



July 26, 2017
File: 1911.032altr.doc

Alameda Unified School District
2060 Challenger Drive
Alameda, California 94501

Attention: Chad Pimentel, Legal Counsel for AUSD

Re: Geotechnical Engineering Investigation
Evaluation of Liquefaction Risk and Liquefaction Induced Settlement Potential
Henry Haight Elementary School Campus
2025 Santa Clara Avenue
Alameda, California

Introduction

This letter summarizes our geotechnical investigation of the Henry Haight Elementary School Campus located at 2025 Santa Clara Avenue in Alameda, California. The approximate site location is presented on Figure 1, Site Location Map. The purpose of our geotechnical investigation is to evaluate the site soil and groundwater conditions and to assess the liquefaction risk and liquefaction induced settlement potential across the school campus. Our scope includes exploring the subsurface conditions with four Cone Penetration Tests (CPTs), conducting engineering analyses to evaluate the liquefaction risk and liquefaction induced settlement potential, and presentation of our geotechnical conclusions in this letter report.

Site Description

The Henry Haight Elementary School campus is located on the northeasterly side of Santa Clara Avenue, and is bordered on the northeast by Lincoln Avenue, as shown on the Site Location Map, Figure 1. The existing campus consists of numerous permanent and portable buildings, paved driveways, parking areas, and play areas, and landscaping improvements, as shown on the Site Plan, Figure 2. The ground surface at the project site and the surrounding area is characterized by nearly level to slightly sloping terrain.

Regional Geology

The site is located within the Coast Range Geomorphic Province of California. The regional bedrock geology consists of complexly folded, faulted, sheared, and altered sedimentary, igneous, and metamorphic rock of the Franciscan Complex. Bedrock is characterized by a diverse assemblage of greenstone, sandstone, shale, chert, and melange, with lesser amounts of conglomerate, calc-silicate rock, schist and other metamorphic rocks.

The regional topography is characterized by northwest-southeast trending mountain ridges and intervening valleys that were formed by movement between the North American and the Pacific Plates. Continued deformation and erosion during the late Tertiary and Quaternary Age (the last several million years) formed the prominent coastal ridges and the inland depression that is now the San Francisco Bay. The more recent seismic activity within the Coast Range

Geomorphic Province is concentrated along the San Andreas Fault zone, a complex group of generally north to northwest trending faults.

Geologic mapping¹ indicates the site is located in an area underlain by dune sands, as shown on Figure 3. No significant artificial (manmade) fills or soft clay (Bay Mud) underlie the subject site.

Surface Conditions

The site is currently developed as an elementary school campus. The attached Site Plan, Figure 2, shows the locations of existing buildings, driveways, and play areas. Most of the ground surface immediately around the existing buildings consists of asphalt paved surfaces.

Seismicity

The San Francisco Bay Region is located in a seismically active area and the existing improvements will therefore experience the effects of future earthquakes. Such earthquakes could occur on any of several active faults within the region. These faults are shown on the Active Fault Map, Figure 4.

Subsurface Exploration and Laboratory Testing

We explored the subsurface soil and groundwater conditions with four Cone Penetration Tests (CPTs) at the approximate locations shown on the Site Plan, Figure 2. The CPTs were conducted with truck-mounted equipment on March 31, 2017. The CPTs were extended to depths of 26 feet to 49 feet below the ground surface. A schematic of the CPT apparatus is provided on Figure A-1 and a CPT Soil Interpretation Chart is provided on Figure A-2. CPT logs are shown on Figures A-3 through A-6.

Subsurface Conditions

The subsurface conditions are consistent with the mapped geology. Review of subsurface data collected from the CPTs conducted at the site indicate that the campus is generally underlain by approximately six to nine feet of loose to medium-dense dune sand underlain by predominantly medium-dense to dense silty sand and sandy silt extending to a depth of 50 feet or more.

Groundwater was measured at approximately the ground surface during our CPT investigations. It is anticipated that the groundwater level beneath the site is influenced by tidal activity in the nearby San Francisco Bay.

¹ Graymer, R. W., "Geologic Map and Map Database of the Oakland Metropolitan Area, Alameda, Contra Costa, and San Francisco Counties, California", 2000, USGS, MF-2342 Version 1.0., Scale 1:50,000.

Given the low site elevations and proximity to San Francisco Bay, the highest historic groundwater elevation is assumed to coincide with the ground surface.

Liquefaction Risk and Liquefaction Induced Settlement Potential

The project site lies within a California Seismic Hazard Zone of Required Investigation for Liquefaction, as mapped by CGS (2003).

Liquefaction refers to the sudden, temporary loss of soil shear strength during strong ground shaking. Liquefaction-related phenomena include liquefaction-induced settlement, flow failure, and lateral spreading. These phenomena can occur where there are saturated, loose, granular deposits. Recent advances in liquefaction studies indicate that liquefaction can occur in granular materials with a high fines content (35 to 50% clayey and silty materials that pass the #200 sieve) provided the fines exhibit a plasticity less than 7. Granular layers with a potential for liquefaction were observed during our subsurface exploration.

To evaluate soil liquefaction, the seismic energy from an earthquake is compared with the ability of the soil to resist pore pressure generation. The earthquake energy is termed the cyclic stress ratio (CSR) and is a function of the maximum credible earthquake peak ground acceleration (PGA) and depth. The soil resistance to liquefaction is based on the relative density, and the amount and plasticity of the fines (silts and clays). The relative density of cohesionless soil is correlated with Cone Penetration Test data measured in the field.

We analyzed the potential for liquefaction utilizing the CPT Liquefaction Assessment software program CLiq (2007, ver. 2.1.6.9), and the procedures outlined by Idriss and Boulanger (2014). The design seismic conditions consisted of a magnitude 7.3 earthquake producing a PGA of 0.63g, which corresponds to the PGA_M per ASCE 7-10 Section 11.8.3, and assuming groundwater at the ground surface. The results of our liquefaction analyses are presented on Figures 5 through 8, and indicate granular soil layers observed between roughly one and nine feet, and discontinuous lenses between roughly 27 and 36 feet below the ground surface classify as liquefiable during the design seismic event. Therefore, we judge the risk of liquefaction at the site is moderate to high.

Potential liquefaction of sandy layers between one and nine feet below the ground surface may result in ground surface settlement of between roughly 0.5-inches to 1.5-inches, based on the liquefaction analyses discussed above, and as shown on Figures 5 through 8. Potential liquefaction induced differential ground surface settlement within a given building footprint area is estimated to be approximately one half of the total settlement (approximately 0.5 to 1.0-inch).

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Based on procedures outlined by Idriss and Boulanger, 2014, the discontinuous and relatively thin layers of potentially liquefiable soil observed 27-feet to 36-feet below the ground surface in the CPT's may experience 0.5-inch to 1.0-inch of post-liquefaction settlement. However, because there is a significant non-liquefiable soil "cap" overlying these deeper potentially liquefiable soil layers, we utilized the procedures outlined by Youd and Garriss (1995) to determine if post-liquefaction settlement will be manifested in the form of ground surface settlement. As shown on Figure 9, based on the relative thicknesses of the non-liquefiable "cap" and the liquefiable layers, post-liquefaction settlements are not expected to result in ground surface settlement from the potentially liquefiable layers located below a depth of 27-feet.

If you have any questions, or if we can be of further assistance, please call us at your convenience.

Yours very truly,
MILLER PACIFIC ENGINEERING GROUP



Daniel S. Caldwell
Geotechnical Engineer #2006
(Expires 9/30/17)

Attachments: Figures 1 through 9, A-1 through A-6



SITE: LATITUDE, 37.7700°
LONGITUDE, -122.2505°

SITE LOCATION
N.T.S.



REFERENCE: Google Earth, 2017



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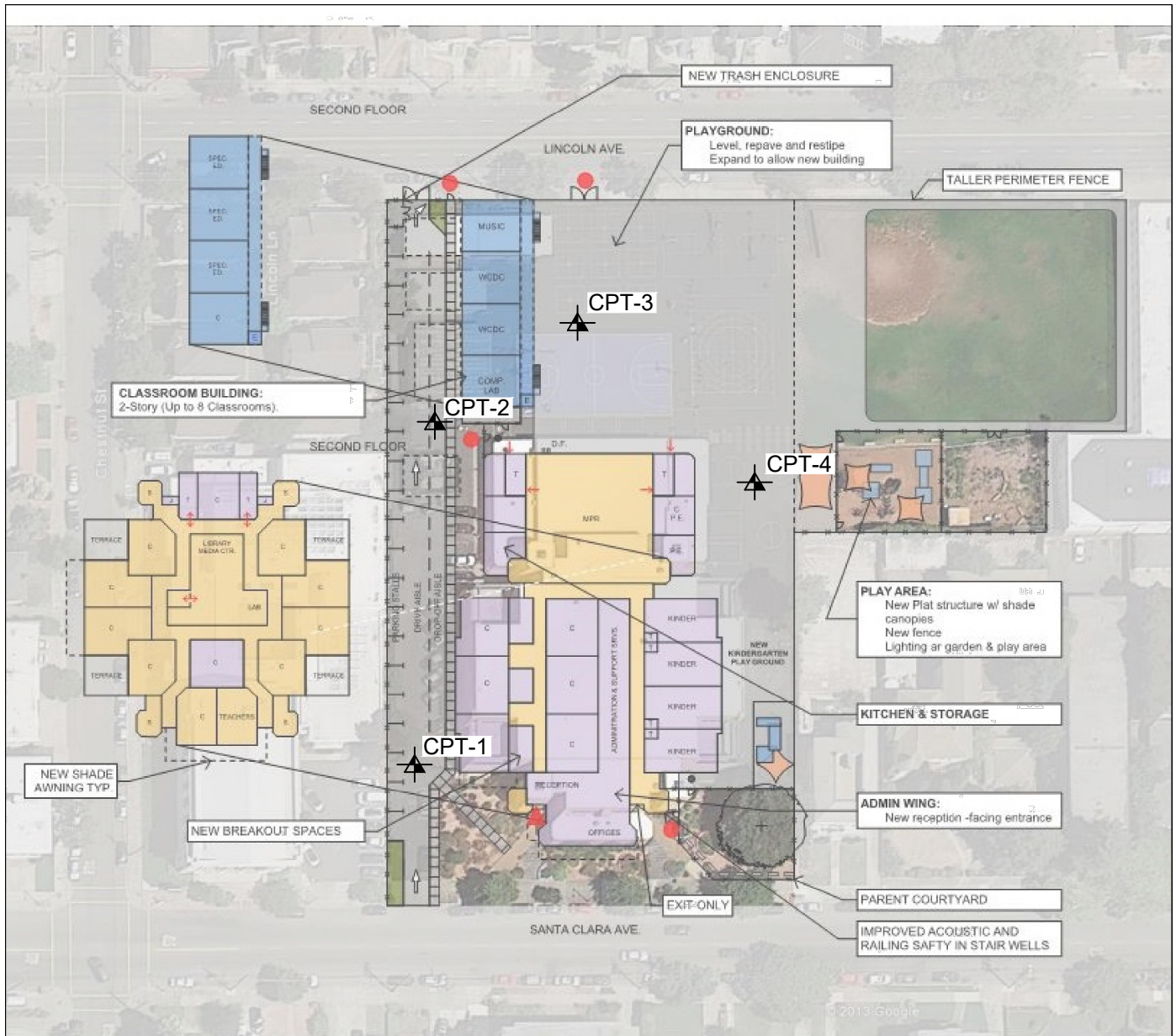
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SITE LOCATION MAP

Henry Haight Elementary School
2025 Santa Clara Avenue
Alameda, California
Project No. 1911.032 Date: 4/7/17

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1
FIGURE

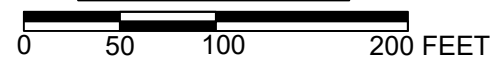


SITE PLAN



Approximate location of CPT completed by MPEG, 2017

APROXIMATE SCALE



REFERENCE: Site Plan provided by QKA.



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SITE PLAN

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2
FIGURE



REGIONAL GEOLOGIC MAP (NOT TO SCALE)



LEGEND

af

ARTIFICIAL FILL (HOLOCENE)

Man made deposit of various materials and ages. Some are compacted and quite firm, but fills made before 1865 are nearly everywhere not compacted and consist simply of dumped materials.

Qds

DUNE SAND (HOLOCENE AND PLEISTOCENE)

Fine-grained, very well sorted, well-drained, eolian deposits. They occur mainly in large sheets, as well as many small hills, most displaying Barchan morphology. Dunes display as much as 30 m of erosional relief and are presently being buried by basin deposits (Qhb) and bay mud (Qhbm). They probably began accumulating after the last interglacial high stand of sea level began to recede about 71 ka, continued to form when sea level dropped to its Wisconsin minimum about 18 ka, and probably ceased to accumulate after sea level reached its present elevation (about 6 ka). Atwater (1982) recognized buried paleosols in the dunes, indicating periods of nondeposition

REFERENCE: Graymer, R.W. (2000), "Geologic Map of the Oakland Metropolitan Area, Alameda, Contra Costa, and San Francisco Counties, California", United States Geological Survey Miscellaneous Field Studies Map MF-2342, Version 1.0, Map Scale 1:50,000.



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REGIONAL GEOLOGIC MAP

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MMT

3

FIGURE



SITE COORDINATES: LAT. 37.7700° LON. -122.2505°

DATA SOURCE:

1) U.S. Geological Survey, U.S. Department of the Interior, "Earthquake Outlook for the San Francisco Bay Region 2014-2043", Map of Known Active Faults in the San Francisco Bay Region, Fact Sheet 2016-3020, Revised August 2016 (ver. 1.1).



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ACTIVE FAULT MAP

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4

FIGURE



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CPT-1 LIQUEFACTION ANALYSIS

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Project No. 1911.032

Date: 4/7/17

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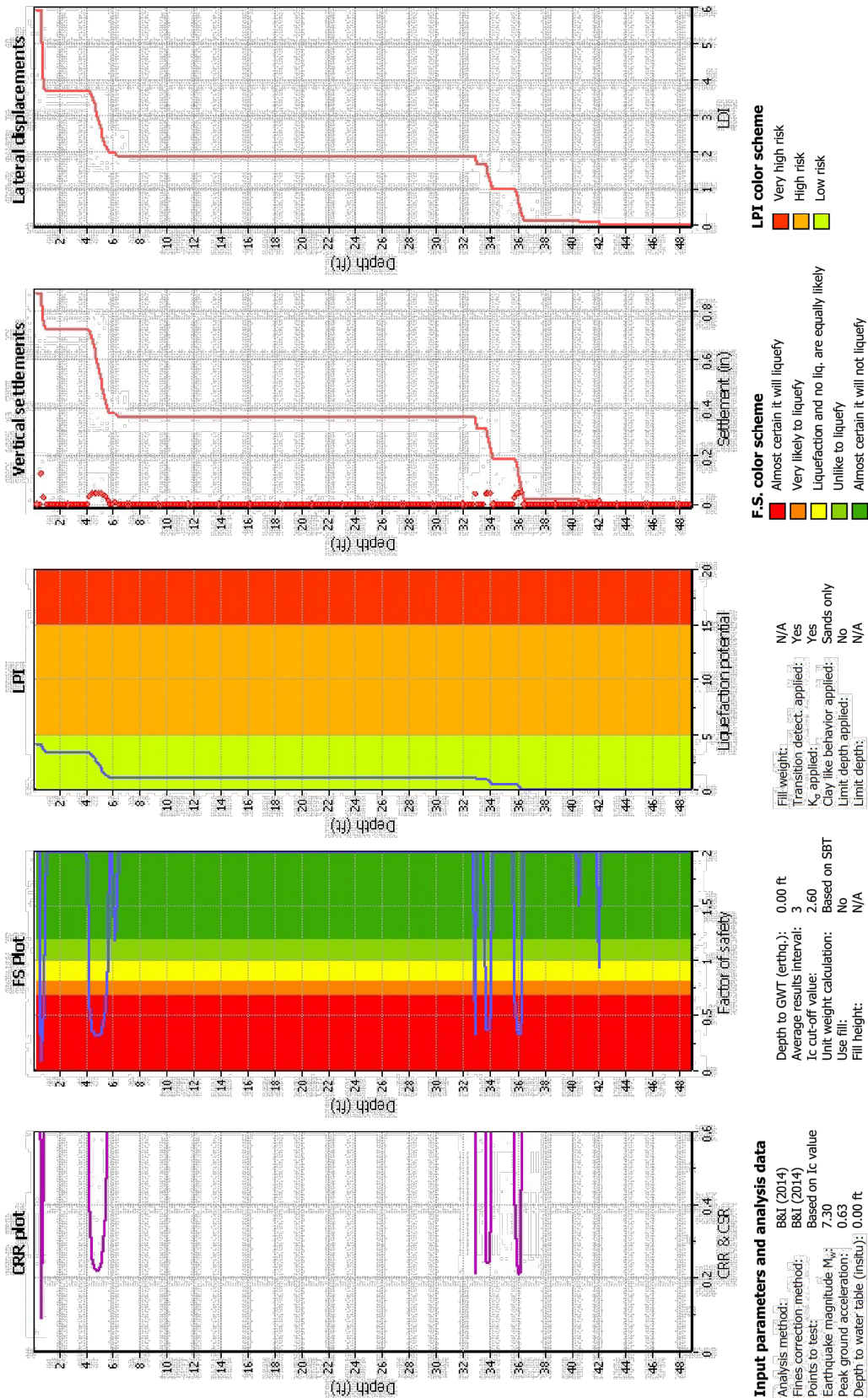
MMT

5
FIGURE

This software is licensed to: Miller Pacific Engineering Group

CPT name: CPT-01

Liquefaction analysis overall plot



CLiQ v.2.1.6.9 - CPT Liquefaction Assessment Software - Report created on: 4/11/2017, 7:57:26 AM
Project file: H:\Jobs\1900-1999\1911.032 Haight ES Classrooms\Analyses\1911.032 CPT\LiQ.dwg

5



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Date: 4/7/17

CPT-2 LIQUEFACTION ANALYSIS

Drawn
Checked

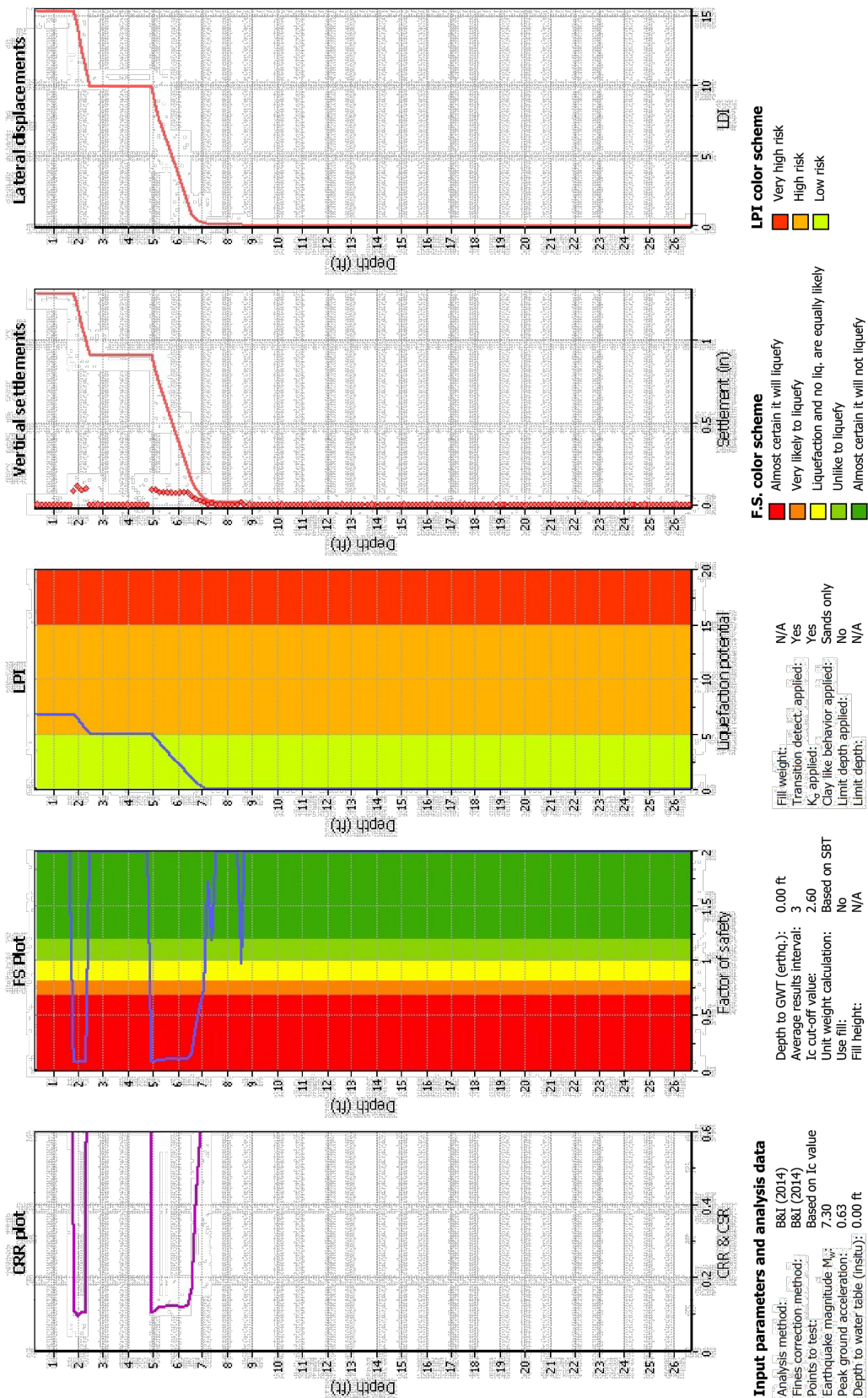
MMT

6
FIGURE

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CPT name: CPT-02

Liquefaction analysis overall plot



CLiQ v.2.1.6.9 - CPT Liquefaction Assessment Software - Report created on: 4/11/2017, 7:57:26 AM
Project file: H:\Jobs\1900-1999\1911.032 Haight ES Classrooms\Analyses\1911.032 CPTLiq.cq



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CPT-3 LIQUEFACTION ANALYSIS

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Date: 4/7/17

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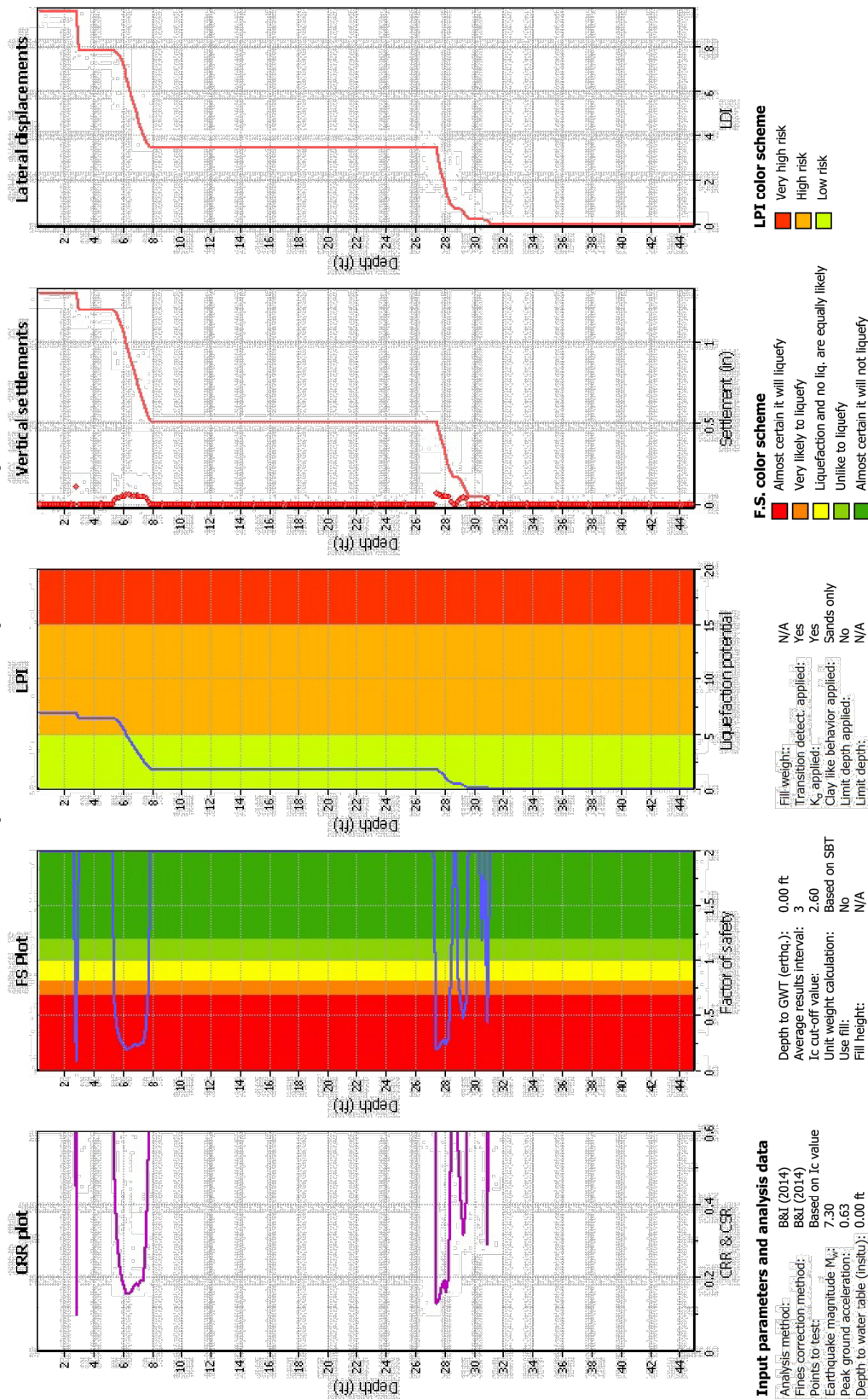
MMT

7
FIGURE

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CPT name: CPT-03

Liquefaction analysis overall plot



Clq v.2.1.6.9 - CPT Liquefaction Assessment Software - Report created on: 4/11/2017, 7:57:27 AM
Project file: H:\00s\1900-1999\1911.032 Haight ES Classrooms\Analyses\1911.032 CPTLiq.dq



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CPT-4 LIQUEFACTION ANALYSIS

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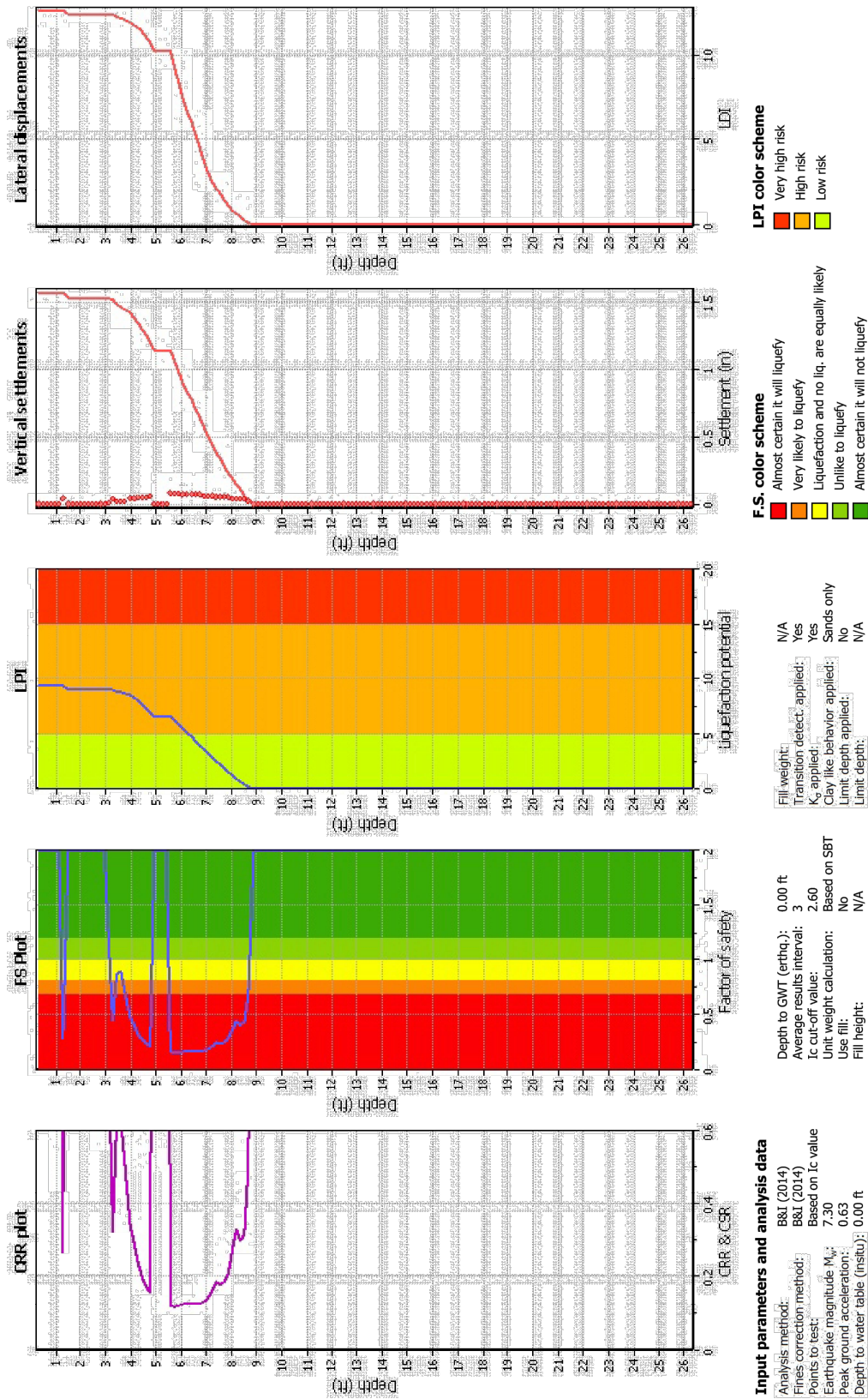
MMT

8
FIGURE

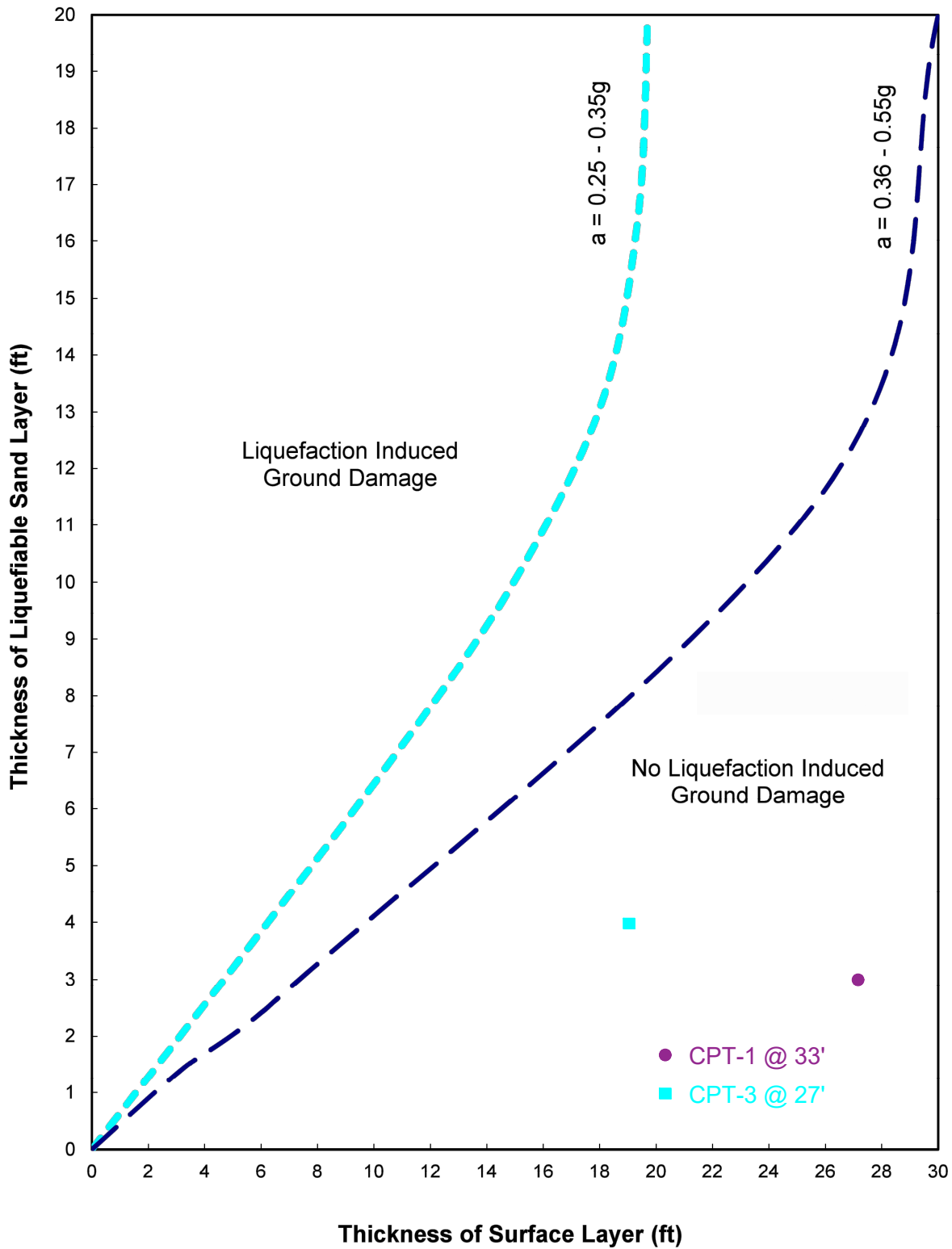
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CPT name: CPT-04

Liquefaction analysis overall plot



Liquefaction-Induced Ground-Surface Distribution
(Youd and Garriss, 1995)



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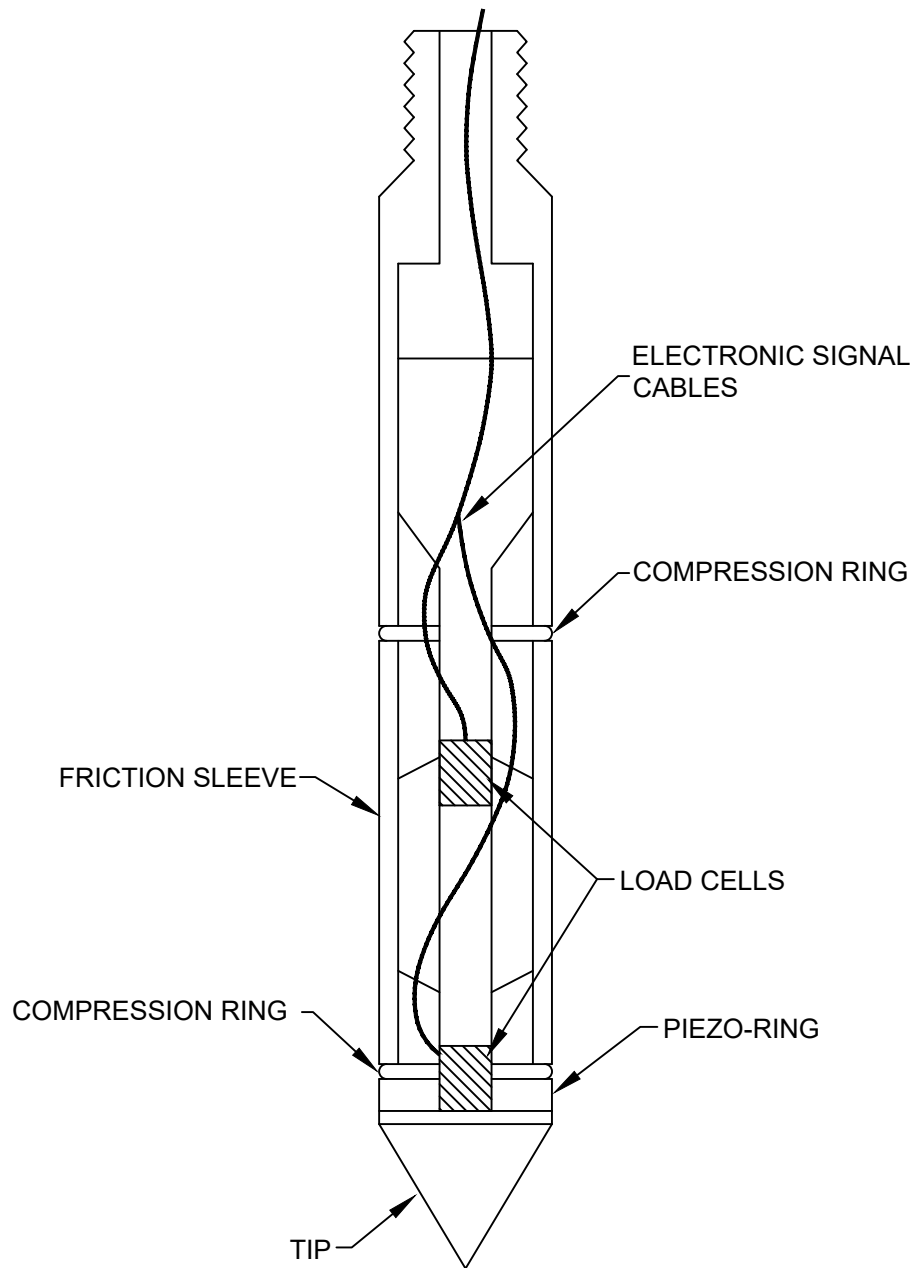
LIQUEFACTION ANALYSIS - SURFACE EFFECTS

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9
FIGURE

APPENDIX A



CONE PENETROMETER

(NO SCALE)



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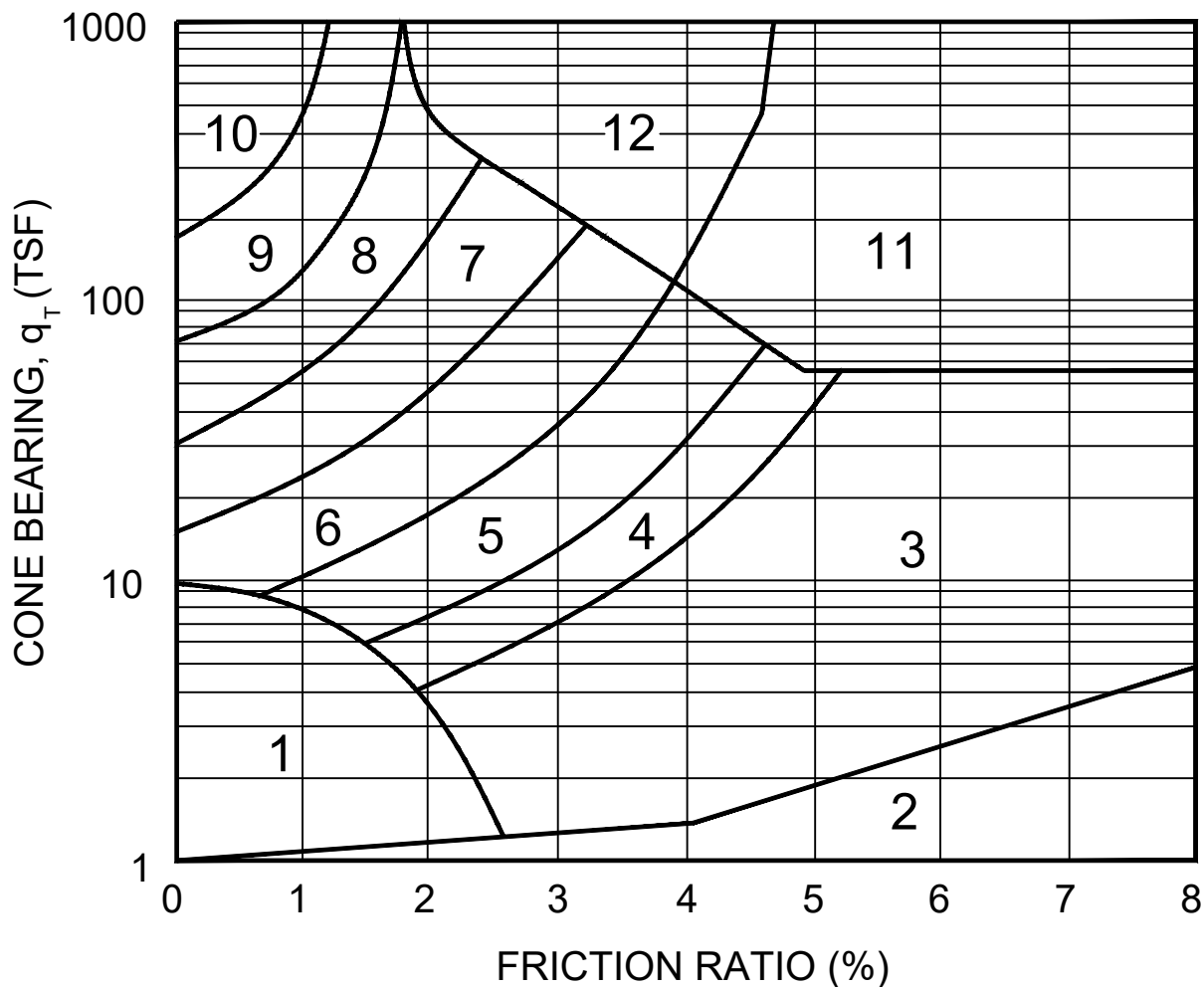
CONE PENETROMETER

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A-1
FIGURE



Zone:	Qc/N	Soil Behavior Type:
1)	2	Sensitive Fine Grained
2)	1	Organic Material
3)	1	Clay
4)	1.5	Silty Clay to Clay
5)	2	Clayey Silt to Silty Clay
6)	2.5	Sandy Silt to Clayey Silt
7)	3	Silty Sand to Sandy Silt
8)	4	Sand to Silty Sand
9)	5	Sand
10)	6	Gravelly Sand to Sand
11)	1	Very Stiff Fine Grained (*)
12)	2	Sand to Clayey Sand (*)

(*) Overconsolidated or Cemented

Reference: Robertson, P.K. (1986), "In-Situ Testing and Its Application to Geotechnical Engineering," Canadian Geotechnical Journal, Vol. 23; No. 23; No. 4, pp. 573-594



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CPT SOIL INTERPRETATION CHART

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A-2
FIGURE

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Project
Job Number
Hole Number
EST GW Depth During Test

Haight Elementary School
2017-8908
CPT-01

Operator
Cone Number
Date and Time

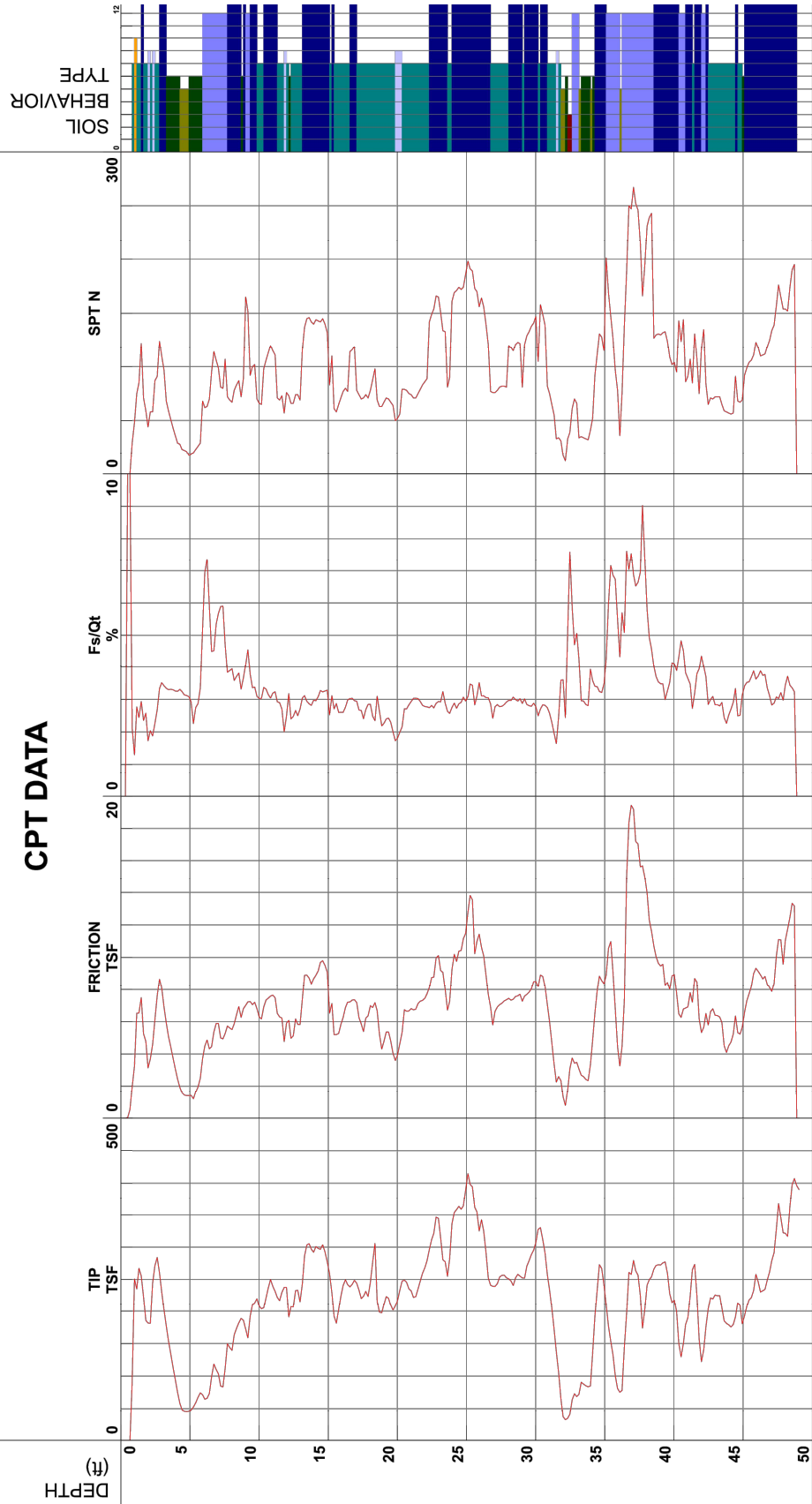
RB KK
DDG1379
3/31/2017 11:04:31 AM

Filename
GPS
Maximum Depth

SDF(019).cpt
49.05 ft

Net Area Ratio .8

CPT DATA



Cone Size 10cm squared



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CPT-1 PLOT

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A-3
FIGURE



Miller Pacific Engineering

Project
Job Number
Hole Number
EST GW Depth During Test

Haight Elementary School
2017-8908
CPT-02

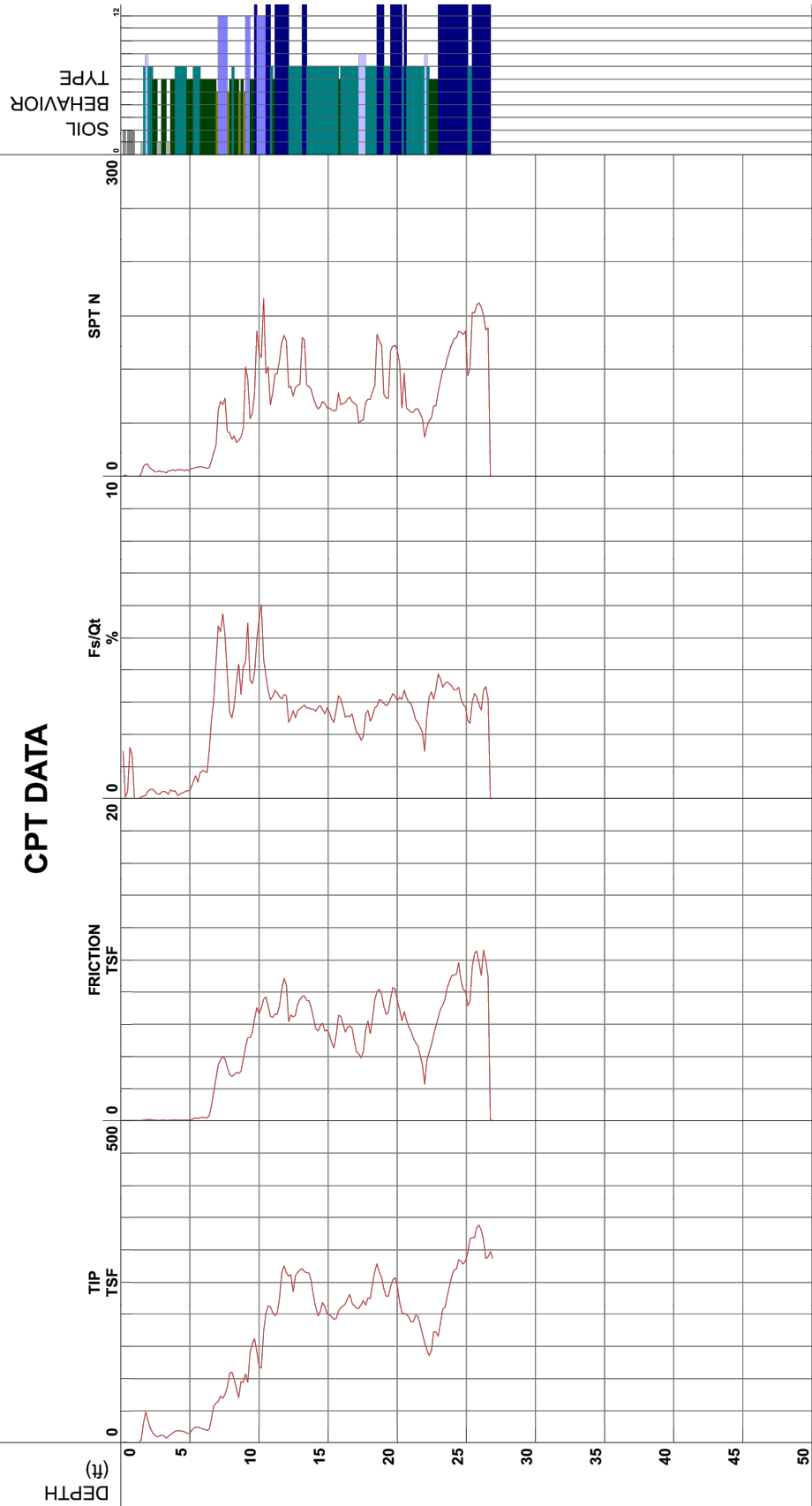
Operator
Cone Number
Date and Time
0.00 ft

Filename
GPS
Maximum Depth

SDF(020).cpt
26.90 ft

Net Area Ratio .8

CPT DATA



S^oSoil behavior type and SPT based on data from UBC-1983

Cone Size 10cm squared



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CPT-2 PLOT

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A-4
FIGURE

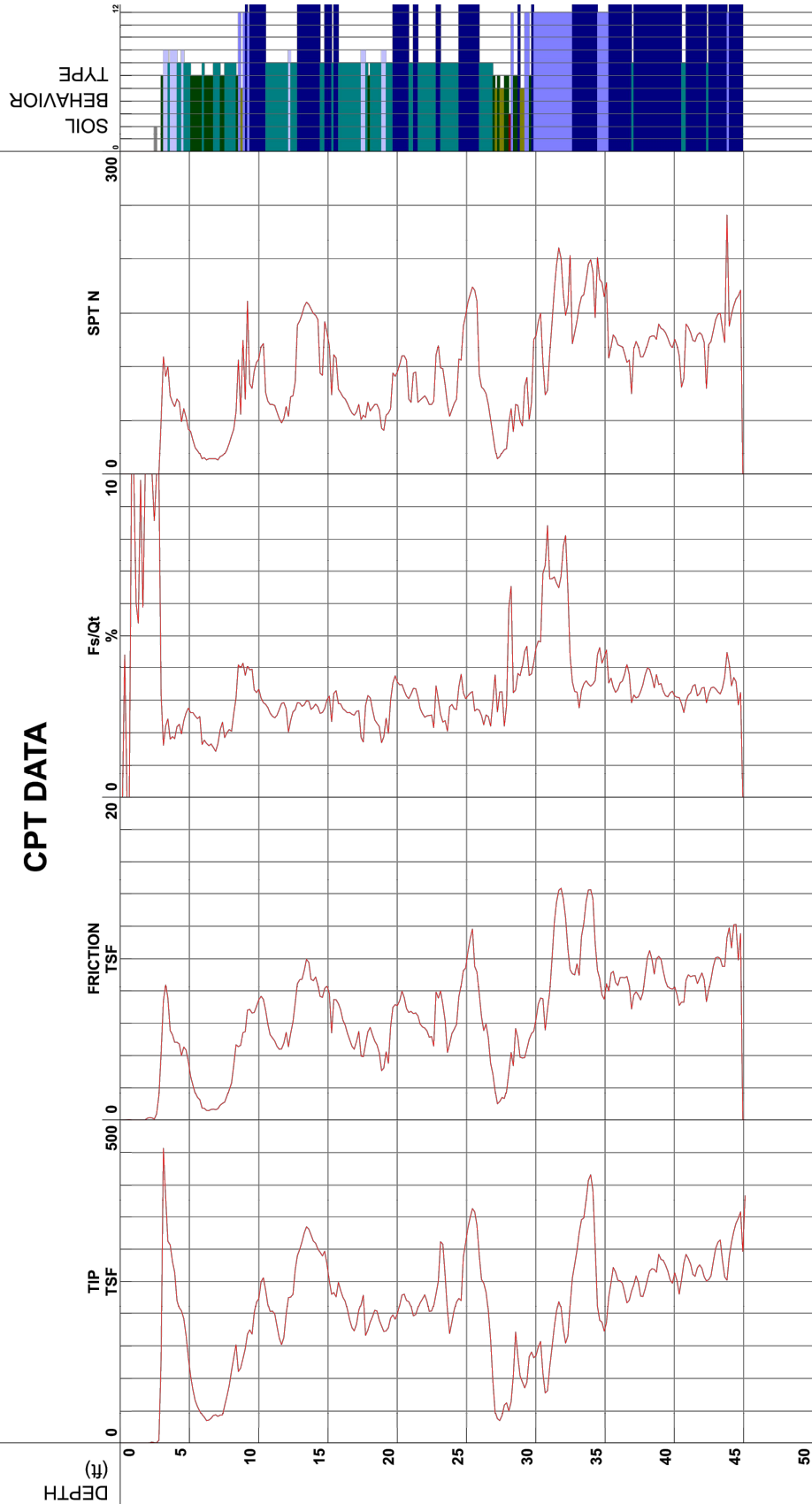
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Project	Haight Elementary School	Operator	RB KK	Filename	SDF(021).cpt
Job Number	2017-8908	Cone Number	DDG1379	GPS	
Hole Number	CPT-03	Date and Time	3/31/2017 1:01:19 PM	Maximum Depth	45.11 ft
EST GW Depth	During Test				

Net Area Ratio .8

CPT DATA



- 1 - sensitive fine grained
- 2 - organic material
- 3 - clay
- 4 - silty clay to clay
- 5 - clayey silt to silty clay
- 6 - sandy silt to clayey silt
- 7 - silty sand to sandy silt
- 8 - sand to silty sand
- 9 - sand
- 10 - gravelly sand to sand
- 11 - very stiff fine grained (*)
- 12 - sand to clayey sand (*)

S*Soil behavior type and SPT based on data from UBC-1983

Cone Size 10cm squared



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CPT-3 PLOT

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A-5
FIGURE

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Project
Job Number
Hole Number
EST GW Depth During Test

Haight Elementary School
2017-8908
CPT-04

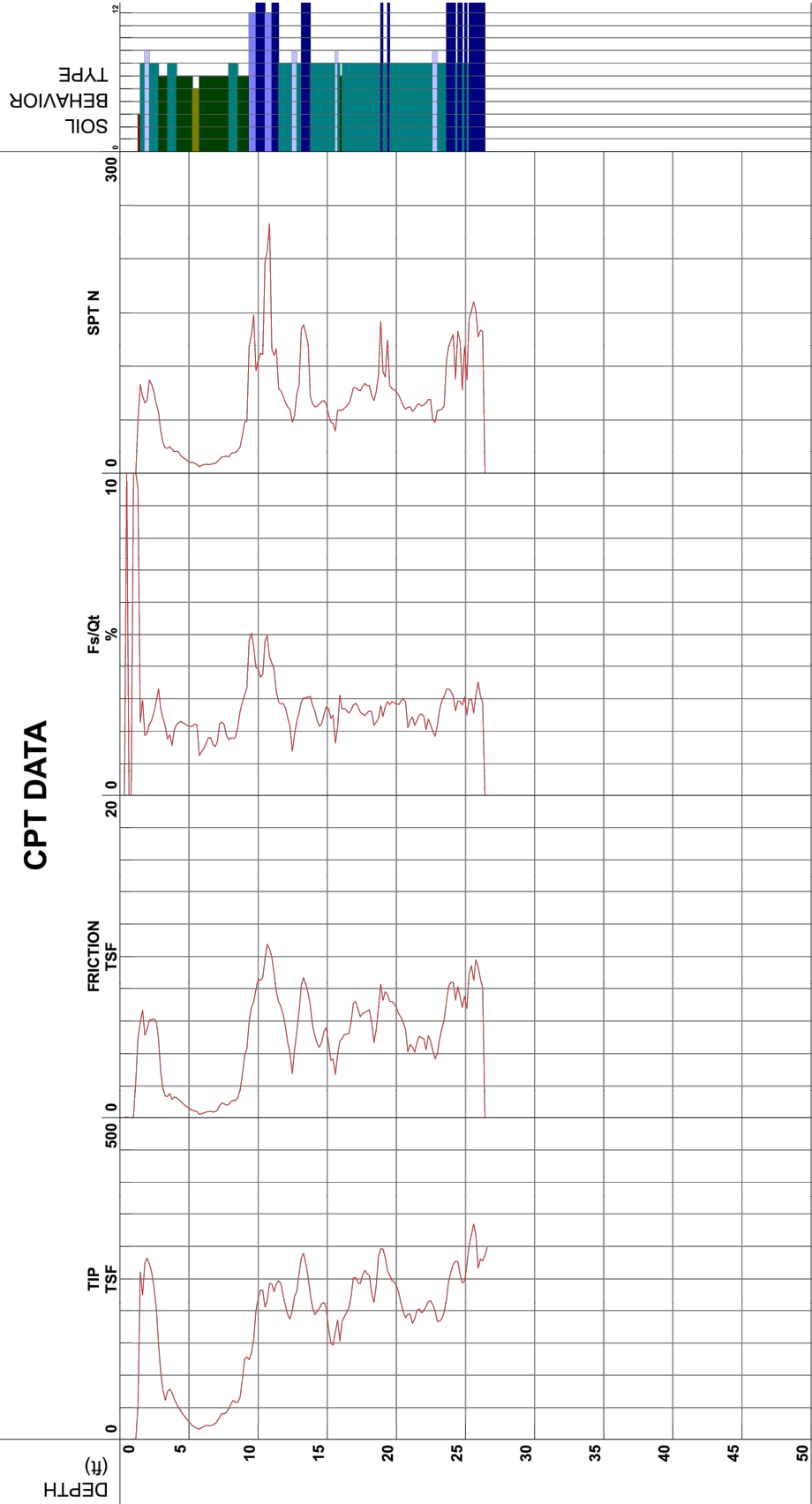
Operator
Cone Number
Date and Time
0.00 ft

Filename
GPS
Maximum Depth

SDF(022).cpt
26.57 ft

Net Area Ratio .8

CPT DATA



S^oSoil behavior type and SPT based on data from UBC-1983

Cone Size 10cm squared



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CPT-4 PLOT

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A-6
FIGURE