANGING OBSTRUCTIONS TO 80" MINIMUM, AND	ECTS
SECTION II – INFORMATION ON FIRE-FLOW AVAILABILITY (To Be Completed by Water Purveyor)	PROTRUDING OBJECTS GREATER THAN 4" PROJECTION FROM WALL AND ABOVE 27" AND LESS THAN 80". ARCHITECT SHALL VERIFY THAT THERE ARE NO	P CH 17
System Information: Water Purveyor: City of Camarillo	BARRIERS IN THE PATH OF TRAVEL.	
Size & Location of Main: 8 Inch Skeel Street Distance to Parcel: 21 Feet Size of Reservoir Serving Test Hydrants: 3 Million Gallons	ARCHITECTS STATEMENT	
Location of Residual Hydrant: 943 Skeel Distance to Parcel: * 6 Feet Location of Flow Hydrant: 903 Skeel Drive Distance to Parcel: * 10 Feet	A. THE P.O.T. IDENTIFIED IN THESE CONSTRUCTION DOCUMENTS MEETS THE	A A A A A A A A A A A A A A A A A A A
Type: Wet Size: 6 Inch # of Outlets: 1 4* 2 2 ½* * Distance to parcel shall be measured along the vehicular access 4* 2 2 ½*	(CBC) ACCESSIBILITY PROVISIONS FOR PATH OF TRAVEL REQUIREMENTS FOR	N A N A N
Test Result Information: Method Used to Obtain Results: Hydraulic Model □ Flow Test Image: Colspan="2">Colspan="2">Flow Test Date of Test: 6-12-2023 Time of Test: 2:00 □ AM Image: PM	OF THIS PROJECT, THE P.O.T. WAS EXAMINED AND ANY ELEMENTS, COMPONENTS	Roesling Nakamura Terada Architects
Static PSI: 108 Residual PSI: 97 Orifice: 2.5 Pitot: 88 Observed GPM: 1583 Calculated GPM @ 20 psi: 4866 Capacity Duration: hrs	WITH THE CBC HAVE BEEN IDENTIFIED AND THE CORRECTIVE WORK NECESSARY	285 N Ventura Ave #102
I have witnessed and/or reviewed this water flow information and by personal knowledge and/or on-site observation certify that the above information is correct.	TO BRING THEM INTO COMPLIANCE HAS BEEN INCLUDED WITHIN THE SCOPE OF THIS PROJECT'S WORK THROUGH DETAILS, DRAWINGS AND SPECIFICATIONS	P805.626.5330 F805.626.5350
Name: Andy Flores Signature:	ELEMENTS, COMPONENTS OR PORTIONS OF THE P.O.T. THAT WILL NOT BE	www.RNTarchitects.com
Phone: (805) 383-5643	OR A FINDING OF UNREASONABLE HARDSHIP ARE INDICATED IN THESE	ED ARCU
Private on-site water system proposed. Separate plan submittal required. Water purveyor approves use of private water system. (Purveyor signature required above). Eire District Record Number:	B. DURING CONSTRUCTION, IF P.O.T. ITEMS WITHIN THE SCOPE OF THE PROJECT	U TYSON CLINE M
	REPRESENTED AS CBC COMPLIANT ARE FOUND TO BE NONCONFORMING BEYOND REASONABLE CONSTRUCTION TOLERANCES, THE ITEMS SHALL BE	TNO 2-34436
Sandary 1, 2023 Fire-flow Verification 625-1	BROUGHT INTO COMPLIANCE WITH THE CBC AS A PART OF THIS PROJECT BY MEANS OF A CONSTRUCTION CHANGE DOCUMENT.	PAREN. 1-3T-25
	FIRE DEPARTMENT NOTES	
FIRE & LIFE SAFETY SITE CONDITIONS SUBMITTAL	A. PROVIDE A MINIMUM, UNOBSTRUCTED WIDTH OF 20 FEET CLEAR TO SKY,	
Division of the State Architect (DSA) documents referenced within this publication are available on the DSA Forms or DSA Publications webpages.	VEHICULAR ACCESS TO WITHIN 150 FEET OF ALL PORTIONS OF THE EXTERIOR WALLS. FIRE CODE 902.2.1	
To facilitate the Division of the State Architect's (DSA) fire and life safety plan review of project site conditions, DSA requires the design professional to provide the following information at time of project submittal for projects consisting of construction of a new campus, construction of new building(s), additions to existing buildings, and	B. EXISTING FIRE DEPARTMENT ACCESS TO REMAIN.C. VEHICULAR ACCESS MUST BE PROVIDED AND MAINTAINED SERVICEABLE	
for site alternate design means for fire department emergency vehicle access, and fire suppression water supply. Information associated with compliance items 1 through 3 below is to be provided for all project types indicated above. Information associated with items 4 through 7 is to be completed when an alternate means is utilized.	THROUGHOUT CONSTRUCTION. D. THE FIRE FLOW FOR ON-SITE HYDRANT MEETS THE REQUIREMENTS FOR	
an alternate design means is being requested. The Project Information and Fire & Life Safety Information sections are to be completed for all projects and	THIS FIRE AUTHORITY. E. INSPECTION FOR COMPLIANCE WITH THESE CONDITIONS AND GENERAL	
imaged onto the fire access site plan. When an alternate design/means is proposed, all sections on pages 1 and 2 are to be completed and imaged on the fire access site plan.	SAFETY WILL BE REQUIRED PRIOR TO OCCUPANCY.	PLEASANT
Project INFORMATION	LEGEND	VALLEY
School District/Owner: Pleasant Valley School District		SCHOOL
Project Address: 777 AILEEN ST. CAMARILLO, CA 93010	(N) ACCESSIBLE PATH OF TRAVEL	DISTRICT
FIRE & LIFE SAFETY INFORMATION 1. Has a fire hydrant flow test been performed within the past 12 months? Yes ☑ No □	EXISTING ACCESSIBLE PATH OF TRAVEL, A# 03-120280, A# 03-104545	ΛΔΙΙΕ
(If yes, provide a copy of the test data.) 2. Was the fire hydrant water flow test performed as part of this LFA review?	$= 150^{\circ} \text{ HOSE DRAG LENGTH.}$	LINDO
3. Is the project located within a designated fire hazard severity zone (FHSZ) as established by Cal-Fire? (If yes, indicate FHSZ classification below.) Yes □ No ☑	20'-0" WIDE MIN 100' MAX. LENGTH FOR TRUCK BACK-UP. 40' MIN. TURNING RADIUS + MIN. WIDTH OF 16'	ELEMENTARY
Refer to the following website for FHSZ locations: Moderate High Very High http://egis.fire.ca.gov/FHSZ/ Very High Very High		SHADE
Wildland Interface Area (WIFA) (If any designations are checked, project design must meet the requirements of CBC Chapter 7A.) WIFA □	EXISTING BUILDINGS	
	(E) EXIT DISCHARGE PATH LIGHTING	777 AILEEN ST. CAMARILLO, CA 93010
DGS DSA 810 (revised 12/29/20) Page 1 of 4 DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA	(E) FIRE ALARM HORN. V.I.F.	
	$(\overline{B}) \qquad (E) FIRE HYDRAN I (\overline{B}) \qquad (E) ACCESSIBLE BOYS' TOILET ROOM, A#03-102898$	100% CDs
FIRE & LIFE SAFETY SITE CONDITIONS SUBMITTAL	(\overline{G}) (E) ACCESSIBLE GIRLS' TOILET ROOM, A#03-102898	03-123817
CONDITION MEANS AND METHODS RESOLUTION ALTERNATE ACCEPTED Yes No N/A N/R	 (E) ACCESSIBLE UNISEX STAFF TOILET ROOM (E) ACCESSIBLE HI-LOW PEDESTAL DRINKING FOUNTAIN, A#03-120280 	
4. Entroyonal resources reading to be interviewed or a requirementation 4. Acceptable Alternate: Emergency vehicle and personnel access as proposed by the project architect is acceptable for providing fire suppression and	 (E) DRINKING FOUNTAIN FE FIRE EXTINGUISHER, BRACKET MOUNTED 	
5. Fire Hydrants: Number and spacing does not meet CFC requirements. ✓	LIST OF BUILDINGS	
5a. Acceptable Alternate: Number of fire hydrants and spacing as proposed by the project architect is acceptable for fire suppression and protection of life and property.	ID BUILDING USE A#' ORG. A# MOD. CERTIFICATION	
6. Fire Hydrants: Water flow and pressure are less than CFC minimum.	BLDG. 100 CLASSROOMS 30725 03-102898 # 2	
Acceptable Atternate. The available now and pressure is acceptable for providing fire suppression and protection of life and property. Acceptable Atternate. The available now and pressure is acceptable for	BLDG. 200 CLASSROOMS 30725 03-102898 # 2 BLDG. 300 KINDER 30725 03-102898 # 2	Sheet Name
standpipe systems does not meet CFC requirements. 7a. Acceptable Alternate: The location of fire department connection serving the fire sprinkler system and/or standpipe system is acceptable for providing fire	BLDG. 400 CLASSROOMS 30725 03-104545 # 2 BLDG. 500 MPR 30725 03-102898 # 2	FIRE ACCESS
suppression and protection of life and property. School District Acceptance of Acceptable Design Alternates	BLDG. 550 KITCHEN 03-105051 - # 1 BLDG. 600 CLASSROOMS 32464 03-102898 # 2	SITE PLAN
By signing this form, the school district acknowledges and accepts the proposed design as an alternative to California Building Code (CBC) and California Fire Code (CFC) minimum requirements, as indicated by one or more of the conditions indicated at items 4a, 5a, 6a or 7a, for providing fire and life safety protection of life and property.	BLDG. 700 CLASSROOMS - # - BLDG. 800 ADMIN 03-104545 - # 3	
Accepted by:	BLDG. 900 CLASSROOMS 03-104136 - # 1 BLDG. KINDERGARTEN 03-120280 - # -*	
LOCAL FIRE AUTHORITY (LFA) INFORMATION	* PENDING EXECUTION OF CCD-01 UNDER THIS CONTRACT	RNT Project No. 19849.18
LFA Agency Name: Ventura County Fire Department LFA Review Official: Brandon Sube	ACCESSIBLE PARKING LOT CALC.	Date 10/27/2023
Title: Fire Inspector II Work Phone: (805) 914-7436 Work Email: Brandon.Sube@ventura.org	LOT 1 TYPE EXISTING REQUIRED PROVIDED	Checked by CY
LFA Reviewer's Signature: Brandon Sube Digitally signed by Brandon Sube Date: 2023.06.26 10:05:36 -07'00' Date: 06/26/23	PARKING42 SPACESN/A42 SPACESACCESSIBLE PARKING2 SPACES2 SPACES2 SPACES	Sheet Number
	ACCESSIBLE VAN PARKING 1 SPACE 1 SPACE 1 SPACE	AS-101
Disk 810 (revised 12/29/20) Page 2 of 4 DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA	TOTAL # OF PARKING SPACES: 44 SPACES	



DESIGN CRITERIA RASELOCATION LOCATED AT POTTOM OF RASEDLATE/TOD OF FOOTING			
DESCRIPTION		DESIGN VALUES	
DEAD AND LIVE LOADS		DEGIGINITIALOLO	
ROOF LIVE LOAD		20 PSF	
ROOF DEAD LOAD (SUPERIMPOSED ON FRAME)		5 PSF MAX	
ROOF PANEL DEAD LOAD	M=1	.1 PSF, G = 1.2 PSF, S = 1.3	PSF
COLLATERAL DEAD LOAD	M = 3	3.9 PSF, G = 3.8 PSF, S = 3.	/ PSF
ROOF LIVE LOAD, L.		20 PSF	
		20101	
ROOF SNOW LOAD			
GROUND SNOW LOAD, Pg		20 PSF	
RISK CATEGORY		I	
ROOF SNOW LOAD: SLOPED, P _s		20 PSF	
FOR SNOW LOAD CONDITIONS ONLY - SITE APPLICATION REVIEWER SHALL VERIFY THE STTRUCTUR	E SHALL BE LOCATED A	AT LEAST 20 FEET	
FROM ANY ADJACENT STRUCTURE FOR SNOW DRIFT.		4.0	
SNOW LOAD SLOPETACTOR, C _s		1.0	
		1.0	
SNOW LOAD IMPORTANCE FACTOR, Is		1.0	
THERIVAL FACTOR, Ct		1.2	
LOWEST ANTICIPATED SERVICE TEMPERATURE		30°	
WIND DESIGN BASIC WIND SPEED (3 SECOND GUST) V V			
EXPOSURE CATEGORY		C	
FACTORS: K _z , K _z , K _d		0.85, 1.0, 0.85	
$q_{\rm h} = 0.00256 \ {\rm K_z} \ {\rm K_z} \ {\rm K_d} \ {\rm V}^2$		18.50 PSF	
C _{NW} PER ASCE FIGURE 27.3-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED	CASEA	(1.1 / -1.2) CASEB (0.0	1 / -0.69)
C _{NL} PER ASCE FIGURE 27.3-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED	CASEA (·	0.17 / -1.09) CASEB (-C).96 / -1.65)
C _N PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (< h)	CASE	, (-0.8 / -1.2) CASEB (0).8 / 0.5)
C _N PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (> h, < 2h)	CASE	(-0.6 / -0.9) CASEB (0).5 / 0.5)
C _N PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (>2h)	CASE	(-0.3 /-0.6) CASEB (0).3 / 0.3)
COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED	 Z0!	NE 3 - $(2.29 / -2.11) / (1.0 / -1.0)$	-3.0)
	ZO	NE 2 - (1.77 / -1.63) / (0.8 / -	-2.3)
	ZOI	NE 1 - (1.15 / -1.05) / (0.5 / -	-1.5)
SEISMIC DESIGN			
LATERAL FORCE RESISTING SYSTEM	STEEL -		
SESIMIC MORTANCE FACTOR. I.			
SEISMIC SITE CLASS			
MCE _R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S _s		2.60	
MCE_R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S ₁		0.90	
SHORT PERIOD SITE COEFFICIENT, Fa		1.20	
LONG PERIOD COEFFICIENT, F_v		1.70	
FUNDAMENTAL PERIOD OF THE STRUCTURE, T (WORST CASE FOR ALL STRUCTURES)		0.152 s	
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD. Sps		2.08 🗆	
		2.00	
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{22} - USED to DETERMINE CS (WITH CAP)		202*070-1456	
PERASCE 7 12.8.1.3) SOIL PROPERTIES MAY NOT BE CLASSIFIED AS SITE CLASS E.		2.00 0.70 - 1.430 🗀	
DESIGN SPECTRAL RÉSPONSE ACCELERATION AT 1-S PERIODS, SD1		1.02	
SEISMIC DESIGN CATEGORY		E	
SITE SPECFIC RESPONSE ANALYSIS NOT REQUIRED PER ASCE 7 11.4.8 EXCEPTION 2	T _s = 0.49 s	T < 1.	.5 * T _s
RESPONSE MODIFICATION FACTOR, R		1.25	
OVERSTRENGTH FACTOR, Ω		1.25	
HORIZONTAL OR VERTICAL IRREGULARITIES		1.0 NONE	
SEISMIC RESPONSE COEFFICIENT, C_{s} (20' WIDE, 30' WIDE, 40' WIDE)	1 16		1.00
DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) (WORST CASE)	10.62 PSF 🛛	12.70 PSF (X)	12.85 PSF 🗌
ALLOWARIES OIL REARING FOR FOUNDATIONS			ARTS
	VAR		טואור
FLOOD DESIGN - DESIGN IS ASSUMED TO NOT BE IN FLOOD HAZARD AREA			
IF PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED & SIGNED FROM A			
JUILO LI VOITVILLI VIO TALQUINLIATO VIALIDATE THE ALLOVVADLE JUIL VIALUEJ JPEUFIED.			

STRUCTURAL SEPARATION

ALL DEFLECTIONS SHOWN ALSO INCLUDE THE P-DELTA ROTATION PER IR PC-7

SEPARATION IS THE SUM OF 2 OF THESE SELECT DEFLECTIONS ARE FOR (1) STRUCTURE

		SOIL CLAS	SES PER CBC TABLE 1806A.2
MAXIMUMDRIFT δmax SIDE COLUMNS		Soil Class 5	Soil Class 4
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	[]2.40	[] 2.55
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	[]2.15	[] 2.30
40' WIDE (8' EAVE , T, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION ($\delta m = Cd \delta max$) Cd = 1.25	(INCHES)	[]2.20	[]2.20
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	[] 3.00	[] 3.19
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	[] 2.69	[] 2.88
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	[] 2.75	[] 2.75
MAXIMUMDRIFT δmax END COLUMNS		<u>Soil Class 5</u>	Soil Class 4
MAXIMUM DRIFT δ _{max} END COLUMNS 20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	<u>Soil Class 5</u> [] 2.40	<u>Soil Class 4</u> [] 2.55
MAXIMUM DRIFT δmax END COLUMNS 20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES) (INCHES)	<u>Soil Class 5</u> [] 2.40 [] 2.15	<u>Soil Class 4</u> [] 2.55 [] 2.30
MAXIMUM DRIFT δmax END COLUMNS 20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES) (INCHES) (INCHES)	<u>Soil Class 5</u> [] 2.40 [] 2.15 [] 2.20	<u>Soil Class 4</u> [] 2.55 [] 2.30 [] 2.20
MAXIMUM DRIFT δ_{max} END COLUMNS 20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION ($\delta_{m} = Cd \delta_{max}$) Cd = 1.25	(INCHES) (INCHES) (INCHES)	<u>Soil Class 5</u> [] 2.40 [] 2.15 [] 2.20	<u>Soil Class 4</u> [] 2.55 [] 2.30 [] 2.20
MAXIMUM DRIFT δ_{max} END COLUMNS 20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION ($\delta_{m} = Cd \ \delta_{max}$) Cd = 1.25	(INCHES) (INCHES) (INCHES)	<u>Soil Class 5</u> [] 2.40 [] 2.15 [] 2.20 [] 3.00	<u>Soil Class 4</u> [] 2.55 [] 2.30 [] 2.20 [] 3.19
MAXIMUM DRIFT om ax END COLUMNS 20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION (om = Cd om ax) Cd = 1.25 20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES) (INCHES) (INCHES) (INCHES)	<u>Soil Class 5</u> [] 2.40 [] 2.15 [] 2.20 [] 3.00 [] 2.69	<u>Soil Class 4</u> [] 2.55 [] 2.30 [] 2.20 [] 3.19 [] 2.88
MAXIMUM DRIFT om ax END COLUMNS 20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION (om = Cd om ax) Cd = 1.25 20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) 40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES) (INCHES) (INCHES) (INCHES) (INCHES)	<u>Soil Class 5</u> [] 2.40 [] 2.15 [] 2.20 [] 3.00 [] 2.69 [] 2.75	Soil Class 4 [] 2.55 [] 2.30 [] 2.20 [] 3.19 [] 2.88 [] 2.75

	BEFORE THE APF	SUBMITTING THESE PRE-CHEC PROVED OPTIONS:	KED DRAWINGS FOR YOUR PI	ROJECT, FOLLOW THE	STEPS BELOW	TO PROPERLY DEFINE		SITE SPEC
	STEP 1:	SELEC T FRAME DIMENSIONS FO -GABLE STRUCTURES UP TO 2	R YOUR PROJECT	ASE FRAME				INSTRUC TIC
		-GABLE STRUCTURES UP TO 4 -MAXIMUM WIDTH IS 40' (SEE ' -THE 24', 44', 64', 84' AND 10	0' WDE USE THE "RG 40" B "ARCHITECTURAL VIEWS" SHE 04' LENGTHS ARE SUGGESTE!	ASE FRAME ET FOR REFERENCE) D BECAUSE THEY ARE	THE MOST C	OMMON		<u>SNOW</u> pg = _ O
		(20' BAYS ARE THE MOST E FRAME LENGTHS ASSUME 2' (CONOMICAL) OVERHANGS (UNO BY ARCHI ERAME DIME	TECT – 2' MAX DIMEN	ISION)			$Pf = \underline{O}$ $Ce = \underline{O}$
					OT	HER		$\frac{WIND}{v = 93}$
	ST	FRAME UDTH	[] 20 30 [] [] 44' 44' 64' []	40 84' [] 104'	[] (4	NO MAX)		kzt =
	STEP 2:	SELECT ROOF DECK FOR YOU	R PROJEC T					EXPOSURE:
		-"M" REPRESENTS McELROY ME -"G" REPRESENTS McELROY ME -"S" REPRESENTS McELROY ME	TAL "MULTI–RIB" ROOF PAN TAL "MEGA–RIB" ROOF PAN TAL "MEDALLION–LOK" 16" §	EL EL TANDING SEAM ROOF	PANEL			
	EP 2	ROOF PANEL TYPE	ROOF P/	ANEL G [] S				NO GEC
	o ا							
	STEP 3:	IDENTIFY THE SS ACCELERATIO -SS VALUE DETERMINES THE R -SS VAULE DEPENDS ON THE F)N (g) FOR YOUR PROJECT EQUIRED SEISMIC DESIGN FOF PROJECTS GEOGRAPHICAL LC	RCES CATION (VALUES RAN	GE FROM 0.00	TO 3.73)	ONE	GEOTEC
		-FIND S\$ VALUES FOR "USGS SEISMIC DESIC	YOUR PROJECT ON THE USC N MAPS")	SS WEBSITE (SEARCH	INTERNET FOR			SITE ULASS: Ss = 1.73
	STEP 3		<u>1.73</u>	<u>Z</u>			SELE	
								PER CH SHORT-
	STEP 4:	IDENTIFY THE SS REGION FOR -THE REGIONS ARE DEPENDAN -THE SS REGION DICTATES THE	YOUR PROJECT T ON THE Ss VALUE DETERM E MAXIMUM DEAD LOAD PERM	INED IN STEP 3 11TTED ON THE FRAME				AS SPEC AREA O'
			Ss REGIO	N Ss REG	IONS	MAX DEAD LOAD		NOT EL SITE CLASS:
	4			0 < Ss < 2.14 < Ss	= 2.14 <= 2.50	5 PSF 5 PSF		Sds = Fa
	STI	DESC RIPTION		2.50 < Ss	<= 2.60	5 PSF		
								Cs= X.X
	STEP 5:	IDENTIFY THE ROOF DEAD LOAD	D FOR YOUR PROJECT					SIESMIC DE *SITE SPE
		 THE COLLATERAL LOAD REPR BE SURE THE TOTAL ROOF I DEAD LOAD SHOWN IN STEP 	RESENTS ADDITIONAL LOAD TO DEAD LOAD FOR YOUR PROJE 4 FOR YOUR Ss VALUE	HAT CAN BE SUPPOR TCT IS LESS THAN OR	TED BY THE F EQUAL TO TH	RAME E MAX		ALLOWED
		- Sds VALUE USED IN CALCUL	ATION IS THE CAPPED Sds (TOTAL ROOF DE	SEE DESIGN CRITERIA)			ABBREVIATION
	د ط	ROOF DECK	DEAD LOAD	M=1.1PSF; G=	EXAMPLES =1.2PSF ;S=1.3	S BPSF (SEE STEP 2)		ACI AISC
	STE		PSF	LIGHTNING,FIRE	SUPPRESSION	,SOLAR PANELS,ETC		ASM
					(MAX 5 PS	F)		AWS
	STEP 6:	IDENTIFY THE FOUNDATION REG -IDENTIFY SOIL CLASS FOR PR -USE THIS TO SELECT CORREC	QUIREMENTS FOR YOUR PROJ OJECT SITE PER SITE SPECIF CT FOUNDATION SIZE ON FOU	ECT IC SOIL CONDITIONS INDATION SHEET				CJP
		A OVER 4000 SQFT REQUIRES	FOUNDATION R					DEG
	STEP 6	SOIL CLASS 5 (BEARING) 1500 F	2SF [] SOIL CLASS 4 ((BEARING) 2000 PSF []	SOIL CL	ASS 3 (BEARING) 3000		DIA DIM
.00		COHESION 130 PSF	00 PSF/FT SOIL CLASS 5 (LAT FRICTION	COEFFICIENT 0.25		S 5 (LATERAL BEARING) FRICTION COEFFICIENT 0.	400 PSF/FT 30	DSA EQ
5 PSF	STED 7.							GA GA
		-MAXIMUM CLEAR HEIGHT IS 12 -MARK UP PC DRAWINGS WITH	2'-0"; (SEE "ARCHITECTURAL SIZE AND LOCATION OF CU	_ VIEWS" SHEET FOR TOUTS BEFORE SUBMI	REFERENCE) ITTING TO DSA		_	IN KSI
			MISC ELLA		DESIGN OP1	NONS	_	MAX MIN
	STEP	CLEAR I ELEC TRIC AL	HEIGHT	[]8' X 10' []YES	[] 12' [] ' (12' MAX)		MISC
		GUTT	ERS	X YES		[] NO		
HESE SELECTED DEFLECCTION	STEP 8:	SELECT APPLICABLE SHEET IN -REFERENCE THE BASE FRAME -IDENTIFY THE APPLICABLE SH	DEX FOR YOUR PROJECT : (STEP 1) AND THE ROOF P IEET INDEX	ANEL TYPE (STEP 2)				
RUCTURE		BASE FRAME	SHEET	INDEX RG 30		N RG 40		TYPE C OCCUP
Soil Class 3		ROOF PANEL TYPE	M G S	M G	S F 1	M G S	_	NUMBE
[] 2.65		GENERAL NOTES	LS1.0 LS1.0 LS1.0	LS1.0 LS1.0	L J LS1.0	LS1.0 LS1.0 LS1.0)	MOST (MOST
₩ ^{2.40}	8	FOUNDATION PLAN FRAMING PLAN	LS2.0 LS2.0 LS2.0	LS3.0 LS3.0 LS3.1 LS3.	LS3.0 LS3.1	LS4.0 LS4.0 LS4.0 LS4.1 LS4.1 LS4.7		AREA
[] 2.30	STI	ROOFING LAYOUT & DETAILS	LS2.1 LS2.1 LS2.1 LS2.2 LS2.5 LS2.4	LS3.1 LS3.1 LS3.2 LS3.3	LS3.1 LS3.4	LS4.1 LS4.1 LS4.7 LS4.2 LS4.3 LS4.4	 	ALLOV
r 1 9 94		MISC DESIGN OPTIONS	LS5.0 LS5.0 LS5.0	LS5.0 LS5.0	AS5.0	LS5.0 LS5.0 LS5.0		<u>RELA</u>
[] 3.31 [X] 3.00		DSA 103 EXAMPLE	LS1.2 LS1.2 LS1.2 LS1.3 LS1.3 LS1.3	LS1.2 LS1.2 LS1.3 LS1.3	LS12 LS1.8	LS1.2 LS1.2 LS1.2 LS1.3 LS1.3 LS1.3		TITLE
[] 2.88		_					Z	2022 2022 2022
	STEP 9:	INCLUDE APPLICABLE SHEETS -INCLUDE 'MISC DESIGN OPTIO	WITH YOUR DSA SUBMITTAL NS' SHEET FOR PROJECTS W	THOUT ELEC TRICAL	UTOUTS OR GI	JTTERS		2022 2022 2022
<u>Soil Class 3</u>	STEP 10	DENTIFY PROJECT NAME AND) LOCATION					2022 2022
[] 2.65		PROJECT NAME:	SCH	OOL DISTRICT:				2022 2022
LXL 2.40 [] 2.30	VALL					10 THE 10T	_	TITLE RFFF
•	STEP 11: SPECIFIC	URUSS UUT EXAMPLE 103 FO	TMIS & INCURPURATE REQUI	XED SPECIAL INSPECT	IUIS IUS FORM	15 IHAI ARE PROJECI		2
[] 3.31								<u>SCOPE</u> THESE
₩ ^{3.00}								PREFAE STRUC ALLOW
[] 2.88								

INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWINGS TO DSA:

SITE SPECIFIC PARAMETERS IDENTIFICATION STAMP ONS: DESIGN PROFESSIONAL SHALL CHECK THE APPROPRIATE SELECTION BOXES BELOW AND ENTER N PARAMETERS APPLICABLE TO THE SPECIFIC PROJECT SITE DIV. OF THE STATE ARCHITEC APP: 03-123817 INC: **REVIEWED FOR** SS 🖌 FLS 🖌 ACS 🖌 DATE: 03/07/2025 mph < XX mph <u><1</u> ICON STD RG/DSA-P cX D□ DRAWN BY JD DATE 3/21/202 REV BASED ON SITE CLASS D TECHNICAL INVESTIGATION REQUIRED REV DATE Fa = 1.2 BASED ON SITE CLASS DETERMINED PER CHAPTER 20 OF ASCE 7-16 HNICAL INVESTIGATION PROVIDED с 🗆 р💢 е🗆 Fa = <u>1.0</u> PER ASCE 7-16 SUPPL 3, TABLE 11.4-1 BASED ON SITE SPECIFIC GROUND MOTION HAZARD ANALYSIS APTER 21 OF ASCE 7-16 PERIOD DESIGN SPECTRAL RESPONSE PARAMETER, Sds, SHALL BE CIFIED IN GEOTECHNICAL INVESTIGATION VER 4000 SQFT REQUIRES A GEOHAZARD REPORT ARCHITECTS ENGINEERS 2700 SATURN ST I BREA, CA 92821 PPROVAL REQUIRED EGIBLE FOR OTC REVIEW T. 714.524.1870 | F. 714.524.1875 www.jrma.com C D E Ss = 1.16 LASS:C or D: 0.7 x Sds* = 0.7 x $1.16 = 0.812 \le X.XX$ Sds = _____ < X.XX LASS E: X USED IN DESIGN SIGN CATEGORY D 🔀 E 🗆 Aug 31, 2023 CIFIC Sds VALUE BEFORE APPLYING REDUCTION BY ASCE 7 SECTION 12.8.1.3 AMERICAN CONCRETE INSTITUTE MPH MILES PER HOUR MERICAN INSTITUTE OF STEEL CONSTRUCTION MULTI-RIB ROOF PANEL (MCELROY) М NOT TO SCALE ASSEMBLY (INTERNAL REFERENCE) NTS NUMBER AMERICAN SOCIETY FOR TESTING AND MAT'LS NO APPROVED AMERICAN WELDING SOCIETY OC ON CENTER DIV. OF THE STATE ARCHITECT CALIFORNIA BUILDING CODE OSHA OCCUPATIONAL HEALTH AND SAFETY ADMIN APP:04-182188 PC COMPLETE JOINT PENETRATION PCF POUNDS PER CUBIC FOOT REVIEWED FOR CLEAR ΡJ PRETENSIONED JOINT SS 🗹 FLS 🗹 ACS 🟹 CG 🗌 PLACES DEGREE PLC S DATE: 09/21/2023 DIAMETER PLT PLATE PSF POUNDS PER SQUARE FOOT DIMENSION POUNDS PER SQUARE INCH DIVISION OF THE STATE ARCHITECT PSI QUANTITY QTY EQUAL REF FEET REFERENCE GAGE SQ SQUARE INC HES SS STANDING SEAM ROOF PANEL (MCELROY) TYP KIPS PER SQUARE INCH TYPIC AL UNO UNLESS NOTED OTHERWISE MAXIMUM U.S. GEOLOGIC AL SURVEY MINIMUM USGS MISC ELLANEOUS W/ WITH \bigcirc \leq EC TURAL REQUIREMENTS AL **DESC RIPTION** DESIGN VAULES ENER, CONSTRUCTION II-B ANC Y CLASSIFIC ATION A-3 R OF STORIES 1 RINKLER SYSTEM NOT BY ICON/WEIGHT NOT INCLUDED IN DESIGN OMMON RG20 MIN/MAX SQ.FT (SEE STEP 1) 480/2,080 OMMON RG30 MIN/MAX SQ.FT (SEE STEP 1) 720/3,120 OMMON RG40 MIN/MAX SQ.FT (SEE STEP 1) 960/4,160 OVER 4000 SQFT REQUIRES GEOHAZARD REPORT ABLE AREA FOR II-B / A-3 IS 9500 SQ.FT <u>FED BUILDING CODES AND STANDARDS</u> 24 CODES: CALIFORNIA ADMINISTRATIVE CODE (CAC)(PART 1, TITLE 24, CCR) CALIFORNIA BUILDING CODE (CBC), PART 2, TITLE 24 CCR UN ..(PART 3, TITLE 24, CCR) CALIFORNIA ELECTRICAL CODE ... CALIFORNIA MECHANICAL CODE (CMC)(PART 4, TITLE 24, CCR) CALIFORNIA PLUMBING CODE (CPC)..... ..(PART 5, TITLE 24, CCR) CALIFORNIA ENERGY CODE(PART 6, TITLE 24, CCR) CALIFORNIA FIRE CODE (CFC)(PART 9, TITLE 24, CCR) CALIFORNIA GREEN BUILDING STANDARDS CODE......(PART 11, TITLE 24, CCR) DISTINCTIVE STEEL SHELTERS WWW.ICONSHELTERS.COM CALIFORNIA REFERENCE STANDARDS CODE.....(PART 12, TITLE 24, CCR) COPYRIGHT 2004, ICON SHELTER 19 CCR, PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS SYSTEMS, INC. RENCE CODE SECTIONS FOR APPLICABLE STANDARDS: 022 CBC, CHAPTER 35 1455 LINCOLN AVE HOLLAND MI, 49423 022 CFC, CHAPTER 80 616.396.0919 OF WORK NARRATIVE 800.748.0985 DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING 616.396.0944 FX RICATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF HOLLOW URAL STEEL MEMBERS SUPPORTED BY CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN THE STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS. $\frown 1$ LJ PRE-CHECK (PC) DOCUMENT • U Code: 2022 CBC A separate project application for construction is required.

PRINTED ON :

GENERAL:

- 1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
- WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE
- CBC, C.A.C. TITLE 24, AND ALL STATE AND FEDERAL REGULATIONS. 3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING
- WITH ANY WORK INVOLVED. 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS, ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS
- PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISIT TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION.
- 6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS, EXCEPT AS AMENDED BY CBC CHAPTER 35. 7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- 8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
- 9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. 10. THE SCHOOL DISTRICT INSPECTOR ON RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF
- INSTALLATION. 11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS
- (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS. SEE CBC CHAPTER 7A FOR REQUIREMENTS 12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
- 13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.

STRUCTURAL AND MISCELLANEOUS STEEL:

- 1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUE OF STEEL CONSTRUCTION (AISC) SPECIFICATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE
- 2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 KSI, GRADE B OR A501 UNLESS NOTED OTHERWISE.
- 3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A-500, GRADE B (OR C), Fy = 46 KSI. MIN. 4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESS CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE
- DRAWINGS (MAXIMUM INCREASE OF 1/8").
- 5. ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A-36, Fy = 36 KSI. 6. ALL PLATE STEEL SHALL CONFORM TO ASTM A-572, Fy= 50 KSI
- 7. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A-653, CS = TYPE B, Fy = 50 KSI Fu = 65 KSI
- 8. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
- 9. ALL ROOF DECKS SHALL HAVE KYNAR 500 METAL COATING
- 10.ALL ROOF DECKS SHALL CONFORM TO ASTM A-792, Fy = 50 KSI. 11. ALL BASE CONNECTIONS ARE A PART OF THE LATERAL FORCE RESISTING SYSTEM

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

- 1. PER TITLE 24, PART 1, SECTION 4-316(e) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS.
- 2. FOR THE SITE SPECIFIC PROJECT, J. R. MILLER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN
- GENERAL RESPONSIBLE CHARGE. 3. FOR THE SITE SPECIFIC PROJECT, J.R. MILLER & ASSOCIATES' RESPONSIBILITY IS LIMITED TO THE
- THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC 4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM J.R. MILLER & ASSOCIATES'
- RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT 5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE, BUT ARE NOT LIMITED TO, STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS,
- AND SIGNING OFF OF THE VERIFIED REPORT FOR COMPLETED WORK. 6. J.R. MILLER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

CONSTRUCTION NOTES

- 1. A DSA-CERTIFIED CLASS 3 (MINIMUM) PROJECT INSPECTOR IS REQUIRED FOR THIS PROJECT. 2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE
- DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.
- 3. A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK, THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
- 4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT. 5. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS ARE THAT ALL THE WORK OF THE ALTERATION, REHABILITATION OR
- RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24. CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA
- BEFORE PROCEEDING WITH THE WORK, (SECTION 4-317(c), PART 1, TITLE 24, CCR) 6. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES

WELDING:

- 1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA. 2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-Ib @ (0° F). 3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE

- PROPER MATERIAL ID AND WELDING. 4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPEC IFIC ATIONS.

<u>BOLTING:</u>

- REQUIRED.

FOUNDATIONS:

- 2. PER CBC SECTION 1803A.2, GEOTECHNICAL REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS. ALLOWABLE FOUNDATION AND LATERAL SOIL PRESSURE VALUES MAY BE DETERMINED FROM TABLE 1806A.2. 3. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD

- 6. PER CBC SECTION 1803A.6, GEOHAZARD REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS.
- 7. GEOHAZRD REPORTS ARE TO COMPLY WITH DSA IR A-4 PER IR-7 SECTION 1.8 8. SITE SPECIFIC GEOTECHNICAL REPORT IS REQUIRED AT THE TIME OF SITE APPLICATION IF USING OTHER THAN
- CLASS 5 SOIL, PER DSA IR PC-7
- NO 1/3 INCREASE HAS BEEN APPLIED.

<u>CONCRETE:</u>

1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

- STRENGTH Pc (28 DAYS) 5000 PSI
- 2. CONCRETE MIX DESIGN PARAMETERS ARE GOOD FOR EXPOSURE CATEGORIES FO, F1 & F2. THE AIR ENTRAINMENT FOR THESE CATEGORIES SHALL BE AS FOLLOWS: F0-0, F1-4.5, F2-6
- 3. CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA. 4. AGGREGATES SHALL CONFORM TO THE ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.005. MAX AGGREGATE SIZE = 1".
- 6. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT.
- ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED. 7. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- 8. CONCRETE DURABILITY SHALL BE PER CBC 1904A.1 ACI 318-19, CHAPTER 19.

 - 9. CONCRETE SHALL BE TESTED PER CBC 1903A, TABLE 1705A.3. AND ACI 318-19, SECTION 26.12. 10. NO ADMIXTURE SHALL CONTAIN CALCIUM CHLORIDE.

1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE HOT DIPPED GALVANIZED ASTM F3125 GRADE A325 HIGH STRENGTH BOLTS (UNO), WITH THE NUTS CONFORMING TO HOT DIPPED GALVANIZED ASTM A-563 GRADE DH.

- 2. HIGH STRENGTH BOLTS SHALL BE VERIFIED AND INSPECTED PER CBC 1705A2.1 3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS - INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE
- 4. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F-436.
- 5. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL
- BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S
- USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6.

A)PRETENSIONED JOINTS MUST BE INSTALLED AND INSPECTED TO MEET ONE OF THE FOLLOWING REQUIREMENTS 1. TURN-OF-NUT PRETENSIONING: PER SECTION 8.2.1 OF THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS, WASHERS ARE NOT REQUIRED FOR THIS METHOD, THE NUT OR HEAD SHALL BE ROTATED AS SPECIFIED IN TABLE 8.2. THE PART NOT TURNED SHALL BE PREVENTED FROM ROTATING. 2. CALIBRATED WRENCH: PER THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS, WASHERS ARE REQUIRED (NOT SUPPLIED BY ICON) THESE SHALL BE INSTALLED PER THE INSTALLATION TORQUE DETERMINED IN THE PRE-INSTALLATION VERIFICATION OF THE FASTENER ASSEMBLY PER SECTION 7. THE PART NOT TURNED SHALL BE PREVENTED FROM ROTATING. 3. IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH "PT REQUIRED"

B) ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF THE SNUG-TIGHTENED JOINTS. SNUG TIGHT CONDITION EXISTS WHEN ALL PLIES IN A CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT BY THE BOLTS IN THE JOINT AND ALL OF THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

- 1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER CBC TABLE 1806A, UNLESS NOTED OTHERWISE. PASSIVE PRESSURE IS ASSUMED TO START 12" BELOW TOP OF FOOTING.
- D-1557 OR AS RECOMMENDED BY THE GEO-TECH ENGINEER. FLOODING NOT PERMITTED. 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL
- BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE. MINIMUM SETBACK FROM TOE OF SLOPE ON AN ASCENDING SLOPE SHALL BE 15 FEET AND MINIMUM SETBACK FROM TOE OF SLOPE ON A DESCENDING SLOPE SHALL BE 40 FEET
- 9. LATERAL BEARING HAS BEEN INCREASED PER CBC 1806A.3.4 FOR THE 1/2" DEFLECTION & HAS BEEN DESIGNED FOR P-DELTA EFFECTS.
- 10. MINIMUM CLEARANCE BETWEEN PIERS SHALL BE 8'-0".

W/C RATIO (NON-AIR ENTRAINED)	W/C RATIO (AIR ENTRAINED)	SLUMP (±1")	UNIT WEIGHT (NORMAL WEIGHT)
0.44	0.35	3"	150 PCF

5. CEMENT SHALL CONFORM TO ASTM C-150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS.

REINFORCING STEEL:

- 1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A-615, AS FOLLOWS:
 - GR 60: (#4 BARS AND LARGER)
 - GR 40: (#3 BARS)
- 2. DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES. 3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
- A. CAST AGAINST EARTH B. CAST AGAINST FORM BELOW GRADE2"
- D. SLABS ON GRADE (FROM TOP OF SLAB)1" 4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE
- COLD 5. REINFORCING SHALL BE LAP SPLICED PER ACI 318-19, SECTION 25.5.
- 6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
- 7. WELDING OF REINFORCING IS NOT ALLOWED. 8. REINFORCING STEEL SHALL BE INSPECTED PER CBC 1705A.3.

POWDER-COAT FINISH SYSTEM

- ALL BUILDINGS THAT HAVE A POWDER-COATED FINISH SHALL MEET THE FOLLOWING SPECIFICATIONS: SPECIFIC ATIONS
- 2. THE STEEL SHALL BE WASHED IN A ZINC PHOSPHATE IN AN MINIMUM THREE STAGE ELECTRO DEPOSITION PRE-TREATEMENT PROCESS. 3. IMMEDIATELY FOLLOWING PRE-TREATMENT THE STEEL SHALL BE TOTALLY COATED IN AN EPOXY PRIMER
- TO A UNIFORM THICKNESS OF A MINIMUM OF 0.7 TO 0.9 MILS. THE E-COATING SHALL PROVIDE A MINIMUM OF 1000 HOURS OF SALT SPRAY CORROSION PROTECTION TO THE STEEL
- 4. THE STEEL SHALL THEN HAVE A TGIC POLYESTER COLOR COAT APPLIED OVER THE E-COATED SURFACE. 5. THE FINISH THICKNESS OF THESE APPLICATIONS SHALL BE A MINIMUM OF 8 TO 12 MILS.
- COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3"(UNLESS NOTED OTHERWISE).

1. THE STEEL FRAME (HSS SECTIONS, COLD FORMED & PLATE STEEL) SHALL BE SHOT-BLASTED TO A NEAR WHITE CONDITION PER SSPC-10

6. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES & COLD FORMED STEEL ETC.) NOT POWDER-COATED SHALL BE PAINTED WITH PRIME



PRE-CHECK (PC) DOCUMENT Code: 2022 CBC A separate project application for construction is required

	DSA 103-22: LISTING (Application Number:	DF STRUCTURAL TEST School Name:	S & SPECIAL INS	SPECTIO	NS, 2022 CBC School District:	DSA Table	A 103-22: LISTIN 2 1705A.6, Table 1703	NG OF STRUCTURAL TES 5A.7, Table 1705A.8	TS & SPECIA	L INSPECTION	DNS (SOILS), 2022 CBC	DSA 1(Table 17	03-22: LISTING 05A.6, Table 1705A.	OF STRUCTURAI 7, Table 1705A.8
	04-122188 DSA File Number:	PC Update Increment Number:			PC Update Date Created: 2023-04-19 08:36:32	Appl 04-12 DSA	lication Number: 22188 File Number:	School Name: PC Update Increment Number:			School District: PC Update Date Created:	Applicati 04-12218 DSA File	ion Number: 38 • Number:	School Name: PC Update Increment Num
		s form is only a summary li	2022	CBC	of the special inspections required for the project	Geo	technical Reports	s: Project has a geotechnica	al report, or CI	Ds indicate so	ils special inspection is required by GE			
	Generally, the structura	al tests and special inspecti	ions noted on this f	orm are th	ose that will be performed by the Geotechnical Engineer		S1. GENERAL:						3. DRIVEN DEEP FOU	NDATIONS (PILES):
	of Record, Laboratory on the DSA approved o	of Record, or Special Inspec locuments. The appendix a	ctor. The actual con at the bottom of thi	s form ide	and inspection program must be performed as detailed ntifies work NOT subject to DSA requirements for special		Test or Special Insp	pection	Type Periodic	Performed By	Code References and Notes * By geotechnical engineer or his or her gualified representative	- a.	Verify pile materials,	sizes and lengths comply
	inspection or structura not limited to, special in	ll testing. The project inspe spections not listed on thi	ector is responsible Storm such as struc	for provid tura l woo	ng inspection of all facets of construction, including but d framing, high-load wood diaphragms, cold-formed steel	V	Site has been prep	pared properly prior to placement of	Feriodic	GL	(See Appendix (end of this form) form for exemptions.)	th	e requirements.	s of test piles and conduc
	fram	ing, anchorage of non-stru	ctural components,	etc., per 7	itle 24, Part 2, Chapter 17A (2022 CBC).		Foundation excava and have reached p	ations are extended to proper depth proper material.	1				Iditional load tests as	required.
	**NOTE: Unde	fined section and table ref	erences found in th	is docume	nt are from the CBC, or California Building Code.		Materials below fo design bearing capa	otings are adequate to achieve the active.				ar	Inspect driving operated accurate records fo	r each pile.
	KEY TO COLUMNS		、									」 □ d. □ cc	Verify locations of pil onfirm type and size o	es and their plumbness, of hammer, record numbe
International and the second	I. TYPE			GE (Geo	rechnical Engineer) – Indicates that the special inspection shall be		Test or Special Inst	pection	Туре	Performed B	y Code References and Notes	- bl	ows per foot of penet enetrations to achieve	ration, determine require e design capacity, record
	Continuous – Indicates that a c	continuous special inspection is		represer	ed by a registered geotechnical engineer or his or her authorized tative.		a. Perform classifica	tion and testing of fill materials.	Test	LOR*	* Under the supervision of the geotechnical engineer.	an	Steel piles.	record any pile damage
				LOR (La	poratory of Record) Indicates that the test or special inspection shall med by a testing laboratory accepted in the DSA Laboratory Evaluation		b. Verify use of prop inspect lift thickness	per materials, densities and ses, placement and compaction	Continuous	s GE*	* By geotechnical engineer or his or her qualified representative. (Refer to specific items identified in the Appendix (end of this form) form for	f.	Concrete piles and cc	oncrete filled piles.
	Periodic – Indicates that a peri	odic special inspection is require	ed	and Acce	ptance (LEA) Program. See CAC Section 4-335.		during placement o	f fill.			exemptions where soils SI and testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager.		For specialty piles, pe	erform additional inspect
				PI (Proje by a proj	ct Inspector) – Indicates that the special inspection may be performed ect						In such cases, the LOR's form DSA 291 shall satisfy the soil SI and test reporting requirements for the exempt items.)	re	sponsible charge.	gistered design profess
	Test – Indicates that a test is re	quired		inspecto	r when specifically approved by DSA.		c. Compaction testi	ng.	Test	LOR*	* Under the supervision of the geotechnical engineer.	- <u></u>	4. CAST-IN-PLACE DE	 EEP FOUNDATIONS (PIE
				by an ap	oropriately qualified/approved special inspector.						(Refer to specific items identified in the Appendix (end of this form) for exemptions where soils testing may be conducted under the	Te	est or Special Inspect	tion
											supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form DSA 291 shall satisfy the soil test	a. ar	Inspect drilling operand accurate records fc	itions and maintain com or each pier.
											reporting requirements for the exempt items.)			
Diff of the second s	DIVISION OF THE STATE ARCHITECT		DEPARTMENT OF GE	NERAL SERVI	CES STATE OF CALIFORNIA	DIVISI	ON OF THE STATE ARCH	ITECT	DEPARTMEN	IT OF GENERAL SER	/ICES STATE OF CALIFORNI	IA DIVISION (OF THE STATE ARCHITEC	T
	DGS DSA 103-22 (Revised 12/01/202	2)	Page 1	of 19		DGS D	05A 103-29 (Revised 12/0)1/2022)		Page 2 of 19		DGS DSA 1	03-22 (Revised 12/01/20	322)
				DECTION								DSA 103	3-22: LISTING C	JF STRUCTURAL
	DSA 103-22: LISTING O Table 1705A.6, Table 1705A.7, 1	able 1705A.8	S & SPECIAL INS	PECHO	15 (SOILS), 2022 CBC	DS/	A 103-22: LISTI	NG OF STRUCTURAL TES	TS & SPECIA	AL INSPECTI	ONS (SOILS), 2022 CBC	Table 1705/ Applicatio	<u>A.3; ACI 318-19 Secti</u> n Number:	ions 26.12 & 26.13 School Name:
Bit How Bardines Bit Market Bit Market Bit Market Bit Market Bit Market Market Mar	Application Number: 04-122188	School Name: PC Update			School District: PC Update	App	lication Number:	School Name:			School District:	- DSA File N	umber:	Increment Numb
Normality State Sta	DSA File Number:	Increment Number:			Date Created: 2023-04-19 08:36:32	DSA	File Number:	Increment Number:			Date Created: 2023-04-19 08:36:32	C1.	 CAST-IN-PLACE CON	
	Test or Special Inspectio	n	Type Peri	formed By	Code References and Note		S6. OTHER SOILS:		\rightarrow			_ Test	or Special Inspectic	
	 b. Verify pier locations, dia diameters (if applicable), la 	meters, plumbness, bell engths and embedment into	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) for exemptions.)		Test or Special Ins	pection	Туре	Performed B	y Code References and Notes	a. Ve	erify use of required d	esign mix.
	volumes.	ord concrete or grout					a. Soil Improvemer	nts	Test	GE*	Submit a comprehensive report documenting final soil improvements constructed, construction observation and the results of the	b. Id	lentifiy, sample, and te	est reinforcing steel.
	C. Confirm adequate end s	trata bearing capacity.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) for exemptions.)						for final acceptance.) C. Di	uring concrete placen	
 I define the second prove t							b. Inspection of Soi	il Improvements	Continuou	ıs GE*	* By geotechnical engineer or his or her qualified representative.	- tests	s, and determine the t	emperature of the
	a. Concrete piers.		Provide tests and ins	pections pe	CONCRETE Section below.		C.					d. Te	est concrete (f'c).	
	S5. RETAINING WALLS:											e. Ba	atch plant inspection:	
Control Cont	Test or Special Inspectio	n	Type Perfe	ormed By	Code References and Notes									
Note: 1 - Note:	a. Placement, compaction	and inspection of backfill.	Continuous	GE*	representative. (See section S2 above).									ato al
1 Comparison of the stand of the stan	b. Placement of soil reinfo devices.	rcement and/or drainage	Continuous	GE*	* By geotechnical engineer or his or her qualified representative.								Plaing of reinforcing s	leel.
Construction of a set of an and a set of a set o	c. Segmental retaining wa	lls; inspect placement of etc.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. See DSA IR 18-2									
<td< td=""><td>d. Concrete retaining wall</td><td>5.</td><td>Provide tests and ins</td><td>pections pe</td><td>CONCRETE section below.</td><td></td><td></td><td></td><td></td><td></td><td></td><td>Test</td><td>c or Special Inspectic</td><td></td></td<>	d. Concrete retaining wall	5.	Provide tests and ins	pections pe	CONCRETE section below.							Test	c or Special Inspectic	
bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC bit 10-22: USTING 0F STRUCTURAL TESTS 6 SPECIAL INSPECTIONS CONCRETE, 2022 CBC	e. Masonry retaining walls		Provide tests and ins	pections pe	MASONRY section below.							a. sa	mple and test prestre	essing tendons and
												b. In	spect placement of p	restressing tendons.
														 \
Applet the Number: Schwol Rever: Schwol Re	DSA 103-22: LISTING (Table 1705A.3; ACI 318-19 Sect	OF STRUCTURAL TEST ions 26.12 & 26.13	S & SPECIAL INS	PECTIO	NS (CONCRETE), 2022 CBC	DSA 1	03-22: LISTING	G OF STRUCTURAL TESTS	S & SPECIAL	INSPECTION	IS (CONCRETE), 2022 CBC	DIVISION OF		
Duble Hundlade: Description Type Performed by Coll Perf	Application Number: 04-122188	School Name: PC Update			School District: PC Update	Table 17 Applica	705A.3; ACI 318-19 Se ition Number:	ections 26.12 & 26.13 School Name:			School District:	DGS DSA 103	-22 (Revised 12/01/2022	2)
Test as Special Inspection Type Performents of the California of the Califori	DSA File Number:	Increment Number:			Date Created: 2023-04-19 08:36:32	DSA Fil	e Number:	Increment Number:			Date Created: 2023-04-19.08:36:32			
c. brownerse Periodic 31 c. bit 1793-A 3 (http://discupred/second/	Test or Special Inspectio	on	Type Per	formed By	Code References and Notes		C4. SHOTCRETE (IN A	DDITION TO SECTION C1):				DSA 10 1705A.2.1,	3-22: LISTING OF Table 1705A.2.1; AISC 303	STRUCTURAL TEST
 Lange Lange Lange	□ c. Verify in-situ concrete s of post-tensioning tendo	trength prior to stressing ns.	Periodic	SI	Table 1705A.3 Item 13. Special inspector to verify specified concrete strength test prior to stressing.	T	est or Special Inspec	ction	Туре	Performed By	Code References and Notes	Applicatic 04-122188 DSA File I)n Number: } Number:	School Name: PC Update Increment Number:
prestriction form Image: Instruction	d. Inspect application of a	post-tensionina or	Continuous	SI	1705A.3.4, Table 1705A.3 Item 9: ACI 318-14 Section 26.13		a. Inspect shotcrete pla application techniques	acement for proper s.	Continuous	SI	1705A.3.9, Table 1705A.3 Item 7, 1908A.1, 1908A.2, 1908A.3. See ACI 506.2-13 Section 3.4, ACI 506R-16.			
Image: Contract Contract Contract Contrate Contract Contract Contract Contract Contrent Contract	prestressing forces and g prestressing tendons.	routing of bonded					o. Sample and test sho	otcrete (f ^r c).	Test	LOR	1908A.2, 1705A.3.9	Tes	st or Special Inspection	materials and
C3. PRECAST CONCEPTE (IN ADDITION TO SECTION C1): Type Performed By Configure 4 C3. Precast concrete (IN ADDITION TO SECTION C1): Type Performed By Configure 4 C3. Precast concrete (IN ADDITION TO SECTION C1): C3. Precast concrete (IN ADDITION TO SECTION C1): Type Performed By Configure 4 C3. Precast concrete (IN ADDITION TO SECTION C1): Test of Special Inspection of precast concrete (IN ADDITION TO SECTION C1): C3. Precast concrete (IN ADDITION TO SECTION C1): C3. Precast concrete (IN ADDITION TO SECTION C1): C4. Prec												⊻ a.V •M wit	ill certificates indicate mar h requirements.	terial properties that comply
Inspectant Special Inspection Type Performed By Code References and Notes Continuous Continuou	C3. PRECAST CONCRETE	IN ADDITION TO SECTION C1	I): Type Per	formed By	Code References and Notes		25. POST-INSTALLED	ANCHORS:	 	Dorforma-15	Code References and Notes	• Ma req	aterial sizes, types and gra juirements.	ides comply with
<td< td=""><td>a. Inspect fabrication of p</td><td>recast concrete members.</td><td>Continuous</td><td>SI</td><td>ACI 318-19 Section 26.13.</td><td></td><td>a. Inspect installation of</td><td>of post-installed anchors</td><td>See Notes</td><td>SI*</td><td>1617A.1.19, Table 1705A.3 Item 4a (Continuous) & 4b (Periodic),</td><td>⊡ b.T ☑ c.F</td><td>escurrigentified materials </td><td>, jS shapes</td></td<>	a. Inspect fabrication of p	recast concrete members.	Continuous	SI	ACI 318-19 Section 26.13.		a. Inspect installation of	of post-installed anchors	See Notes	SI*	1617A.1.19, Table 1705A.3 Item 4a (Continuous) & 4b (Periodic),	⊡ b.T ☑ c.F	escurrigentified materials 	, jS shapes
Image: Construction of the continuity of reinforcement at joints classified as moderate or high deformability elements (MDE of HDE) in structures assigned as moderate or high deformability elements (MDE of HDE) in structures assigned to Seismic Design Category DE, Gor F, inspect Si Table 1705A3; ACI 318-19 Section 26.13.1.3; ACI 550.5 Image: Continue of the embedded parts as Continue of the embedded parts as Completion of the continuity of reinforcement in the field for: Image: Continue of the embedded parts as Completion of the continuity of reinforcement as continue of the field for: Test or Special Inspection Type Performed By Code References and Notes Image: Content at spice of completion content on s	b. Inspect erection of pre	cast concrete members.	Periodic	SI*	Table 1705A.3 Item 10. * May be performed by PI when specifically approved by DSA						1705A.3.8 (See Appendix (end of this form) for exemptions). ACI 318-14 Sections 17.8 & 26.13. * May be performed by the project	d. v apr	/erify and document steel	fabrication per DSA- iments.
reinforcement at joints classified as moderate or high decompility elements (MDE or HDE) in structures assigned to Seising Category D, E or F, inspect such connections and reinforcement in the field for: Test LOR 1910A.S. (See Appendix (end of this form) for exemptions.) 1. Installation of the embedded parts across joints. C. Ompletion of the continuity of reinforcement across joints. C. Other CONCRETE: 3. Completion of connections in the field d. Inspect installation tolerances of precast concrete dlaphragm connections for compliance with ACI 550.5. SI Table 1705A.3: ACI 318-19 Section 26.13.1.3; ACI 550.5	C. For precast concrete di	aphragm connections or	Continuous	SI	Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5						inspector when specifically approved by DSA.	□ e. B	uckling restrained braces	
assigned to besting bes	reinforcement at joints cl deformability elements (1	assified as moderate or high MDE or HDE) in structures					o. Test post-installed a	nchors.	Test	LOR	1910A.5. (See Appendix (end of this form) for exemptions.)	S/A	.2. HIGH-STRENGTH BOL	.TS:
1. Installation of the embedded parts 2. Completion of the continuity of reinforcement across joints. 3. Completion of connections in the field. 1. Installation tolerances of precast concrete diaphragm connections for compliance with ACI 550.5. New York Connections in the field. Image: Content Conte	assigned to Seismic Designed to Seismic Design	nforcement in the field for:											/erify identification markin	ngs and manufacturer's
across joints. 3. Completion of connections in the field. image: second se	1. Installation of the em	bedded parts ntinuity of reinforcement					C6. OTHER CONCRET	E:	Turc	Parformed De	Code References and Notes	spe	rest high-strength bolts r	ad documents.
Image: Serie Series Image: S	across joints. 3. Completion of conne	ctions in the field.					escor special inspec		гуре	г епоттеа Ву		⊂. E	Bearing-type ("snug tight") connections.
diaphragm connections for compliance with ACI 550.5.				C)								d. 1	Pretensioned and slip-criti	ical connections.
	diaphragm connections f	or compliance with ACI 550.5.	Periodic	21	Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5									

	C3. PRECAST CONCRETE (IN ADDITION TO SECTION C1):					С
	Test or Special Inspection	Туре	Performed By	Code References and Notes		Т
	a. Inspect fabrication of precast concrete members.	Continuous	SI	ACI 318-19 Section 26.13.		a.
	b. Inspect erection of precast concrete members.	Periodic	SI*	Table 1705A.3 Item 10. * May be performed by PI when specifically approved by DSA.		
	c. For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high	Continuous	SI	Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5		
	deformability elements (MDE or HDE) in structures assigned to Seismic Design Category D, E or F, inspect					b
	such connections and reinforcement in the field for:					_
	1. Installation of the embedded parts					C
	Completion of the continuity of reinforcement across joints					Т
	3. Completion of connections in the field.					a.
	d. Inspect installation tolerances of precast concrete diaphragm connections for compliance with ACI 550.5.	Periodic	SI	Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5		

DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022)

DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022)

RAL TESTS & SPECIAL INSPECTIONS (SOILS), 2022 CBC IDENTIFICATION STAMP School District: DIV. OF THE STATE ARCHITECT PC Update Date Created: lumber: APP: 03-123817 INC: 2023-04-19 08:36:32 **REVIEWED FOR** SS 🗹 FLS 🗹 ACS 🗹 DATE: 03/07/2025 Performed By Code References and Notes Туре nply with * By geotechnical engineer or his or her qualified representative. GE* Continuous duct Test LOR* * Under the supervision of the geotechnical engineer. ICON STD RG/DSA-PC * By geotechnical engineer or his or her qualified representative. GE* mplete Continuous DRAWN BY JD 3/21/202 DATE Continuous GE* * By geotechnical engineer or his or her qualified representative. mber of quired ord tip REV REV DATE Provide tests and inspections per STEEL section below. Provide tests and inspections per CONCRETE section below. * * As defined on drawings or specifications. ections essional in (PIERS): Performed By Code References and Note Туре mplete GE* * By geotechnical engineer or his or her qualified representative. Continuous ARCHITECTS ENGINEERS (See Appendix (end of this form) for exemptions.) 2700 SATURN STIBREA, CA 92821 T. 714.524.1870 | F. 714.524.1875 www.jrma.com DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA Page 3 of 19 AL TESTS & SPECIAL INSPECTIONS (CONCRETE), 2022 CBC Aug 31, 2023 School District: PC Update Date Created: 2023-04-19 08:36:32 Performed By Code References and Notes Туре Table 1705A.3 Item 5, 1910A.1. Periodic SI Test 1910A.2; ACI 318-19 Ch.20 and Section 26.6.1.2; DSA IR 17-10. (See LOR Appendix (end of this form) for exemptions.) LOR Table 1705A.3 Item 6; ACI 318-19 Sections 26.5 & 26.12. Test APPROVED DIV. OF THE STATE ARCHITECT APP:04-122188 PC REVIEWEDFOR 1905A.1.17; ACI 318-19 Section 26.12. LOR Test SS 🗹 FLS 🗹 ACS 🗷 CG 🗖 DATE: 09/21/2023 See Notes SI Default of 'Continuous' per 1705A.3.3. If approved by DSA, batch plant inspection may be reduced to 'Periodic' subject to requirements in Section 1705A.3.3.1, or eliminated per 1705A.3.3.2. See IR 17-13. (See Appendix (end of this form) for exemptions.) Provide special inspection per STEEL, Category S/A4(d) & (e) and/or S/A5(g) & (h) below. CRETE (IN ADDITION TO SECTION C1): Туре Performed By Code References and Notes Test LOR 1705A.3.4, 1910A.3 1705A.3.4, Table 1705A.3 Items 1 & 9. Periodic SI M \bigcirc $\overline{}$ STATE OF CALIFORNIA DEPARTMENT OF GENERAL SERVICES Page 6 of 19 \triangleleft S \square ESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC 8-16, AISC 360-16; AIS \$100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8 School District: PC Update Date Created: 2023-04-19 08:36:32 ND ALUMINUM USED FOR STRUCTURAL PORPOSES Type Performed By Code References and Notes Table 1705A.2.1 Hem 3a–3c. 2202A.1; AISI S100-20 Section A3.1 & A3.2, AISI S240-20 Section A3 & A5, AISI S220-20 Sections A4 & A6. * By special inspector or qualified technician when performed off-site. Periodic <u>_</u> $\underline{\mathbb{N}}$ Test LOR 2202A.1. SI Periodic DSA IR 17-3. Ś Periodic SI Not applicable to cold-formed steel light-frame construction, except for trusses (1705A.2.4). lter Test LOR Testing and special inspections in accordance with IR 22-4. Type Performed By Code References and Notes DISTINCTIVE STEEL SHELTERS Table 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3.3, Periodic S WWW.ICONSHELTERS.COM J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9. COPYRIGHT 2004, ICON SHELTER SYSTEMS, INC. Test Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR LOR 1455 LINCOLN AVE 17-8. HOLLAND MI, 49423 Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, Periodic SI M2.5 & N5.6; RCSC 2014 Section 9.1; DSA IR 17-9. Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2; AISC 360-16 616.396.0919 S J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. 800.748.0985 *"Continuous" or "Periodic" depends on the tightening method used. 616.396.0944 FX STATE OF CALIFORNIA DEPARTMENT OF GENERAL SERVICES Page 9 of 19

PRE-CHECK (PC) DOCUMENT Code: 2022 CBC A separate project application for construction is required

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	& SPECIAL INSPECT	IONS (STEEL AND ALUMNINUM), 2022 CBC	DSA 103-22: LISTING OF STRUCTURAL TESTS 1705A.2.1. Table 1705A.2.1: AISC 303-16, AISC 341-16, AISC 358-16, A	& SPECIAL	INSPECTIO	NS (STEEL AND ALUMNINUM), 2022 CBC 14: AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8	DSA 103-22: LISTING OF STRUCTURAL T 1705A.2.1. Table 1705A.2.1: AISC 303-16, AISC 341-16, AISC 3
Application Number: School Name:	<u>ISC 360-16; AIST ST00-20; RCSC</u>	School District: PC Update	Application Number:School Name:04-122188PC Update	,	,	School District: PC Update	Application Number:School Name:04-122188PC Update
DSA File Number: Increment Number:		Date Created: 2023-04-19.08:36:32	DSA File Number: Increment Number:			Date Created: 2023-04-19 08:36:32	DSA File Number: Increment Number
			Test or Special Inspection	Туре	Performed By	Code References and Notes	Test or Special Inspection
S/A3. WELDING:			S/A5. FIELD WELDING (IN ADDITION TO SECTION S/A3):	I			S/A6. NONDESTRUCTIVE TESTING:
Test or Special Inspection	Type Performed	By Code References and Notes	Test or Special Inspection	Type	Performed By	Code References and Notes	Test or Special Inspection
AWS designation listed on the DSA-approved documents	Periodic SI	structural steel; AWS D1.2 for Aluminum; AWS D1.3 for cold-formed	fillet welds > 5/16", plug and slot welds.	Continuous	51	applicable); DSA IR 17-3.	
b. Verify weld filler material manufacturer's certificate of	Reriodic SI	DSA IR 17-3.	b. Inspect single-pass fillet welds $\leq 5/16''$.	Periodic	SI	Table 1705A.2.1 Item 5a.5; AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.	b. Magnetic Particle
compliance.			 c. Inspect end-welded studs (ASTM A-108) installation 	Periodic	SI	2213A.2; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1; DSA IR	
c. Verify WPS, welder qualifications and equipment.		DSA IR 17-3.	(including bend test).	Periodic	SI	17-3. 1705A.2.2. Table 1705A.2.1 Item 5a.6: AISC 360-16 (AISC 341-16 as	□ c.
S/A4. SHOP WELDING (IN ADDITION TO SECTION S/A3):						applicable); AWS D1.3; DSA IR 17-3.	
Test or Special Inspection	Type Performed	By Sode References and Notes	e. Inspect welding of structural cold-formed steel.	Periodic	SI*	1705A.2.5; AWS D1.3; DSA IR 17-3. The quality control provisions of AISI S240-20 Chapter D shall also apply. * May be performed by the	S/A7. STEEL JOISTS AND TRUSSES:
fillet welds > 5/16", plug and slot welds.	Continuous Si	applicable), DSA IR 17-3.				project inspector when specifically approved by DSA.	Test or Special Inspection
\checkmark b. Inspect single-pass fillet welds $\leq 5/16''$, floor and roof deck welds.	Periodic SI	1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6; AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.	f. Inspect welding of stairs and railing systems.	Periodic	SI*	1705A.2.1; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3. * May be performed by the project inspector when	a. Verify size, type and grade for all chord and web members as well as connectors and weld filler mate
C. Inspect welding of stairs and railing systems.	Periodic SI	1705A.2.1; AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 &				specifically approved by DSA.	verify joist profile, dimensions and camber (if applic verify all weld locations, lengths and profiles; mark
d. Verification of reinforcing steel weldability	Periodic SI	D1.3; DSA IR 17-3. 1705A.3.1; AWS D1.4; DSA IR 17-3. Verify sarbon equivalent reported	G , Verification of reinforcing steel weldability.	Periodic	SI	1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.	each joist.
other than ASTM A706.	Continuous	on mill certificates.	h. Inspect welding of reinforcing steel.	Continuous	SI	Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A 8: AWS D1 4: DSA IB 17-3	
e. Inspect weiding of reinforcing steel.		1903A.8; AWS D1.4; DSA IR 17-3.					
		ERVICES STATE OF CALIFORNIA	DIVISION OF THE STATE ARCHITECT	DEPARTMENT	OF GENERAL SERV	CES STATE OF CALIFORNIA	DIVISION OF THE STATE ARCHITECT
DGS DSA 103-22 (Revised 12/01/2022)	Page 10 of 19		DGS DSA 103-22 (Revised 12/01/2022)	Р	Page 11 of 19		DGS DSA 103-22 (Revised 12/01/2022)
DSA 103-221 ISTING OF STRUCTURAL TESTS	& SPECIAL INSPECT	IONS (STEEL AND ALLIMNINUM) 2022 CBC					
1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, A	ISC 360-16; AISI S100-20; RCSC	2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8	DSA 103-22: LISTING OF STRUCTURAL TESTS & 1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC	& SPECIAL I SC 360-16; AISI S	100-20; RCSC 2014	IS (STEEL AND ALUMININUM), 2022 CBC I; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8	DSA 103-22: LISTING OF STRUCTURAL TE Application Number: School Name:
Application Number:School Name:04-122188PC UpdateDCA File Number:PC Update		PC Update	Application Number: School Name: 04-122188 PC Update			School District: PC Update	04-122188 PC Update DSA File Number: Increment Number:
DSA File Number: Increment Number:		Date Created: 2023-04-19 08:36:32	DSA File Number: Increment Number:			Date Created: 2023-04-19 08:36:32	
Test or Special Inspection	Type Performed f	By Code References and Notes	Test or Special Inspection	Туре	Performed By	Code References and Notes	X1. OTHER:
S/A8. SPRAYED FIRE-RESISTANT MATERIALS:	Turne Destaura		C. Storage rack anchorage installation.	Periodic	SI	ANSI/MH16.1 Section 7.3.2; Table 1705A.13.7	Test or Special Inspection
a. Examine structural steel surface conditions, inspect	Periodic SI	Code References and Notes 1705A.15, 1705A.1, 1705A.2, 1705A.3, 1705A.4.	 d. Completed storage rack system to indicate compliance 	Periodic	SI*	Table 1705A.13.7; * May be preformed by the project inspector when	a. Load test for identified product(s):
application, take samples, measure thickness and verify compliance of all aspects of application with DSA-			with the approved construction documents.			specifically approved by DSA.	b Installation torque for non HS holts
approved documents.	T I I OD		S/A11. Other Steel		\rightarrow		
D. Test density.	Test LOR	1705A.15.1, 1705A.15.5, ASTM E730	Test or Special Inspection	Туре	Performed By	Code References and Notes	
			□ a.				
S/A9. ANCHOR BOLTS AND ANCHOR RODS:							□ c.
Test or Special Inspection	Type Performed F	By Code References and Notes					
a. Anchor Bolts and Anchor Rods	Test LOR	Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-11.					
b. Threaded rod not used for foundation anchorage.	Test LOR	Sample and test threaded rods not readily identifiable per procedures					
S/A10. STORAGE RACK SYSTEMS:							
Test or Special Inspection	Type Performed E	By Code References and Notes					
Test or Special Inspection a. Materials used, to verify compliance with one or more of the material test reports in accordance with the	Type Performed I Periodic SI	By Code References and Notes Table 1705A.13.7					
 Test or Special Inspection a. Materials used, to verify compliance with one or more of the material test reports in accordance with the approved construction documents. b. Fabricated storage rack elements 	Type Performed I Periodic SI Periodic SI	By Code References and Notes Table 1705A.13.7					
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Test or Special Inspection a. Materials used, to verify compliance with one or more of the material test reports in accordance with the approved construction documents. b. Fabricated storage rack elements. DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022) Appendix: Work Exempt from DSA Requirem Application Number: School Name: 04-122188 PSA File Number: Increment Number: DSA File Number: Increment Number: DSA File Number: Increment Number: Exempt items given in DSA IR A-22 or the 2019 CBC (in design professional are NOT subject to DSA requireme be identified on the approved construction docum construction documents. SOILS: 1 1. Deep foundations acting as a cantilever footing wigeotechnical report for the following cases: A) free st poles, flag poles, poles supporting open mesh fences or D) covered walkway structure with an apex height 2. Shallow foundations, etc. are exempt from special a geotechnical report and meeting the exception itee (not exceeding 12" depth per CBC Section 1804A.6), exterior non-structural flatwork (e.g., sidewalks, site or areas, or E) utility trench backfill. CONCRETE/MASONRY: 1. Post-installed anchors for the following: A) exempt item 7 for "Welding" in the Appendix below) given in wall partitions meeting criteria listed in exempt item	Type Performed Periodic SI Periodic SI Periodic SI DEPARTMENT OF GENERAL SE Page 13 of 19 nents for Structural T Icluding DSA amendment ents for the structural tests ents. The project inspector th a design based on minim anding sign or scoreboard, F s, etc.), C) single-story structu less than 10'-0" above adjac inspections and testing by a m #1 criteria in CBC Section 3) soil scarification/recompa- oncrete ramps, site stairs, pa non-structural components CBC Section 1617A.1.18 (wh 3 for "Welding" in the Apper	By Code References and Notes Table 1705A.13.7 1704A.2.5; Table 1705A.13.7 IRVICES State of CALIFORNIA rests / Special Inspections School District: PC Update Date Created: 2023-04-19 08:36:32 s) and those items identified below with a check mark by the / special inspections noted. Items marked as exempt shall or shall verify all construction complies with the approved um allowable pressures per CBC Table 1806A.2 and without a 3) cell or antenna towers and poles less than 35'-0" tall (e.g., lighting the with dead load less than 5 psf (e.g., open fabric shade structure), tent grade. Geotechnical Engineer for the following cases: A) buildings without 1803A.2 supported by native soil (any excavation depth) or fill soil ction not exceeding 12" depth, C) native or fill soil supporting arking lots, driveways, etc.), D) unpaved landscaping and playground arking lots, driveways, etc.), D) unpaved landscaping and playground	Appendix: Work Exempt from DSA Requirem Application Number: School Name: 04-122188 PC Update DSA File Number: Increment Number:	nents for St s given in CBC S mum leaf span cated above ci oof. amps associate 1705A.2.1); fille panning less th onry, stone, or a member sha rolled or cold- ions of such fra 'or S/A5 of listin on, etc.) for me	tructural Te Section 1910A.2 section 1910A.2 of 10', and gate irculation or occ ed with walking et welds shall no han 15'-0", such terra cotta vene ill not exceed th formed steel (i.e ames to superst ng above).	sts / Special Inspections School District: PC Update Date Created: 2023-04-19 08:36:32 subject to the requirements and limitations subject to the requirements and limitations swith a maximum rolling section of 10' all having an apex height upied space below, these gates/fences are not located within 1.5x surfaces less than 30" above adjacent grade (excluding post base t be ground flush. as in interior partitions, interior soffits, etc. supporting only selfer on more than 5/8" thickness and apex less than 20'-0" in height e equivalent of that occurring from a 10'x10' opening in a 15' tall, light gauge) for mechanical, electrical, or plumbing equipment ructure elements using welding will require special inspection as cal, or plumbing hanger support and bracing (connections of such particular to the such as the such	DSA 103-22: LISTING OF STRUCTURAL T Application Number: School Name: 04-122188 School Name: DSA File Number: DSA File Number: Name of Architect or Engineer in general responsible charge: Increment Number Name of Structural Engineer (When structural design has been deleged) Signature of Architect or Structural Engineer: Note: To facilitate DSA electronic mark-ups and identities Structural Engineer
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Geotechnical Engineer for the following cases: A) buildings without t803A.2 supported by native soil (any excavation depth) or fill soil supporting arking lots, driveways, etc.), D) unpaved landscaping and playground arking lots, driveways, etc.), D) unpaved landscaping and playground landscaping and playground structurel to the requirements and limitations roys testing and special inspection items as allowed per DSA each applicable wall condition.	Appendix: Work Exempt from DSA Requirem Application Number: School Name: 04-122188 PC Update DSA File Number: Increment Number: CONCRETE/MASONRY: 5. Testing of reinforcing bars is not required for items in that section. WELDING: 1. Solid-clad and open-mesh fences, gates with maxir less than 8'-0" above lowest adjacent grade. When logate/fence height (max 8'-0") to the edge of floor or r 2. Handrails, guardrails, and modular or relocatable ra connections per the 'Exception' language in Section 1' 3. Non-structural interior cold-formed steel framing s weight and light-weight finishes or adhered tile, mas and not over an exit way. Maximum tributary load to wall for a header or king stud. 4. Manufactured support frames and curbs using hot weighing less than 2000# (equipment only) (connect noted in selected item(s) for Sections S/A3, S/A4 and/ 5. Manufactured components (e.g., Tolco, B-Line, Afco components to superstructure elements using weldir of listing above). 6. TV Brackets, projector mounts with a valid listing (s etc.) (connections of such elements to superstructure s/A3, S/A4 and/or S/A5 located in the Steel/Aluminur	nents for St s given in CBC S given in CBC S mum leaf span cated above ci oof. amps associate 1705A.2.1); fille panning less th onry, stone, or a member sha rolled or cold- ions of such fra 'or S/A5 of listin on, etc.) for me ng will require see DSA IR A-5) e elements usin m category of li	tructural Te Section 1910A.2 Section 1910A.2 a of 10', and gate irculation or occ ed with walking et welds shall no han 15'-0", such terra cotta vene ill not exceed th formed steel (i.e ames to superst ng above). echanical, electri special inspecti and recreationant isting above).	sts / Special Inspections School District: PC Update Date Created: 2023-04-19 08:36:32 subject to the requirements and limitations swith a maximum rolling section of 10' all having an apex height upied space below, these gates/fences are not located within 1.5x surfaces less than 30" above adjacent grade (excluding post base t be ground flush. as in interior partitions, interior soffits, etc. supporting only self ser no more than 5/8" thickness and apex less than 20'-0" in height e equivalent of that occurring from a 10'x10' opening in a 15' tall at ing the gauge) for mechanical, electrical, or plumbing equipment ructure elements using welding will require special inspection as cal, or plumbing hanger support and bracing (connections of such on as noted in selected item(s) for Sections 5/A3, 5/A4 and/or 5/A5 al equipment (e.g., playground structures, basketball backstops, equire special inspection as noted in selected item(s) for sections	DSA 103-22: LISTING OF STRUCTURAL Application Number: 04-122188 DSA File Number: Name of Architect or Engineer in general responsible charge: Name of Structural Engineer (When structural design has been deleter) Signature of Architect or Structural Engineer: Note: To facilitate DSA electronic mark-ups and identitive in the structural in the stru

DEPARTMENT OF GENERAL SERVICES Page 16 of 19

STATE OF CALIFORNIA

DIVISION OF THE STATE ARCHITECT DGS DSA 103-22 (Revised 12/01/2022)

PC Update
Increment Number:

Inspection	Туре	Performed By	Code References and Notes	
ELDING (IN ADDITION TO SECTION S/A3):				
Inspection	Туре	Performed By	Code References and Notes	
ve welds, multi-pass fillet welds, single pass ′16", plug and slot welds.	Continuous	SI	Table 1705A.2.1 Items 5a.1–4; AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.	
e-pass fillet welds ≤ 5/16″.	Periodic	SI	Table 1705A.2.1 Item 5a.5; AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.	
welded studs (ASTM A-108) installation d test).	Periodic	SI	2213A.2; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1; DSA IR 17-3.	
and roof deck welds.	Periodic	SI	1705A.2.2, Table 1705A.2.1 Item 5a.6; AISC 360-16 (AISC 341-16 as applicable); AWS D1.3; DSA IR 17-3.	
ing of structural cold-formed steel.	Periodic	SI*	1705A.2.5; AWS D1.3; DSA IR 17-3. The quality control provisions of AISI S240-20 Chapter D shall also apply. * May be performed by the project inspector when specifically approved by DSA.	
ing of stairs and railing systems.	Periodic	SI*	1705A.2.1; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3. * May be performed by the project inspector when specifically approved by DSA.	
of reinforcing steel weldability.	Periodic	SI	1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.	
ing of reinforcing steel.	Continuous	SI	Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8: AWS D1.4: DSA IR 17-3.	

following: A) when supported on a floor/roof, <400# and resulting composite center of mass (including component's center of mass) ≤4' above supporting floor/roof, B) when hung from a wall or roof/floor, <20# for discrete units or <5 plf for distributed systems.

STATE OF CALIFORNIA





RG30 - PIER				
8 height - End Columns	8' height - End Columns	8' height - End Columns	8' End Columns	
Soil Class 5 - 1500 ps f Bearing	Soil Class 4 - 2000 psf Bearing	Soil Class 3 - 3000 psf Bearing	Tension Rebar Dimensions V	eld
Vertical	Vertical	Vertical		let
Dia (A) Depth (B) Rebar Rebar	Dia (A) Depth (B) Rebar Reba	r Dia (A) Depth (B) Rebar Rebar	r X(in) Y (in) Rebar Size V	eld e to the
(in) (in) Qty Size	(in) (in) Qty Size	(in) (in) Qty Size		
30 122 8 6	30 104 8 6	30 94 8 6	11 47 6	/16
8' height - Side Columns	8' height - SIde Columns	8' height - Side Columns	8' Side Columns	SEISMIC THE HOOK D
Soil Class 5 - 1500 psf Bearing	Soil Class 4 - 2000 psf Bearing	Soil Class 3-3000 psf Bearing	Tension Rebar Dimensions V	eld
Vertical	Vertical	Vertical		let
Dia (A) Depth (B) Rebar Rebai	Dia (A) Depth (B) Rebar Reba	r Dia (A) Depth (B) Rebar Rebar	r X (in) Y (in) Rebar Size V	
(in) (in) Qty Size	(in) Qty Size	(in) (in) Qty Size		
48 138 12 8	36 132 8 8	36 118 8 8	17 65 8	
8' Eave - 1500 psf []	8' Eave - 2000 psf []	8' Eave - 3000 psf []	8' Eave - Rebar & Weld	
10' height - End Columns	10' height - End Columns	10' height - End Columns	10' End Columns	
Soil Class 5 - 1500 psf Bearing	Soil Class 4 - 2000 psf Bearing	Soil Class 3 - 3000 psf Bearing	Tension Rebar Dimensions V	VERTICAL TENSION TIE
Vertical	Vertical	Vertical		et ONE ON EACH SIDE OF EACH ANCHOR
Dia (A) Depth (B) Rebar Rebai	Dia (A) Depth (B) Rebar Reba	r Dia (A) Depth (B) Rebar Rebar	r X (in) Y (in) Rebar Size V	
(in) (in) Oty Size	(in) (in) Qty Size	(in) (in) Qty Size		N" BASE PLATE / W / Y
30 116 8 6	30 100 8 6	30 90 8 6	11 59 7	/16
10' height - Side Columns 🔪	10' height - SIde Columns	10' height - Side Columns	10' Side Columns	
Soil Class 5 - 1500 psf Bearing	Soil Class 4 - 2000 psf Bearing	Soil Class 3 - 3000 psf Bearing	Tension Rebar Dimensions V	
Vertical	Vertical	Vertical		
Dia (A) Depth (B) Bebar Rebai	Dia (A) Depth (B) Rebar Reba	r Dia (A) Depth (B) Rebar Rebar	r X (in) Y (in) Rebar Size V	
(in) (in) Qty Size	(in) (in) Qty Size	(in) (in) Qty Size		
42 138 10 8	36 124 8 8	36 112 8 8	17 65 8	
10' Eave - 1500 psf []	10' Eave - 2000 psf [)	10' Eave - 3000 psf [🗙]	10' Eave - Rebar & Weld	
12' height - End Columns	12' height - End Columns	12' height - End Columns	12' End Colymns	
Soil Class 5 - 1500 psf Bearing	Soil Class 4 - 2000 psf Bearing	Soil Class 3 - 3000 psf Bearing	Tension Rebar Dimensions V	
Vertical	Vertical	Vertical		
Dia (A) Depth (B) Rebar Rebar	Dia (A) Depth (B) Rebar Reba	r Dia (A) Depth (B) Rebar Rebar	r X (in) Y (in) Rebar Size V	
(in) (in) Qty Size	(in) (in) Qty Size	(in) (in) Qty Size		V"
30 122 8 6	30 104 8 6	30 94 8 6	13 59 7	/16
12' height - Side Columns	12' height - SIde Columns	12 th eight - Side Columns	12' Side Columns	
Soil Class 5 - 1500 psf Bearing	Soil Class 4 - 2000 psf Bearing	Soil Class 3-3000 psf Bearing	Tension Rebar Dimensions V	eld I
Vertical	Vertical	Vertical		let
Dia (A) Depth (B) Rebar Rebar	Dia (A) Depth (B) Rebar Reba	r Dia (A) Depth (B) Rebar Rebar	T X (in) Y (in) Rebar Size V	eld Footing
(in) (in) Qty Size	Tin) (in) Qty Size	(in) (in) Qty Size		<u>V"</u>
36 140 8 8	36 120 8 8	36 108 8 8		<u>SEE DETAILS BP1, BP2 OR BF</u>
	1 101- 0000 11 1	1 4015 0000 (1 1		

PRINTED ON :





____ 0 PLAN - PURLIN & MID RAFTER CONNECTIONS @ MID COLUMN ₹4° 3⁄4 ؼ"HOLE — 15⁄8" —► -(7) PLCS 0 0 0 BEND LINE — 6⁷⁄8"· PURLIN PLATE DETAIL - 14GA 143.13° 1/2" — -SECTION PROPERTIES in^2 Fy=36 ksi lx=0.087 in^4 Sx=0.03 in^3 ly=2.92 in⁴ Sy=0.97 in³ RIDGE BEAM CLIF (6" WIDE BEAM)

PRELIMINARY IVIL ENGINEERING • PLANNING CONSTRUCTION ENGINEERING BY: _____NOT FOR CONSTRUCTION

PLOTTED: Saturday, March 22, 2025 1:24:55 PM

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W.O. 20007 2007_SHELTER GRADING EXHIBIT.DWG

DATE