SAN ANTONIO ELEMENTARY SCHOOL SHADE STRUCTURE

1721 E SAN ANTONIO ST, SAN JOSE, CA 95116

ALUM ROCK UNION SCHOOL DISTRICT

DSA FILE NUMBER 43-4
DSA APPLICATION NUMBER 01-119834
PROJECT TRACKING NUMBER 69369-88



GENERAL NOTES

PRE-BID SITE VISIT

CONTRACTOR SHALL VISIT THE PROJECT AREA IN ORDER TO BECOME FAMILIAR WITH EXISTING CONDITIONS AND THE REQUIREMENTS OF THE PROJECT. THE CONTRACTOR MAY CONTACT THE ARCHITECT DURING THE BIDDING PHASE REGARDING CLARIFICATIONS AND PROJECT REQUIREMENTS.

SAFETY

IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.

DAMAGE TO STRUCTURE OR SYSTEMS TO REMAIN

CONTRACTOR SHALL REIMBURSE THE OWNER FOR REPAIR AND REPLACEMENT, INCLUDING ARCHITECT'S FEES, FOR ANY DAMAGE CAUSED TO STRUCTURES, LANDSCAPE, SITE WORK, OR EXISTING SYSTEMS TO REMAIN, AS THE RESULT OF CONSTRUCTION OPERATIONS.

EXISTING CONDITIONS

ALL EXISTING CONDITIONS ARE SHOWN BASED ON THE BEST INFORMATION AVAILABLE AT THE TIME, BUT WITHOUT GUARANTEE OF ACCURACY. THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS, DIMENSIONS, AND BUILDING DATA AT THE JOB SITE. ANY DISCREPANCIES REQUIRING MODIFICATION TO THE CONSTRUCTION DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT IMMEDIATELY. NO MODIFICATIONS SHALL BE MADE BY THE CONTRACTOR WITHOUT PRIOR APPROVAL FROM THE ARCHITECT.

CONTRACTOR'S EQUIPMENT

COORDINATE WITH OWNER'S REPRESENTATIVE FOR APPROVED LOCATION OF JOB SITE ACCESS, PARKING, AND LOCATION OF CONTRACTOR'S EQUIPMENT AND MATERIAL STORAGE AREA. SEE SITE PLAN FOR ADDITIONAL NOTES.

UTILITY SHUT-DOWNS AND CONNECTIONS

ALL REQUIRED UTILITY SHUT DOWNS SHALL HAVE PRIOR APPROVAL FROM THE OWNER'S REPRESENTATIVE. REQUEST SHALL BE SUBMITTED WITH ADEQUATE ADVANCE NOTICE PER PROJECT REQUIREMENTS.

ASBESTOS AND ASBESTOS PRODUCTS

THE OWNER/OPERATOR AND CONTRACTOR SHALL BE AWARE THAT BUILDINGS CONSTRUCTED PRIOR TO 1978 (OR THERE ABOUT) POSSIBILITY CONTAIN ASBESTOS IN SOME EXISTING CONSTRUCTION MATERIALS, AND WILL LIKELY BE ENCOUNTERED DURING ALTERATIONS OR REMODELING.

UNDER CALIFORNIA TITLE 8, THE OWNER AND CONTRACTOR BOTH HAVE RESPONSIBILITIES TO DETERMINE THE EXISTENCE OF ASBESTOS CONTAINING MATERIALS IN AREAS TO BE ALTERED OR REMODELED PRIOR TO COMMENCEMENT OF WORK AND TO TAKE APPROPRIATE MEASURES TO PROTECT PERSONNEL. CAL-OSHA HAS JURISDICTION OVER ASBESTOS RELATED WORK. ASBESTOS RELATED WORK SHALL BE DONE IN ACCORDANCE WITH CALIFORNIA GENERAL INDUSTRIAL SAFETY ORDERS, TITLE 8, SECTION 341.6 THROUGH 341.14. ASBESTOS IN THE WORK ENVIRONMENT IS REGULATED BY TITLE 8, SECTION 5208.

THE BAY AREA AIR QUALITY MANAGEMENT DISTRICT AND DISTRICT REGULATION 11-2-401.3 REQUIRES EVERY RENOVATION INVOLVING THE REMOVAL OF 100 SQ.FT., LN.FT, OR GREATER OF REGULATED ASBESTOS CONTAINING MATERIAL, AND FOR EVERY DEMOLITION (EVEN WHEN NO ASBESTOS IS PRESENT), A NOTIFICATION MUST BE SENT TO THE BAAQMD AT LEAST 10 WORKING DAYS PRIOR TO COMMENCEMENT OF DEMOLITION / RENOVATION.

ALL BUILDING MATERIALS MUST BE ASBESTOS FREE.

THESE DOCUMENTS DO NOT ADDRESS CONTAINMENT FOR EXISTING AREAS OF ASBESTOS WHICH MAY BE DISCOVERED DURING CONSTRUCTION. THE OWNER'S ABATEMENT SUBCONTRACTOR IS SOLELY RESPONSIBLE FOR THE DETECTION, REMOVAL, AND THE DISPOSAL OF ANY EXISTING ASBESTOS MATERIAL. ARCHITECTURAL AND ENGINEERING FEES FOR ADDITIONAL DESIGN EFFORT TO OBTAIN STATE APPROVALS, AS WELL AS THE COST OF ANY REPAIRS, FOR DAMAGE CAUSED OR REPLACEMENT OF EXISTING SYSTEMS TO REMAIN, DUE TO WORK PERFORMED BY THE ASBESTOS ABATEMENT SUBCONTRACTOR, SHALL BE THE RESPONSIBILITY OF SAID SUBCONTRACTOR.

CONSTRUCTION SCHEDULING

CONTRACTOR SHALL COORDINATE ALL CONSTRUCTION OPERATIONS WITH OWNER'S REPRESENTATIVE PRIOR TO SCHEDULING AND START OF THE WORK. CONTRACTOR SHALL PROVIDE PROTECTION TO ALL EXISTING SPACES AND SYSTEMS WHICH ARE IN USE, ADJOINING THE PROJECT, AND NOT PART OF THE PROJECT.

INTERIOR FINISHES

INTERIOR FINISHES AND ALL WALL COVERING MATERIAL SHALL CONFORM TO CCR TITLE 24, PART 2, CHAPTER 8.

PIPES, DUCTS AND CONDUIT - SUPPORT AND BRACING

PIPES, DUCTS, AND CONDUITS SHALL BE SUPPORTED AND BRACED PER THE SMACNA "GUIDELINES FOR SEISMIC RESTRAINTS OF MECHANICAL SYSTEMS AND PLUMBING PIPING SYSTEMS", OPM 0052-13 SEISMIC BRACING AND SUPPORT SYSTEMS.

DRILLED-IN EXPANSION ANCHORS

WHEN INSTALLING DRILLED-IN ANCHORS AND/OR POWDER DRIVEN PINS IN EXISTING NON-PRESTRESSED REINFORCED CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. WHEN INSTALLING THEM INTO EXISTING PRESTRESSED CONCRETE (PRE- OR POST-TENSIONED), LOCATE THE PRESTRESSED TENDONS BY USING A NON-DESTRUCTIVE METHOD PRIOR TO INSTALLATION. EXERCISE EXTREME CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE TENDONS DURING INSTALLATION. MAINTAIN A MINIMUM CLEARANCE OF ONE INCH BETWEEN THE REINFORCEMENT AND THE DRILLED-IN ANCHOR AND/OR PIN.

TITLE 24 COMPLIANCE

THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS IS THAT THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CALIFORNIA CODE OF REGULATIONS (2019 CBC). SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK, SHALL BE SUBMITTED TO AND APPROVED BY THE DSA BEFORE PROCEEDING WITH THE WORK.

ADMINISTRATIVE REQUIREMENTS FROM PART 1, TITLE 24, C.C.R.

- CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDUM OR A CONSTRUCTION CHANGE DOCUMENT, AND APPROVED BY DSA, AS PER 4-338
- A DSA CERTIFIED PROJECT INSPECTOR EMPLOYED BY THE DISTRICT AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK, PER SECTION 4-342.
- A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT SHALL CONDUCT ALL REQUIRED TEST AND INSPECTIONS FOR THE PROJECT.
- SPECIAL INSPECTION PER SECTION 4-333 (C)
- CONTRACTOR SHALL SUBMIT VERIFIED REPORT OR SECTION 4-336 & 4-343 (C)
- ADMINISTRATION OR CONSTRUCTION PER PART 1, TITLE 24, C.C.R.
  - DUTIES OF ARCHITECT, STRUCTURAL ENGINEER, OR PROFESSIONAL ENGINEER PER SECTION 4-333 (A) AND 4-341
  - DUTIES OF CONTRACTOR PER SECTION 4-343
  - VERIFIED REPORTS PER SECTION 4-343 AND 4-336
- A COPY OF PARTS 1 TO 5 OF TITLE 24 SHALL BE KEPT AND AVAILABLE IN THE FIELD DURING CONSTRUCTION
- DSA SHALL BE NOTIFIED AT START OF CONSTRUCTION AND PRIOR TO PLACEMENT OF CONCRETE PER SECTION 4-331
- SUPERVISION BY DSA PER SECTION 4-334
- DSA IS NOT SUBJECT TO ARBITRATION

GENERAL NOTES, cont.

- ALL WORK SHALL CONFORM TO 2019 TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).
- THE SCOPE OF WORK - CLEARLY INDICATE THE SCOPE OF WORK ON THE COVER SHEET
  OR GENERAL NOTE SHEET OF THE DRAWINGS.
- FABRICATION AND INSTALLATION OF DEFERRED SUBMITTAL ITEMS SHALL NOT BE STARTED UNTIL CONTRACTOR'S DRAWINGS, SPECIFICATIONS, AND ENGINEERING CALCULATIONS FOR THE ACTUAL SYSTEMS TO BE INSTALLED HAVE BEEN ACCEPTED AND SIGNED BY THE ARCHITECT OR STRUCTURAL ENGINEER AND APPROVED BY THE DSA.
  LIST DEFERRED SUBMITTAL ITEMS FOR THIS PROJECT.
- CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDUM OR A CONSTRUCTION CHANGED DOCUMENT (CCD) APPROVED BY THE DIVISION OF THE STATE ARCHITECT, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.
- A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY THE DSA SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
- DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT
- THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS IS THAT THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. (SECTION 4-317(C), PART 1, TITLE 24, CCR) GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES.

ABBREVIATIONS

(REFER TO CONSULTANT DRAWINGS FOR ADDITIONAL ABBREVIATIONS)

A.F.F.	ABOVE FINISHED FLOOR
A.P.	ACCESS PANEL
ACT	ACOUSTIC TILE
ADJ.	ADJUSTABLE
ALUM.	ALUMINUM
A.B.	ANCHOR BOLT
APPROX.	APPROXIMATELY
ARCH.	ARCHITECT
AC	ASPHALTIC CONCRETE
@	AT
B.M.	BENCH MARK
BLKG.	BLOCKING
BD.	BOARD
B.W.	BOTH WAYS
BOT.	BOTTOM
BLDG.	BUILDING
B.U.R.	BUILT-UP ROOFING
C.B.	CATCH BASIN
CLG.	CEILING
CEM.	CEMENT
C.C or O.C.	CENTER TO CENTER
	CENTERLINE
CER. TILE	CERAMIC TILE
C.O.	CLEANOUT
C.O.T.G.	CLEANOUT TO GRADE
CLR.	CLEAR
C.A.H.R.	CLEAR ALL HEART
	REDWOOD
C.W.	COLD WATER
COL.	COLUMN
COM.	COMMON
CONC.	CONCRETE
CONST.	CONSTRUCTION
C.H.	CONSTRUCTION HEART
C.J.	CONSTRUCTION JOINT
CONT.	CONTINUOUS
CONTR.	CONTRACTOR
CTR.	COUNTER
CTSK.	COUNTER SUNK
DET.	DETAIL
DIA. or Ø	DIAMETER
DIM.	DIMENSION
D.A.	DISABLED ACCESS
DR.	DOOR
D.S.	DOWNSPOUT
DWG.	DRAWING
D.F.	DRINKING FOUNTAIN
	and/or DOUGLAS FIR
EA.	EACH
E.W.	EACH WAY
ELEC.	ELECTRIC or ELECTRICAL
EL. or	
ELEV.	ELEVATION
ENCL.	ENCLOSE and/or ENCLOSURE
EQ.	EQUAL
EQUIP.	EQUIPMENT
(E)	EXISTING
EX.	EXPANSION
E.J.	EXPANSION JOINT
EXP.	EXPOSED
EXT.	EXTERIOR
F.O.C.	FACE OF CONCRETE
F.O.M.	FACE OF MASONRY
F.O.S.	FACE OF STUD
F.O.F.	FACE OF FINISH
FIN.	FINISH
F.F.	FINISHED FLOOR
F.S.	FINISH SLAB
F.E.	FIRE EXTINGUISHER
F.E.C.	FIRE EXTINGUISHER CABINET
F.H.	FIRE HYDRANT
F.H.M.S.	FLAT HEAD METAL SCREW
F.H.W.S.	FLAT HEAD WOOD SCREW
FL. or FLR.	FLOOR
F.D.	FLOOR DRAIN
FTG.	FOOTING
FND.	FOUNDATION
GALV.	GALVANIZED
G.I.	GALVANIZED IRON
GA.	GAUGE
GL.	GLASS
GLU-LAM	GLUE-LAMINATED
GRD.	GRADE
GYP. BD.	GYPSUM BOARD
HDW.	HARDWARE
HT.	HEIGHT
H.C.	HOLLOW CORE
H.M.	HOLLOW METAL
HORIZ.	HORIZONTAL
H.B.	HOSE BIBB
I.D.	INSIDE DIAMETER
INSUL.	INSULATION
INT.	INTERIOR
INV.	INVERT
JT	JOINT
J.H.	JOIST HANGER
K.D.	KILN DRIED
LAM.	LAMINATE
LAV.	LAVATORY
M.B.	MACHINE BOLT
M.S.	MACHINE SCREW
M.H.	MANHOLE
MFG.	MANUFACTURER
M.B.	MARKER BOARD
MATL.	MATERIAL
MAX.	MAXIMUM
MECH.	MECHANICAL
MTL.	METAL
MIN.	MINIMUM
MISC.	MISCELLANEOUS
MTD.	MOUNTED
(N)	NEW
NOM.	NOMINAL
N.I.C.	NOT IN CONTRACT
N.T.S.	NOT TO SCALE
NO. or #	NUMBER
OCC.	OCCUPANT(CY)
O.C.	ON CENTER
OPNG.	OPENING
OPP.	OPPOSITE
O.H.	OPPOSITE HAND
O.F.O.S.	OUTSIDE FACE OF STUD
O.H.W.S.	OVAL HEAD WOOD SCREW
O.D.	OVERFLOW DRAIN and/or
	OUTSIDE DIAMETER
O.F.C.I.	OWNER FURNISHED and
	CONTRACTOR INSTALLED
PR.	PAIR
PART.	PARTITION
PL.	PLATE
d	PENNY (NAILS)
PLAS.	PLASTER
PLYWD.	PLYWOOD
P.V.C.	POLY VINYL CHLORIDE
P.T.	PRESSURE TREATED
P.L.	PROPERTY LINE
R. or RAD.	RADIUS
R.W.L.	RAIN WATER LEADER
RWD./R.W.	REDWOOD
REINF.	REINFORCING
REQ'D	REQUIRED
R.A.G.	RETURN AIR GRILLE
R.E.	RIM ELEVATION
R.D.	ROOF DRAIN
RM.	ROOM
R.O.	ROUGH OPENING
RND.	ROUND
R.H.M.S.	ROUND HEAD METAL SCREW
R.H.W.S.	ROUND HEAD WOOD SCREW
SSD.	SEE STRUCTURAL DRAWINGS
S.T.S.M.S.	SELF TAPPING SHEET
	METAL SCREW
SHEATH.	SHEATHING
S.M.	SHEET METAL
S.M.S.	SHEET METAL SCREW
S.O.V.	SHUT OFF VALVE
SIM.	SIMILAR
S.C.	SOLID CORE
SPEC.	SPECIFICATION
SQ.	SQUARE
S.F.	SQUARE FEET
STAG.	STAGGERED
STD.	STANDARD
S.S.	STAINLESS STEEL
STL.	STEEL
STOR.	STORAGE
STRUCT.	STRUCTURAL
S.A.G.	SUPPLY AIR GRILLE
THRES.	THRESHOLD
T&G	TONGUE & GROOVE
T.J.	TOOLED JOINT
T.O.B.	TOP OF BEAM
T.O.C.	TOP OF CURB or CONCRETE
T.O.S.	TOP OF STEEL or SHEATHING
T.O.W.	TOP OF WALK
TYP.	TYPICAL
U.O.N.	UNLESS OTHERWISE NOTED
U.O.S.	UNLESS OTHERWISE SHOWN
V.T.R.	VENT THROUGH ROOF
VERT.	VERTICAL
V.G.	VERTICAL GRAIN
V.I.F.	VERIFY IN FIELD
V.C.T.	VINYL COMPOSITION TILE
V.W.C.	VINYL WALL COVERING
V.O.I.P.	VOICE OVER INTERNET PROTOCOL
W.C.	WATER CLOSET
W.H.	WATER HEATER
WP.	WATERPROOF
W.R.	WATER RESISTANT
W.W.M.	WELDED WIRE MESH
W.D.	WINDOW DIMENSION
W/	WITH
W/O	WITHOUT
WD.	WOOD

BUILDING CODES AND STANDARDS:

2019	CALIFORNIA ADMINISTRATIVE CODE, PART 1, TITLE 24 C.C.R.	
2019	CALIFORNIA BUILDING CODE (CBC), PART 2, TITLE 24, C.C.R. (2018 INTERNATIONAL BUILDING CODE, VOLUMES 1 AND 2, WITH 2019 CALIFORNIA AMENDMENTS.)	
2019	CALIFORNIA ELECTRIC CODE (CEC), PART 3, TITLE 24, C.C.R. (2018 NATIONAL ELECTRIC CODE WITH 2019 CALIFORNIA AMENDMENTS).	
2019	CALIFORNIA MECHANICAL CODE (CMC), PART 4, TITLE 24, C.C.R. (2018 UNIFORM MECHANICAL CODE WITH 2019 CALIFORNIA AMENDMENTS).	
2019	CALIFORNIA PLUMBING CODE (CPC), PART 5, TITLE 24, C.C.R. (2018 UNIFORM PLUMBING CODE WITH 2019 CALIFORNIA AMENDMENTS).	
2019	CALIFORNIA ENERGY CODE (CENC), PART 6, TITLE 24, C.C.R.	
2019	CALIFORNIA FIRE CODE (CFC), PART 9, TITLE 24, C.C.R. (2018 INTERNATIONAL FIRE CODE WITH 2019 CALIFORNIA AMENDMENTS).	
2019	CALIFORNIA GREEN BUILDING STANDARDS CODE, PART 11, TITLE 24, C.C.R.	
2019	CALIFORNIA REFERENCED STANDARDS, PART 12, TITLE 24, C.C.R.	
2016	ASME A17.1 (W/A17.1a/CSA B44a-08 ADDENDA) SAFETY CODE FOR ELEVATORS AND ESCALATORS	
2010	ADA STANDARDS FOR ACCESSIBLE DESIGN (28 CFR PART 35 FOR TITLE II ENTITIES)	

CCR TITLE-19, PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS.

NFPA 13	INSTALLATION OF SPRINKLER SYSTEMS (CA AMENDED)	2016 EDITION
NFPA 14	INSTALLATION OF STANDPIPE & HOSE SYSTEMS (CA AMENDED)	2016 EDITION
NFPA 17	DRY CHEMICAL EXTINGUISHING SYSTEMS	2017 EDITION
NFPA 17A	WET CHEMICAL EXTINGUISHING SYSTEM	2017 EDITION
NFPA 20	STATIONARY FIRE PUMPS TO FIRE PROTECTION	2016 EDITION
NFPA 22	WATER TANKS FOR PRIVATE FIRE PROTECTION	2013 EDITION
NFPA 24	PRIVATE FIRE SERVICE MAINS (CA AMENDED).	2016 EDITION
NFPA 25	INSPECTION, TESTING AND MAINTENANCE OF WATER BASED FIRE PROTECTION SYSTEMS	2013 CALIFORNIA EDITION
NFPA 72	NATIONAL FIRE ALARM CODE (CA AMENDED)	2016 EDITION
NFPA 80	FIRE DOORS AND OTHER OPENING PROTECTIVES	2016 EDITION
NFPA 92	STANDARD FOR SMOKE CONTROL SYSTEMS	2015 EDITION
NFPA 110	EMERGENCY AND STANDBY POWER SYSTEMS	2016 EDITION
NFPA 170	STANDARD FOR FIRE SAFETY AND EMERGENCY SYMBOLS	2018 EDITION
NFPA 253	CRITICAL RADIANT FLUX OF FLOOR COVERING SYSTEMS	2015 EDITION
NFPA 2001	CLEAN AGENT FIRE EXTINGUISHING SYSTEMS	2015 EDITION
ICC 300	STANDARDS FOR BLEACHERS, FOLDING AND TELESCOPIC SEATING, AND GRANDSTANDS	2017 EDITION
SFM 12-10-1	POWER OPERATED EXIT DOORS	
SFM 12-10-2	SINGLE POINT LATCHING OR LOCKING DEVICES	
SFM 12-10-3	EMERGENCY EXIT & PANIC HARDWARE	
UL 38	MANUAL OPERATING SIGNAL BOXES	1999/2005 EDITION
UL 268	SMOKE DETECTORS FOR FIRE PROTECTIVE SIGNALING SYSTEMS	2009 EDITION
UL 268A	SMOKE DETECTORS DUCT APPLICATIONS	1998/2003 EDITION
UL 300	FIRE TESTING OF FIRE EXTINGUISHING SYSTEMS FOR PROTECTION OF COMMERCIAL COOKING EQUIPMENT	2005 (R2010)
UL 305	PANIC HARDWARE	2012 EDITION
UL 464	AUDIBLE SIGNALING DEVICES FOR FIRE ALARM AND SIGNALING SYSTEMS, AND ACCESSORIES	2003 EDITION
UL 521	HEAT DETECTORS FOR FIRE PROTECTIVE SIGNALING SYSTEMS	1999 EDITION
UL 864	CONTROL UNITS FOR FIRE PROTECTIVE SIGNALING SYSTEMS (W/ REVISIONS THROUGH DEC. 2014)	2003 EDITION
UL 1971	SIGNALING DEVICES FOR THE HEARING IMPAIRED	2002 EDITION

COMPLIANCE WITH CFC CHAPTER 33, FIRE SAFETY DURING CONSTRUCTION AND DEMOLITION AND CBC CHAPTER 33, SAFETY DURING CONSTRUCTION WILL BE ENFORCED.

SYMBOLS LEGEND

- SECTION / EXTERIOR ELEVATION — 1 / A8.1 — SECTION IDENTIFICATION; SHEET WHERE SECTION IS DRAWN
- DETAIL — 4 / A9.1 — DETAIL IDENTIFICATION; SHEET WHERE DETAIL IS DRAWN
- INTERIOR ELEVATION — 1 / A7.1 — INDICATES ELEVATION SHOWN; SHEET WHERE ELEVATION IS DRAWN
- ROOM IDENTIFICATION — CLASSROOM / 102 — ROOM NAME; ROOM NUMBER
- SPECIFIC NOTE — 3
- DOOR DESIGNATION — 102A
- WINDOW DESIGNATION — A
- ADDENDUM REVISION — 1 — CLOUD AROUND REVISION
- CCD REVISION — 1 / CCD — CLOUD AROUND REVISION
- FINISH NUMBER — 127 — SEE SPECS AND I.E. DWGS.
- EQUIPMENT LETTER — A — SEE EQUIPMENT SCHEDULE
- CEILING HEIGHT — +8'-0"
- WALL TYPE — 1
- MATCH LINE
- ELEV. HEIGHT — +8'-0"
- CENTER OF
- FACE OF

PROJECT SUMMARY

INSTALL 40' X 50'X12' DSA #04-119454 PC SHADE STRUCTURE AT SAN ANTONIO ELEMENTARY SCHOOL AND ASSOCIATED SITE WORK.

THERE ARE NO DEFERRED SUBMITTALS FOR THIS PROJECT.

DESIGN TEAM

ARCHITECT
SUGIMURA FINNEY ARCHITECTS
2155 SOUTH BASCOM AVENUE SUITE 200
CAMPBELL, CALIFORNIA 95008
(408) 879-9600
(408) 377-6066 FAX
ATTN: MARK FINNEY MARK@SUGIMURA.COM

SHEET TOTAL = 12

DRAWING INDEX

T1	TITLE SHEET
T2	SITE PLAN - ACCESS COMPLIANCE
T3	SITE PLAN - FIRE LIFE SAFETY

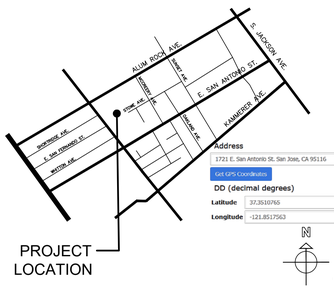
ARCHITECTURAL

A0.1	DEMOLITION SITE PLAN ENLARGED
A0.2	NEW SITE PLAN ENLARGED
A0.3	SITE DETAILS

MANUFACTURER'S DRAWINGS PC #04-119454

P.C. T-1.0	TITLE SHEET
P.C. T-2.0	DSA 103 SAMPLE FORM
P.C. T-2.1	DSA 103 SAMPLE FORM
25.1-1000	PRODUCT INFORMATION
25.2-1001	DETAILS
25.3-2000	REACTIONS

VICINITY MAP



STATEMENT OF GENERAL CONFORMANCE

FOR ARCHITECTS / ENGINEERS WHO UTILIZE PLANS, INCLUDING BUT NOT LIMITED TO SHOP DRAWINGS, PREPARED BY OTHER LICENSED DESIGN PROFESSIONALS AND / OR OTHER CONSULTANTS

APPLICATION NO.: 01-119834 FILE NO.: 43-4

☒ THE DRAWINGS OR SHEETS LISTED ON THE COVER OR INDEX SHEET
☐ THIS DRAWING, PAGE OF SPECIFICATIONS / CALCULATIONS

HAVE BEEN PREPARED BY OTHER DESIGN PROFESSIONALS OR CONSULTANTS WHO ARE LICENSED AND / OR AUTHORIZED TO PREPARE SUCH DRAWINGS IN THIS STATE. IT HAS BEEN EXAMINED BY ME FOR:

1) DESIGN INTENT AND APPEARS TO MEET THE APPROPRIATE REQUIREMENTS OF TITLE 24, CALIFORNIA CODE OF REGULATIONS AND THE PROJECT SPECIFICATIONS PREPARED BY ME, AND

2) COORDINATION WITH MY PLANS AND SPECIFICATIONS AND IS ACCEPTABLE FOR INCORPORATION INTO THE CONSTRUCTION OF THIS PROJECT.

THE STATEMENT OF GENERAL CONFORMANCE "SHALL NOT BE CONSTRUED AS RELIEVING ME OF MY RIGHTS, DUTIES, AND RESPONSIBILITIES UNDER SECTIONS 17302 AND 81138 OF THE EDUCATION CODE AND SECTIONS 4-336, 4-341 AND 4-344" OF TITLE 24, PART 1, *(TITLE 24, PART 1, SECTION 4-317(B))*

I FIND THAT:

☒ ALL DRAWINGS OR SHEETS LISTED ON THE COVER OR INDEX SHEET
☐ THIS DRAWING OR PAGE

☒ IS / ARE IN GENERAL CONFORMANCE WITH THE PROJECT DESIGN, AND
☒ HAS / HAVE BEEN COORDINATED WITH THE PROJECT PLANS AND SPECIFICATIONS.

MARK FINNEY — 09/07/2021
C-24673 — 9/30/2023
LICENSE NUMBER — EXPIRATION DATE

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01 -119834 INC: 0
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 5/20/2022
(DSA STAMP AREA)

SUGIMURA FINNEY ARCHITECTS
SFA
ARCHITECTURE INTERIORS PLANNING
2155 SOUTH BASCOM AVE.
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LICENSED ARCHITECT
MARK C. FINNEY
NO. C-24673
RENEW 9-30-23
STATE OF CALIFORNIA

TITLE SHEET

SAN ANTONIO ELEMENTARY SCHOOL SHADE STRUCTURE
1721 E SAN ANTONIO ST., SAN JOSE, CA 95116
ALUM ROCK UNION SCHOOL DISTRICT

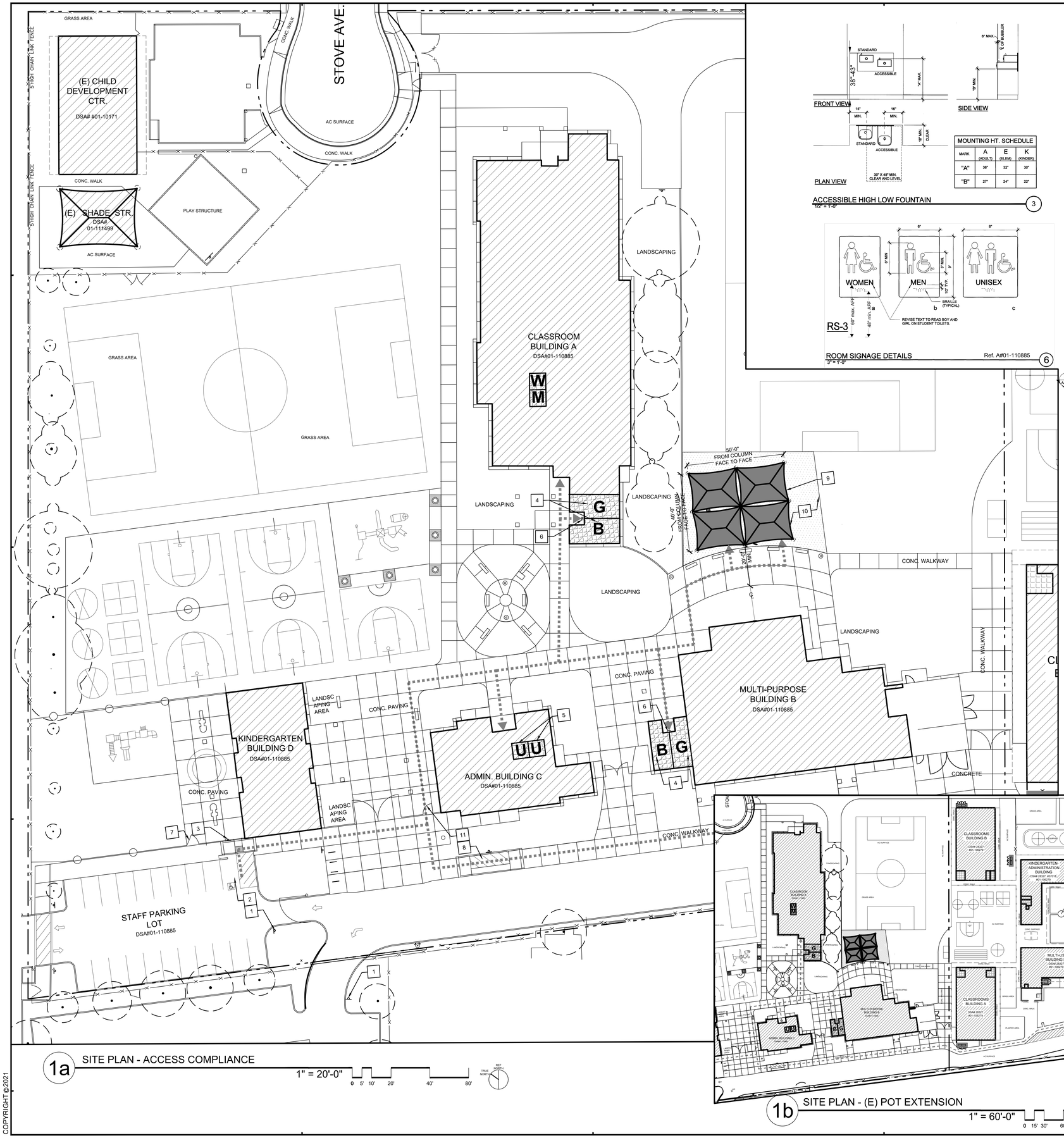
REVISIONS

NO.	ITEM	DATE

DRAWN BY:	MK
CHECKED BY:	NJ
SFA JOB NO:	DATE:
21077	08/27//2021

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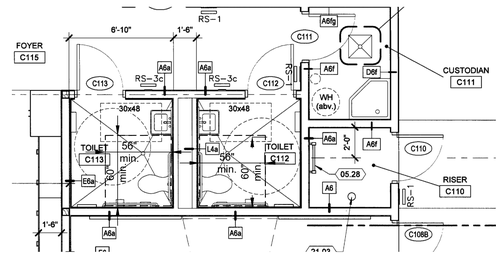
1a SITE PLAN - ACCESS COMPLIANCE
1" = 20'-0"
0 5' 10' 20' 40' 80'

1b SITE PLAN - (E) POT EXTENSION
1" = 60'-0"
0 15' 30' 60' 120' 180'

MOUNTING HT. SCHEDULE			
MARK	A (ADULT)	E (ELEM)	K (KINDER)
"A"	36"	32"	30"
"B"	27"	24"	22"

ACCESSIBLE HIGH LOW FOUNTAIN 3
1/2" = 1'-0"

ROOM SIGNAGE DETAILS Ref. A#01-110885 6
3" = 1'-0"



2 ADMIN. BUILDING C UNISEX ACCESSIBLE RESTROOMS PER DSA #01-110885

1 ENLARGED FLOOR PLAN BOYS AND GIRLS ACCESSIBLE RESTROOMS PER DSA #01-110885
1/4" = 1'-0"

GENERAL NOTES

A. **THIS SHEET IS FOR ACCESS COMPLIANCE CODE RELATED ITEMS. FOR SCOPE OF WORK SEE SHEETS A0.1 AND A0.2.**
B. ACCESSIBLE PATH OF TRAVEL (P.O.T.), AS INDICATED, IS A COMMON BARRIER FREE ACCESS ROUTE WITHOUT ANY ABRUPT VERTICAL CHANGES EXCEEDING A 1/2" BEVELED AT 1:2 MAXIMUM SLOPE, EXCEPT THAT LEVEL CHANGES DO NOT EXCEED 1/4" VERTICAL AND AT LEAST 48" WIDE. THE PATH SURFACE IS SLIP RESISTANT, STABLE, FIRM, AND SMOOTH. PASSING SPACES (11B-403.5.3) AT LEAST 60"X60" ARE LOCATED NOT MORE THAN 200' APART. PARTS OF P.O.T WITH CONTINUOUS GRADIENTS HAVE 60" LEVEL AREAS (11B-403.7) NOT MORE THAN 400' APART. THE CROSS SLOPE DOES NOT EXCEED 2% AND SLOPE IN THE DIRECTION OF TRAVEL AND IS LESS THAN 5% UNLESS OTHERWISE INDICATED. P.O.T. SHALL BE MAINTAINED FREE OF OVERHANGING OBSTRUCTIONS TO 80" MINIMUM (11B-307.4) AND PROTRUDING OBJECTS GREATER THAN 4" PROJECTION FROM WALL AND ABOVE 27" AND LESS THAN 80" (11B-307.2).
C. GATES IN THE PATH OF TRAVEL SHALL MEET DOOR REQUIREMENTS PER CBC SECTION 11B-404. ALL GATES TO HAVE ACCESSIBLE HARDWARE AND 10" MIN. SMOOTH BOTTOM OR KICK PLATES. PANIC HARDWARE AND EXIT SIGN MAY BE REQUIRED. COORDINATE WITH FIRE AND LIFE SAFETY.
D. ARCHITECT TO VERIFY ALL BARRIERS IN P.O.T. HAVE BEEN REMOVED.
E. ALL EXTERIOR ENTRANCES AND EXITS IDENTIFIED WITH A TRIANGULAR SYMBOL ON THIS PLAN ARE ACCESSIBLE AND COMPLY WITH CBC 11B-401 AND INCLUDE A 32" CLEAR OPENING, THE REQUIRED STRIKE EDGE CLEARANCE AT PULL SIDE OF DOOR, LEVEL LANDINGS WITH A 2% MAX. SLOPE, AND AN ACCESSIBLE THRESHOLD, HARDWARE, CLOSER AND KICK PLATE.
F. REMOVE AND REINSTALL ALL (E) ACCESSIBLE RESTROOMS ACCESSORIES INCLUDING GRAB BARS, TOILET SEAT DISPENSERS, PAPER TOWEL DISPENSERS IF MOUNTING HEIGHTS FOUND NON COMPLIANT TO MEET CBC 2019 REQUIREMENTS PER DET. 9/A0.3
G. DESIGN PROFESSIONAL TO VERIFY ALL ACCESSIBLE RESTROOM SIGNS AND THEIR MOUNTING HEIGHTS ARE PER CBC 2019 REQUIREMENTS. IF FOUND EXISTING SIGNS AND THEIR MOUNTING HEIGHTS ARE NOT CODE COMPLIANT REMOVE AND INSTALL SIGNS PER DET. 16/A0.3.

SITE PLAN - ACCESS COMPLIANCE NOTES

1. EXISTING ENTRY WARNING SIGN PER DSA #01-110885. UPDATE PER DET. 3/A0.3 IF DOES NOT COMPLY WITH CBC 2019 REQUIREMENTS.
2. (E) ACCESSIBLE PARKING STALL PER DSA #01-110885.
3. (E) ACCESSIBLE PARKING SIGN PER DSA #01-110885. UPDATE PER DET. 4/A0.3 IF DOES NOT COMPLY WITH CBC 2019 REQUIREMENTS.
4. (E) ACCESSIBLE BOYS & GIRLS RESTROOMS PER DSA 01-110885. SEE DET. 1/-
5. (E) ACCESSIBLE UNISEX RESTROOMS BLDG. "C" PER DSA 01-110885. SEE DET 2/-
6. (E) ACCESSIBLE DRINKING FOUNTAIN PER DSA #01-110885. SEE DET. 3/-
7. (E) ACCESSIBLE GATE PER DSA #01-110885.
8. (E) LOADING ZONE PER DSA #01-110885
9. (N) SHADE STRUCTURE PC #04-119454. SEE MANUFACTURER'S DRAWINGS.
10. (N) AC PAVING. SEE SHEET A0.2
11. (N) ACCESSIBLE GATE DECORATIVE FENCE. SEE NOTE 5 SHEET A0.2
12. (N) ACCESSIBLE GATE IN (E) CHAINLINK FENCE PER DET. 14/A03.

PARKING COUNT

PER DSA # 01-110885
PER 2019 CBC,TABLE 11B-208.2

PARKING LOT VISITOR & STAFF

TOTAL PARKING SPACES (INCLUDING ALL ACCESSIBLE PARKING SPACES) = 21
MINIMUM ACCESSIBLE PARKING SPACES REQUIRED = 1
TOTAL STANDARD ACCESSIBLE SPACES + TOTAL VAN ACCESSIBLE SPACES* PROVIDED = 1
THEREFORE, OKAY.
* FOR EVERY SIX STANDARD ACCESSIBLE SPACES REQUIRED, AT LEAST ONE SHALL BE A VAN PARKING SPACE.

GRAPHIC KEY

- EXISTING PROPERTY LINE
- ACCESSIBLE PATH OF TRAVEL
- ROOF OVERHANG
- CHAIN LINK FENCE
- WOOD FENCE
- DECORATIVE FENCE
- NEW BUILDING
- EXISTING BUILDING
- EXISTING RESTROOMS
- ACCESSIBLE ENTRANCE, SEE GENERAL NOTE 'F'
- (E) DRY STAND PIPE
- DRINKING FOUNTAIN
- (E) FIRE HYDRANT
- (E) SIGN
- M (E) MENS TOILET ROOM
- W (E) WOMENS TOILET ROOM
- G (E) GIRLS TOILET ROOM
- B (E) BOYS TOILET ROOM
- U (E) UNISEX TOILET ROOM
- K (E) KINDERGARTEN TOILET ROOM

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01 -119834 INC: 0
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 5/20/2022
(DSA STAMP AREA)

SUGIMURA FINNEY ARCHITECTS
SFA
ARCHITECTURE INTERIORS PLANNING
2155 SOUTH BASCOM AVE.
SUITE 200
CAMPBELL, CA 95008
PHONE: 408-879-0606
FAX: 408-377-6066

LICENSED ARCHITECT
MARK C. FINNEY
No. C-24573
REN. 9-30-23
STATE OF CALIFORNIA

SITE PLAN
ACCESS COMPLIANCE

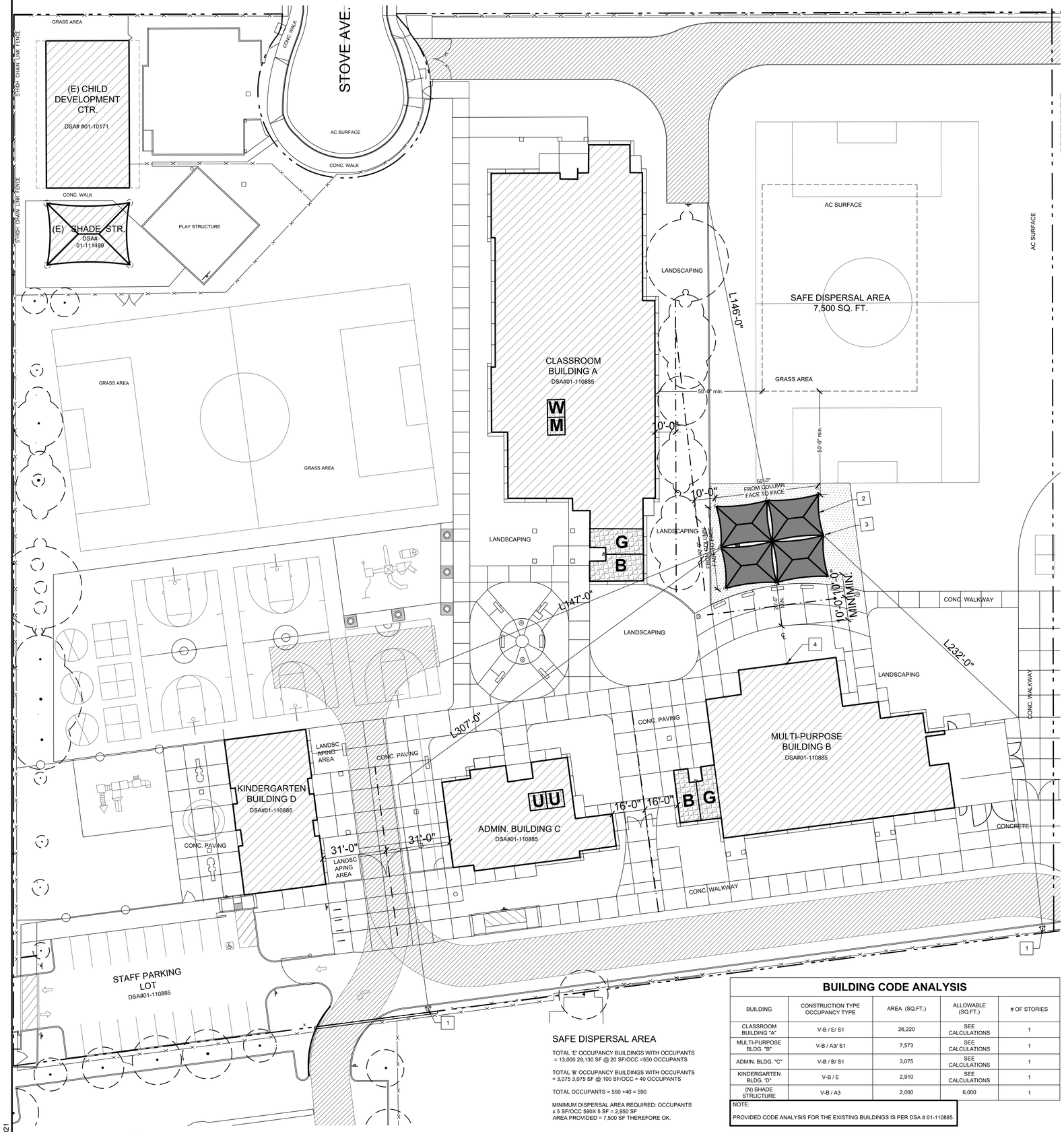
SAN ANTONIO ELEMENTARY SCHOOL SHADE STRUCTURE
1721 E SAN ANTONIO ST., SAN JOSE, CA 95116
ALUM ROCK UNION SCHOOL DISTRICT

REVISIONS
NO. ITEM DATE

DRAWN BY: MK
CHECKED BY: NJ
SFA JOB NO: 21077
DATE: 08/27//2021

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BUILDING CODE ANALYSIS

BUILDING	CONSTRUCTION TYPE OCCUPANCY TYPE	AREA (SQ.FT.)	ALLOWABLE (SQ.FT.)	# OF STORIES
CLASSROOM BUILDING "A"	V-B / E/ S1	26,220	SEE CALCULATIONS	1
MULTI-PURPOSE BLDG. "B"	V-B / A3/ S1	7,573	SEE CALCULATIONS	1
ADMIN. BLDG. "C"	V-B / B/ S1	3,075	SEE CALCULATIONS	1
KINDERGARTEN BLDG. "D"	V-B / E	2,910	SEE CALCULATIONS	1
(N) SHADE STRUCTURE	V-B / A3	2,000	6,000	1

NOTE:
PROVIDED CODE ANALYSIS FOR THE EXISTING BUILDINGS IS PER DSA # 01-110885.

SAFE DISPERSAL AREA

TOTAL 'E' OCCUPANCY BUILDINGS WITH OCCUPANTS = 13,000 29,130 SF @ 20 SF/OCC =550 OCCUPANTS

TOTAL 'B' OCCUPANCY BUILDINGS WITH OCCUPANTS = 3,075 3,075 SF @ 100 SF/OCC = 40 OCCUPANTS

TOTAL OCCUPANTS = 550 +40 = 590

MINIMUM DISPERSAL AREA REQUIRED: OCCUPANTS x 5 SF/OCC 590X 5 SF = 2,950 SF
AREA PROVIDED = 7,500 SF THEREFORE OK.

CODE ANALYSIS

BUILDING A - CLASSROOMS

OCCUPANCY CLASSIFICATION GROUPS
MAJOR GROUP = E
MINOR GROUP = S-1

TYPE OF CONSTRUCTION
TYPE = V-B
WITH AUTOMATIC SPRINKLER SYSTEM (SYSTEM USED FOR BUILDING HEIGHT AND STORY INCREASES ONLY)

ALLOWABLE FLOOR AREA - **BASIC ALLOWANCES PER STORY PER TABLE 503**
OCCUPANT GROUP E = 9,500SF
OCCUPANT GROUP S-1 = 9,000SF

AREA MODIFICATION - **ALLOWED PER SECTION 506**
FRONTAGE INCREASE - **ALLOWED PER EQUATION 5-2**
If = [F/P - 0.25] W/30
Where F = 643'-7"
P = 643'-7"
W = 30
If = [643'-7"/643'-7" - 0.25] 30/30 = 0.75

ALLOWABLE FLOOR AREA - **ALLOWED PER EQUATION 5-1**
Aa = (At + [At * If] + [At * Ip])
OCCUPANT GROUP E
Where At = 9,500
If = 0.75 (from above)
Ip = 0
Aa = (9,500 + [9,500*0.75] + [9,500*0]) = 16,625sf per story
Maximum floor area may be increased by 200% per Section 506.4
MAX. FLOOR AREA GROUP E = Aa*2 = 16,625*2 = **33,250sf**
OCCUPANT GROUP S-1
Where At = 9,000
If = 0.75 (from above)
Ia = 2 (per Section 506.3)
Aa = (9,000 + [9,000*0.75] + [9,000*2]) = 15,750sf per story
Maximum floor area may be increased by 200% per Section 506.4
MAX. FLOOR AREA GROUP S-1 = Aa*2 = 15,750*2 = **31,500sf**

MAIN LEVEL MIXED OCCUPANCY RATIOS PER CBC 508.3			
OCCUPANCY	ACTUAL AREA (X)	MAX ALLOWABLE (Y)	RATIO (X/Y)
E	12,912sf	16,625sf	.78
S-1	351sf	15,750sf	.02
		SUBTOTAL	.80

UPPER LEVEL MIXED OCCUPANCY RATIOS PER CBC 508.3			
OCCUPANCY	ACTUAL AREA (X)	MAX ALLOWABLE (Y)	RATIO (X/Y)
E	12,912sf	16,625sf	.78
S-1	351sf	15750sf	.02
		SUBTOTAL	.80
		TOTAL = .80 + .80 =	**1.60**

PER CBC SECTION 508.4.1, NOTE 2, THE MIXED RATIO OF THIS BUILDING SHALL BE LESS THAN 2 - SINCE 1.60 < 2 : **OK**

HEIGHT AND NUMBER OF STORIES - **ALLOWED PER TABLE 503**
OCCUPANT GROUP E = 1 STORY @ 40'-0" ABOVE GRADE
OCCUPANT GROUP S-1 = 1 STORY @ 40'-0" ABOVE GRADE

HEIGHT MODIFICATION - **ALLOWED PER SECTION 504.2**
AUTOMATIC SPRINKLER SYSTEM INCREASE - THE HEIGHT IS INCREASED BY 20'-0" AND THE MAXIMUM NUMBER OF STORIES IS INCREASED BY ONE

MAXIMUM HEIGHT ALLOWABLE
OCCUPANT GROUP E = 40' + 20' = 60'-0" ABOVE GRADE
OCCUPANT GROUP S-1 = 40' + 20' = 60'-0" ABOVE GRADE

ACTUAL BUILDING INFORMATION
OCCUPANT GROUP E = 2-STORY @ 36'-0" ABOVE GRADE
OCCUPANT GROUP S-1 = 2-STORY @ 36'-0" ABOVE GRADE

BUILDING B - MULTIPURPOSE

OCCUPANCY CLASSIFICATION GROUPS
MAJOR GROUP = A-3
MINOR GROUP = S-1

TYPE OF CONSTRUCTION
TYPE = V-B
WITH AUTOMATIC SPRINKLER SYSTEM

ALLOWABLE FLOOR AREA - **BASIC ALLOWANCES PER STORY PER TABLE 503**
OCCUPANT GROUP A-3 = 9,000SF
OCCUPANT GROUP S-1 = 9,000SF

AREA MODIFICATION - **ALLOWED PER SECTION 506**
FRONTAGE INCREASE - **ALLOWED PER EQUATION 5-2**
If = [F/P - 0.25] W/30
Where F = 491'-7"
P = 491'-7"
W = 30
If = [491'-7"/491'-7" - 0.25] 30/30 = 0.75

ALLOWABLE FLOOR AREA - **ALLOWED PER EQUATION 5-1**
Aa = (At + [At * If] + [At * Ip])
OCCUPANT GROUP A-3
Where At = 9,000
If = 0.75 (from above)
Ia = 3 (per Section 506.3)
Aa = (9,000 + [9,000*0.75] + [9,000*3]) = 28,500sf
OCCUPANT GROUP S-1
Where At = 9,000
If = 0.75 (from above)
Ia = 3 (per Section 506.3)
Aa = (9,000 + [9,000*0.75] + [9,000*3]) = 42,750sf

MAIN LEVEL MIXED OCCUPANCY RATIOS PER CBC 508.3			
OCCUPANCY	ACTUAL AREA (X)	MAX ALLOWABLE (Y)	RATIO (X/Y)
A-3	7,227sf	28,500sf	.254
S-1	437sf	42,750sf	.010
		TOTAL	.255

PER CBC SECTION 506.4.1, NOTE 1, THE MIXED RATIO OF THIS BUILDING SHALL BE LESS THAN 1 - SINCE 0.255 < 1 : **OK**

HEIGHT AND NUMBER OF STORIES - **ALLOWED PER TABLE 503**
OCCUPANT GROUP A-3 = 1 STORY @ 40'-0" ABOVE GRADE
OCCUPANT GROUP S-1 = 1 STORY @ 40'-0" ABOVE GRADE

ACTUAL BUILDING INFORMATION
OCCUPANT GROUP A-3 = 1-STORY @ 33'-9 1/2" ABOVE GRADE
OCCUPANT GROUP S-1 = 1-STORY @ 33'-9 1/2" ABOVE GRADE

BUILDING C - ADMINISTRATION

OCCUPANCY CLASSIFICATION GROUPS
MAJOR GROUP = B
MINOR GROUP = S-1

TYPE OF CONSTRUCTION
TYPE = V-B
WITH AUTOMATIC SPRINKLER SYSTEM

ALLOWABLE FLOOR AREA - **BASIC ALLOWANCES PER STORY PER TABLE 503**
OCCUPANT GROUP B = 9,000SF
OCCUPANT GROUP S-1 = 9,000SF

AREA MODIFICATION - **ALLOWED PER SECTION 506**
FRONTAGE INCREASE - **ALLOWED PER EQUATION 5-2**
If = [F/P - 0.25] W/30
Where F = 293'-6"
P = 293'-6"
W = 30
If = [293'-6"/293'-6" - 0.25] 30/30 = 0.75

ALLOWABLE FLOOR AREA - **ALLOWED PER EQUATION 5-1**
Aa = (At + [At * If] + [At * Ip])
OCCUPANT GROUP B
Where At = 9,000
If = 0.75 (from above)
Ia = 3 (per Section 506.3)
Aa = (9,000 + [9,000*0.75] + [9,000*3]) = 42,750sf
OCCUPANT GROUP S-1
Where At = 9,000
If = 0.75 (from above)
Ia = 3 (per Section 506.3)
Aa = (9,000 + [9,000*0.75] + [9,000*3]) = 42,750sf

MAIN LEVEL MIXED OCCUPANCY RATIOS PER CBC 508.3			
OCCUPANCY	ACTUAL AREA (X)	MAX ALLOWABLE (Y)	RATIO (X/Y)
B	3,003sf	42,750sf	.071
S-1	102sf	42,750sf	.002
		TOTAL	.073

PER CBC SECTION 506.4.1, NOTE 1, THE MIXED RATIO OF THIS BUILDING SHALL BE LESS THAN 1 - SINCE 0.073 < 1 : **OK**

HEIGHT AND NUMBER OF STORIES - **ALLOWED PER TABLE 503**
OCCUPANT GROUP B = 2 STORY @ 40'-0" ABOVE GRADE
OCCUPANT GROUP S-1 = 1 STORY @ 40'-0" ABOVE GRADE

ACTUAL BUILDING INFORMATION
OCCUPANT GROUP B = 1-STORY @ 21'-1" ABOVE GRADE
OCCUPANT GROUP S-1 = 1-STORY @ 21'-1" ABOVE GRADE

BUILDING D - KINDERGARTEN

OCCUPANCY CLASSIFICATION GROUPS
MAJOR GROUP = E

TYPE OF CONSTRUCTION
TYPE = V-B
WITH AUTOMATIC SPRINKLER SYSTEM

ALLOWABLE FLOOR AREA - **BASIC ALLOWANCES PER STORY PER TABLE 503**
OCCUPANT GROUP E = 9,000SF

AREA MODIFICATION - **ALLOWED PER SECTION 506**
FRONTAGE INCREASE - **ALLOWED PER EQUATION 5-2**
If = [F/P - 0.25] W/30
Where F = 295'-0"
P = 295'-0"
W = 30
If = [295'-0"/295'-0" - 0.25] 30/30 = 0.75

ALLOWABLE FLOOR AREA - **ALLOWED PER EQUATION 5-1**
Aa = (At + [At * If] + [At * Ip])
OCCUPANT GROUP E
Where At = 9,500
If = 0.75 (from above)
Ia = 3 (per Section 506.3)
Aa = (9,500 + [9,500*0.75] + [9,500*3]) = 43,125sf

MAIN LEVEL MIXED OCCUPANCY RATIOS PER CBC 508.3			
OCCUPANCY	ACTUAL AREA (X)	MAX ALLOWABLE (Y)	RATIO (X/Y)
E	2,963sf	43,125sf	.069

PER CBC SECTION 506.4.1, NOTE 1, THE MIXED RATIO OF THIS BUILDING SHALL BE LESS THAN 1 - SINCE 0.069 <1 : **OK**

HEIGHT AND NUMBER OF STORIES - **ALLOWED PER TABLE 503**
OCCUPANT GROUP E = 1-STORY @ 40'-0" ABOVE GRADE

ACTUAL BUILDING INFORMATION
OCCUPANT GROUP E = 1-STORY @ 18'-0" ABOVE GRADE

PROJECT SUMMARY

INSTALL 40' X 50'X12' SHADE STRUCTURE AT SAN ANTONIO ELEMENTARY SCHOOL AND ASSOCIATED SITE WORK.

COPIED FROM TITLE PAGE. SAN JOSE LOCAL FIRE REQUESTED THIS BE ADDED SO THEY HAVE A RECORD OF THE PROJECT SCOPE.

GENERAL NOTES

A. **THIS SHEET IS FOR FIRE LIFE SAFETY CODE RELATED ITEMS.**
B. **THE PROVISIONS OF CBC CHAPTER 33, SAFEGUARDS DURING CONSTRUCTION AND CFC CHAPTER 33, FIRE SAFETY DURING CONSTRUCTION AND DEMOLITION SHALL BE ENFORCED ON THIS PROJECT.**

SITE PLAN - FIRE LIFE SAFETY NOTES

1. EXISTING FIRE HYDRANT.
2. (N) SHADE STRUCTURE PC #04-119454. SEE MANUFACTURER'S DRAWINGS.
3. INSTALL (N) SIGNS TO READ: "COOKING PROHIBITED" & "NOT TO BE USED FOR STORAGE".
4. (E) OUTSIDE FIRE HORN.

DSA **810**

FIRE & LIFE SAFETY SITE CONDITIONS SUBMITTAL

Division of the State Architect (DSA) documents referenced within this publication are available on the DSA Forms or DSA Publications webpages.

To facilitate the Division of the State Architect's (DSA) fire and life safety plan review of project site conditions, DSA requires the design professional to provide the following information at time of project submittal for projects consisting of construction of a new campus, construction of new building(s), additions to existing buildings, and for site alternate design means for fire department emergency vehicle access, and fire suppression water supply.

Information associated with compliance items 1 through 3 below is to be provided for all project types indicated above. Information associated with items 4 through 7 is to be completed when an alternate means is utilized. Acknowledgement by the school district and signature from the Local Fire Authority (LFA) is only required when an alternate design means is being requested.

The Project Information and Fire & Life Safety Information sections are to be completed for all projects and imaged onto the fire access site plan. When an alternate design/means is proposed, all sections on pages 1 and 2 are to be completed and imaged on the fire access site plan.

For additional information refer to the instructions at the end of this form and DSA Policy *PL 09-01: Fire Flow for Buildings*.

PROJECT INFORMATION	
School District/Owner:	ALUM ROCK ELEMENTARY SCHOOL DISTRICT
Project Name/School:	SAN ANTONIO ELEMENTARY SCHOOL SHADE STRUCTURE
Project Address:	1721 E SAN ANTONIO ST, SAN JOSE, CA 95116

FIRE & LIFE SAFETY INFORMATION				
1.	Has a fire hydrant flow test been performed within the past 12 months? *(If yes, provide a copy of the test data.)*	Yes ☐		No ☑
2.	Was the fire hydrant water flow test performed as part of this LFA review?	Yes ☐		No ☑
3.	Is the project located within a designated fire hazard severity zone (FHSZ) as established by Cal-Fire? *(If yes, indicate FHSZ classification below.)*	Yes ☐		No ☑
	Refer to the following website for FHSZ locations: http://egis.fire.ca.gov/FHSZ/	Moderate ☐	High ☐	Very High ☐
	Wildland Interface Area (WIFA) *(If any designations are checked, project design must meet the requirements of CBC Chapter 7A.)*			WIFA ☐

CONDITION MEANS AND METHODS RESOLUTION		ALTERNATE ACCEPTED			
		Yes	No	N/A	N/R
4.	Emergency vehicle access roadways do not meet CFC requirements.			✔	
4a.	**Acceptable Alternate:** Emergency vehicle and personnel access as proposed by the project architect is acceptable for providing fire suppression and protection of life and property.				
5.	**Fire Hydrants:** Number and spacing does not meet CFC requirements.			✔	
5a.	**Acceptable Alternate:** Number of fire hydrants and spacing as proposed by the project architect is acceptable for fire suppression and protection of life and property.				
6.	**Fire Hydrants:** Water flow and pressure are less than CFC minimum.			✔	
6a.	**Acceptable Alternate:** The available flow and pressure is acceptable for providing fire suppression and protection of life and property.				
7.	Location of fire department connection(s) serving fire sprinkler systems or standpipe systems does not meet CFC requirements.			✔	
7a.	**Acceptable Alternate:** The location of fire department connection serving the fire sprinkler system and/or standpipe system is acceptable for providing fire suppression and protection of life and property.				

School District Acceptance of Acceptable Design Alternates

By signing this form, the school district acknowledges and accepts the proposed design as an alternative to California Building Code (CBC) and California Fire Code (CFC) minimum requirements, as indicated by one or more of the conditions indicated at items 4a, 5a, 6a or 7a, for providing fire and life safety protection of life and property.

Accepted by: _____ Title: _____

Signature: _____ Date: _____

LOCAL FIRE AUTHORITY (LFA) INFORMATION	
LFA Agency Name:	
LFA Review Official:	
Title:	Work Phone:
Work Email:	

LFA Reviewer's Signature: _____ Date: _____

GRAPHIC KEY

- EXISTING PROPERTY LINE
- ROOF OVERHANG
- CHAIN LINK FENCE
- WOOD FENCE
- DECORATIVE FENCE
- NEW BUILDING
- FIRE DEPARTMENT ACCESS. PER DSA #01-110885
- FIRE DEPARTMENT ACCESS IS 20'-0" WIDE AND RATED FOR 96,000 LBS.
- (E) DRY STAND PIPE
- (E) FIRE HYDRANT
- DRINKING FOUNTAIN
- (E) SIGN
- EXISTING BUILDING
- EXISTING RESTROOMS

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01 -119834 INC: 0
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 5/20/2022
(DSA STAMP AREA)

SUGIMURA FINNEY ARCHITECTS
SFA
ARCHITECTURE INTERIORS PLANNING
2155 SOUTH BASCOM AVE.
SUITE 200
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PHONE: 408-879-0609
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LICENSED ARCHITECT
MARK C. FINNEY
NO. C-24573
RENEW 9-30-21
STATE OF CALIFORNIA

SITE PLAN
FIRE LIFE SAFETY

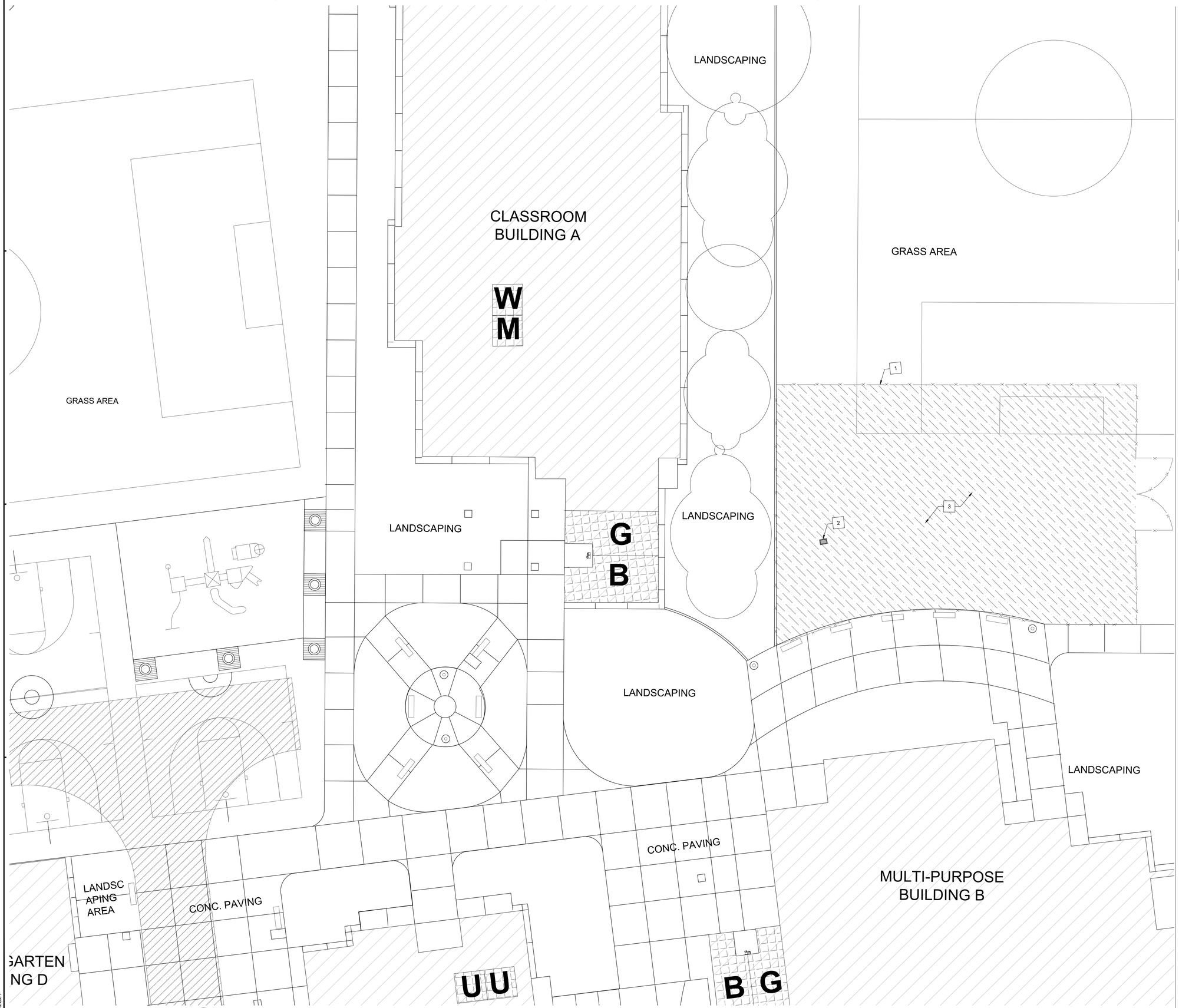
SAN ANTONIO ELEMENTARY SCHOOL SHADE STRUCTURE
1721 E SAN ANTONIO ST., SAN JOSE, CA 95116
ALUM ROCK UNION SCHOOL DISTRICT

REVISIONS
NO. ITEM DATE

DRAWN BY: MK
CHECKED BY: NJ
SFA JOB NO: 21077 DATE: 08/27/2021

T3

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GENERAL NOTES

A. CONTRACTOR VERIFY EXISTING UNDERGROUND UTILITIES LOCATION PRIOR TO DRILLING FOR CONCRETE FOOTINGS OF THE NEW SHADE STRUCTURE INSTALLATION. SEE REFERENCE DRAWINGS A2 GRADING AND STORM DRAINAGE & P1 PLUMBING PLOT PLAN DSA #21116

B. GENERAL CONTRACTOR IS RESPONSIBLE FOR REROUTING (E) IRRIGATION SYSTEM TO PREPARE DESIGNATED AREA TO RECEIVE (N) AC PAVING.

DEMOLITION SITE PLAN NOTES

1. CONTRACTOR TO PROVIDE TEMPORARY FENCE PER DET. 1/A0.3 TO REMAIN DURING ENTIRE CONSTRUCTION PERIOD. SEE A0.2 FOR DIRECT SHADE STRUCTURE LOCATION.
2. (E) STORM DRAIN. SHOWN DIAGRAMMATICALLY. FOR EXACT LOCATION SEE REFERENCE DRAWING C.5.1 STORM DRAIN AND SANITARY SEWER PLAN DSA #01-110885.
3. REROUTE (E) IRRIGATION SYSTEM.

GRAPHIC KEY

- ROOF OVERHANG
- CHAINLINK FENCE
- DECORATIVE FENCE
- AREA OF WORK
- EXISTING BUILDING
- EXISTING RESTROOMS
- (E) DRY STAND PIPE
- DRINKING FOUNTAIN
- (E) FIRE HYDRANT
- (E) SIGN
- **M** (E) MENS TOILET ROOM
- **W** (E) WOMENS TOILET ROOM
- **G** (E) GIRLS TOILET ROOM
- **B** (E) BOYS TOILET ROOM
- **U** (E) UNISEX TOILET ROOM

1 DEMOLITION SITE PLAN ENLARGED

1" = 20'-0"

0 5' 10' 20' 40' 80'

TRUE NORTH / REF NORTH

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IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01 -119834 INC: 0
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 5/20/2022
(DSA STAMP AREA)

SUGIMURA FINNEY ARCHITECTS
SFA
ARCHITECTURE INTERIORS PLANNING
2155 SOUTH BASCOM AVE.
SUITE 200
CAMPBELL, CA 95008
PHONE: 408-879-6009
FAX: 408-377-6066

LICENSED ARCHITECT
MARK C. FINNEY
NO. C-24673
REN. 9-30-23
STATE OF CALIFORNIA

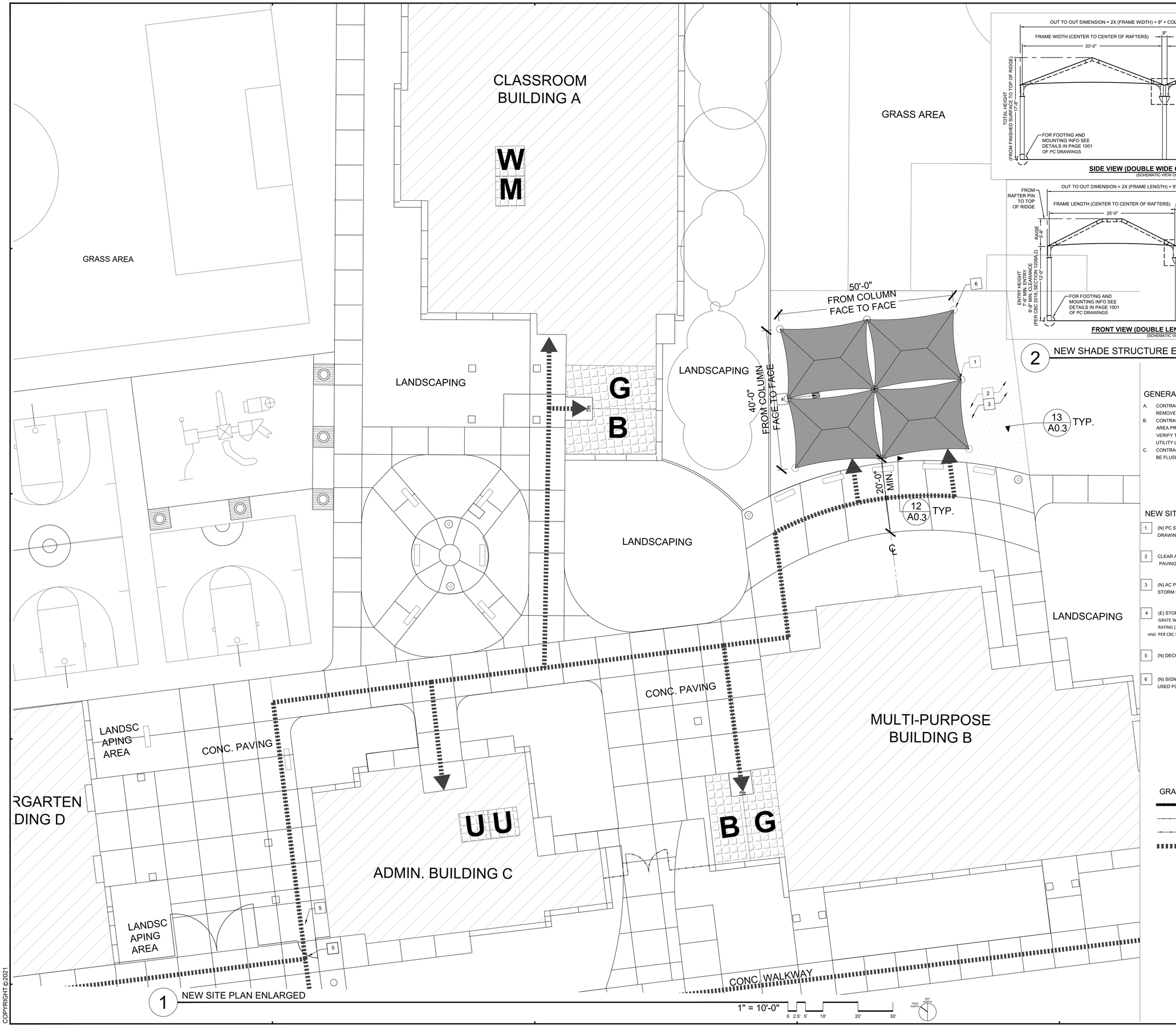
DEMOLITION SITE PLAN ENLARGED

SAN ANTONIO ELEMENTARY SCHOOL SHADE STRUCTURE
1721 E SAN ANTONIO ST., SAN JOSE, CA 95116
ALUM ROCK UNION SCHOOL DISTRICT

REVISIONS
NO. ITEM DATE

DRAWN BY: MK
CHECKED BY: NJ
SFA JOB NO: 21077
DATE: 08/27//2021

A0.1



SIDE VIEW (DOUBLE WIDE CONFIGURATION)
(SCHEMATIC VIEW ONLY)

FRONT VIEW (DOUBLE LENGTH CONFIGURATION)
(SCHEMATIC VIEW ONLY)

2 NEW SHADE STRUCTURE ELEVATIONS

N.T.S

GENERAL NOTES

A. CONTRACTOR TO VERIFY ALL BARRIERS IN P.O.T. HAVE BEEN REMOVED.
B. CONTRACTOR SHALL SURVEY (N) SHADE STRUCTURE INSTALLATION AREA PRIOR TO DRILLING FOR CONCRETE FOOTING POUR TO VERIFY THERE IS NO CONFLICT WITH THE EXISTING UNDERGROUND UTILITY LINES.
C. CONTRACTOR TO RAISE THE (E) STORM DRAIN AS NECESSARY TO BE FLUSH WITH THE (N) AC PAVING.

NEW SITE PLAN NOTES

1. (N) PC SHADE STRUCTURE DSA #04-119454, SEE MANUFACTURER'S DRAWINGS.
2. CLEAR AND GRADE (E) AREA AND PREPARE TO RECEIVE (N) AC PAVING .
3. (N) AC PAVING PER. DET 2/A0.3 & 5/A0.3. SLOPE (N) AC PAVING TO (E) STORM DRAIN FROM ALL DIRECTIONS. 2 % MAX.
4. (E) STORM DRAIN. SHOWN DIAGRAMMATICALLY. REPLACE EXISTING GRATE WITH A NEW ADA COMPLIANT AREA DRAIN GRATE IN PEDESTRIAN LOAD RATING (JENSEN PRECAST OR EQ.) MAX. SLOT OPENING 1/2" IN DIRECTION OF TRAVEL AND PER CBC SECTION 11B-302.3 FLUSH WITH NEW AC PAVING.
5. (N) DECORATIVE FENCE ACCESSIBLE GATE. SEE DET. 8/A0.3.
6. (N) SIGN POSTED TO READ "COOKING PROHIBITED" & "NOT TO BE USED FOR STORAGE".

GRAPHIC KEY

- ROOF OVERHANG
- CHAINLINK FENCE
- DECORATIVE FENCE
- PATH OF TRAVEL. SEE T2 FOR ENTIRE LENGTH
- NEW BUILDING
- EXISTING BUILDING
- (E) DRY STAND PIPE
- DRINKING FOUNTAIN
- (E) FIRE HYDRANT
- (E) SIGN
- **M** (E) MENS TOILET ROOM
- **W** (E) WOMENS TOILET ROOM
- **G** (E) GIRLS TOILET ROOM
- **B** (E) BOYS TOILET ROOM
- **U** (E) UNISEX TOILET ROOM
- **K** (E) KINDERGARTEN TOILET ROOM

1 NEW SITE PLAN ENLARGED

1" = 10'-0"

0 2.5' 5' 10' 20' 30'

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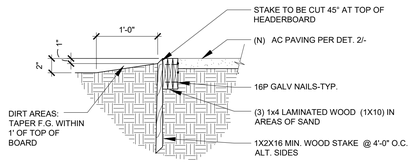
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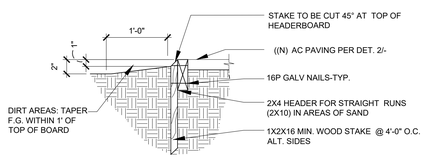
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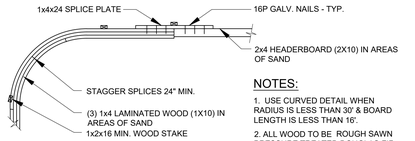
A0.2



CURVED SECTION



STRAIGHT SECTION

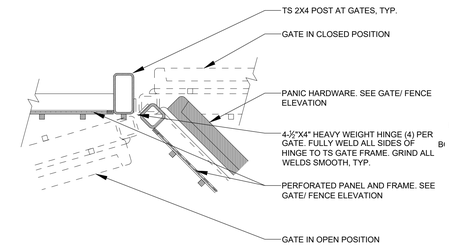


NOTES:

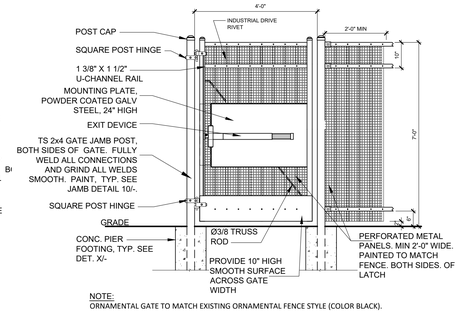
1. USE CURVED DETAIL WHEN RADIUS IS LESS THAN 30' & BOARD LENGTH IS LESS THAN 16'.
2. ALL WOOD TO BE ROUGH SAWN PRESSURE TREATED DOUGLAS FIR.

PLAN VIEW

13 WOOD HEADERBOARD — N.T.S

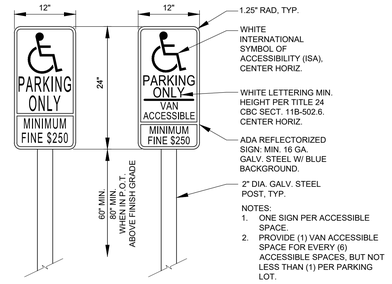


10 JAMB DETAIL — 1-1/2"=1'-0"



NOTE:
ORNAMENTAL GATE TO MATCH EXISTING ORNAMENTAL FENCE STYLE (COLOR BLACK).

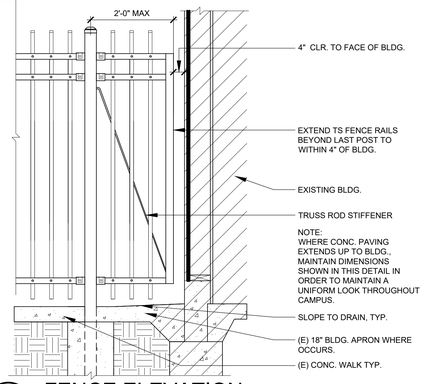
8 DECORATIVE FENCE SINGLE GATE — WITH PANIC HARDWARE — 1/2"=1'-0"



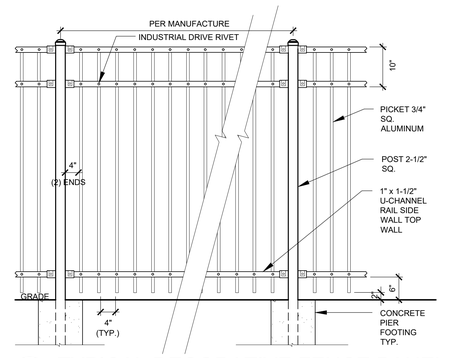
NOTES:

1. ONE SIGN PER ACCESSIBLE SPACE.
2. PROVIDE (1) VAN ACCESSIBLE SPACE FOR EVERY (6) ACCESSIBLE SPACES, BUT NOT LESS THAN (1) PER PARKING LOT.

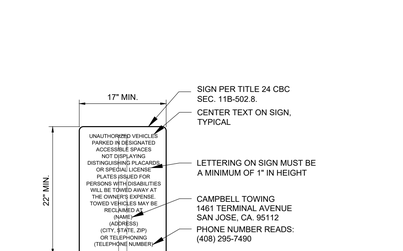
4 ACCESSIBLE PARKING SIGNS — ELEVATION — 1"=1'-0"



9 FENCE ELEVATION — AT BUILDING EXTERIOR WALL — 3/4"=1'-0"



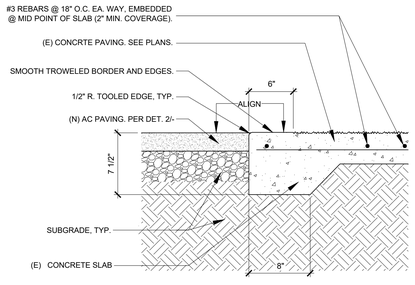
7 TYPICAL DECORATIVE FENCE PANEL — 3/4"=1'-0"



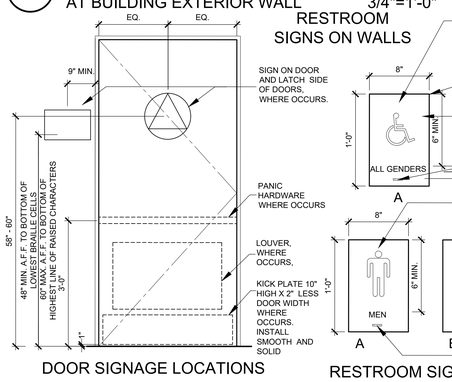
NOTE:

1. SIGN SHALL BE REFLECTORIZED FINISH.

3 ENTRY WARNING SIGN — 1"=1'-0"

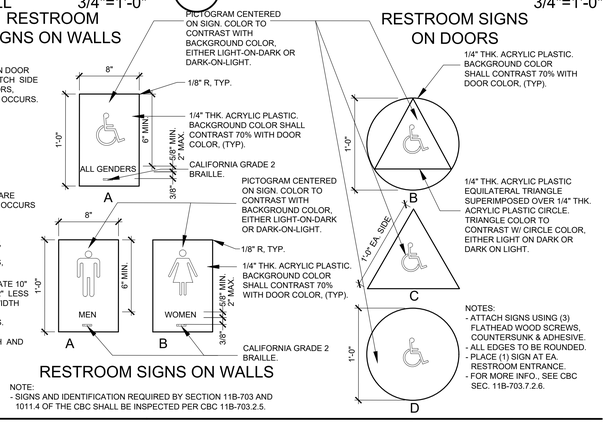


12 ASPHALT / CONCRETE JOINT — 1-1/2"=1'-0"



DOOR SIGNAGE LOCATIONS

RESTROOM SIGNS ON WALLS

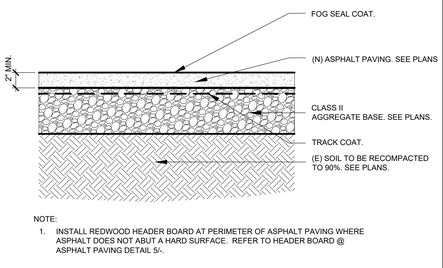


RESTROOM SIGNS ON DOORS

NOTES:
- ATTACH SIGNS USING (3) FLATHEAD WOOD SCREWS, COUNTERSUNK & ADHESIVE.
- ALL EDGES TO BE ROUNDED.
- PLACE (1) SIGN AT EA. RESTROOM ENTRANCE.
- FOR MORE INFO., SEE CBC SEC. 11B-703.7.2.6.

NOTE:
- SIGNS AND IDENTIFICATION REQUIRED BY SECTION 11B-703 AND 1011.4 OF THE CBC SHALL BE INSPECTED PER CBC 11B-703.2.5.

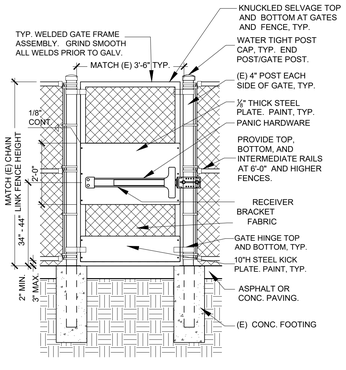
6 TYPICAL SIGNAGE — N.T.S.



NOTE:

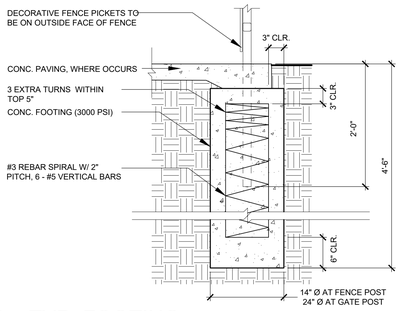
1. INSTALL REDWOOD HEADER BOARD AT PERIMETER OF ASPHALT PAVING WHERE ASPHALT DOES NOT ABUT A HARD SURFACE. REFER TO HEADER BOARD @ ASPHALT PAVING DETAIL 5/-.

2 (N) ASPHALT PAVING — 1-1/2"=1'-0"

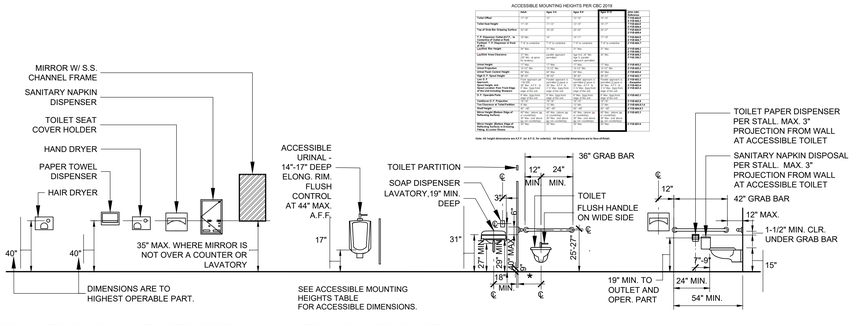


14 (N) CHAIN LINK SINGLE GATE — W/ PANIC HARDWARE — 1/2"=1'-0"

HARDWARE LOCKEY USA: D-6100 OUTSIDE TRIM
MAMMOTH180 SELF CLOSING HINGE SET
PANIC BAR: Detex V40 Value Series Panic Bar (#V40x628xHDx36 or V40x628xHDx48)



11 TYP. FOOTING — NTS

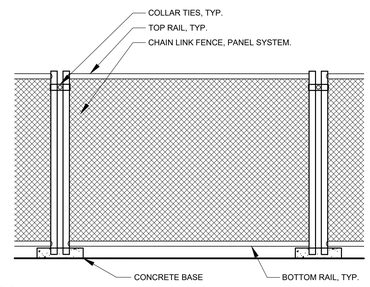


ACCESSIBLE MOUNTING HEIGHTS PER CBC 2019

	Adult	Ages 5-12	Ages 2-4	2001 DSA
Toilet Offset	12" min.	12" min.	12" min.	11B-604.7.1
Toilet Seat Height	17"-19"	11"-12"	9"-10"	11B-604.4
Top of Rail Bar Keying Surface	33"-36"	23"-25"	18"-20"	11B-604.5
T. P. Dispenser Centerline, to "a" Toilet Centerline, to Front of Fixture, Clear	7" min.	7" min.	7" min.	11B-604.7
T. P. Dispenser Mtng. Height	15" min.	10" min.	8" min.	11B-604.7
Lav/Sink Rim Height	31" max.	24" max.	20" max.	11B-606.3
Urinal Height	17" max.	11" max.	9" max.	11B-605.2
Urinal Flush Control Height	44" max.	30" max.	24" max.	11B-605.2
Lav Clear Knee Space Depth	25" min.	19" min.	13" min.	11B-606.2
Lav Clear Knee Space Width	30" min.	24" min.	20" min.	11B-606.2
Shelf, Coat Hook, Soap Dispenser, Towel Dispenser, Mirror Bottom Edge, and Trash Receptacle Opening Height	40" max.	30" max.	24" max.	11B-606.3
"a" Grab Bars	33"-36"	23"-25"	18"-20"	11B-604.5
Toilet Seat to "b" Grab Bar	12" min.	12" min.	12" min.	11B-604.5
Toilet Seat to "c" Grab Bar	24" min.	24" min.	24" min.	11B-604.5
Rear Wall Length	54" min.	42" min.	36" min.	11B-604.5
Side Wall Length (Measure Edge of Toilet to Partition Surface)	54" min.	42" min.	36" min.	11B-604.5
Mirror Height (Bottom Edge of Reflective Surface)	40" max.	30" max.	24" max.	11B-603.3
Mirror Width (Minimum)	40" min.	30" min.	24" min.	11B-603.3

Note: All height dimensions are A.F.F. or A.F.G. for exterior. All horizontal dimensions are to face of finish.

5 TYPICAL FIXTURE MOUNTING HEIGHTS — SCALE: 1/4"=1'-0"



1 REQ'D TEMPORARY FENCING — CONSTRUCTION FENCING — 1/2"=1'-0"

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SITE DETAILS

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A0.3

USASHADE & Fabric Structures®

FABRIC SHADE STRUCTURE

DSA P.C. 04-119454

SITE SPECIFIC APPLICATION TITLE SHEET SHALL INCLUDE:

PARTIAL LIST OF APPLICABLE CODES

- 2019 CALIFORNIA ADMINISTRATIVE CODE (CAC), PART 1, TITLE 24 C.C.R.
- 2019 CALIFORNIA BUILDING CODE (CBC), PART 2, TITLE 24 C.C.R. (2018 INTERNATIONAL BUILDING CODE VOLUMES 1-2 AND 2019 CALIFORNIA AMENDMENTS)
- 2019 CALIFORNIA ELECTRICAL CODE (CEC) , PART 3, TITLE 24 C.C.R. (2017 NATIONAL ELECTRICAL CODE AND 2019 CALIFORNIA AMENDMENTS)
- 2019 CALIFORNIA MECHANICAL CODE (CMC), PART 4, TITLE 24 C.C.R. (2018 IAPMO UNIFORM MECHANICAL CODE AND 2019 CALIFORNIA AMENDMENTS)
- 2019 CALIFORNIA PLUMBING CODE (CPC), PART 5, TITLE 24 C.C.R. (2018 IAPMO UNIFORM PLUMBING CODE AND 2019 CALIFORNIA AMENDMENTS)
- 2019 CALIFORNIA ENERGY CODE (CEC), PART 6, TITLE 24 C.C.R.
- 2019 CALIFORNIA FIRE CODE, PART 9, TITLE 24 C.C.R. (2018 INTERNATIONAL FIRE CODE AND 2019 CALIFORNIA AMENDMENTS)
- 2019 CALIFORNIA EXISTING BUILDING CODE (CEBC), PART 10, TITLE 24 C.C.R. (2018 INTERNATIONAL EXISTING BUILDING CODE AND 2019 CALIFORNIA AMENDMENTS)
- 2019 CALIFORNIA GREEN BUILDING STANDARDS CODE (CALGREEN), PART 11, TITLE 24 C.C.R.
- 2019 CALIFORNIA REFERENCED STANDARDS CODE, PART 12, TITLE 24 C.C.R.
- TITLE 19 C.C.R., PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS
- 2016 ASME A17.1/CSA B44-13 SAFETY CODE FOR ELEVATORS AND ESCALATORS (PER 2019 CBC, PART 2, CHAPTER 35)

NOTE: CALOSHA ELEVATOR UNIT ENFORCES C.C.R. TITLE 8 AND USES THE 2004 ASME A17.1 BY ADOPTION

PARTIAL LIST OF APPLICABLE STANDARDS

NFPA 13	STANDARD FOR THE INSTALLATION OF SPRINKLER SYSTEMS (CA AMENDED)	2016 EDITION
NFPA 14	STANDARD FOR THE INSTALLATION OF STANDPIPE AND HOSE SYSTEMS	2016 EDITION
NFPA 17	STANDARD FOR DRY CHEMICAL EXTINGUISHING SYSTEMS	2017 EDITION
NFPA 17A	STANDARD FOR WET CHEMICAL EXTINGUISHING SYSTEMS	2017 EDITION
NFPA 20	STANDARD FOR THE INSTALLATION OF STATIONARY PUMPS FOR FIRE PROTECTION	2016 EDITION
NFPA 22	STANDARD FOR WATER TANKS FOR PRIVATE FIRE PROTECTION	2013 EDITION
NFPA 24	STANDARD FOR THE INSTALLATION OF PRIVATE FIRE SERVICE MAINS AND THEIR APPURTENANCES	2016 EDITION
NFPA 72	NATIONAL FIRE ALARM & SIGNALING CODE (CA AMENDED)	2016 EDITION
NFPA 80	STANDARD FOR FIRE DOORS AND OTHER OPENING PROTECTIVES	2016 EDITION
NFPA 2001	STANDARD ON CLEAN AGENT FIRE EXTINGUISHING SYSTEMS	2015 EDITION
UL 300	STANDARD FOR FIRE TESTING OF FIRE EXTINGUISHING SYSTEMS FOR PROTECTION OF COMMERCIAL COOKING EQUIPMENT	2005 (R2010)
UL 464	AUDIBLE SIGNALING DEVICES FOR FIRE ALARM AND SIGNALING SYSTEMS, INCLUDING ACCESSORIES	2003 EDITION
UL521	STANDARD FOR HEAT DETECTORS FOR FIRE PROTECTIVE SIGNALING SYSTEMS	1999 EDITION
UL 1971	STANDARD FOR SIGNALING DEVICES FOR THE HEARING IMPAIRED	2002 (R2010)
ICC 300	SANDARD FOR BLEACHERS, FOLDING AND TELESCOPIC SEATING AND GRANDSTANDS	2017 EDITION

FOR A COMPLETE LIST OF APPLICABLE NFPA STANDARDS REFER TO 2019 CBC (SFM) CHAPTER 35 AND CALIFORNIA FIRE CODE CHAPTER 80.

SEE CALIFORNIA BUILDING CODE, CHAPTER 35, FOR STATE OF CALIFORNIA AMENDMENTS TO THE NFPA STANDARDS.

SEE INDIVIDUAL STRUCTURAL DRAWINGS FOR SPECIFIC DESIGN NOTES AND LOADING.

ALL WORK SHALL CONFORM TO 2019 EDITION TITLE 24, CALIFORNIA CODE OF REGULATIONS (C.C.R.).

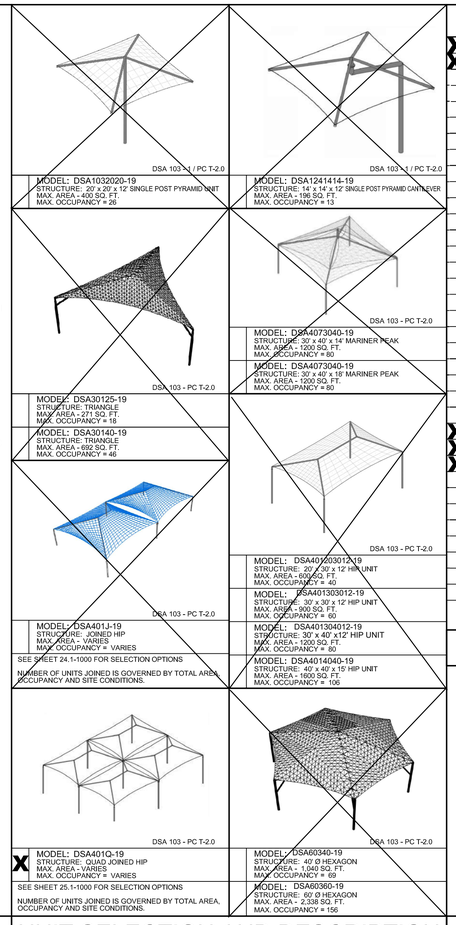
ALL WORK SHALL BE IN COMPLIANCE WITH CFC CHAPTER 33 - FIRE SAFETY DURING CONSTRUCTION AND DEMOLITION.

BUILDING CODE DATA

SITE SPECIFIC APPLICATION SITE PLAN SHALL INCLUDE:

1. ACTUAL DIMENSIONS OF SHADE STRUCTURES.
2. DIMENSIONS FROM ADJACENT STRUCTURES AND PROXIMITY OF ASSUMED OR ACTUAL PROPERTY LINES.
3. PROVIDE CODE ANALYSIS INCLUDING ACTUAL SHADE STRUCTURE AREA (SQ. FT.), OCCUPANCY TYPE (A-3), AND TYPE OF CONSTRUCTION (V-B). INDICATE OCCUPANT LOAD FACTOR per 2019 CBC, SECTION 1004.
4. INDICATE LOCATIONS OF FIRE EXTINGUISHER WITHIN 75 FEET.
5. SHOW LOCATIONS OF AUDIBLE FIRE ALARM.
6. INDICATE DIMENSIONS FROM THE ROOF TO THE HIGHER STRUCTURE OR TERRAIN FEATURE. MINIMUM DIMENSION OF 20' FOR SNOW LOAD MODEL (ASCE 7-16).
7. ACTUAL SITE ELEVATION (FT.) TO DETERMINE SITE OCCURS AT OR BELOW THE UPPER ELEVATION LIMIT FOR THE GROUND SNOW LOAD SHOWN IN ASCE 7-16 (FOR SNOW LOAD MODEL).
8. FOR RECESSED BASE PLATE (RBP) OPTION: ARCHITECT/ENGINEER OF RECORD TO SPECIFY THE LOWEST ANTICIPATED SERVICE TEMPERATURE (LAST), AS DEFINED IN AISC 341-10 SECTION A.3.4b, A4.1 AND A4.2 PER NOTE ON EACH INDIVIDUAL MODEL ENGINEERING DRAWING WHICH RELATES TO DEMAND CRITICAL WELD AND "L.A.S.T." TEMPERATURE (EITHER STRUCTURAL STEEL NOTE #14).
9. COMPLETE SCOPE OF WORK INCLUDING THE SHADE STRUCTURE MODEL NUMBER, P.C. NUMBER, AND SPECIFIC SIZE OF SHADE STRUCTURE.
10. ALL SADDLES, CLAMPS AND FITTINGS SHALL CONFORM TO THE GUIDELINES AS SPECIFIED IN APPENDICES "A, B & C" RESPECTIVELY IN ASCE 19-16, "STRUCTURAL APPLICATIONS OF STEEL CABLES FOR BUILDINGS."
11. ARCHITECTS OF RECORD TO DETERMINE IF SPECIFIC SITE IS IN MAPPED GEOLOGIC HAZARD ZONE. GEOHAZARD REPORT REQUIREMENTS PER DSA IR A-4.
12. ARCHITECTS OF RECORD TO DETERMINE IF SPECIFIC SITE IS IN A MAPPED FIRE HAZARD SEVERITY ZONE OR WILDLAND INTERFACE AREA.

GENERAL NOTES



UNIT SELECTION AND DESCRIPTION

	DRAWING NUMBER	DRAWING DESCRIPTION	STRUCTURE TYPE	MAX SIZE	MODEL NUMBER
X	P.C. T-1.0	P.C. TITLE SHEET			
X	P.C. T-2.0	DSA 103 SAMPLE FORM			
	P.C. T-2.1	DSA 103 SAMPLE FORM			
	P.C. T-3.0	DSA 103 SAMPLE FORM			
	P.C. T-3.1	DSA 103 SAMPLE FORM			
	16.1-1000	PRODUCT INFORMATION	SINGLE POST PYRAMID	20 X 20	DSA1032020-19
	16.2-2000	REACTIONS	SINGLE POST PYRAMID	20 X 20	DSA1032020-19
	17.1-1000	PRODUCT INFORMATION	SINGLE POST PYRAMID CANTILEVER	14 X 14	DSA1241414-19
	17.2-2000	REACTIONS	SINGLE POST PYRAMID CANTILEVER	14 X 14	DSA1241414-19
	18.1-1000	PRODUCT INFORMATION	TRIANGLE	25 X 25	DSA30125-19
	18.2-2000	REACTIONS	TRIANGLE	25 X 25	DSA30125-19
	19.1-1000	PRODUCT INFORMATION	TRIANGLE	40 X 40	DSA30140-19
	19.2-2000	REACTIONS	TRIANGLE	40 X 40	DSA30140-19
	20.1-1000	PRODUCT INFORMATION	HIP	20 X 30	DSA401203012-19
	20.2-2000	REACTIONS	HIP	20 X 30	DSA401203012-19
	21.1-1000	PRODUCT INFORMATION	HIP	30 X 30	DSA401303012-19
	21.2-2000	REACTIONS	HIP	30 X 30	DSA401303012-19
	22.1-1000	PRODUCT INFORMATION	HIP	30 X 40	DSA401304012-19
	22.2-2000	REACTIONS	HIP	30 X 40	DSA401304012-19
	23.1-1000	PRODUCT INFORMATION	HIP	40 X 40	DSA4014040-19
	23.2-2000	REACTIONS	HIP	40 X 40	DSA4014040-19
	24.1-1000	PRODUCT INFORMATION	JOINED HIPS	VARIES	DSA401J-19
	24.1-1001	DETAILS	JOINED HIPS	VARIES	DSA401J-19
	24.3-2000	REACTIONS	JOINED HIPS	VARIES	DSA401J-19
X	25.1-1000	PRODUCT INFORMATION	QUAD JOINED HIPS	VARIES	DSA401Q-19
X	25.2-1001	DETAILS	QUAD JOINED HIPS	VARIES	DSA401Q-19
X	25.3-2000	REACTIONS	QUAD JOINED HIPS	VARIES	DSA401Q-19
	26.1-1000	PRODUCT INFORMATION	HEXAGON	40 Ø	DSA60340-19
	26.2-2000	REACTIONS	HEXAGON	40 Ø	DSA60340-19
	27.1-1000	PRODUCT INFORMATION	HEXAGON	60 Ø	DSA60360-19
	27.2-2000	REACTIONS	HEXAGON	60 Ø	DSA60360-19
	28.1-1000	PRODUCT INFORMATION	MARINER PEAK	30 X 40	DSA4073040-19
	28.2-2000	REACTIONS	MARINER PEAK	30 X 40	DSA4073040-19

TOTAL SHEET COUNT: 33 SHEETS

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ARCHITECT

ENGINEER

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01 -119834 INC: 0
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 5/20/2022

THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN

USASHADE & Fabric Structures®

CORPORATE HEADQUARTERS
2580 ESTERS BLVD. SUITE 100
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800-966-5005

CERTIFICATIONS:

IAS CERTIFICATION No: FA-428

CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:

Alum Rock Union ES Dist

PROJECT NAME:

San Antonio Elementary

LOCATION:

1721 E San Antonio St.
San Jose, CA 95116

MODEL NUMBER:

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-119454 PC
REVIEWED FOR
SS ☑ FLS ☑ ACS ☒ CG ☐
DATE: 03/26/2021

STRUCTURE TYPE:

SCALE : VARIES

DRAWING SIZE: D

PRE-CHECK (PC) DOCUMENT
Code : 2019 CBC
A separate project application for construction is required.

Eng. By :	**DWH**	**10/16/20**
Design By :	**DWH**	**10/16/20**
Approved By :	**DWH**	**10/16/20**

DRAWING DESCRIPTION:

P.C. TITLE SHEET

DWG.

SHEET

P.C. T-1.0

REV.

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS, 2019 CBC

Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

2019 CBC

IMPORTANT: This form is only a summary list of structural tests and some of the special inspections required for the project. Generally, the structural tests and special inspections noted on this form are those that will be performed by the Geotechnical Engineer of Record, Laboratory of Record, or Special Inspector. The actual complete test and inspection program must be performed as detailed on the DSA approved documents. The appendix at the bottom of this form identifies work NOT subject to DSA requirements for special inspection or structural testing. The project inspector is responsible for providing inspection of all facets of construction, including but not limited to, special inspections not listed on this form such as structural wood framing, high-load wood diaphragms, cold-formed steel framing, anchorage of non-structural components, etc., per Title 24, Part 2, Chapter 17A (2019 CBC).

****NOTE:** Undefined section and table references found in this document are from the CBC, or California Building Code.

KEY TO COLUMNS

1. TYPE	2. PERFORMED BY
Continuous – Indicates that a continuous special inspection is required	**GE** – Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative.
Periodic – Indicates that a periodic special inspection is required	**LOR** – Indicates that the test or special inspection shall be performed by a testing laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program. See CAC Section 4-335.
Test – Indicates that a test is required	**PI** – Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA.
	SI – Indicates that the special inspection shall be performed by an appropriately qualified/approved special inspector.

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2019 CBC

Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

Geotechnical Reports: Project does NOT have and does NOT require a geotechnical report

	1. GENERAL:	Table 1705A.6		
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Verify that: • Site has been prepared properly prior to placement of controlled fill and/or excavations for foundations. • Foundation excavations are extended to proper depth and have reached proper material. • Materials below footings are adequate to achieve the design bearing capacity.	**See Notes**	**PI**	Refer to specific items identified in the Appendix listing exemptions for limitations. Placement of controlled fill exceeding 12" depth under foundations is not permitted without a geotechnical report.

	2. SOIL COMPACTION AND FILL:	Table 1705A.6		
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Verify use of proper materials, densities and inspect lift thicknesses, placement and compaction during placement of fill.	**Continuous**	**LOR***	* Under the supervision of a geotechnical engineer or LOR's engineering manager. Refer to specific items identified in the Appendix listing exemptions for limitations.
☑	**b.** Compaction testing.	**Test**	**LOR***	* Under the supervision of a geotechnical engineer or LOR's engineering manager. Refer to specific items identified in the Appendix listing exemptions for limitations.

	3. DRIVEN DEEP FOUNDATIONS (PILES):	Table 1705A.7

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2019 CBC

Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Placement, compaction and inspection of backfill.	**Continuous**	**GE***	**1705A.6.1.** * By geotechnical engineer or his or her qualified representative. (See Section 2 above).
☐	**b.** Placement of soil reinforcement and/or drainage devices.	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative.
☐	**c.** Segmental retaining walls; inspect placement of units, dowels, connectors, etc.	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative. See DSA IR 16-3.
☐	**d.** Concrete retaining walls.	Provide tests and inspections per CONCRETE section below.		
☐	**e.** Masonry retaining walls.	Provide tests and inspections per MASONRY section below.		

	4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS):	Table 1705A.8		
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2019 CBC

Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

☑	**a.** Inspect drilling operations and maintain complete and accurate records for each pier.	**Continuous**	**PI**	Continuous inspection to be provided by project inspector. Refer to specific items identified in the Appendix listing exemptions for limitations.
☑	**b.** Verify pier locations, diameters, plumbness and lengths.Record concrete or grout volumes.	**Continuous**	**PI**	Continuous inspection to be provided by project inspector. Refer to specific items identified in the Appendix listing exemptions for limitations.
☑	**c.** Concrete piers.	Provide tests and inspections per CONCRETE section below.		

	5. RETAINING WALLS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Placement, compaction and inspection of backfill.	**Continuous**	**GE***	**1705A.6.1.** * By geotechnical engineer or his or her qualified representative. (See Section 2 above).
☐	**b.** Placement of soil reinforcement and/or drainage devices.	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative.
☐	**c.** Segmental retaining walls; inspect placement of units, dowels, connectors, etc.	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative. See DSA IR 16-3.
☐	**d.** Concrete retaining walls.	Provide tests and inspections per CONCRETE section below.		
☐	**e.** Masonry retaining walls.	Provide tests and inspections per MASONRY section below.		

	6. OTHER SOILS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2019 CBC

Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
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☐	**a.** Soil Improvements	**Test**	**GE***	Submit a comprehensive report documenting final soil improvements constructed, construction observation and the results of the confirmation testing and analysis to CGS for final acceptance. * By geotechnical engineer or his or her qualified representative.
☐	**b.** Inspection of Soil Improvements	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative.
☐	**c.**			

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Concrete), 2019 CBC

Table 1705A.3; ACI 318-14 Sections 26.12 & 26.13
Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Sample and test prestressing tendons and anchorages.	**Test**	**LOR**	**1705A.3.4, 1910A.3**
☐	**b.** Inspect placement of prestressing tendons.	**Periodic**	**SI**	**1705A.3.4, Table 1705A.3 Items 1 & 9.**
☐	**c.** Verify in-situ concrete strength prior to stressing of post-tensioning tendons.	**Periodic**	**SI**	**Table 1705A.3 Item 11.** Special inspector to verify specified concrete strength test prior to stressing.
☐	**d.** Inspect application of post-tensioning or prestressing forces and grouting of bonded prestressing tendons.	**Continuous**	**SI**	**1705A.3.4, Table 1705A.3 Item 9;** ACI 318-14 Section 26.13

	9. PRECAST CONCRETE (in addition to Cast-in-Place Concrete tests and inspections):			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Inspect fabrication of precast concrete members.	**Continuous**	**SI**	ACI 318-14 Section 26.13.
☐	**b.** Inspect erection of precast concrete members.	**Periodic**	**SI***	**Table 1705A.3 Item 10.** * May be performed by PI when specifically approved by DSA.

	10. SHOTCRETE (in addition to Cast-in-Place Concrete tests and inspections):			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Concrete), 2019 CBC

Table 1705A.3; ACI 318-14 Sections 26.12 & 26.13
Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

☐	**a.** Inspect shotcrete placement for proper application techniques.	**Continuous**	**SI**	**1705A.19, Table 1705A.3 Item 7, 1908A.6, 1908A.7, 1908A.8, 1908A.9, 1908A.11, 1908A.12.** See ACI 506.2-13 Section 3.4, ACI 506R-16.
☐	**b.** Sample and test shotcrete (f'c).	**Test**	**LOR**	**1908A.5, 1908A.10.**

	11. POST-INSTALLED ANCHORS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Inspect installation of post-installed anchors	**See Notes**	**SI***	**1617A.1.19, Table 1705A.3 Item 4a (Continuous) & 4b (Periodic), 1705A.3.8** (See Appendix for exemptions). ACI 318-14 Sections 17.8 & 26.13. * May be performed by the project inspector when specifically approved by DSA.
☐	**b.** Test post-installed anchors.	**Test**	**LOR**	**1910A.5.** (See Appendix for exemptions.)

	12. OTHER CONCRETE:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.**			

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Concrete), 2019 CBC

Table 1705A.3; ACI 318-14 Sections 26.12 & 26.13
Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

☐	**a.** Inspect shotcrete placement for proper application techniques.	**Continuous**	**SI**	**1705A.19, Table 1705A.3 Item 7, 1908A.6, 1908A.7, 1908A.8, 1908A.9, 1908A.11, 1908A.12.** See ACI 506.2-13 Section 3.4, ACI 506R-16.
☐	**b.** Sample and test shotcrete (f'c).	**Test**	**LOR**	**1908A.5, 1908A.10.**

	11. POST-INSTALLED ANCHORS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Inspect installation of post-installed anchors	**See Notes**	**SI***	**1617A.1.19, Table 1705A.3 Item 4a (Continuous) & 4b (Periodic), 1705A.3.8** (See Appendix for exemptions). ACI 318-14 Sections 17.8 & 26.13. * May be performed by the project inspector when specifically approved by DSA.
☐	**b.** Test post-installed anchors.	**Test**	**LOR**	**1910A.5.** (See Appendix for exemptions.)

	12. OTHER CONCRETE:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.**			

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Concrete), 2019 CBC

Table 1705A.3; ACI 318-14 Sections 26.12 & 26.13
Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

☐	**a.** Inspect shotcrete placement for proper application techniques.	**Continuous**	**SI**	**1705A.19, Table 1705A.3 Item 7, 1908A.6, 1908A.7, 1908A.8, 1908A.9, 1908A.11, 1908A.12.** See ACI 506.2-13 Section 3.4, ACI 506R-16.
☐	**b.** Sample and test shotcrete (f'c).	**Test**	**LOR**	**1908A.5, 1908A.10.**

	11. POST-INSTALLED ANCHORS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Inspect installation of post-installed anchors	**See Notes**	**SI***	**1617A.1.19, Table 1705A.3 Item 4a (Continuous) & 4b (Periodic), 1705A.3.8** (See Appendix for exemptions). ACI 318-14 Sections 17.8 & 26.13. * May be performed by the project inspector when specifically approved by DSA.
☐	**b.** Test post-installed anchors.	**Test**	**LOR**	**1910A.5.** (See Appendix for exemptions.)

	12. OTHER CONCRETE:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.**			

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Steel and Aluminum), 2019 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-16
Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

☐	**b.** Test high-strength bolts, nuts and washers.	**Test**	**LOR**	**Table 1705A.2.1 Item 1c, 2213A.1;** RCSC 2014 Section 7.2; DSA IR 17-8.
	Inspection of High-Strength Bolt Installation:			
☐	**c.** Bearing-type ("snug tight") connections.	**Periodic**	**SI**	**Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2**; AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Section 9.1; DSA IR 17-9.
☐	**d.** Pretensioned and slip-critical connections.	*	**SI**	**Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2**; AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. * "Continuous" or "Periodic" depends on the tightening method used.

	19. WELDING:	1705A.2.5, Table 1705A.2.1 Items 4 & 5; AWS D1.1 and AWS D1.8 for structural steel; AWS D1.2 for Aluminum; AWS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-3 (See Appendix for exemptions.)		
	Verification of Materials, Equipment, Welders, etc.:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Verify weld filler material identification markings per AWS designation listed on the DSA-approved documents and the WPS.	**Periodic**	**SI**	DSA IR 17-3.
☑	**b.** Verify weld filler material manufacturer's certificate of compliance.	**Periodic**	**SI**	DSA IR 17-3.
☑	**c.** Verify WPS, welder qualifications and equipment.	**Periodic**	**SI**	DSA IR 17-3.

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Steel and Aluminum), 2019 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-16
Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

	19.1 SHOP WELDING:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Inspect groove welds, multi-pass fillet welds, single pass fillet welds > 5/16", plug and slot welds.	**Continuous**	**SI**	**Table 1705A.2.1 Items 5a.1–4;** AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.
☑	**b.** Inspect single-pass fillet welds ≤ 5/16", floor and roof deck welds.	**Periodic**	**SI**	**1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6;** AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.
☐	**c.** Inspect welding of stairs and railing systems.	**Periodic**	**SI**	**1705A.2.1;** AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3.
☐	**d.** Verification of reinforcing steel weldability other than ASTM A706.	**Periodic**	**SI**	**1705A.3.1;** AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.
☐	**e.** Inspect welding of reinforcing steel.	**Continuous**	**SI**	**Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8;** AWS D1.4; DSA IR 17-3.

	19.2 FIELD WELDING:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Inspect groove welds, multi-pass fillet welds, single pass fillet welds > 5/16", plug and slot welds.	**Continuous**	**SI**	**Table 1705A.2.1 Items 5a.1–4;** AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.
☐	**b.** Inspect single-pass fillet welds ≤ 5/16".	**Periodic**	**SI**	**Table 1705A.2.1 Item 5a.5;** AISC 360-16 (AISC 341-16 as applicable), DSA IR 17-3.

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Steel and Aluminum), 2019 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-16
Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
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☐	**c.** Inspect end-welded studs (ASTM A-108) installation (including bend test).	**Periodic**	**SI**	**2213A.2**; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1; DSA IR 17-3.
☐	**d.** Inspect floor and roof deck welds.	**Periodic**	**SI**	**1705A.2.2, Table 1705A.2.1 Item 5a.6;** AISC 360-16 (AISC 341-16 as applicable); AWS D1.3; DSA IR 17-3.
☐	**e.** Inspect welding of structural cold-formed steel.	**Periodic**	**SI***	**1705A.2.5; AWS D1.3; DSA IR 17-3.** The quality control provisions of AISI S240-15 Chapter D shall also apply. * May be performed by the project inspector when specifically approved by DSA.
☐	**f.** Inspect welding of stairs and railing systems.	**Periodic**	**SI***	**1705A.2.1;** AISC 360-16 (AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3. * May be performed by the project inspector when specifically approved by DSA.
☐	**g.** Verification of reinforcing steel weldability.	**Periodic**	**SI**	**1705A.3.1**; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.
☐	**h.** Inspect welding of reinforcing steel.	**Continuous**	**SI**	**Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8;** AWS D1.4; DSA IR 17-3.

	20. NONDESTRUCTIVE TESTING: **1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-16**			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Ultrasonic	**Test**	**LOR**	**1705A.2.1, 1705A.2.5;** AISC 341-16 J6.2. AISC 360-16 N5.5; ANSI/ ASNT CP-189, SNT-TC-1A; AWS D1.1, AWS D1.8; DSA IR 17-2.

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Steel and Aluminum), 2019 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-16
Application Number: 04-119454 | **School Name:** TO BE DETERMINED | **School District:** USA SHADE
DSA File Number: | **Increment Number:** | **Date Created:** 2021-03-26 08:15:52

	17. STRUCTURAL STEEL, COLD-FORMED STEEL AND ALUMINUM USED FOR STRUCTURAL PURPOSES			
	Material Verification and Testing:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Verify identification of all materials and: • Mill certificates indicate material properties that comply with requirements. • Material sizes, types and grades comply with requirements.	**Periodic**	*	**Table 1705A.2.1 Item 3a–3c**, 2202A.1; AISI S100-16 Section A3.1 & A3.2, AISI S240-15 Section D1.1 & A5, AISI S220-15 Sections A4 & A6. * By special inspector or qualified technician when performed off-site.
☑	**b.** Test unidentified materials	**Test**	**LOR**	**2202A.1.**
☑	**c.** Examine seam welds of HSS shapes	**Periodic**	**SI**	DSA IR 17-3.
	Inspection:			
☑	**d.** Verify and document steel fabrication per DSA-approved construction documents.	**Periodic**	**SI**	Not applicable to cold-formed steel light-frame construction, except for trusses (**1705A.2.4**).

	18. HIGH-STRENGTH BOLTS: RCSC 2014			
	Material Verification and Testing of High-Strength Bolts, Nuts and Washers:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Verify identification markings and manufacturer's certificates of compliance conform to ASTM standards specified in the DSA-approved documents.	**Periodic**	**SI**	**Table 1705A.2.1 Items 1a & 1b, 2202A.1;** AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.

THE SAMPLE DSA-103 FORM SHOWN ON THIS SHEET IS FOR ILLUSTRATION PURPOSES ONLY TO ASSIST IN THE COMPLETION OF FUTURE PROJECTS SPECIFIC FORM DSA-103. A CURRENT DSA-103 FORM IS TO BE COMPLETED FOR EACH APPLICATION THAT THIS PC IS BEING INCORPORATED INTO AND ALL SAMPLE DSA-103 SHEETS ARE TO BE CROSSED OUT ON THIS DRAWING.

ADDITIONAL TESTING AND INSPECTION NOTES:

1. THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE SELECTED BY THE SCHOOL DISTRICT AND APPROVED BY DSA AND THE ARCHITECT OF RECORD.
2. THE SITE PROJECT INSPECTOR SHALL BE CLASS 2.
3. THE COSTS OF THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE BORN BY THE SCHOOL DISTRICT.
4. COPIES OF VERIFIED REPORTS SHALL BE SENT TO DSA, THE ARCHITECT, THE SCHOOL DISTRICT, THE CONTRACTOR, AND THE PROJECT INSPECTOR.
5. THE IN PLANT INSPECTOR SHALL BE WELDING SPECIAL INSPECTOR FOR MATERIAL VERIFICATION AND WELDING.
6. PER 2019 CBC, SECTION 1705A.3.3, BATCH PLANT INSPECTION MAY BE WAIVED WHEN THE FOLLOWING REQUIREMENTS ARE MET:
   - 6.1. A LICENSED WEIGHMASTER SHALL POSITIVELY IDENTIFY QUANTITY OF MATERIALS AND CERTIFY EACH LOAD BY A BATCH TICKET.
   - 6.2. BATCH TICKETS, INCLUDING MATERIAL QUANTITIES AND WEIGHTS SHALL ACCOMPANY THE LOAD, SHALL BE TRANSMITTED TO THE INSPECTOR OF RECORD BY THE TRUCK DRIVER WITH LOAD IDENTIFIED THEREON. THE LOAD SHALL NOT BE PLACED WITHOUT A BATCH TICKET IDENTIFYING THE MIX. THE INSPECTOR OF RECORD SHALL KEEP A DAILY RECORD OF PLACEMENTS, IDENTIFYING EACH TRUCK IT'S LOAD, AND TIME OF RECEIPT AT THE JOBSITE, AND APPROXIMATE LOCATION OF DEPOSIT IN THE STRUCTURE AND SHALL MAINTAIN A COPY OF THE DAILY RECORD AS REQUIRED BY THE ENFORCEMENT AGENCY.

LICENSED ARCHITECT — DAVID HIGGINSON — NO. C19788 — REN 10-31-22 — STATE OF CALIFORNIA

SAMPLE DSA 103 - STATEMENT OF STRUCTURAL TESTS AND INSPECTIONS FORM - FOR ALL UNITS EXCEPT CANTILEVER AND SINGLE POST UNITS

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01 -119834 INC: 0
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 5/20/2022

THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN

USA SHADE & Fabric Structures
CORPORATE HEADQUARTERS
2580 ESTERS BLVD. SUITE 100
DFW AIRPORT, TX, 75261
800-966-5005

CERTIFICATIONS:
IAS CERTIFICATION No: FA-428
CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:
Alum Rock Union ES Dist

PROJECT NAME:
San Antonio Elementary

LOCATION:
1721 E San Antonio St.
San Jose, CA 95116

MODEL NUMBER:

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-119454 PC
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑ CG ☐
DATE: 03/26/2021

STRUCTURE TYPE:

SCALE : VARIES

DRAWING SIZE: D

PRE-CHECK (PC) DOCUMENT
Code : 2019 CBC
A separate project application for construction is required.

Eng. By :	DWH	10/16/20
Design By :	DWH	10/16/20
Approved By :	DWH	10/16/20

DRAWING DESCRIPTION:
DSA 103 SAMPLE FORM

DWG.

SHEET P.C. T-2.0

REV.

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Steel and Aluminum), 2019 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-16

Application Number: 04-119454
DSA File Number:
School Name: TO BE DETERMINED
Increment Number:
School District: USA SHADE
Date Created: 2021-03-26 08:15:52

☐	**b.** Magnetic Particle	**Test**	**LOR**	**1705A.2.1, 1705A.2.5;** AISC 341-16 J6.2. AISC 360-16 N5.5; ANSI/ ASNT CP-189, SNT-TC-1A; AWS D1.1, AWS D1.8; DSA IR 17-2.
☐	**c.**	**Test**	**LOR**	

	21. STEEL JOISTS AND TRUSSES: 1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-16			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Verify size, type and grade for all chord and web members as well as connectors and weld filler material; verify joist profile, dimensions and camber (if applicable); verify all weld locations, lengths and profiles; mark or tag each joist.	**Continuous**	**SI**	**1705A.2.3, Table 1705A.2.3;** AWS D1.1; DSA IR 22-3 for steel joists only. **1705A.2.4;** AWS D1.3 for cold-formed steel trusses.

	22. SPRAY APPLIED FIRE-PROOFING: 1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-16			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Examine structural steel surface conditions, inspect application, take samples, measure thickness and verify compliance of all aspects of application with DSA-approved documents.	**Periodic**	**SI**	**1705A.14.**
☐	**b.** Test bond strength.	**Test**	**LOR**	**1705A.14.6.**

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Steel and Aluminum), 2019 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-16

Application Number: 04-119454
DSA File Number:
School Name: TO BE DETERMINED
Increment Number:
School District: USA SHADE
Date Created: 2021-03-26 08:15:52

☐	**c.** Test density.	**Test**	**LOR**	**1705A.14.5.**

	23. ANCHOR BOLTS AND ANCHOR RODS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Anchor Bolts and Anchor Rods	**Test**	**LOR**	Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-11.
☐	**b.** Threaded rod not used for foundation anchorage.	**Test**	**LOR**	Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11.

	Other Steel			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.**			

Appendix: Work Exempt from DSA Requirements for Structural Tests / Special Inspections

Application Number: 04-119454
DSA File Number:
School Name: TO BE DETERMINED
Increment Number:
School District: USA SHADE
Date Created: 2021-03-26 08:15:52

Exempt items given in DSA IR A-22 or the 2019 CBC (including DSA amendments) and those items identified below with a check mark by the design professional are NOT subject to DSA requirements for the structural tests / special inspections noted. **Items marked as exempt shall be identified on the approved construction documents**. The project inspector shall verify all construction complies with the approved construction documents.

	SOILS:
☐	1. Deep foundations acting as a cantilever footing designed based on maximum allowable pressures per CBC Table 1806A.2 and having no geotechnical report for the following cases: A) free standing sign or scoreboard, B) cell or antenna towers and poles less than 35'-0" tall (e.g., lighting poles, flag poles, poles supporting open mesh fences, etc.), C) single-story structure with dead load less than 5 psf (e.g., open fabric shade structure), or D) covered walkway structure with an apex height less than 10'-0" above adjacent grade.
☐	2. Shallow foundations, etc. are exempt from special inspections and testing by a Geotechnical Engineer for the following cases: A) buildings without a geotechnical report and meeting the exception item #1 criteria in CBC Section 1803A.2 supported by native soil (any excavation depth) or fill soil (not exceeding 12" depth per CBC Section 1804A.5), B) soil scarification/recompaction not exceeding 12" depth, C) native or fill soil supporting exterior non-structural flatwork (e.g., sidewalks, site concrete ramps, site stairs, parking lots, driveways, etc.), D) unpaved landscaping and playground areas, or E) utility trench backfill.

	CONCRETE/MASONRY:
☐	1. Post-installed anchors for the following: A) exempt non-structural components (e.g., mechanical, electrical, plumbing equipment - see item 7 for "Welding" given in CBC Section 1617A.1.18 (which replaces ASCE 7-16, Section 13.1.4) or B) interior nonstructural wall partitions meeting criteria listed in exempt item 3 for "Welding."
☐	2. Concrete batch plant inspection is not required for items given in CBC Section 1705A.3.3.2 subject to the requirements and limitations in that section.

Appendix: Work Exempt from DSA Requirements for Structural Tests / Special Inspections

Application Number: 04-119454
DSA File Number:
School Name: TO BE DETERMINED
Increment Number:
School District: USA SHADE
Date Created: 2021-03-26 08:15:52

☐	3. Non-bearing non-shear masonry walls may be exempt from certain DSA masonry testing and special inspection items as allowed per DSA IR 21-1.16. Refer to construction documents for specific exemptions accordingly for each applicable wall condition.
☐	4. Epoxy shear dowels in site flatwork and/or other non-structural concrete.
☐	5. Testing of reinforcing bars is not required for items given in CBC Section 1910A.2 subject to the requirements and limitations in that section.

	Welding:
☐	1. Solid-clad and open-mesh gates with maximum leaf span or rolling section for rolling gates of 10' and apex height less than 8'-0" above lowest adjacent grade. When located above circulation or occupied space below, these gates are not located within 1.5x gate/fence height (max 8'-0") to the edge of floor or roof.
☐	2. Handrails, guardrails, and modular or relocatable ramps associated with walking surfaces less than 30" above adjacent grade (excluding post base connections per the "Exception" language in Section 1705A.2.1); fillet welds shall not be ground flush.
☐	3. Non-structural interior cold-formed steel framing spanning less than 15'-0", such as in interior partitions, interior soffits, etc. supporting only self weight and light-weight finishes or adhered tile, masonry, stone, or terra cotta veneer no more than 5/8" thickness and apex less than 20'-0" in height and not over an exit way. Maximum tributary load to a member shall not exceed the equivalent of that occurring from a 10'x10' opening in a 15' tall wall for a header or king stud.
☐	4. Manufactured support frames and curbs using hot rolled or cold-formed steel (i.e., light gauge) for mechanical, electrical, or plumbing equipment weighing less than 2000# (equipment only) (connections of such frames to superstructure elements using welding will require special inspection as noted in selected item(s) for Sections 19, 19.1 and/or 19.2 of listing above).
☐	5. Manufactured components (e.g., Tolco, B-Line, Aflcon, etc.) for mechanical, electrical, or plumbing hanger support and bracing (connections of such components to superstructure elements using welding will require special inspection as noted in selected item(s) for Sections 19, 19.1 and/or 19.2 of listing above).

Appendix: Work Exempt from DSA Requirements for Structural Tests / Special Inspections

Application Number: 04-119454
DSA File Number:
School Name: TO BE DETERMINED
Increment Number:
School District: USA SHADE
Date Created: 2021-03-26 08:15:52

☐	6. TV Brackets, projector mounts with a valid listing (see DSA IR A-5) and recreational equipment (e.g., playground structures, basketball backstops, etc.) (connections of such elements to superstructure elements using welding will require special inspection as noted in selected item(s) for section 19, 19.1 and/or 19.2 located in the Steel/Aluminum category).
☐	7. Any support for exempt non-structural components given in CBC Section 1617A.1.18 (which replaces ASCE 7-16, Section 13.1.4) meeting the following: A) when supported on a floor/roof, <400# and resulting composite center of mass (including component's center of mass) ≤4' above supporting floor/roof, B) when hung from a wall or roof/floor, <20# for discrete units or <5 plf for distributed systems.

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS(SIGNATURE), 2019 CBC

Application Number: 04-119454
DSA File Number:
School Name: TO BE DETERMINED
Increment Number:
School District: USA SHADE
Date Created: 2021-03-26 08:15:52

Name of Architect or Engineer in general responsible charge:

Name of Structural Engineer (When structural design has been delegated):

Signature of Architect or Structural Engineer: Date:

Note: To facilitate DSA electronic mark-ups and identification stamp application, DSA recommends against using secured electronic or digital signatures.

DSA STAMP

DSA 103-19: LIST OF REQUIRED VERIFIED REPORTS, CBC 2019

Application Number: 04-119454
DSA File Number:
School Name: TO BE DETERMINED
Increment Number:
School District: USA SHADE
Date Created: 2021-03-26 08:15:52

1. Structural Testing and Inspection: Laboratory Verified Report Form DSA 291
2. Concrete Batch Plant Inspection: Laboratory Verified Report Form DSA 291
3. Shop Welding Inspection: Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292

THE SAMPLE DSA-103 FORM SHOWN ON THIS SHEET IS FOR ILLUSTRATION PURPOSES ONLY TO ASSIST IN THE COMPLETION OF FUTURE PROJECTS SPECIFIC FORM DSA-103, A CURRENT DSA-103 FORM IS TO BE COMPLETED FOR EACH APPLICATION THAT THIS PC IS BEING INCORPORATED INTO AND ALL SAMPLE DSA-103 SHEETS ARE TO BE CROSSED OUT ON THIS DRAWING.

ADDITIONAL TESTING AND INSPECTION NOTES:

1. THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE SELECTED BY THE SCHOOL DISTRICT AND APPROVED BY DSA AND THE ARCHITECT OF RECORD.
2. THE SITE PROJECT INSPECTOR SHALL BE CLASS 2.
3. THE COSTS OF THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE BORN BY THE SCHOOL DISTRICT.
4. COPIES OF VERIFIED REPORTS SHALL BE SENT TO DSA, THE ARCHITECT, THE SCHOOL DISTRICT, THE CONTRACTOR, AND THE PROJECT INSPECTOR.
5. THE IN PLANT INSPECTOR SHALL BE WELDING SPECIAL INSPECTOR FOR MATERIAL VERIFICATION AND WELDING.
6. PER 2019 CBC, SECTION 1705A.3.3, BATCH PLANT INSPECTION MAY BE WAIVED WHEN THE FOLLOWING REQUIREMENTS ARE MET:
   - 6.1. A LICENSED WEIGHMASTER SHALL POSITIVELY IDENTIFY QUANTITY OF MATERIALS AND CERTIFY EACH LOAD BY A BATCH TICKET.
   - 6.2. BATCH TICKETS, INCLUDING MATERIAL QUANTITIES AND WEIGHTS SHALL ACCOMPANY THE LOAD, SHALL BE TRANSMITTED TO THE INSPECTOR OF RECORD BY THE TRUCK DRIVER WITH LOAD IDENTIFIED THEREON. THE LOAD SHALL NOT BE PLACED WITHOUT A BATCH TICKET IDENTIFYING THE MIX. THE INSPECTOR OF RECORD SHALL KEEP A DAILY RECORD OF PLACEMENTS, IDENTIFYING EACH TRUCK IT'S LOAD, AND TIME OF RECEIPT AT THE JOBSITE, AND APPROXIMATE LOCATION OF DEPOSIT IN THE STRUCTURE AND SHALL MAINTAIN A COPY OF THE DAILY RECORD AS REQUIRED BY THE ENFORCEMENT AGENCY.

SAMPLE DSA 103 - STATEMENT OF STRUCTURAL TESTS AND INSPECTIONS FORM - FOR ALL UNITS EXCEPT CANTILEVER AND SINGLE POST UNITS

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01 -119834 INC: 0
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 5/20/2022

THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN

USA SHADE & Fabric Structures
CORPORATE HEADQUARTERS
2580 ESTERS BLVD. SUITE 100
DFW AIRPORT, TX, 75261
800-966-5005

CERTIFICATIONS:
IAS CERTIFICATION No: FA-428
CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:
Alum Rock Union ES Dist

PROJECT NAME:
San Antonio Elementary

LOCATION:
1721 E San Antonio St.
San Jose, CA 95116

MODEL NUMBER:

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-119454 PC
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑ CG ☐
DATE: 03/26/2021

STRUCTURE TYPE:

SCALE : VARIES

DRAWING SIZE: D

PRE-CHECK (PC) DOCUMENT
Code : 2019 CBC
A separate project application for construction is required.

Eng. By :	**DWH**	**10/16/20**
Design By :	**DWH**	**10/16/20**
Approved By :	**DWH**	**10/16/20**

DRAWING DESCRIPTION:
DSA 103 SAMPLE FORM

DWG.

SHEET **P.C. T-2.1**

REV.

LICENSED ARCHITECT
DAVID HIGGINSON
NO. C19168
REN. 10-31-21
STATE OF CALIFORNIA

GENERAL NOTES
DESIGN LOADS

BUILDING CODE	CBC 2019 (BASED ON IBC 2018)
LIVE LOADS	5 PSF
SNOW LOAD	5 PSF
WIND LOADS	115 MPH (3-Sec. Gust); EXPOSURE C; TOPOGRAPHIC FACTOR , Kzt = 1.0

1.- SPECIAL INSPECTION REQUIREMENTS SHALL FOLLOW THE ATTACHED SAMPLE TEST AND INSPECTION LIST (T & I LIST) APPROVED BY DSA. THE SHOP WELDING INSPECTION SHALL INCLUDE WELDING OF ALL STEEL MEMBERS AND IDENTIFICATION OF STEEL THROUGH MILL CERTIFICATE OR MATERIAL TESTING, UNCERTIFIED STEEL SHALL BE TESTED TO THE REQUIREMENTS OF CBC 2019 CHAPTER 17A. THE FIELD SPECIAL INSPECTION SHALL INCLUDE COMPRESSION CYLINDER TESTS FOR THE CONCRETE FOUNDATION.

2.- STRUCTURE SHALL BE IN THE LOCATION SHOWN ON THE SITE SPECIFIC DSA APPLICATION DRAWING.

3.- FOUNDATION DESIGN BASED ON CBC 2019, TABLE 1806A.2, SOIL CLASS 5 (ALLOWABLE FOUNDATION PRESSURE 1500 PSF)

4.- DESIGN PER FOLLOWING CODES: CBC 2019, ASCE 7-16, AISC 360-16, AISC 341-16, ACI 318-14, ASCE 55-16 & ASCE 19-16

STRUCTURAL STEEL

1.- FABRICATION OF THE STEEL STRUCTURES SHALL BE PERFORMED BY SHADE STRUCTURES OR AN AUTHORIZED LICENSEE. MATERIAL TESTING (OR MILL CERTIFICATES) AND INSPECTION OF WELDING SHALL BE CONDUCTED PER CBC 2019 SECTIONS 1704A, 1705A, 1705A.2, AND TABLE 1705A.2.1.

2.- ONLY CALIFORNIA LICENSED CONTRACTORS AUTHORIZED BY SHADE STRUCTURES SHALL INSTALL THE SHADE STRUCTURES.

3.- ALL WORK SHALL CONFORM TO CBC 2019 EDITION, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR)

4.- ALL GALVANIZED STEEL TUBE PRODUCTS MANUFACTURED BY ALLIED TUBE & CONDUIT FOR THIS STRUCTURE SHALL BE, AND CONFORM TO ASTM A500-16, IN ITS' ENTIRETY.
TYPICAL MECHANICAL PROPERTIES ARE:
ROUND TUBE 42,000 PSI YIELD STRESS MINIMUM / 48,000 PSI TENSILE STRESS MINIMUM

5.- ALL STRUCTURAL SHAPES SHALL BE COLD FORMED HSS ASTM A500 GRADE B, UNLESS OTHERWISE NOTED. TYPICAL MECHANICAL PROPERTIES ACHIEVED FOR HSS PRODUCTS:
SQUARE AND RECTANGULAR 46,000 PSI YIELD STRESS / 58,000 PSI TENSILE STRESS
ROUND PIPE 42,000 PSI YIELD STRESS / 58,000 PSI TENSILE STRESS

6.- ALL PLATES PRODUCTS SHALL COMPLY WITH ASTM A572 GRADE 50.

7.- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH A.I.S.C. SPECIFICATIONS.

8.- ALL WELDING TO CONFORM WITH AMERICAN WELDING SOCIETY STANDARDS AND SHALL BE INSPECTED BY AN AWS/CWI INSPECTOR. AWS D1.1 FOR HOT ROLLED. AWS D1.3 FOR SHEET/COLD FORMED. AWS D1.8 SEISMIC SUPPLEMENT.

9.- ALL FULL PENETRATION WELD SHALL BE CONTINUOUSLY INSPECTED PER AWS D1.1 & D1.8.

10.- SHOP CONNECTIONS SHALL BE WELDED UNLESS NOTED OTHERWISE. FIELD CONNECTIONS SHALL BE AS INDICATED ON THE DRAWINGS (IF REQUIRED), ALL FILLET WELDS SHALL BE A MINIMUM OF 3/16" ER70SX ELECTRODES UNLESS OTHERWISE NOTED. EITHER SMAW OR GMAW IS ACCEPTABLE.

11.- ALL STAINLESS STEEL BOLTS SHALL COMPLY WITH ASTM F-593, YIELD STRENGTH= 65 KSI, TENSILE STRENGTH=100 KSI MINIMUM, ALLOY GROUP 1, CONDITION CW1. ALL NUTS SHALL COMPLY WITH ASTM F-594 ALLOY GROUP 1, CONDITION CW1, REFERRING TO RCSC, ASTM F-593 IS NOT CONSIDERED AS HIGH STRENGTH BOLTS.

12.- ALL HIGH STRENGTH BOLTS SHALL COMPLY WITH ASTM F3125 GRADE A325 N (GALVANIZED). ALL NUTS SHALL COMPLY WITH ASTM A563DH, AND WASHERS SHALL COMPLY WITH ASTM F436. ALL HIGH STRENGTH BOLTS SHALL BE TIGHTENED TO A SNUG TIGHT CONDITION.

13.- ALL STRUCTURAL STEEL (ITEMS FROM NOTE 5) SHALL BE POWDER COATED WITH ONE SHOP COAT (2.5 MILS MIN.) OF ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT, OR EQUIVALENT PAINT SYSTEM. THIS COAT IS A WEATHER RESISTANT POWDER COATING BASED ON POLYESTER TGIC (MANUFACTURED BY SHERWIN WILLIAMS, ASKO NOBEL, PPG OR TIGER DRYLAC). TO ACHIEVE OPTIMUM ADHESION, IT IS RECOMMENDED THAT THE PROPER TREATMENT AND DRYING TAKE PLACE BEFORE COATING. POLYESTER POWDER (TGIC) SPECIFICATIONS SHALL BE AS FOLLOWS:
- PENCIL HARDNESS (ASTM D-3363). - HUMIDITY (ASTM D-2247).
- SOLVENT RESISTANCE (PCI METHOD) - 50 DBL RUBS SL. SOFTNESS.

14.- ALL STEEL ROUND TUBING (ITEMS FROM NOTE 4) SHALL BE TRIPLE COATED FOR RUST PROTECTION USING THE IN-LINE ELECTROPLATING COAT PROCESS. TUBING SHALL BE INTERNALLY COATED WITH ZINC AND ORGANIC COATINGS TO PREVENT CORROSION AS MANUFACTURED BY ALLIED TUBE & CONDUIT.

15.- COLD-FORMED STEEL MEMBERS SHALL BE 55% ALUMINUM ZINC ALLOY COATED PER ASTM A792/A792M STANDARD IN ACCORDANCE TO AISI S200 TABLE A4-1, CP 90 COATING DESIGNATION. ALL EXPOSED STEEL FASTENERS SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM), HOT DIP GALVANIZED (ASTM A153, CLASS D MINIMUM OR ASTM F2329), OR PROTECTED WITH CORROSION PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT.

FABRIC SPECIFICATION

1.- FABRIC SHALL BE MANUFACTURED BY MULTIKNIT LTD. OR OTHER COMPANY WHO CAN MANUFACTURE FABRIC, WHICH MEETS THE SPECIFICATIONS LISTED ON PAGE 2000, AND SHALL BE FABRICATED FROM POLYETHYLENE MATERIALS.

2.- THE FABRIC SHALL RETAIN 80% OF ITS TENSILE AND TEARING STRENGTH AFTER ULTRAVIOLET EXPOSURE PER ASTM G53 USING A 313 NM LIGHT SOURCE FOR 500 HOURS WHILE MOISTENED FOR 1 HOUR EVERY 12 HOURS.

3.- PROVIDE CERTIFICATION BY MANUFACTURER AND STATE FIRE MARSHAL TO SCHOOL'S DISTRICT INSPECTOR OF RECORD AT SITE SPECIFIC INSTALLATION. COPY OF FIRE CERTIFICATION SHALL BE SENT TO DSA.

4.- FABRIC SHALL REQUIRE ANNUAL INSPECTION AND MAINTENANCE BY THE DISTRICT. FABRICS SAMPLES OF THE SAME MATERIAL WHICH ARE MAINTAINED AT THE PROJECTS SITE SHALL BE TESTED TO BE IN COMPLIANCE WITH ASTM D5034 AND D2261. THE ANNUAL TESTING ON THE APPROVED PLANS SHALL BE COMPARED TO THE FABRIC SPECIFICATIONS INDICATED IN NOTE 1 OF "FABRIC SPECIFICATION" ON THE APPROVED PLANS. THE FABRIC SHALL BE REPLACED WHEN THE TEST RESULTS RETURN LESS THAN 50% OF THE ULTIMATE VALUES IN NOTE 1 OF "FABRIC SPECIFICATION", FIRE TEST ON FABRIC: NFPA 701 TEST 2 AND ASTM E 84 EXTENDED 30 MINUTES TEST. FLAME SPREAD INDEX (FSI): 10. SMOKE DEVELOPED INDEX (SDI): 50. FABRIC IS ACCEPTABLE FOR USE IN WILDLIFE URBAN INTERFACE AREA.

5.- FABRIC TOP NEEDS TO BE REMOVED IF SNOW EXCEEDING 5 PSF ARE ANTICIPATED, FABRIC TOP NEEDS TO BE REMOVED IF WINDS EXCEEDING 115 MPH ARE ANTICIPATED.

6.- A VISUAL INSPECTION LOOKING FOR TEAR AND ABNORMAL WEAR IN FABRIC MATERIAL AND THREAD IS REQUIRED PRIOR TO RE-INSTALLATION. USA SHADE & FABRIC STRUCTURES SHALL BE NOTIFIED IF SIGNIFICANT DAMAGE IS PRESENT BEFORE RE-INSTALLATION.

AIRCRAFT CABLE

1.- FOR FABRIC ATTACHMENT USE 3/8" 7x19 GALV. CABLE PER ASTM A1023A, ASTM 1023M-02, WITH A BREAKING STRENGTH VALUE OF 14,400 LBS. CABLE SHALL BE TENSIONED TO 250 LBS MINIMUM. THE MAXIMUM CALCULATED CABLE TENSIONS ARE IN TABLE 1, ITEM 22. THE MAXIMUM CALCULATED CABLE ALLOWABLE CAPACITY IS Sa=4909 LB.

2.- CABLES SHALL BE FED THROUGH THE FABRIC SLEEVES AROUND THE PERIMETER OF THE CANOPY AND TENSIONED UNTIL THE FABRIC PANELS (DESIGNED PURPOSELY UNDERSIZED) REACH A TAUT APPEARANCE. ANY LONG TERM CABLE SAG SHALL BE MINIMIZED DURING THE MAINTENANCE RE-TIGHTING VISITS AS REQUIRED.

CONCRETE SPECIFICATION

1.- CONCRETE SHALL BE SAMPLED AND TESTED PER CBC 2019 SECTION 1903A & SHALL BE INSPECTED PER SECTION 1903A.

2.- CONCRETE TO BE F'c= 4500 PSI, TYPE V CEMENT, WATER/CEMENT RATIO OF 0.45, PER ACI 318-14 CHAPTER 5. REINFORCING STEEL TO BE Fy= 60000 PSI , MIN. GR. 60

3.- ALL ANCHOR BOLTS SET IN NEW CONCRETE (WHEN APPLICABLE) SHALL COMPLY WITH ASTM F-1554 GRADE 55 (GALVANIZED PER ASTM A153, CLASS D MINIMUM OR ASTM F2329).
ANCHOR BOLT'S EMBEDMENT NEEDS TO BE AS FOLLOW:
A) ANCHOR BOLT Ø1 1/4" 30 IN (MINIMUM EMBEDMENT)

4.- CERTIFIED MILL TEST REPORTS ARE TO BE PROVIDED FOR EACH SHIPMENT OF REINFORCEMENT.

5.- ALL NON-SHRINK GROUT SHALL HAVE A MINIMUM 28 DAYS COMPRESSIVE STRENGTH OF 5000 PSI, AND SHALL COMPLY THE REQUIREMENTS OF ASTM C109, ASTM C939, ASTM C1090, ASTM C1107, WHEN APPLICABLE.

2019 CBC PC DESIGN NOTES

FLOOR LIVE LOAD	N/A	
ROOF LIVE LOAD	RLL	5 PSF
ALLOWABLE SOIL PRESSURE:		
DL + LL (CONC FTG)		1500 PSF
DL + LL + SEISMIC (CONC FTG)		1500 PSF
LATERAL BEARING DESIGN VALUE		100 PSF/FT BELOW NATURAL GRADE, PER TABLE 1806A.2
TWO TIMES THE TABULAR VALUE IS USED (200 PSF/FT) PER CBC SECTION 1806A.3.4.		
ALLOWABLE PIER FRICTIONAL RESISTANCE 250 PSF MAXIMUM BASED ON SECTION 1810A.3.3.1.4 (ONE-SIXTH OF THE BEARING VALUE).		
UPLIFT FRICTIONAL RESISTANCE HAVE A SAFETY FACTOR OF 3.		
ROOF SNOW LOAD	5 PSF	
ICE LOAD	ZERO PSF	
FLOOD HAZARD AREA	NO	

WHEN A SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC ARE STILL APPLICABLE.

WIND DESIGN DIRECTIONAL PROCEDURE: ASCE 7-16, SECTION 27,3,2		
-BASIC DESIGN WIND SPEED (3 SEC GUST)	V	115 MPH
-WIND EXPOSURE FACTOR	C	
-TOPOGRAPHIC FACTOR	Kzt	1
-RISK CATEGORY	II	
-VELOCITY PRESSURE EXPOSURE COEFFICIENT	Kz	SEE TABLE 1, ITEM 25
-VELOCITY PRESSURE	qz	SEE TABLE 1, ITEM 24
SEISMIC DESIGN:		
-SITE CLASS	D	
	SS	3.00g
	S1	1.389g
-SPECTRAL RESPONSE COEFFICIENTS	SDS	2.00
	SD1	1.39

-LATERAL FORCE RESISTING SYSTEM G.2 ORDINARY CANTILEVERED COLUMN SYSTEM.

-SEISMIC IMPORTANCE FACTOR	I	1.0
-DESIGN BASE SHEAR	V	SEE TABLE 1, ITEM 23
-SEISMIC RESPONSE COEFFICIENTS	Cs	1.6
-RESPONSE MODIFICATION FACTOR	R	1.25
-ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE	
-RISK CATEGORY	II	
-SEISMIC DESIGN CATEGORY		E
-SITE COEFFICIENT CATEGORY	Fa	1
	Fv	1.5

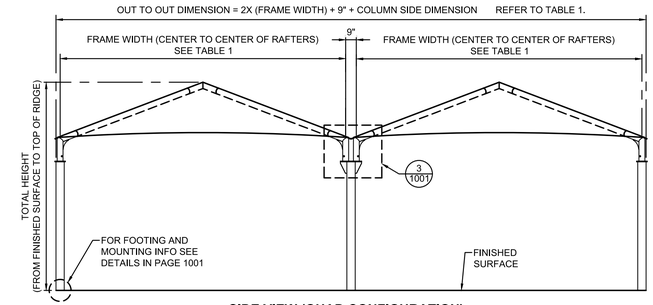
GEOHAZARD REPORT IS NOT REQUIRED FOR OPEN FABRIC STRUCTURES 1,600 SQF OR LESS COMPLYING WITH THE REQUIREMENTS OF IR A-4 SECTION 3.1.1. OPEN FABRIC SHADE STRUCTURES GREATER THAN 1,600 SQUARE FEET UP TO A MAXIMUM OF 4,000 SQUARE FEET AND COMPLYING WITH THE REQUIREMENTS NOTED IN IR A-4 SECTION 3,1,1 DO NOT REQUIRE A GEOHAZARD REPORT PROVIDED A GEOTECHNICAL REPORT INDICATES THAT NO LIQUEFACTION POTENTIAL EXISTS.

ARCHITECT OF RECORD TO DETERMINE IF SPECIFIC SITE IS IN GEOLOGIC HAZARD ZONE. GEOHAZARD REPORT REQUIREMENTS PER DSA IR A-4.

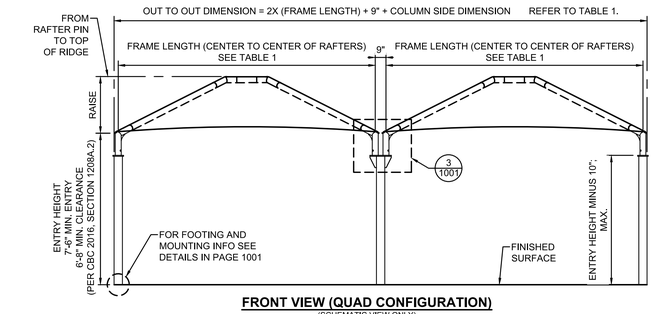
PC OPTIONS SHALL NOT INCLUDE LIQUEFIABLE SOIL (EXCEPTION: OPEN FABRIC SHADE STRUCTURES 1,600 SQUARE FEET OR LESS COMPLYING WITH REQUIREMENTS OF IR A-4 SECTION 3.1.1). IF STRUCTURE IS LOCATED IN AN AREA WITH LIQUEFIABLE SOIL OR SITE CLASS F, OVER-THE-COUNTER SUBMITTAL IS NOT ALLOWED AND REGULAR PROJECT SUBMITTAL IS REQUIRED. IF SITE IS NOT IN A MAPPED LIQUEFACTION HAZARD ZONE, IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE UNLESS A SITE-SPECIFIC GEOTECHNICAL REPORT IDENTIFIES SUCH HAZARD.

MINIMUM FOUNDATION SETBACK LIMIT IN ADJACENT SLOPE: THE DEPTH OF REQUIRED PIER EMBEDMENT SHALL START FROM AN ELEVATION THAT CORRESPONDS WITH A HORIZONTAL CLEAR DISTANCE OF 14 FEET THAT INTERSECT WITH THE SLOPE (DAYLIGHTING) FOR FOOTING TYPE "A" (2'-0" DIAMETER) AND A CLEAR DISTANCE OF 17'-6" FOR FOOTING TYPE "B" (2'-6" DIAMETER). IF SETBACK LIMITS ARE SMALLER THAN CBC REQUIRES, A SITE-SPECIFIC SOILS REPORT IS REQUIRED.

MINIMUM CLASS 2 PROJECT INSPECTOR REQUIRED.



SIDE VIEW (QUAD CONFIGURATION)
(SCHEMATIC VIEW ONLY)



FRONT VIEW (QUAD CONFIGURATION)
(SCHEMATIC VIEW ONLY)

TABLE 1

ITEM	DESCRIPTION	QUAD FRAMES 20'X25'X12'e	QUAD FRAMES 20'X30'X12'e	QUAD FRAMES 30'X30'X12'e	QUAD FRAMES 30'X40'X12'e	QUAD FRAMES 20'X25'X15'e	QUAD FRAMES 20'X30'X15'e	QUAD FRAMES 30'X30'X15'e	QUAD FRAMES 30'X40'X15'e
	PRODUCT NAME	DSA401Q202512-16	DSA401Q203012-16	DSA401Q303012-16	DSA401Q304012-16	DSA401Q2025-16	DSA401Q2030-16	DSA401Q3030-16	DSA401Q3040-16
	DOUBLE LENGTH CONFIGURATIONS (WxL)	40'X50'	40'X60'	60'X60'	60'X80'	40'X50'	40'X60'	60'X60'	60'X80'
	SELECTION CHECKBOX								
	MAXIMUM INDIVIDUAL FRAME LENGTH	25'-0"	30'-0"	30'-0"	40'-0"	25'-0"	30'-0"	30'-0"	40'-0"
	MAXIMUM INDIVIDUAL FRAME WIDTH	20'-0"	20'-0"	30'-0"	30'-0"	20'-0"	20'-0"	30'-0"	30'-0"
	MAXIMUM ENTRY HEIGHT	12'-0"	12'-0"	12'-0"	12'-0"	15'-0"	15'-0"	15'-0"	15'-0"
	MAXIMUM DISTANCE "A"	7'-6"	7'-6"	11'-3"	11'-3"	7'-6"	7'-6"	11'-3"	11'-3"
	MAXIMUM RAISE	3'-10"	3'-10"	5'-8"	5'-8"	3'-10"	3'-10"	5'-8"	5'-8"
	MAXIMUM TOTAL HEIGHT	15'-10"	17'-8"	17'-8"	17'-8"	18'-10"	18'-10"	20'-8"	20'-8"
1	COLUMN	HSS 5.0 x 5.0 x 0.250	HSS 5.0 x 5.0 x 0.250	HSS 6.0 x 6.0 x 0.375	HSS 7.0 x 7.0 x 0.375	HSS 5.0 x 5.0 x 0.250	HSS 7.0 x 7.0 x 0.250	HSS 7.0 x 7.0 x 0.375	HSS 7.0 x 7.0 x 0.375
2	CUP CONNECTOR (6" LG)	HSS 3.5 x 0.25	HSS 4.0 x 0.25	HSS 4.5 x 0.375	HSS 5.0 x 0.375	HSS 4.0 x 0.25	HSS 4.0 x 0.25	HSS 4.5 x 0.375	HSS 5.0 x 0.375
3	RAFTER	HSS 4.0 x 0.188	4.50 GA 7 RD. TUBE (4.5 x 0.188)	5.00 GA 7 RD. TUBE (5.0 x 0.188)	HSS 5.563 x 0.258	4.50 GA 7 RD. TUBE (4.5 x 0.188)	4.50 GA 7 RD. TUBE (4.5 x 0.188)	5.00 GA 7 RD. TUBE (5.0 x 0.188)	HSS 5.563 x 0.258
4	EXTENSION	HSS 4.0 x 0.188	4.50 GA 7 RD. TUBE (4.5 x 0.188)	5.00 GA 7 RD. TUBE (5.0 x 0.188)	HSS 5.563 x 0.258	4.50 GA 7 RD. TUBE (4.5 x 0.188)	4.50 GA 7 RD. TUBE (4.5 x 0.188)	5.00 GA 7 RD. TUBE (5.0 x 0.188)	HSS 5.563 x 0.258
5	CROSSPIECE	HSS 4.0 x 0.188	4.50 GA 7 RD. TUBE (4.5 x 0.188)	5.00 GA 7 RD. TUBE (5.0 x 0.188)	HSS 5.563 x 0.258	4.50 GA 7 RD. TUBE (4.5 x 0.188)	4.50 GA 7 RD. TUBE (4.5 x 0.188)	5.00 GA 7 RD. TUBE (5.0 x 0.188)	HSS 5.563 x 0.258
6	RIDGE	HSS 4.0 x 0.188	4.50 GA 7 RD. TUBE (4.5 x 0.188)	5.00 GA 7 RD. TUBE (5.0 x 0.188)	HSS 5.563 x 0.258	4.50 GA 7 RD. TUBE (4.5 x 0.188)	4.50 GA 7 RD. TUBE (4.5 x 0.188)	5.00 GA 7 RD. TUBE (5.0 x 0.188)	HSS 5.563 x 0.258
7	FABRIC TOP	FR COLOURSHADE Z25	FR COLOURSHADE Z25	FR COLOURSHADE Z25	FR COLOURSHADE Z25	FR COLOURSHADE Z25	FR COLOURSHADE Z25	FR COLOURSHADE Z25	FR COLOURSHADE Z25
8	CABLE (GALVANIZED)	Ø3/8" CABLE	Ø3/8" CABLE	Ø3/8" CABLE	Ø3/8" CABLE	Ø3/8" CABLE	Ø3/8" CABLE	Ø3/8" CABLE	Ø3/8" CABLE
9	CABLE CLAMP (GALVANIZED)	Ø3/8" CABLE CLAMP	Ø3/8" CABLE CLAMP	Ø3/8" CABLE CLAMP	Ø3/8" CABLE CLAMP	Ø3/8" CABLE CLAMP	Ø3/8" CABLE CLAMP	Ø3/8" CABLE CLAMP	Ø3/8" CABLE CLAMP
10	HEX BOLT	Ø1/2"-13NCx5" HEX BOLT 18-8 SS	Ø5/8"-11NCx6" HEX BOLT 18-8 SS	Ø5/8"-11NCx6 1/2" HEX BOLT 18-8 SS	Ø3/4"-10NCx7" HEX BOLT ASTM F3125 GRADE A325 GALV.	Ø5/8"-11NCx6" HEX BOLT 18-8 SS	Ø5/8"-11NCx6" HEX BOLT 18-8 SS	Ø5/8"-11NCx6 1/2" HEX BOLT 18-8 SS	Ø3/4"-10NCx7" HEX BOLT ASTM F3125 GRADE A325 GALV.
11	HEX NUT	Ø1/2"-13NC HEX NUT 18-8 SS	Ø5/8"-11NC HEX NUT 18-8 SS	Ø5/8"-11NC HEX NUT 18-8 SS	Ø3/4"-10NC HEX NUT ASTM A563 GALV.	Ø5/8"-11NC HEX NUT 18-8 SS	Ø5/8"-11NC HEX NUT 18-8 SS	Ø5/8"-11NC HEX NUT 18-8 SS	Ø3/4"-10NC HEX NUT ASTM A563 GALV.
12	SPLIT LOCK WASHER	Ø1/2" SPLIT LOCK WASHER 18-8 SS	Ø5/8" SPLIT LOCK WASHER 18-8 SS	Ø5/8" SPLIT LOCK WASHER 18-8 SS	Ø3/4" SPLIT LOCK WASHER ASTM F436 GALV.	Ø5/8" SPLIT LOCK WASHER 18-8 SS	Ø5/8" SPLIT LOCK WASHER 18-8 SS	Ø5/8" SPLIT LOCK WASHER 18-8 SS	Ø3/4" SPLIT LOCK WASHER ASTM F436 GALV.
13	DERLIN WASHER (ACETAL)	Ø1/2" FLAT WASHER DELRIN	Ø5/8" FLAT WASHER DELRIN	Ø5/8" FLAT WASHER DELRIN	N/A	Ø5/8" FLAT WASHER DELRIN	Ø5/8" FLAT WASHER DELRIN	Ø5/8" FLAT WASHER DELRIN	N/A
14	FLAT WASHER	Ø1/2" FLAT WASHER 18-8 SS	Ø5/8" FLAT WASHER 18-8 SS	Ø5/8" FLAT WASHER 18-8 SS	Ø3/4" FLAT WASHER ASTM F436 GALV.	Ø5/8" FLAT WASHER 18-8 SS	Ø5/8" FLAT WASHER 18-8 SS	Ø5/8" FLAT WASHER 18-8 SS	Ø3/4" FLAT WASHER ASTM F436 GALV.
15	DRILLED PIER FOOTING DETAIL	FOOTING TYPE A (Ø2'-0")	FOOTING TYPE A (Ø2'-0")	FOOTING TYPE B (Ø2'-6")	FOOTING TYPE B (Ø2'-6")	FOOTING TYPE A (Ø2'-0")	FOOTING TYPE A (Ø2'-0")	FOOTING TYPE B (Ø2'-6")	FOOTING TYPE B (Ø2'-6")
16	FOOTING DEPTH	7'-6"	8'-0"	9'-0"	10'-6"	7'-6"	8'-6"	9'-3"	10'-6"
17	ALT. SPREAD FOOTING SIZE	4'-0" SQ	4'-6" SQ	5'-0" SQ	6'-0" SQ	4'-0" SQ	5'-0" SQ	6'-0" SQ	6'-6" SQ
18	HORIZONTAL REBAR	7#5 E.W. TOP AND BOTTOM	8#5 E.W. TOP AND BOTTOM	9#5 E.W. TOP AND BOTTOM	10#5 E.W. TOP AND BOTTOM	7#5 E.W. TOP AND BOTTOM	9#5 E.W. TOP AND BOTTOM	10#5 E.W. TOP AND BOTTOM	11#5 E.W. TOP AND BOTTOM
19	WELD "A" (T&B COLUMN)	3/16"	3/16"	5/16"	5/16"	3/16"	3/16"	5/16"	5/16"
20	WELD "B" (UPPERFRAME)	3/16"	3/16"	3/16"	1/4"	3/16"	3/16"	3/16"	1/4"
21	WELD "C" (CUP CONNECTOR)	3/16"	3/16"	5/16"	5/16"	3/16"	3/16"	5/16"	5/16"
22	MAXIMUM CABLE TENSION	2259 LB	2603 LB	2888 LB	3633 LB	2342 LB	2741 LB	3144 LB	3763 LB
23	DESIGN BASE SHEAR	5674 LB	6314 LB	9885 LB	14280 LB	25296 LB / 37 COLUMNS	32112 LB / 37 COLUMNS	46222 LB / 37 COLUMNS	58530 LB / 37 COLUMNS
24	VELOCITY PRESSURE qz	24.46 PSF	24.46 PSF	24.46 PSF	24.46 PSF	25.32 PSF	25.32 PSF	25.32 PSF	25.32 PSF
25	VELOCITY PRESSURE EXPOSURE COEFF.Kz	0.85	0.85	0.85	0.85	0.88	0.88	0.88	0.88

NOTE: PER DSA IR A-4, CGS APPROVAL OF GEOHAZARD REPORT IS REQUIRED FOR SHADE STRUCTURES EXCEEDING 4,000 SQUARE FEET.

NOTE: PER DSA IR A-4, CGS APPROVAL OF GEOHAZARD REPORT IS REQUIRED FOR SHADE STRUCTURES EXCEEDING 4,000 SQUARE FEET.

CODE ANALYSIS					
BUILDING	OCCUPANCY	CONST. TYPE	AREA (SQ. FT.)	OCCUPANT LOAD FACTOR	OCCUPANT LOAD
SHADE STRUCTURE					

MAXIMUM OCCUPANT LOAD (PER CBC 2019 TABLE 1604A.5)
-K-12: 250 PERSONS
-PUBLIC ASSEMBLY: 300 PERSONS
-EDUCATIONAL OCCUPANCIES ABOVE 12TH GRADE: 500 PERSONS





IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01 -119834 INC: 0
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 5/20/2022

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USASHADE & Fabric Structures
CORPORATE HEADQUARTERS
2580 ESTERS BLVD. SUITE 100
DFW AIRPORT, TX, 75261
800-966-5005

CERTIFICATIONS:
IAS CERTIFICATION No: FA-428
CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:
Alum Rock Union ES Dist

PROJECT NAME:
San Antonio Elementary

LOCATION:
1721 E San Antonio St.
San Jose, CA 95116

MODEL NUMBER:
DSA401Q-19

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-119454 PC
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑ CG ☐
DATE: 03/26/2021

STRUCTURE TYPE:
QUAD JOINED HIPS DSA
SIZE: SEE TABLE 1

SCALE : NONE

DRAWING SIZE:
D

PRE-CHECK (PC) DOCUMENT
Code : 2019 CBC
A separate project application for construction is required.

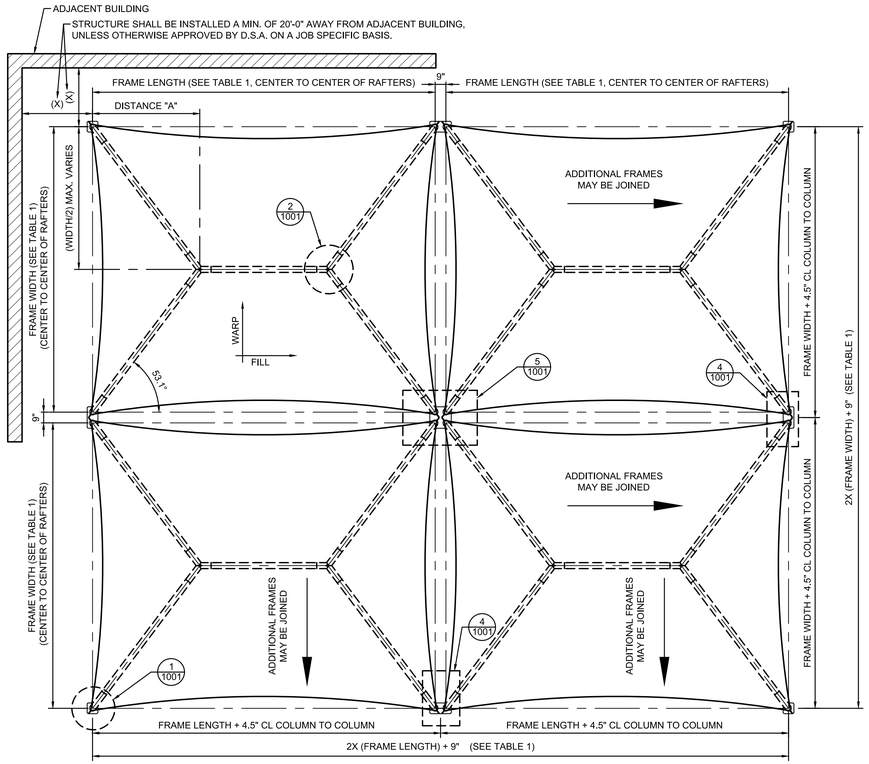
Eng. By :	JO	08/15/20
Design By :	MP	08/15/20
Approved By :	JO	08/15/20

DRAWING DESCRIPTION:
PRODUCT INFORMATION

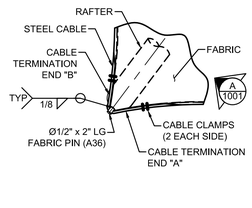
DWG.
DSA401Q-19

SHEET
25.1-1000

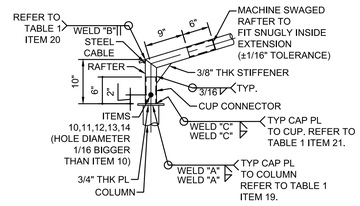
REV.
NC



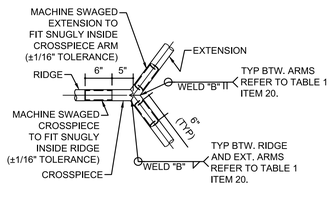
QUAD JOINED CONFIGURATION
TOP VIEW (4 FRAMES SCHEMATIC VIEW ONLY)



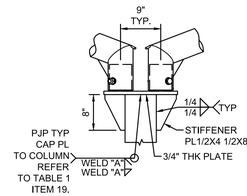
DETAIL 1
REFER TO TOP VIEW



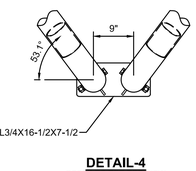
VIEW-A
REFER TO DETAIL-1



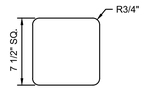
DETAIL-2
REFER TO TOP VIEW



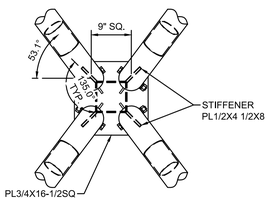
DETAIL 3
REFER TO FRONT/SIDE VIEW



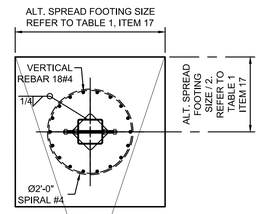
DETAIL-4
REFER TO TOP TOP VIEW



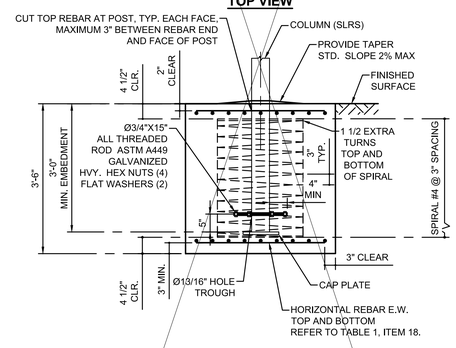
CAP PLATE
(3/4" THK)
TOP FOR ALL SINGLE COLUMNS (TOP & BOT. OF SINGLE PIH COLUMNS)



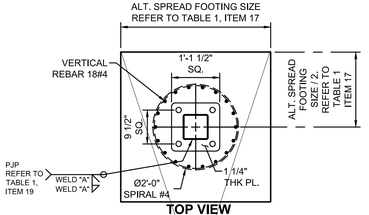
DETAIL-5
REFER TO TOP QUAD CONFIGURATION TOP VIEW



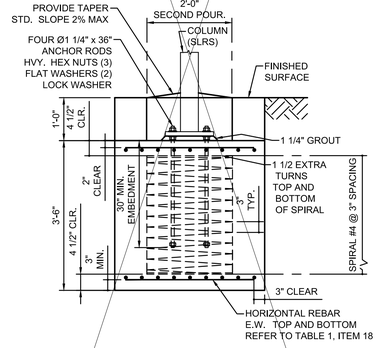
TOP VIEW



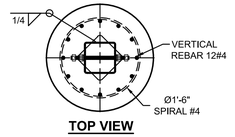
ALTERNATE SPREAD FOOTING
(EMBEDDED, PIH)
(OPTIONAL)



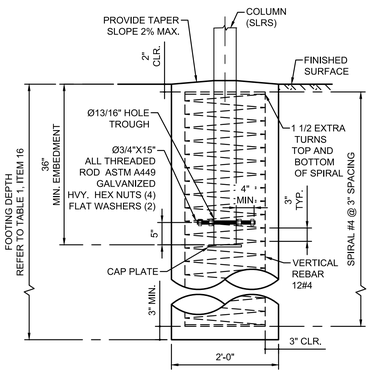
TOP VIEW



ALTERNATE SPREAD FOOTING
(RECESSED BASE PLATE, RBP)
(OPTIONAL)

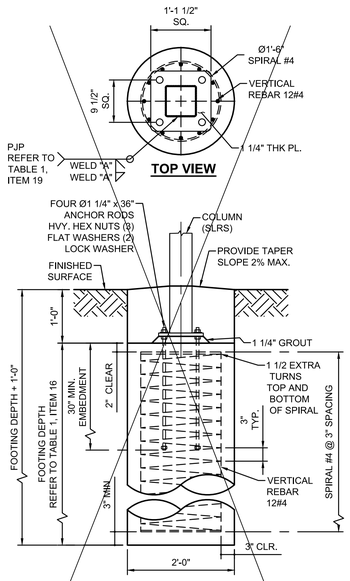


TOP VIEW



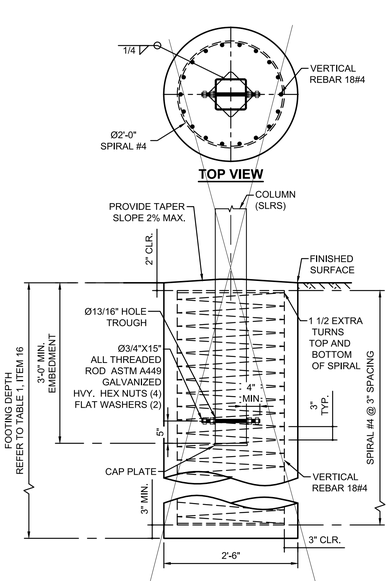
DRILLED PIER FOOTING-PIH
(EMBEDDED, PIH)
(USE FOR NON-CONSTRAINED CASES)

DRILLED PIER DETAIL TYPE "A"
REFER TO TABLE 1, ITEM 15



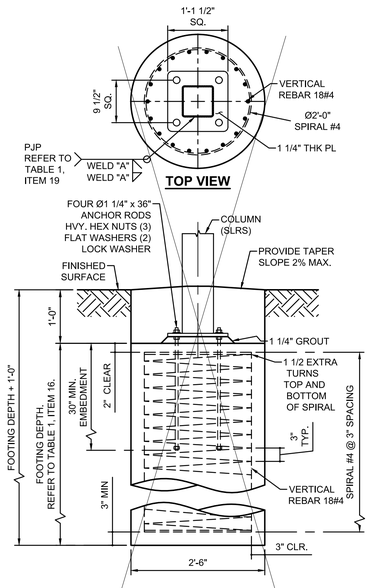
DRILLED PIER FOOTING-RBP
(RECESSED BASE PLATE, RBP)
(USE FOR NON-CONSTRAINED CASES)
(OPTIONAL)

DRILLED PIER DETAIL TYPE "A"
REFER TO TABLE 1, ITEM 15



DRILLED PIER FOOTING-PIH
(EMBEDDED, PIH)
(USE FOR NON-CONSTRAINED CASES)

DRILLED PIER DETAIL TYPE "B"
REFER TO TABLE 1, ITEM 15



DRILLED PIER FOOTING-RBP
(RECESSED BASE PLATE, RBP)
(USE FOR NON-CONSTRAINED CASES)
(OPTIONAL)

DRILLED PIER DETAIL TYPE "B"
REFER TO TABLE 1, ITEM 15

LICENSED ARCHITECT DAVID HIGGINSON NO. C19108 REN. 10-31-21 STATE OF CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER MARSHALL BEAU KRAUSE No. 3693 STRUCTURAL STATE OF CALIFORNIA

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01 -119834 INC: 0
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 5/20/2022

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USA SHADE & Fabric Structures
CORPORATE HEADQUARTERS
2580 ESTERS BLVD. SUITE 100
DFW AIRPORT, TX, 75261
800-966-5005

CERTIFICATIONS:
IAS CERTIFICATION No: FA-428
CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:
Alum Rock Union ES Dist

PROJECT NAME:
San Antonio Elementary

LOCATION:
1721 E San Antonio St.
San Jose, CA 95116

MODEL NUMBER:
DSA401Q-19

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-119454 PC
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑ CG ☐
DATE: 03/26/2021

STRUCTURE TYPE:
QUAD JOINED HIPS
DSA
SIZE: SEE TABLE 1

SCALE : NONE

DRAWING SIZE: D

PRE-CHECK (PC) DOCUMENT
Code : 2019 CBC
A separate project application for construction is required.

Eng. By :	JO	08/15/20
Design By :	MP	08/15/20
Approved By :	JO	08/15/20

DRAWING DESCRIPTION:
DETAILS

DWG. DSA401Q-19

SHEET 25.2-1001

REV. NC

USA SHADE & Fabric Structures
Dallas, TX

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RESULTS

Project: Model: DSA401Q202512-19_V1 Date: 9/16/2020

■ 4.1 NODES - SUPPORT FORCES

Result Combinations

Node			Support Forces [kip]			Support Moments [kipft]			
No.	RC		P_X	P_Y	P_Z	M_X	M_Y	M_Z	
86	RC3	Max	0.850	0.586	0.307	1.441	9.433	1.030	Foundation Loads
		Min	0.000	-0.161	-1.307	-4.783	0.000	-0.441	Foundation Loads
300	RC3	Max	0.391	0.587	0.452	2.592	3.205	0.215	Foundation Loads
		Min	-0.850	-0.252	-1.307	-4.789	-9.434	-1.030	Foundation Loads
508	RC3	Max	0.968	0.530	0.588	3.000	9.502	0.440	Foundation Loads
		Min	-0.173	-0.286	-1.165	-5.154	-1.693	-0.594	Foundation Loads
726	RC3	Max	0.391	0.530	0.588	4.185	3.204	0.593	Foundation Loads
		Min	-0.968	-0.513	-1.143	-5.156	-9.502	-0.348	Foundation Loads
850	RC3	Max	0.615	0.780	0.666	2.001	5.201	0.029	Foundation Loads
		Min	0.000	-0.264	-2.500	-6.070	-0.002	-0.150	Foundation Loads
856	RC3	Max	0.615	1.009	1.164	3.747	5.201	0.150	Foundation Loads
		Min	-0.001	-0.550	-2.249	-9.478	-0.006	-0.030	Foundation Loads
858	RC3	Max	1.086	0.597	0.903	0.001	8.908	0.000	Foundation Loads
		Min	0.000	0.000	-2.229	-5.124	0.000	-0.125	Foundation Loads
862	RC3	Max	0.454	0.597	0.902	0.000	3.843	0.125	Foundation Loads
		Min	-1.086	0.000	-2.229	-5.124	-8.909	0.000	Foundation Loads
864	RC3	Max	0.784	1.049	1.855	0.001	6.252	0.001	Foundation Loads
		Min	-0.001	0.000	-4.413	-9.447	-0.005	-0.001	Foundation Loads

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RESULTS

Project: Model: DSA401Q203012-19_V1 Date: 9/16/2020

■ 4.1 NODES - SUPPORT FORCES

Result Combinations

Node			Support Forces [kip]			Support Moments [kipft]			
No.	RC		P_X	P_Y	P_Z	M_X	M_Y	M_Z	
86	RC3	Max	0.878	0.601	0.371	1.956	9.480	1.327	Foundation Loads
		Min	-0.026	-0.228	-1.478	-4.609	0.000	-0.429	Foundation Loads
300	RC3	Max	0.469	0.601	0.514	3.159	4.005	0.263	Foundation Loads
		Min	-0.878	-0.332	-1.478	-4.610	-9.483	-1.327	Foundation Loads
508	RC3	Max	1.066	0.698	0.712	2.217	9.992	0.429	Foundation Loads
		Min	-0.331	-0.215	-1.367	-6.443	-3.001	-0.759	Foundation Loads
726	RC3	Max	0.469	0.698	0.711	3.765	4.005	0.757	Foundation Loads
		Min	-1.066	-0.498	-1.367	-6.444	-9.993	-0.395	Foundation Loads
850	RC3	Max	0.691	0.859	0.773	2.647	5.864	0.003	Foundation Loads
		Min	-0.001	-0.366	-2.787	-7.161	-0.008	-0.133	Foundation Loads
856	RC3	Max	0.691	1.342	1.395	3.480	5.864	0.133	Foundation Loads
		Min	0.000	-0.548	-2.676	-11.854	-0.003	-0.003	Foundation Loads
858	RC3	Max	1.245	0.677	1.121	0.001	9.628	0.000	Foundation Loads
		Min	-0.072	0.000	-2.564	-6.276	0.000	-0.098	Foundation Loads
864	RC3	Max	0.580	0.676	1.121	0.000	4.996	0.098	Foundation Loads
		Min	-1.245	0.000	-2.564	-6.274	-9.633	0.000	Foundation Loads
866	RC3	Max	0.904	1.423	2.287	0.000	7.165	0.000	Foundation Loads
		Min	0.000	0.000	-5.094	-11.905	-0.003	-0.001	Foundation Loads

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RESULTS

Project: Model: DSA401303012-19_V1 Date: 9/21/2020

■ 4.1 NODES - SUPPORT FORCES

Result Combinations

Node			Support Forces [kip]			Support Moments [kipft]			
No.	RC		P_X	P_Y	P_Z	M_X	M_Y	M_Z	
86	RC3	Max	1.453	1.264	0.497	4.946	18.471	1.537	Foundation Loads
		Min	-0.054	-0.552	-2.317	-12.279	0.000	-1.131	Foundation Loads
300	RC3	Max	0.716	1.288	0.874	9.276	5.732	0.218	Foundation Loads
		Min	-1.452	-0.909	-2.318	-12.294	-18.468	-1.532	Foundation Loads
508	RC3	Max	1.830	1.146	1.080	8.529	19.412	1.127	Foundation Loads
		Min	-0.443	-0.670	-2.056	-11.320	-4.760	-0.515	Foundation Loads
726	RC3	Max	0.716	1.144	1.080	12.278	5.728	0.875	Foundation Loads
		Min	-1.829	-1.287	-1.875	-11.303	-19.407	-0.709	Foundation Loads
850	RC3	Max	2.059	1.071	1.496	0.001	19.339	0.161	Foundation Loads
		Min	-0.092	0.000	-3.871	-10.566	0.000	-0.209	Foundation Loads
856	RC3	Max	0.886	1.071	1.698	0.002	7.449	0.214	Foundation Loads
		Min	-2.059	0.000	-3.816	-10.559	-19.333	-0.161	Foundation Loads
858	RC3	Max	1.091	1.510	1.243	7.892	9.755	0.003	Foundation Loads
		Min	0.000	-0.989	-4.437	-13.109	-0.001	-0.476	Foundation Loads
864	RC3	Max	1.091	2.177	2.113	11.978	9.754	0.475	Foundation Loads
		Min	-0.003	-1.417	-3.631	-20.453	-0.028	0.000	Foundation Loads
866	RC3	Max	1.413	1.703	3.226	0.004	13.121	0.003	Foundation Loads
		Min	-0.001	0.000	-7.503	-18.062	-0.018	-0.002	Foundation Loads

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RESULTS

Project: Model: DSA401304012-19_V1 Date: 9/17/2020

■ 4.1 NODES - SUPPORT FORCES

Result Combinations

Node			Support Forces [kip]			Support Moments [kipft]			
No.	RC		P_X	P_Y	P_Z	M_X	M_Y	M_Z	
86	RC3	Max	1.682	1.374	0.573	9.146	21.824	2.130	Foundation Loads
		Min	-0.397	-0.994	-2.988	-13.216	-2.489	-1.321	Foundation Loads
299	RC3	Max	1.202	1.388	0.989	14.294	10.991	0.570	Foundation Loads
		Min	-1.683	-1.407	-2.988	-13.242	-21.838	-2.125	Foundation Loads
506	RC3	Max	2.271	1.956	1.442	6.464	24.038	1.318	Foundation Loads
		Min	-1.084	-0.690	-2.630	-19.452	-10.974	-0.540	Foundation Loads
723	RC3	Max	1.202	1.957	1.440	13.116	10.992	1.225	Foundation Loads
		Min	-2.269	-1.387	-2.550	-19.460	-24.025	-0.845	Foundation Loads
846	RC3	Max	1.568	2.045	1.522	14.330	15.222	0.004	Foundation Loads
		Min	-0.001	-1.728	-5.605	-17.824	-0.016	-0.605	Foundation Loads
852	RC3	Max	1.568	3.638	2.898	14.246	15.221	0.605	Foundation Loads
		Min	-0.002	-1.677	-4.852	-34.057	-0.016	0.000	Foundation Loads
854	RC3	Max	2.833	1.514	2.068	0.005	26.822	0.099	Foundation Loads
		Min	-0.699	0.000	-4.985	-16.739	-4.394	-0.399	Foundation Loads
856	RC3	Max	1.753	1.516	2.067	0.000	15.933	0.405	Foundation Loads
		Min	-2.832	0.000	-4.984	-16.765	-26.824	-0.098	Foundation Loads
858	RC3	Max	2.198	3.029	4.184	0.005	20.499	0.004	Foundation Loads
		Min	-0.002	0.000	-9.694	-29.943	-0.019	-0.002	Foundation Loads

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RESULTS

Project: Model: DSA401J2025-19-R1 Date: 9/25/2020

■ 4.1 NODES - SUPPORT FORCES

Result Combinations

Node			Support Forces [kip]			Support Moments [kipft]			
No.	RC		P_X	P_Y	P_Z	M_X	M_Y	M_Z	
1	RC3	Max	0.658	0.683	0.501	0.842	8.059	1.649	Foundation Loads
		Min	0.000	-0.091	-1.372	-6.397	0.000	-0.368	Foundation Loads
5	RC3	Max	0.690	0.511	0.402	1.330	7.660	0.655	Foundation Loads
		Min	-0.163	-0.194	-1.237	-4.508	-1.987	-0.942	Foundation Loads
9	RC3	Max	0.722	0.715	0.541	0.802	8.665	1.631	Foundation Loads
		Min	0.000	-0.088	-1.385	-6.748	0.000	-0.393	Foundation Loads
13	RC3	Max	0.755	0.505	0.408	1.388	8.580	0.691	Foundation Loads
		Min	-0.114	-0.204	-1.227	-4.435	-1.330	-0.893	Foundation Loads
179	RC3	Max	0.659	0.807	0.792	1.182	6.062	0.077	Foundation Loads
		Min	0.000	-0.160	-2.387	-8.242	0.000	-0.055	Foundation Loads
181	RC3	Max	0.659	0.964	1.045	2.843	6.062	0.055	Foundation Loads
		Min	0.000	-0.417	-2.485	-10.106	0.000	-0.077	Foundation Loads
183	RC3	Max	0.707	0.885	0.867	1.153	6.617	0.172	Foundation Loads
		Min	0.000	-0.160	-2.431	-7.733	0.000	-0.047	Foundation Loads
185	RC3	Max	0.707	0.922	0.988	2.951	6.617	0.151	Foundation Loads
		Min	0.000	-0.419	-2.443	-9.566	0.000	-0.090	Foundation Loads
349	RC3	Max	0.473	0.683	0.501	1.388	4.415	0.368	Foundation Loads
		Min	-0.625	-0.132	-1.372	-6.390	-8.058	-1.649	Foundation Loads
353	RC3	Max	0.473	0.511	0.403	2.653	4.415	0.943	Foundation Loads
		Min	-0.690	-0.323	-1.190	-4.508	-7.660	-0.655	Foundation Loads
364	RC3	Max	0.658	0.885	0.867	1.153	6.040	0.080	Foundation Loads
		Min	-0.139	-0.160	-2.431	-7.733	-1.719	-0.173	Foundation Loads
366	RC3	Max	0.658	0.922	0.988	2.883	6.040	0.070	Foundation Loads
		Min	-0.147	-0.420	-2.444	-9.566	-1.769	-0.150	Foundation Loads
449	RC3	Max	0.472	0.715	0.540	1.353	4.395	0.392	Foundation Loads
		Min	-0.665	-0.128	-1.385	-6.741	-8.663	-1.631	Foundation Loads
453	RC3	Max	0.472	0.505	0.408	2.723	4.395	0.894	Foundation Loads
		Min	-0.755	-0.330	-1.150	-4.435	-8.580	-0.691	Foundation Loads
457	RC3	Max	0.704	0.747	0.515	1.259	7.844	1.268	Foundation Loads
		Min	0.000	-0.122	-1.334	-7.244	0.000	-0.388	Foundation Loads
465	RC3	Max	0.967	0.710	0.931	0.000	8.617	0.000	Foundation Loads
		Min	0.000	0.000	-2.326	-6.057	0.000	-0.158	Foundation Loads
471	RC3	Max	0.705	0.550	0.441	2.122	7.693	0.388	Foundation Loads
		Min	-0.173	-0.223	-1.262	-5.931	-1.790	-0.737	Foundation Loads
905	RC3	Max	0.719	0.964	0.842	1.713	6.774	0.031	Foundation Loads
		Min	0.000	-0.203	-2.540	-8.343	0.000	-0.045	Foundation Loads
908	RC3	Max	0.896	1.032	1.982	0.000	7.923	0.000	Foundation Loads
		Min	0.000	0.000	-4.612	-10.343	0.000	0.000	Foundation Loads
911	RC3	Max	0.719	0.961	0.912	3.581	6.774	0.045	Foundation Loads
		Min	0.000	-0.476	-2.412	-10.016	0.000	-0.031	Foundation Loads
1327	RC3	Max	0.511	0.746	0.514	2.251	4.915	0.215	Foundation Loads
		Min	-0.636	-0.200	-1.334	-7.235	-7.841	-1.268	Foundation Loads
1334	RC3	Max	0.596	0.710	0.931	0.000	5.675	0.158	Foundation Loads
		Min	-0.881	0.000	-2.325	-6.057	-7.607	0.000	Foundation Loads
1339	RC3	Max	0.511	0.550	0.441	3.749	4.915	0.738	Foundation Loads
		Min	-0.705	-0.410	-1.262	-5.931	-7.692	-0.361	Foundation Loads
1639	RC3	Max	0.631	0.654	0.431	1.349	5.944	1.199	Foundation Loads
		Min	0.000	-0.130	-1.364	-6.186	0.000	-0.434	Foundation Loads
1647	RC3	Max	0.813	0.666	0.917	0.000	7.238	0.076	Foundation Loads
		Min	-0.003	0.000	-2.360	-6.180	0.000	-0.287	Foundation Loads
1653	RC3	Max	0.631	0.560	0.431	2.056	5.944	0.434	Foundation Loads
		Min	-0.143	-0.194	-1.279	-6.138	-1.379	-0.758	Foundation Loads
1657	RC3	Max	0.635	0.697	0.450	1.678	6.035	1.222	Foundation Loads
		Min	0.000	-0.155	-1.379	-6.706	0.000	-0.440	Foundation Loads
1665	RC3	Max	0.833	0.692	0.911	0.464	7.578	0.050	Foundation Loads
		Min	-0.002	-0.041	-2.369	-5.932	0.000	-0.252	Foundation Loads
1675	RC3	Max	0.833	0.680	0.920	0.714	7.578	0.078	Foundation Loads
		Min	-0.002	-0.065	-2.352	-7.143	0.000	-0.256	Foundation Loads
1681	RC3	Max	0.635	0.607	0.433	2.439	6.034	0.440	Foundation Loads
		Min	-0.150	-0.216	-1.303	-6.774	-1.485	-0.771	Foundation Loads
2055	RC3	Max	0.490	0.653	0.430	2.367	4.647	0.234	Foundation Loads
		Min	-0.459	-0.207	-1.364	-6.178	-5.297	-1.199	Foundation Loads
2062	RC3	Max	0.579	0.666	0.917	0.000	5.476	0.287	Foundation Loads
		Min	-0.727	0.000	-2.360	-6.184	-5.622	-0.076	Foundation Loads
2067	RC3	Max	0.490	0.560	0.431	3.704	4.647	0.760	Foundation Loads
		Min	-0.548	-0.399	-1.278	-6.137	-5.433	-0.234	Foundation Loads
2071	RC3	Max	0.493	0.696	0.449	2.950	4.668	0.202	Foundation Loads
		Min	-0.459	-0.251	-1.379	-6.697	-5.360	-1.222	Foundation Loads
2078	RC3	Max	0.605	0.692	0.911	0.888	5.801	0.252	Foundation Loads
		Min	-0.735	-0.080	-2.369	-5.936	-5.834	-0.081	Foundation Loads
2086	RC3	Max	0.605	0.680	0.920	1.022	5.801	0.257	Foundation Loads
		Min	-0.727	-0.095	-2.351	-7.145	-5.509	-0.059	Foundation Loads
2091	RC3	Max	0.493	0.607	0.434	4.321	4.668	0.773	Foundation Loads
		Min	-0.551	-0.446	-1.303	-6.772	-5.482	-0.223	Foundation Loads

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RESULTS

Project: Model: DSA401J2030-19_V1 Date: 9/22/2020

■ 4.1 NODES - SUPPORT FORCES

Result Combinations

Node			Support Forces [kip]			Support Moments [kipft]			
No.	RC		P_X	P_Y	P_Z	M_X	M_Y	M_Z	
297	RC3	Max	1.172	0.670	0.654	2.315	15.269	0.778	Foundation Loads
		Min	-0.333	-0.188	-1.504	-7.074	-4.484	-1.083	Foundation Loads
299	RC3	Max	0.999	0.749	0.286	1.919	14.867	2.058	Foundation Loads
		Min	-0.001	-0.177	-1.688	-6.975	0.000	-0.542	Foundation Loads
513	RC3	Max	0.640	0.749	0.436	3.101	6.244	0.497	Foundation Loads
		Min	-1.000	-0.251	-1.688	-6.975	-14.871	-2.058	Foundation Loads
515	RC3	Max	0.640	0.670	0.652	4.314	6.243	1.080	Foundation Loads
		Min	-1.172	-0.432	-1.438	-7.075	-15.270	-0.778	Foundation Loads
721	RC3	Max	1.243	0.666	0.662	2.400	16.394	0.833	Foundation Loads
		Min	-0.281	-0.200	-1.501	-6.788	-3.633	-0.993	Foundation Loads
723	RC3	Max	1.043	0.751	0.300	1.872	15.587	2.008	Foundation Loads
		Min	0.000	-0.174	-1.688	-7.000	0.000	-0.583	Foundation Loads
1149	RC3	Max	0.642	0.751	0.442	3.051	6.274	0.585	Foundation Loads
		Min	-1.043	-0.248	-1.688	-6.998	-15.591	-2.008	Foundation Loads
1151	RC3	Max	0.642	0.666	0.660	4.449	6.273	0.990	Foundation Loads
		Min	-1.243	-0.440	-1.428	-6.788	-16.396	-0.833	Foundation Loads
1275	RC3	Max	0.833	1.010	0.753	2.505	8.579	0.076	Foundation Loads
		Min	-0.001	-0.281	-2.929	-11.618	-0.011	-0.212	Foundation Loads
1277	RC3	Max	0.832	1.298	1.510	4.085	8.579	0.211	Foundation Loads
		Min	0.000	-0.497	-2.793	-15.967	-0.003	-0.077	Foundation Loads
1279	RC3	Max	0.896	1.020	0.760	2.419	9.472	0.140	Foundation Loads
		Min	0.000	-0.278	-2.979	-10.691	0.000	-0.218	Foundation Loads
1281	RC3	Max	0.896	1.217	1.484	4.510	9.472	0.218	Foundation Loads
		Min	0.000	-0.519	-2.746	-14.803	0.000	-0.084	Foundation Loads
1287	RC3	Max	0.838	1.020	0.762	2.420	8.638	0.089	Foundation Loads
		Min	-0.185	-0.278	-2.979	-10.691	-2.817	-0.216	Foundation Loads
1289	RC3	Max	0.838	1.217	1.484	4.321	8.638	0.215	Foundation Loads
		Min	-0.175	-0.510	-2.746	-14.803	-2.620	-0.137	Foundation Loads
1369	RC3	Max	1.009	0.819	0.300	2.872	14.730	1.585	Foundation Loads
		Min	-0.012	-0.241	-1.676	-7.982	0.000	-0.602	Foundation Loads
1583	RC3	Max	0.675	0.819	0.463	4.882	6.795	0.384	Foundation Loads
		Min	-1.010	-0.372	-1.675	-7.982	-14.734	-1.585	Foundation Loads
1709	RC3	Max	0.898	1.091	0.756	3.878	9.508	0.000	Foundation Loads
		Min	-0.001	-0.378	-3.028	-10.836	-0.012	-0.250	Foundation Loads
1795	RC3	Max	1.194	0.771	0.680	3.607	15.425	0.601	Foundation Loads
		Min	-0.358	-0.248	-1.496	-9.819	-4.633	-0.770	Foundation Loads
2013	RC3	Max	0.675	0.771	0.678	5.995	6.794	0.783	Foundation Loads
		Min	-1.194	-0.547	-1.496	-9.820	-15.426	-0.552	Foundation Loads
2139	RC3	Max	0.898	1.449	1.428	5.329	9.508	0.250	Foundation Loads
		Min	0.000	-0.585	-2.816	-17.933	-0.004	-0.003	Foundation Loads
2141	RC3	Max	1.363	0.855	1.091	0.000	15.170	0.000	Foundation Loads
		Min	-0.047	0.000	-2.773	-9.384	0.000	-0.179	Foundation Loads
2143	RC3	Max	0.782	0.855	1.090	0.001	8.208	0.179	Foundation Loads
		Min	-1.363	0.000	-2.773	-9.382	-15.179	0.000	Foundation Loads
2145	RC3	Max	1.092	1.459	2.294	0.000	11.253	0.000	Foundation Loads
		Min	0.000	0.000	-5.316	-17.294	-0.004	-0.001	Foundation Loads
2221	RC3	Max	0.832	0.795	0.244	3.055	10.787	1.716	Foundation Loads
		Min	-0.031	-0.253	-1.682	-7.640	0.000	-0.536	Foundation Loads
2223	RC3	Max	0.649	0.795	0.438	5.096	6.404	0.101	Foundation Loads
		Min	-0.771	-0.383	-1.682	-7.640	-10.791	-1.716	Foundation Loads
2431	RC3	Max	0.944	0.811	0.654	3.264	11.363	0.535	Foundation Loads
		Min	-0.330	-0.223	-1.515	-10.414	-4.196	-1.060	Foundation Loads
2437	RC3	Max	0.649	0.812	0.652	5.463	6.403	1.056	Foundation Loads
		Min	-0.943	-0.510	-1.515	-10.416	-11.365	-0.388	Foundation Loads
2561	RC3	Max	1.097	0.839	1.074	0.001	11.006	0.265	Foundation Loads
		Min	-0.076	0.000	-2.786	-10.196	-0.105	-0.668	Foundation Loads
2563	RC3	Max	0.752	0.839	1.074	0.000	7.782	0.668	Foundation Loads
		Min	-1.097	0.000	-2.786	-10.194	-11.013	-0.265	Foundation Loads
2647	RC3	Max	0.837	0.816	0.250	3.629	10.950	1.728	Foundation Loads
		Min	-0.036	-0.292	-1.697	-7.951	0.000	-0.503	Foundation Loads
2649	RC3	Max	0.652	0.816	0.446	6.065	6.432	0.092	Foundation Loads
		Min	-0.777	-0.449	-1.696	-7.952	-10.954	-1.728	Foundation Loads
2985	RC3	Max	1.123	0.876	1.079	0.956	11.463	0.231	Foundation Loads
		Min	-0.085	-0.070	-2.801	-9.345	-0.273	-0.585	Foundation Loads
2987	RC3	Max	0.782	0.876	1.078	1.640	8.226	0.585	Foundation Loads
		Min	-1.123	-0.122	-2.801	-9.342	-11.471	-0.231	Foundation Loads
3071	RC3	Max	0.949	0.886	0.652	3.736	11.479	0.502	Foundation Loads
		Min	-0.348	-0.254	-1.538	-11.548	-4.499	-1.069	Foundation Loads
3076	RC3	Max	0.652	0.886	0.650	6.139	6.431	1.065	Foundation Loads
		Min	-0.949	-0.555	-1.537	-11.550	-11.482	-0.458	Foundation Loads
3200	RC3	Max	1.071	0.924	1.071	0.937	11.243	0.201	Foundation Loads
		Min	-0.085	-0.071	-2.791	-11.951	-0.272	-0.592	Foundation Loads
3202	RC3	Max	0.782	0.924	1.070	1.219	8.226	0.592	Foundation Loads
		Min	-1.065	-0.094	-2.791	-11.950	-10.359	-0.201	Foundation Loads

USA SHADE & Fabric Structures
Dallas, TX

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RESULTS

Project: Model: DSA401J3030-19_V1 Date: 9/25/2020

■ 4.1 NODES - SUPPORT FORCES

Result Combinations

Node			Support Forces [kip]			Support Moments [kipft]			
No.	RC		P_X	P_Y	P_Z	M_X	M_Y	M_Z	
1	RC3	Max	1.392	1.515	0.687	4.553	22.136	2.195	Foundation Loads
		Min	0.000	-0.431	-2.555	-18.815	0.000	-1.272	Foundation Loads
5	RC3	Max	1.650	1.089	0.782	6.849	22.544	1.270	Foundation Loads
		Min	-0.350	-0.627	-2.275	-10.865	-4.571	-0.737	Foundation Loads
9	RC3	Max	1.449	1.556	0.710	4.468	22.536	2.164	Foundation Loads
		Min	0.000	-0.425	-2.563	-19.389	0.000	-1.291	Foundation Loads
13	RC3	Max	1.692	1.082	0.781	7.059	23.215	1.290	Foundation Loads
		Min	-0.279	-0.643	-2.273	-10.755	-3.453	-0.684	Foundation Loads
179	RC3	Max	1.213	1.910	1.317	6.870	12.649	0.068	Foundation Loads
		Min	0.000	-0.749	-4.542	-21.262	-0.002	-0.297	Foundation Loads
181	RC3	Max	1.213	1.915	1.980	14.300	12.651	0.294	Foundation Loads
		Min	0.000	-1.342	-3.817	-23.708	-0.002	-0.068	Foundation Loads
183	RC3	Max	1.279	2.105	1.337	6.525	13.592	0.105	Foundation Loads
		Min	0.000	-0.729	-4.585	-23.990	0.000	-0.318	Foundation Loads
185	RC3	Max	1.279	1.806	1.925	15.295	13.593	0.314	Foundation Loads
		Min	-0.002	-1.398	-3.806	-21.825	-0.005	-0.105	Foundation Loads
349	RC3	Max	0.885	1.513	0.689	8.530	8.151	0.625	Foundation Loads
		Min	-1.373	-0.696	-2.555	-18.782	-22.132	-2.196	Foundation Loads
353	RC3	Max	0.886	1.089	0.783	13.362	8.160	1.164	Foundation Loads
		Min	-1.650	-1.080	-1.963	-10.863	-22.540	-0.793	Foundation Loads
364	RC3	Max	1.215	2.105	1.332	6.525	12.643	0.116	Foundation Loads
		Min	-0.262	-0.729	-4.585	-23.991	-3.861	-0.336	Foundation Loads
366	RC3	Max	1.215	1.806	1.925	14.410	12.643	0.332	Foundation Loads
		Min	-0.182	-1.354	-3.815	-21.827	-2.713	-0.116	Foundation Loads
449	RC3	Max	0.885	1.553	0.709	8.435	8.152	0.688	Foundation Loads
		Min	-1.397	-0.690	-2.563	-19.355	-22.533	-2.166	Foundation Loads
453	RC3	Max	0.886	1.082	0.782	13.493	8.161	1.148	Foundation Loads
		Min	-1.692	-1.088	-1.943	-10.753	-23.212	-0.858	Foundation Loads
457	RC3	Max	1.611	1.845	0.768	6.201	23.196	1.804	Foundation Loads
		Min	-0.022	-0.534	-2.515	-23.843	0.000	-1.263	Foundation Loads
465	RC3	Max	2.133	1.268	1.537	0.004	25.864	0.000	Foundation Loads
		Min	-0.041	0.000	-4.121	-13.126	0.000	-0.269	Foundation Loads
471	RC3	Max	1.813	1.220	0.906	10.838	24.851	1.262	Foundation Loads
		Min	-0.451	-0.898	-2.255	-14.525	-6.056	-0.423	Foundation Loads
913	RC3	Max	1.291	2.353	1.305	9.680	13.731	0.003	Foundation Loads
		Min	0.000	-0.929	-4.695	-27.989	-0.002	-0.339	Foundation Loads
917	RC3	Max	1.583	1.657	3.435	0.000	16.539	0.002	Foundation Loads
		Min	0.000	0.000	-7.777	-22.466	-0.003	-0.002	Foundation Loads
921	RC3	Max	1.291	2.137	1.828	18.711	13.732	0.335	Foundation Loads
		Min	0.000	-1.622	-3.847	-25.890	-0.002	-0.002	Foundation Loads
1349	RC3	Max	0.937	1.842	0.767	11.629	8.941	0.464	Foundation Loads
		Min	-1.461	-0.891	-2.514	-23.802	-23.191	-1.805	Foundation Loads
1357	RC3	Max	1.103	1.268	1.537	0.001	11.031	0.271	Foundation Loads
		Min	-2.068	0.000	-4.031	-13.127	-24.873	-0.002	Foundation Loads
1363	RC3	Max	0.937	1.220	0.906	18.528	8.951	1.039	Foundation Loads
		Min	-1.813	-1.416	-2.036	-14.519	-24.849	-0.814	Foundation Loads
1683	RC3	Max	1.313	1.550	0.608	6.693	17.459	2.128	Foundation Loads
		Min	-0.052	-0.563	-2.543	-19.687	0.000	-1.186	Foundation Loads
1691	RC3	Max	1.787	1.196	1.578	0.003	20.322	0.401	Foundation Loads
		Min	-0.124	0.000	-4.111	-14.280	0.000	-0.602	Foundation Loads
1697	RC3	Max	1.441	1.220	0.842	8.594	18.897	1.185	Foundation Loads
		Min	-0.435	-0.743	-2.298	-15.648	-5.847	-0.843	Foundation Loads
1701	RC3	Max	1.356	1.637	0.622	7.255	17.397	2.097	Foundation Loads
		Min	-0.060	-0.599	-2.552	-21.019	0.000	-1.082	Foundation Loads
1709	RC3	Max	1.898	1.240	1.602	1.537	22.189	0.429	Foundation Loads
		Min	-0.149	-0.102	-4.109	-13.021	-0.212	-0.535	Foundation Loads
1719	RC3	Max	1.896	1.236	1.588	2.779	22.164	0.328	Foundation Loads
		Min	-0.150	-0.202	-4.109	-16.314	-0.215	-0.520	Foundation Loads
1725	RC3	Max	1.450	1.284	0.847	9.910	19.052	1.081	Foundation Loads
		Min	-0.448	-0.830	-2.311	-16.673	-6.069	-0.874	Foundation Loads
2099	RC3	Max	0.912	1.548	0.669	12.025	8.585	0.144	Foundation Loads
		Min	-1.111	-0.906	-2.543	-19.648	-17.452	-2.129	Foundation Loads
2107	RC3	Max	1.101	1.196	1.596	0.000	11.058	0.603	Foundation Loads
		Min	-1.762	0.000	-4.061	-14.280	-20.011	-0.400	Foundation Loads
2113	RC3	Max	0.913	1.219	0.843	16.729	8.594	1.176	Foundation Loads
		Min	-1.441	-1.281	-2.058	-15.641	-18.897	-0.671	Foundation Loads
2117	RC3	Max	0.909	1.634	0.668	12.578	8.530	0.047	Foundation Loads
		Min	-1.106	-0.939	-2.552	-20.977	-17.384	-2.098	Foundation Loads
2125	RC3	Max	1.140	1.240	1.601	2.093	11.619	0.539	Foundation Loads
		Min	-1.817	-0.138	-4.075	-12.991	-21.063	-0.427	Foundation Loads
2135	RC3	Max	1.139	1.235	1.588	2.668	11.617	0.519	Foundation Loads
		Min	-1.752	-0.184	-4.079	-16.302	-19.713	-0.328	Foundation Loads
2141	RC3	Max	0.910	1.283	0.848	17.662	8.540	1.069	Foundation Loads
		Min	-1.450	-1.338	-2.069	-16.663	-19.052	-0.727	Foundation Loads

USA SHADE & Fabric Structures
Dallas, TX

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RESULTS

Project: Model: DSA401J3040-19_V1 Date: 9/28/2020

■ 4.1 NODES - SUPPORT FORCES

Result Combinations

Node			Support Forces [kip]			Support Moments [kipft]			
No.	RC		P_X	P_Y	P_Z	M_X	M_Y	M_Z	
295	RC3	Max	1.952	1.452	1.398	4.203	24.558	1.441	Foundation Loads
		Min	-0.853	-0.439	-2.754	-16.400	-10.684	-1.375	Foundation Loads
297	RC3	Max	1.536	1.250	0.553	6.750	24.104	3.558	Foundation Loads
		Min	-0.266	-0.672	-3.093	-12.408	-1.354	-1.446	Foundation Loads
510	RC3	Max	1.270	1.249	0.939	10.655	13.358	0.824	Foundation Loads
		Min	-1.537	-0.939	-3.094	-12.401	-24.119	-3.553	Foundation Loads
512	RC3	Max	1.270	1.452	1.396	9.637	13.360	1.708	Foundation Loads
		Min	-1.951	-0.950	-2.607	-16.408	-24.547	-1.239	Foundation Loads
635	RC3	Max	1.598	1.788	1.524	9.721	17.698	0.003	Foundation Loads
		Min	-0.001	-1.147	-5.551	-19.408	-0.016	-0.429	Foundation Loads
637	RC3	Max	1.598	2.895	3.060	9.916	17.696	0.429	Foundation Loads
		Min	-0.002	-1.138	-5.062	-34.101	-0.018	-0.002	Foundation Loads
719	RC3	Max	2.011	1.448	1.410	4.278	25.482	1.461	Foundation Loads
		Min	-0.807	-0.460	-2.753	-16.185	-9.819	-1.275	Foundation Loads
721	RC3	Max	1.571	1.251	0.567	6.716	24.688	3.491	Foundation Loads
		Min	-0.263	-0.670	-3.092	-12.433	-1.317	-1.467	Foundation Loads
1057	RC3	Max	1.674	1.807	1.540	9.628	18.681	0.078	Foundation Loads
		Min	0.000	-1.137	-5.617	-18.468	0.000	-0.449	Foundation Loads
1059	RC3	Max	1.674	2.765	3.031	10.856	18.682	0.448	Foundation Loads
		Min	-0.011	-1.196	-4.998	-32.011	-0.143	0.000	Foundation Loads
1147	RC3	Max	1.279	1.251	0.944	10.592	13.496	0.935	Foundation Loads
		Min	-1.572	-0.935	-3.092	-12.431	-24.704	-3.486	Foundation Loads
1149	RC3	Max	1.279	1.448	1.407	9.791	13.498	1.681	Foundation Loads
		Min	-2.010	-0.961	-2.599	-16.193	-25.471	-1.323	Foundation Loads
1272	RC3	Max	1.634	1.807	1.541	9.628	18.181	0.003	Foundation Loads
		Min	-0.267	-1.140	-5.617	-18.468	-3.960	-0.433	Foundation Loads
1274	RC3	Max	1.634	2.765	3.031	10.347	18.179	0.432	Foundation Loads
		Min	-0.196	-1.168	-4.998	-32.013	-2.856	-0.080	Foundation Loads
1358	RC3	Max	1.579	1.359	0.589	9.034	24.331	2.920	Foundation Loads
		Min	-0.321	-0.826	-3.093	-13.971	-2.190	-1.512	Foundation Loads
1569	RC3	Max	1.354	1.359	0.993	14.613	14.607	0.661	Foundation Loads
		Min	-1.581	-1.207	-3.093	-13.972	-24.348	-2.916	Foundation Loads
1694	RC3	Max	1.705	1.914	1.546	13.390	19.144	0.003	Foundation Loads
		Min	-0.001	-1.384	-5.707	-19.992	-0.015	-0.519	Foundation Loads
1780	RC3	Max	2.027	1.801	1.456	6.177	25.454	1.508	Foundation Loads
		Min	-0.966	-0.543	-2.749	-21.477	-12.139	-0.978	Foundation Loads
1995	RC3	Max	1.354	1.802	1.453	12.590	14.608	1.602	Foundation Loads
		Min	-2.026	-1.155	-2.718	-21.486	-25.438	-1.161	Foundation Loads
2120	RC3	Max	1.705	3.291	2.955	12.630	19.143	0.519	Foundation Loads
		Min	-0.002	-1.329	-5.115	-38.513	-0.022	-0.001	Foundation Loads
2122	RC3	Max	2.434	1.617	2.144	0.007	26.541	0.001	Foundation Loads
		Min	-0.562	-0.001	-5.141	-20.371	-3.936	-0.322	Foundation Loads
2124	RC3	Max	1.784	1.617	2.143	0.001	19.713	0.327	Foundation Loads
		Min	-2.434	0.000	-5.140	-20.402	-26.541	-0.002	Foundation Loads
2126	RC3	Max	2.256	3.129	4.423	0.005	24.181	0.003	Foundation Loads
		Min	-0.002	0.000	-9.978	-36.583	-0.023	-0.002	Foundation Loads
2204	RC3	Max	1.357	1.310	0.491	9.053	18.998	3.278	Foundation Loads
		Min	-0.306	-0.826	-3.096	-13.308	-1.976	-1.416	Foundation Loads
2206	RC3	Max	1.274	1.310	0.947	14.634	13.375	0.206	Foundation Loads
		Min	-1.262	-1.201	-3.096	-13.309	-19.006	-3.274	Foundation Loads
2413	RC3	Max	1.693	1.835	1.398	5.431	20.056	1.411	Foundation Loads
		Min	-0.927	-0.405	-2.764	-21.990	-11.517	-1.521	Foundation Loads
2419	RC3	Max	1.274	1.835	1.396	11.349	13.373	1.698	Foundation Loads
		Min	-1.691	-1.065	-2.731	-21.998	-20.046	-1.074	Foundation Loads
2542	RC3	Max	2.076	1.692	2.123	0.006	20.902	0.449	Foundation Loads
		Min	-0.577	0.000	-5.171	-22.375	-4.135	-1.041	Foundation Loads
2544	RC3	Max	1.695	1.694	2.122	0.002	18.393	1.047	Foundation Loads
		Min	-2.076	0.000	-5.170	-22.405	-20.903	-0.448	Foundation Loads
2628	RC3	Max	1.360	1.333	0.502	9.888	19.124	3.247	Foundation Loads
		Min	-0.319	-0.882	-3.117	-13.652	-2.148	-1.331	Foundation Loads
2630	RC3	Max	1.276	1.334	0.956	15.890	13.381	0.162	Foundation Loads
		Min	-1.265	-1.285	-3.117	-13.654	-19.132	-3.243	Foundation Loads
2964	RC3	Max	2.127	1.636	2.135	1.994	21.747	0.420	Foundation Loads
		Min	-0.615	-0.147	-5.203	-20.320	-4.721	-0.883	Foundation Loads
2966	RC3	Max	1.756	1.636	2.135	3.188	19.194	0.888	Foundation Loads
		Min	-2.127	-0.239	-5.202	-20.348	-21.748	-0.420	Foundation Loads
3050	RC3	Max	1.698	1.947	1.395	5.791	20.161	1.346	Foundation Loads
		Min	-0.962	-0.434	-2.771	-23.677	-12.039	-1.513	Foundation Loads
3179	RC3	Max	2.024	2.003	2.122	1.091	20.924	0.312	Foundation Loads
		Min	-0.615	-0.088	-5.202	-25.568	-4.721	-0.839	Foundation Loads
3181	RC3	Max	1.756	2.005	2.121	1.777	19.194	0.844	Foundation Loads
		Min	-2.024	-0.139	-5.201	-25.595	-19.890	-0.311	Foundation Loads
3194	RC3	Max	1.276	1.947	1.393	12.058	13.380	1.583	Foundation Loads
		Min	-1.696	-1.112	-2.760	-23.685	-20.150	-1.205	Foundation Loads

Multiknit INTERNATIONAL

190/F5 Fire rated specifications

Standard range — Revision 0 — 28-Oct-12

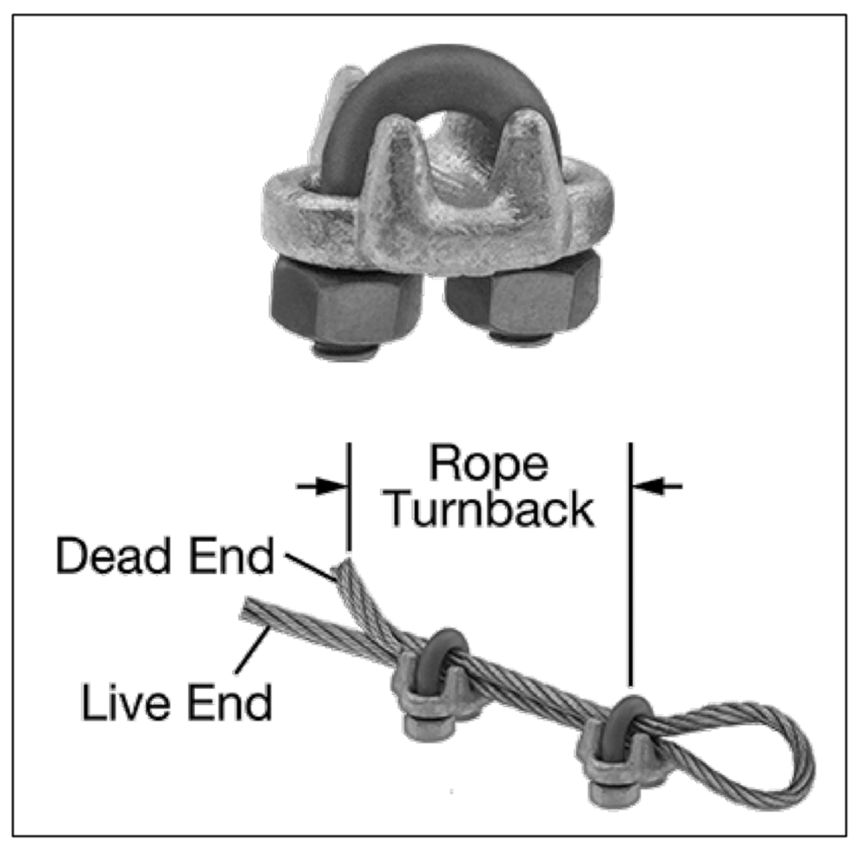
Colour	Shade %	UV Block %	Average GSM	Average Warp break strength kgs	Average Elongation %	Average Weft break strength kgs	Average Elongation %	Average Burst Kpa	Average Burst to Mass ratio
Desert Sand	80	92	185	50	40	72	73	156	0.84
Blue	80	85	185	50	40	72	73	156	0.84
Brown	85		185	50	40	72	73	156	0.84
Green	80	85	185	50	40	72	73	156	0.84
Red	80	86	185	50	40	72	73	156	0.84
Silver	80	81	185	50	40	72	73	156	0.84
Terracotta	75	82	185	50	40	72	73	156	0.84
Yellow	80	89	185	50	40	72	73	156	0.84
				110 LB		159 LB		3258 PSF	

CONVERSION TO IMPERIAL UNITS:
185 GSM = .0378 psf
50 KGS = 110 Lb
72 KGS = 159 Lb
156 Kpa = 3258 psf

Notes:
- 190/F5 conforms to The California State Fire Marshal Title 19 Test for Small scale Fabrics
- Tear tests are done using a 50mm wide strip and a cross head speed of 500mm/min
- This report has been compiled using the mean results from all tests conducted on the given sample by our Quality Control Laboratory, the information provided is considered to be a good reflection of the relevant properties of the fabric tested. These results must only be used as an indication of the quality and characteristics of the fabric tested.
Company cannot be held responsible or liable in any way whatsoever should this information differ to that of a registered testing institution.

PP. Deon Joubert
General Manager - Multiknit (Pty) Ltd

Tommy Rogers
Managing Director - Multiknit (Pty) Ltd



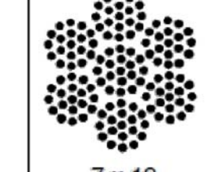
FORGED WIRE ROPE CLAMP

FITTING TYPE ROPE CLAMP
FABRICATION: FORGED
MATERIAL: GALVANIZED STEEL
FOR WIRE ROPE DIAMETER 3/8"
NUMBER OF CLAMPS REQUIRED: 2
ROPE TURNBACK: 6 1/2"
FOR WIRE ROPE CONSTRUCTION 7 × 19
ATTACHMENT TYPE: LOOP
CLAMP:WIDTH 2", HEIGHT 1 15/16", THICKNESS 1 11/16"
REQUIRED INSTALLATION TOOL TORQUE WRENCH
REQUIRED TORQUE 45 FT.-LBS.
CAPACITY 80% OF THE ROPE'S CAPACITY
SPECIFICATIONS MET ASME B30.26, FED. SPEC. FF-C-450

Aircraft Cable

Preformed, made in accordance with commercial specifications military and federal specification rope available.

Carbon Steel (Aircraft Cable) - Galvanized cable has the highest strength and greatest fatigue life of the materials offered. It has good to fair corrosion resistance in rural to industrial atmosphere environments. This material is most widely used for small diameter cables. Tin over galvanized cable offers greater corrosion resistance and reduced friction over pulleys.



7 x 19

7 x 19		Galvanized Min. Breaking Strengths (lbs)
Dia. (In)	Approx. Wt 1000 Ft/lbs	
3/32	17.	1,000
1/8	29.	2,000
5/32	45.	2,800
3/16	65.	4,200
7/32	86.	5,600
1/4	110.	7,000
9/32	139.	8,000
5/16	173.	9,800
3/8	243.	14,400

BASIC LOAD CASES

DEAD LOAD	0.0378 PSF (FABRIC)
FLOOR LIVE LOAD	N/A
ROOF LIVE LOAD	5 PSF
ROOF SNOW LOAD	5 PSF
SUPERIMPOSED LOADS	N/A
WIND LOAD	
BASIC DESIGN WIND SPEED (3 SEC GUST)	115 MPH
VELOCITY PRESSURE qz	SEE TABLE 1 ITEM 24
COMPONENT AND CLADDING qz (CABLE AND CABLE HARDWARE ONLY)	SEE TABLE 1 ITEM 24
SEISMIC LOAD	
SEISMIC RESPONSE COEFFICIENTS Cs	1.6
DESIGN BASE SHEAR	SEE TABLE 1 ITEM 23

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01 -119834 INC: 0
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 5/20/2022

THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN PERMISSION.

USA SHADE & Fabric Structures
CORPORATE HEADQUARTERS
2580 ESTERS BLVD. SUITE 100
DFW AIRPORT, TX, 75261
800-966-5005

CERTIFICATIONS:
IAS CERTIFICATION No: FA-428
CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:
Alum Rock Union ES Dist

PROJECT NAME:
San Antonio Elementary

LOCATION:
1721 E San Antonio St.
San Jose, CA 95116

MODEL NUMBER:
DSA401Q-19

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-119454 PC
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑ CG ☐
DATE: 03/26/2021

STRUCTURE TYPE:
QUAD JOINED HIPS
DSA
SIZE: SEE TABLE 1

SCALE : NONE

DRAWING SIZE: D

PRE-CHECK (PC) DOCUMENT
Code : 2019 CBC
A separate project application for construction is required.

Eng. By :	JO	08/15/20
Design By :	MP	08/15/20
Approved By :	JO	08/15/20

DRAWING DESCRIPTION:
REACTIONS

LICENSED ARCHITECT
DAVID HIGGINSON
NO.C19168
REN. 10-31-21
STATE OF CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
MARK ALBERT LOWE
No. 3693
STRUCTURAL
STATE OF CALIFORNIA

DWG. DSA401Q-19

SHEET 25.3-2000

REV. NC