BULK STORAGE BUILDING

Liberty Public School District

1138 Southview Drive Liberty, MO

CONSTRUCTION DOCUMENTS

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C200 SITE DETAILS

ARCHITECTURE
A101 FLOOR PLAN, ELEVATIONS & DETAILS

STRUCTURAL
S001 GENERAL NOTES
S101 FOUNDATION PLAN
S300 SECTIONS

TYPICAL CONCRETE DETAILS

PHONE: 913.317.9390

STRUCTURAL ENGINEER:
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1828 Walnut Street Ste 922
Kansas City, MO 64108
CONTACT: Lily Riehl, PE
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PHONE: 816.442.7700
FAX: 816.599.2545

ARCHITECT:

1828 Walnut Street Ste 922

CONTACT: Rick Sander, AIA

EMAIL: RSander@HollisandMiller.com

Kansas City, MO 64108

PHONE: 720.949.1689

CIVIL ENGINEER:

11827 W. 112th St. Ste 200

CONTACT: Braden Taylor, PE EMAIL: BTaylor@MKEC.com

Overland Park, KS 66210

FAX: 816.599.2545

Bulk Storage Building Liberty Public Schools 1138 Southview Drive, Liberty, MO Bulk Storage Building Liberty Public Schools Bulk Storage Building Liberty Public Schools Bulk Storage Building Liberty Public Schools Date

we design the future

115 Wilcox Street Suite 210

HOLLISANDMILLER.COM

PROJECT INFORMATION
PROJECT NUMBER: 21022, Work Order: 93061

PROJECT NAME:

Liberty Bulk Storage Building

AUTHORITY HAVING JURISDICTION:

Building Division - City of Libety, MO 101 E. Kansas Street Liberty, MO 64068

RESPONDING FIRE SERVICE:

Liberty Fire Department
101 E. Kansas Street
Liberty, MO 64068

ANTICIPATED OCCUPANCY: August, 2023

APPLICABLE ADOPTED CODES:

OCCUPANCY CLASSIFICATION:

CONSTRUCTION TYPE:

II B

SPRINKLERED PER NFPA 13 REQ'D. (903.2.9)?

No

CODE INFORMATION

ALLOWABLE HEIGHT (TABLE 504.3): 55 FT. (
BUILDING HEIGHT: 29 FT. (

ALLOWABALE STORIES (TABLE 504.4): 2 (S-1 C

BUILDING STORIES: 1 (Comp.)

FIRE RESISTANCE RATINGS(TABLES 601, 705.5): 0-Hour

ALLOWABLE BUILDING AREA (TABLE 506.2): 17,500 Stories (Section 1): 17,500 Stories (Section 1): 17,500 Stories (Section 1): 17,500 Stories (Comp.)

BUILDING AREA:

2 (S-1 OCC., NS) 1 (Complies) 705.5): 0-Hour 17,500 SF (S-1 OCC., NS) 1,622 SF (Complies)

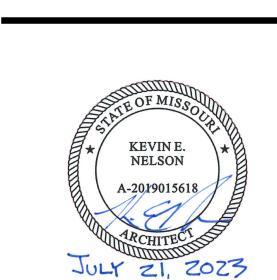
55 FT. (S OCC., NS)

29 FT. (Complies)

VICINITY MAP



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JOB NO: 21022 DRAWN BY: RDS CHECKED BY: JB DATE: 07/21/2023

G000

COVER SHEET

ii dii

BRADEN L. TAYLOR LICS# 2021001896

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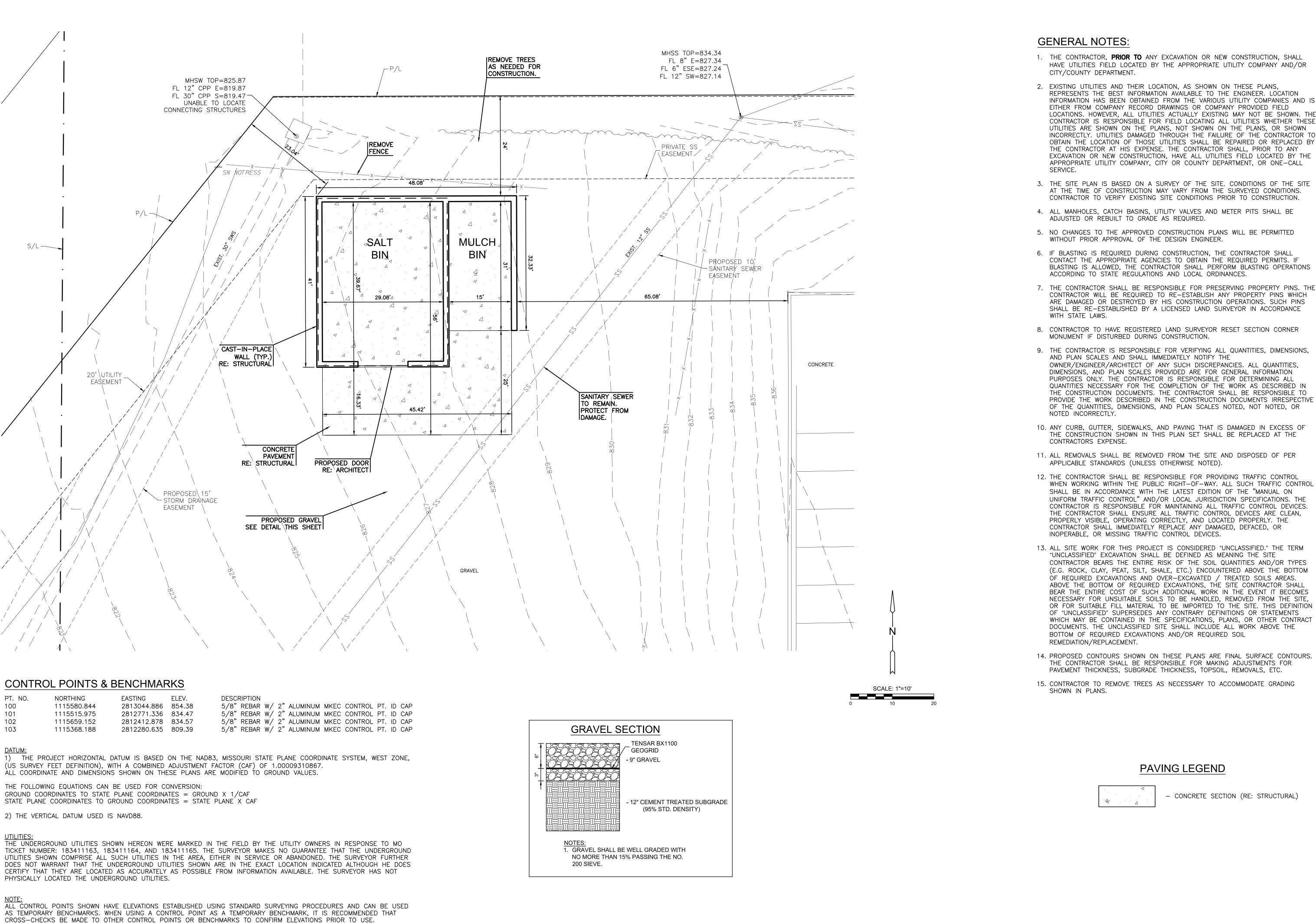
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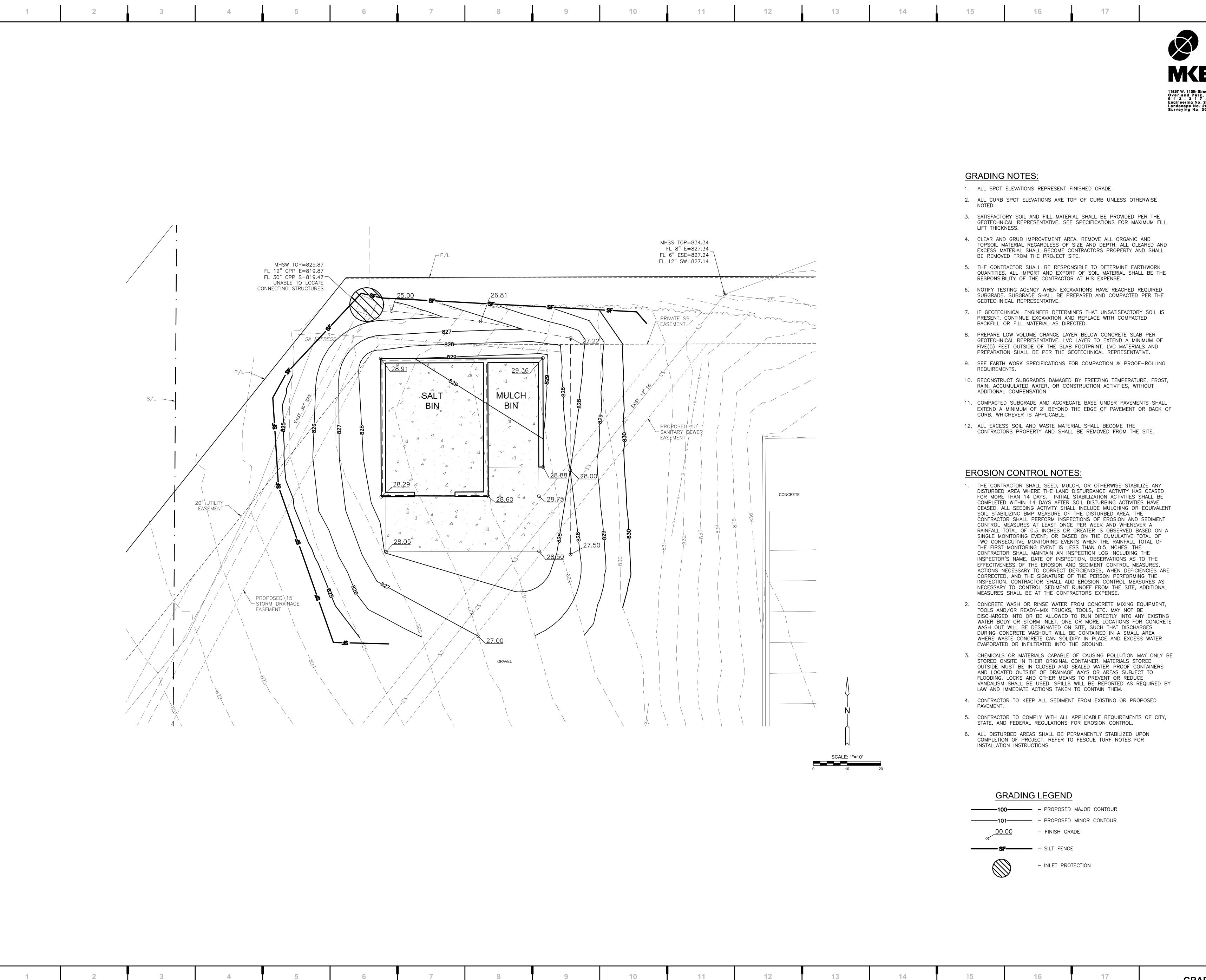
Description Date

PE-2021001896

JOB NO: 21022

SITE DIMENSION PLAN





MKEC 11827 W. 112th Street, Suite 200 Overland Park, KS 66210 9 1 3 . 3 1 7 . 9 3 9 0 Engineering No. 2001009364 Landscape No. 2006027139 Surveying No. 2006027138

we design the future 1828 Walnut Street Suite 922 Kansas City, MO 64108

т 816.442.7700 115 Wilcox Street Suite 210 Castle Rock, CO 80104 т 720.949.1689

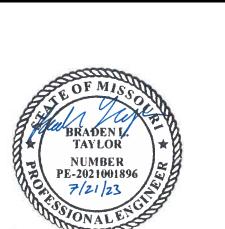
HOLLISANDMILLER.COM

Hollis + Miller Architects Missouri State Certificate of Authority Architecture # 0000161 Structure # 2006031333

MKEC Engineering, Inc. Civil Engineering
State Certificate of Authority #: Engineering: 2001009364 11827 W. 112th St, Ste 200

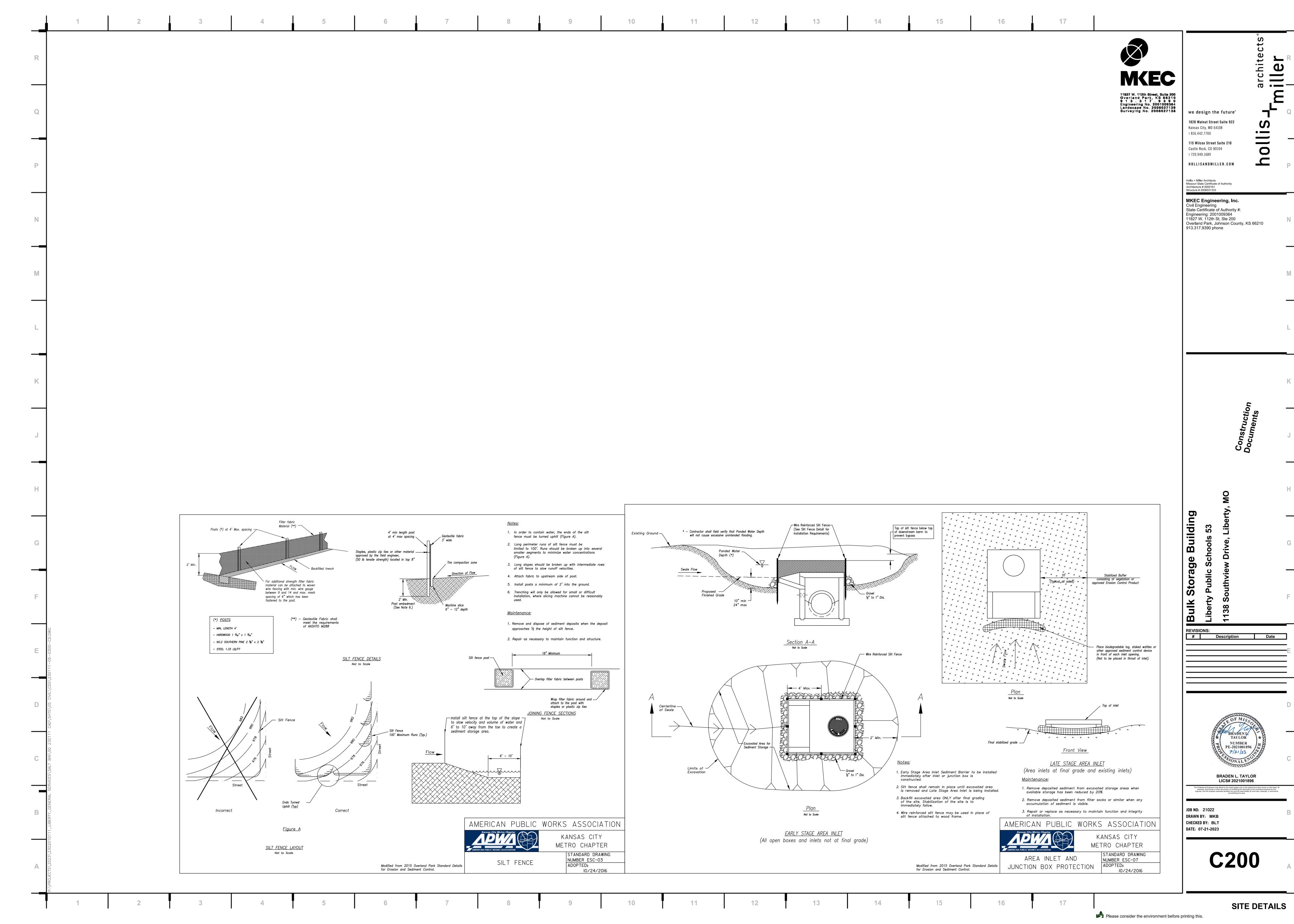
Overland Park, Johnson County, KS 66210 913.317.9390 phone

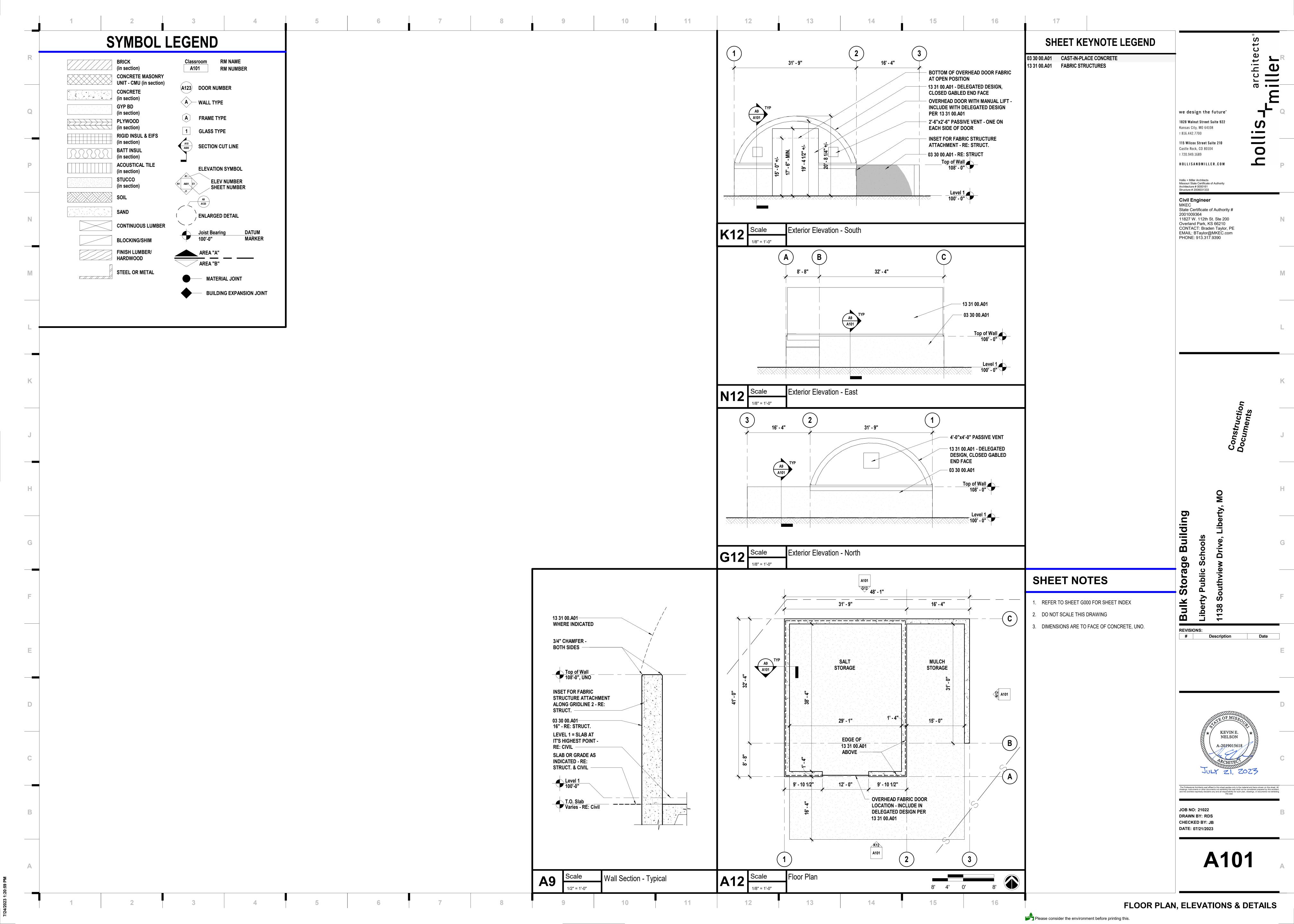
Description Date



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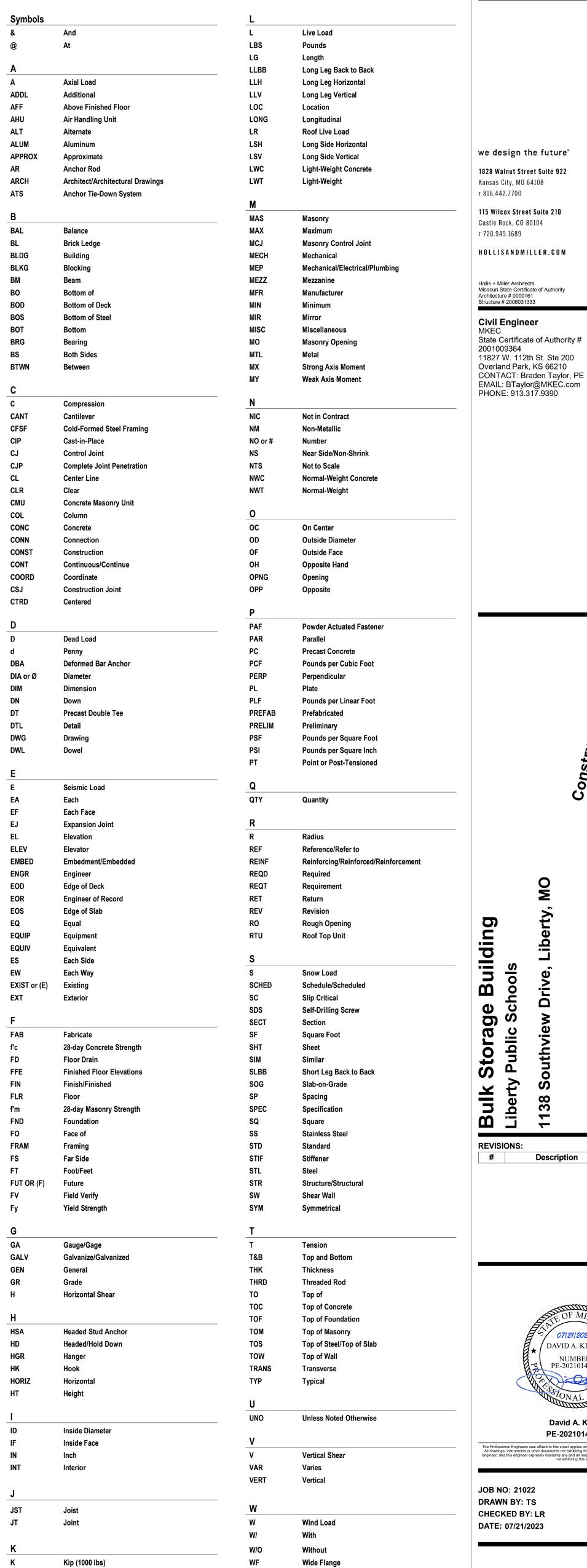


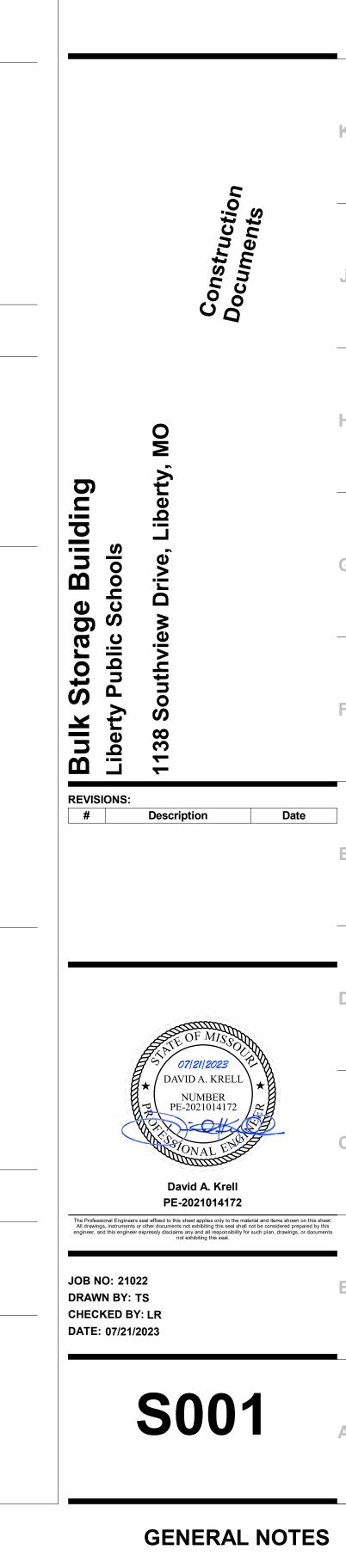
		1 2	3 4		5		6	6		7	8		1	9		
Α.	Bui 1.	Iding Code The design and construction shall conform to the 2018 City of Liberty, MO.	International Building Code (IBC) as amended by the	•		Concrete c Concrete e	•	id exposed t th or weathe	to earth er #5 and small er #6 and large				•			
В.	Des	sign Loads			6. All		'		· ·		5 's on each side, in ϵ	each corner	of the opening]		
	1.	This project is designed to resist the most critical loads of the code.	resulting from the basic load combinations outlined in section	n 1605	7. The	ne Contractor	shall furnish a	n additional	(5) bars of each		ning. the structural drawii All costs for material			t		
	2.	Dead Loads a. The roof mounted equipment weights used for des submit actual weights for all roof mounted equipment. b. Total service roof dead load:	sign are indicated on the contract documents. The Contractor ent for review by the Engineer. 3 psf (Includes 0.25 psf collateral load)	rshall	incl	cluded in the b		ricate and d	eliver only whe	en ordered by the Er		·				
	3.	Live Loads	o por (morados o.20 por conatoral loda)	E.	Post Ins	nstalled Anch	nors									
-		 a. Code Loads 1. Roof 2. Vertical Skid Loader Wheel Load 3. Herizontal Parrier Load 	12 psf 5385 lb point load over an area of 12"x 6"			l post installed Install expa	d anchors shal ansion anchors	ll be installe	d per the manu	racked concrete at t ufacturers recomme commended standa		ss otherwis	e noted in			
	4.	 Horizontal Barrier Load Snow - The snow load is in accordance with ASCE 7 with wi	6000 lb point load over an area of 12" x 12" ith the following criteria:		b.	The embed	•			oe defined as the dis e anchor is placed b	stance from the surf	ace of the lo	oaded			
		a. Ground snow loadb. Exposure Factor	$p_g = 20 \text{ psf},$ $C_e = 1.0$		3. All		,	•		•	Kwik Bolt 3 or appro	oved equal.				
		c. Importance Factord. Thermal Factore. Roof Slope Factor	$I_s = 1.0$ $C_t = 1.2$ $C_s = 1.0$		4. All	l adhesive and	chors embedd			•	nd capacity of the Hil		00-R V3			
		e. Roof Slope Factor f. Flat Roof Snow Load g. Minimum Snow Load	$p_{f} = 16.8 \text{ psf}$ $p_{m} = 20 \text{ psf}$			thesive Ancho		steel at exte	erior exposed o	conditions						
	5.	Wind - The wind load is in accordance with ASCE 7 wit		-			iii bo stairiioss	Sicol di CAI	choi exposed o	oriditions.						
		a. Basic wind speedb. Allowable Stress Design Wind Speed	V = 110 mph $V_{asd} = 85 \text{ mph}$	F.	Miscella 1 Per		servation by fi	eld renreser	ntatives of Holli	s and Miller Archite	cts, if provided, is so	olely for the	nurnose of			
•		c. Risk Category d. Exposure Catergory e. Internal Pressure Coefficient	Vasd = 0.5 mpn II C ± 0.55		det limi	etermining if the nited site obse	he work of the ervation should	contractor is d not be con	s proceeding in strued as exha	n general accordanc austive or continuou	e with the structural s to check the qualit s in the work of the o	contract do by or quantit	cuments. Thi y of the work,	but		
	6.	f. Components & Cladding Force Seismic - The seismic design is in accordance with the	per code		eng doc	igineer shall nocuments.	not be consider	red inspection	ons, and in no	way relieves the cor	ntractor of any requi	rements of	the contract			
		a. Importance Factorb. Risk Category	I _E = 1.00		per	rmanent brac	cing, and exteri	ior load bea	ring walls (whe	re applicable) are c	tructurally stable unicomplete and have a	chieved the	ir design strer	igth.		
		c. 0.2 sec Spectral Response Accelerationd. 1.0 sec Spectral Response Acceleratione. Soil Site Class	$S_S = 9.5\%$ $S_1 = 6.9\%$ D (Assumed)		furr and	rnishing all ter id/or sequence	mporary bracir ces. Temporar	ng and/or su y bracing pl	ipport that may ans should inc	be required as the lude installation and	ection and constructi result of the contract removal sequencin	tor's constr	uction method	S		
		f. Design 0.2sec Spectral Response Accelerationg. Design 1.0sec Spectral Response Accelerationh. Seismic Design Category	$S_{DS} = 10.1\%$ $S_{D1} = 11.0\%$			0 ,				ork is complete.	gs without receiving	written app	roval from the			
		i. Basic Seismic Force Resisting Systemj. Design Base Shear	Ground Supported Cantilever Walls 23 kips		eng	igineer. Wher	n conflicts occ	ur between	the drawings a	nd specifications, th	ne strictest interpreta	ation shall g	overn.			
		k. Seismic Response Coefficientl. Response Modification Coefficientm. Analysis Procedure	C _S = 0.081 R = 1.25 Equivalent Lateral Force		not res	ot indicate the sponsible for a	method or me all construction	eans of cons n means, m	struction. The o	contractor shall sup lures, techniques, s	tructure, and except ervise and direct the equence, and safety or, subcontractor, or	work and some some some some and some some some some some some some some	shall be solely s and progran	ns.		
C.	Fou	undations			any	y of the work,	, or for the failu	ure of any of	f them to carry	out the work in acco	ordance with the cor	ntract docur	nents.	•		
	1.	Geotechnical Report a. A geotechnical report was not provided for this pro	oject.		The	nese structura	al drawings are	intended to	be utilized as	a complete set of de	the structural work ocuments that repre	sent the bu	ilding's structu	ıral		
	2.	Spread Footings, Trench Footing and Grade Beams a. All shallow foundations have been designed to bear on undisturbed soil or engineered fill for a net allowable bearing pressure of 1500 psf based on presumptive values per IBC table 1806.2.			systems. No single sheet or series of sheets is intended to "stand alone". Typical details may or may not be cut at specific locations throughout the drawings, but are to be applied where required. These structural drawings are intended to be included in a complete set of construction documents, including but not limited to, architectural drawings, and civil drawings. Contractor shall verify coordination of these drawings with contents of above sets specified and only proceed with bidding and											
	 3. Retaining Walls a. Retaining walls have been designed assuming a maximum unrestrained equivalent fluid pressure of 45 psf/ft. b. Retaining walls are designed assuming that a drained, non-hydrostatic condition exists behind the walls. 				construction after such has taken place. 6. All existing field and building conditions shall be verified by the Contractor before any other work shall begin. Coordinate with Engineer of Record regarding any discrepancy with existing building dimensions.											
	4.		ng soil shall attain full design strength prior to any backfill bein is used. Use only hand operated tools for compaction adjacer		a.	Supplemen	ntal Instruction	s (ASIs), St	ructural Supple	emental Drawings (S	nts. This includes a SSD's), and Reques duplication, in any w	ts for Inforn	nation (RFI's).			
D.	Cor	ncrete			C.	drawing tha	at is not origina	al will be rej	ected and retui	rned without review.	oying or electronic so nall review the shop					
	1.	All concrete and reinforcing details shall conform to AC	I 318 and CRSI "Manual of Standard Practice".			the means, other trade	s, methods, tec es and shall inc	hniques, se clude respor	quences, and oneses to all Conf	operations of constr tractor directed que	ruction. The submitta stions. After all asp	al shall be c ects of the (oordinated wit Contractor's re	h all view		
	2.	Strength - The following areas shall have a minimum 28 a. Exterior flatwork concrete: b. Footing and grade beams:	8 day compressive strength: 4000 psi 4000 psi			•	ect or Structura		•		p drawings and thos ring the Contractor's	•	•			
	3	c. Walls:	4000 psi site unless specifically withheld at the batch plant. The workal	hility	d.	project. Pr	rovide the follo	wing design	s shall be signe n calculations fong ng and Connec	or review:	professional enginee	er licensed i	n the State of	the		
	4.	should be attained through the use of water-reducing ag		Dirity	e.	1. Concre	s - Provide the treate Mix Designer Reinforcing	n and Mater	bmittals for revials	riew:						
	••	a. Grade1. Typical reinforcing	ASTM A615, Grade 60		f.	4. Fabric	edded Items (pl c Canopy Supe ons are allowed	erstructure	,	ce the specification	s for timing of subm	ission				
		Table (A5 / S530) unless indicated elsewhere in the	ASTM A706 nent shall be per the Typical Reinforcing Splice Length ne drawings and specifications. Lap welded wire reinforcing or	ne full G.	Special		s (based on 2	•	•	- _F 230411011	g 5, 545111					
		<u> </u>	ASTM A1064 shall be supported on metal chairs specifically designed for so	bil	•	•	ion reports sha pertinent entity			ding Official, Owner	, Architect, Enginee	r, Contracto	r, Sub-Contra	ctor		
		OC. Pulling reinforcing up during concrete pla	shall be supported by metal chairs with a maximum spacing			•	•	•	•	, ,	ht to the attention of I inspector shall noti	•				
		similar conditions located elsewhere on the project e. All synthetic and steel fiber reinforcement shall be	t.	.o will!	•	•	•	•	•		oort delineating that oct documents and a			of the		
							ıll retain specia assist with spec	•		e items listed below	v. The Contractor sh	nall provide	light general la	abor		
						oncrete See Sched	dule of Special	Inspections	Table this she	eet.						
					6. Pos	ost installed A	anchors									
•																

Req'd	Inspection Task	Continuous	Periodio
Yes	Inspect reinforcing steel, including prestressing tendons, and		Х
Yes	2. Inspection of reinforcing steel welding in accordance with Steel Construction section above.		X
Yes	3. Inspection of anchors cast in concrete.		Х
Yes	4. Inspection of anchors post-installed in hardened concrete members.	X	
Yes	5. Verify use of approved design mix.		Х
Yes	6. Prior to placement fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	Х	
Yes	7. Inspect concrete and shotcrete placement for proper application techniques.	X	
Yes	8. Inspect for maintenance of specified curing temperature and techniques.		Х
No	9. Inspection of prestressed concrete:		
No	9.a. Application of prestressing forces	Х	
No	9.b. Grouting of bonded prestressing tendons in the	Х	
No	10. Erection of precast structural members		Х
No	11. Verification of in-situ concrete strength, prior to		Х
Yes	12. Inspection formwork for shape, location and		Х

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GROUT HATCH

BUILDING SECTION CUT OR DETAIL - SHEET NUMBER - ENLARGED DETAIL OR

PLAN NUMBER - SHEET NUMBER

Kips per Square Foot

Kips per Square Inch

Work Point/Waterproofing

Welded Wire Reinforcing

EARTH HATCH

GRAVEL HATCH

CONCRETE HATCH

SLAB STEP SLAB SLOPE

SLOPE ARROW ELEVATION SYMBOL

DRAWING REVISION NUMBER

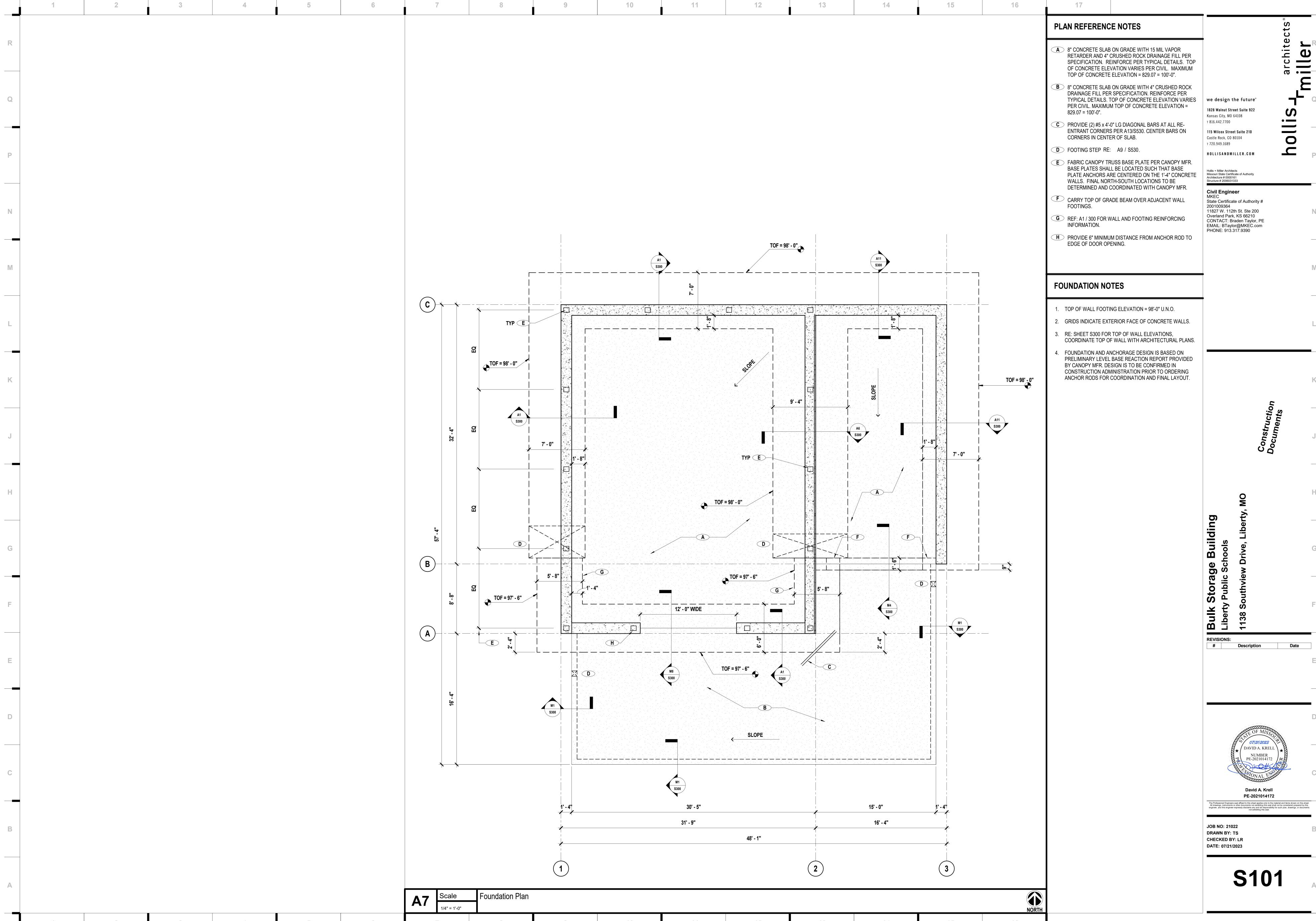
PLAN NOTE

FLOOR OR ROOF

SYMBOLS LEGEND

REVISION CLOUD

WELDED WIRE FABRIC



FOUNDATION PLAN

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