

227

x
227.3

x
227.1

PORTION OF 373 TALC
TO REMAIN AS S

APPROX. REVISED ZO
PROPOSED PROF

PORTION OF 371 TALCO
CHANGED TO C

MAP 4

JTF PROF

DETENTION BASIN (	
STRUCTURE (w/ TR	
RACK)	
TF=228.6±	
2" ORIFICE INV.=22	
6" ORIFICE INV.=22	
8' WEIR ELEV.=226	

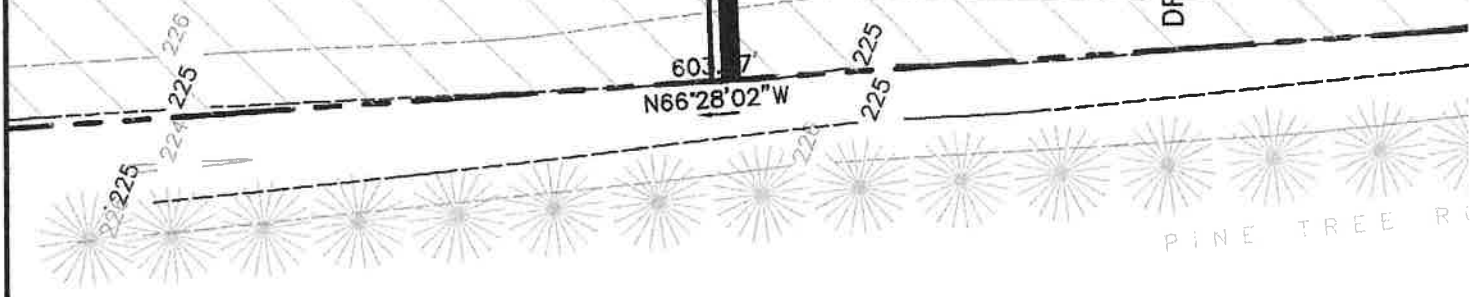
UTILITY RIGHT OF WAY &
DRAINAGE EASEMENT (REFER
TO MAP REF #1)

LOT 6B

RIDRAP @

603.17'
N66°28'02"W

PINE TREE RI



Graph Look Up

ITETripGen v1.0.0.0 - 4/20/22

Query: Empty

DATA SOURCE

Trip Generation Manual, 11th Ed

SEARCH BY LAND USE CODE

948

LAND USE GROUP

(900-949) Services

LAND USE

948 - Automated Car Wash

LAND USE SUBCATEGORY

All Sites

RETHINKLOCATION

General Urban/Suburban

INTERMEDIATE VARIABLE (PP)

Car Wash Tunnels

TRAIL SPACING

Saturday, Peak Hour of Generator

TRIP TYPE

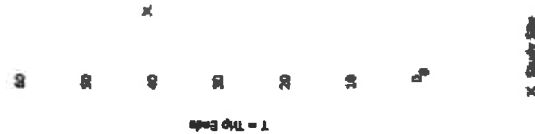
Vehicle

ENTER IN VALUE TO CALCULATE TRIP

2

Calculate

Data Plot and Equation



Custom - Small Sample Size

Land Use	Automated Car Wash (948) (See Trip Generation Manual, 11th Ed)
Inter. Data File	
Independent Variable	Car Wash Tunnels
Trail Period	
Peak Hour of Generator	
Re-think location	
General Urban/Suburban	
Intermediate Variable (PP)	Car Wash Tunnels
Trail Spacing	
Vehicle	
Number of Tunnels	1
Avg. Peak of Car Wash Tunnels	1
Re-think Rate	41.00
Range of Rates	41.00 - 41.00
Standard Deviation	0.00
Fitted Curve Equation	Y = 5X
Max Gen	10
R ²	1.00
Dispersed Distribution	40% entering, 60% exiting
Calculated Trip Rate	Average Rate: 20 (Trip), 20 (Rate), 44 (Rate)

Use the mouse wheel to Zoom Out or Zoom In.
Hover the mouse pointer on data points to view X and Y values.

Graph Look Up

ITE TripGen Web-based App

Graph Look Up

1. Trip Generation

2. Trip Distribution

3. Trip Assignment

4. Trip Delay

5. Trip Delay

6. Trip Delay

Category

DATA SOURCE:

Trip Generation Manual, 11th Ed

SEARCH BY LAND USE CODE:

541

LAND USE GROUP:

(500-600) Services

LAND USE:

541 - Quick Lubrication Vehicle Shop

LAND USE SUBCATEGORY:

AS Shop

SETTLEMENT:

General Urban/Suburban

INDEPENDENT VARIABLE (P):

1000 Sq. Ft. GFA

TIME PERIOD:

Weekday, Full Peak Hour of Generator

TRIP TYPE:

Vehicle

ENTER A VALUE TO CALCULATE TRIP:

3.84

Calculate

Data Plot and Equation

30

25

20

X

T = Trip Ends

15

10

5

0

2

4

6

8

10

X = 1000 Sq. Ft. GFA

Reset Zoom

Zoom

X Study Site

Average Rate

Use the mouse wheel to Zoom Out or Zoom In.
Hover the mouse pointer on data points to view X and Y values.

DATA STATUS: 22

Land User:

Quick Lubrication Vehicle Shop (541) Click for

Explanatory and Data Files

Independent Variables:

1000 Sq. Ft. GFA

Time Period:

Weekday

Full Peak Hour of Generator

Settlement:

General Urban/Suburban

Trip Type:

Vehicle

Number of Studies:

1

1

Avg. 1000 Sq. Ft. GFA:

1

Average Rate:

0.42

Range of Rates:

0.42 - 0.42

Standard Deviation:

...

Planned Curve Significant:

Not Given

p²:

...

Directional Classification:

40% entering, 60% exiting

Calculated Trip Ends:

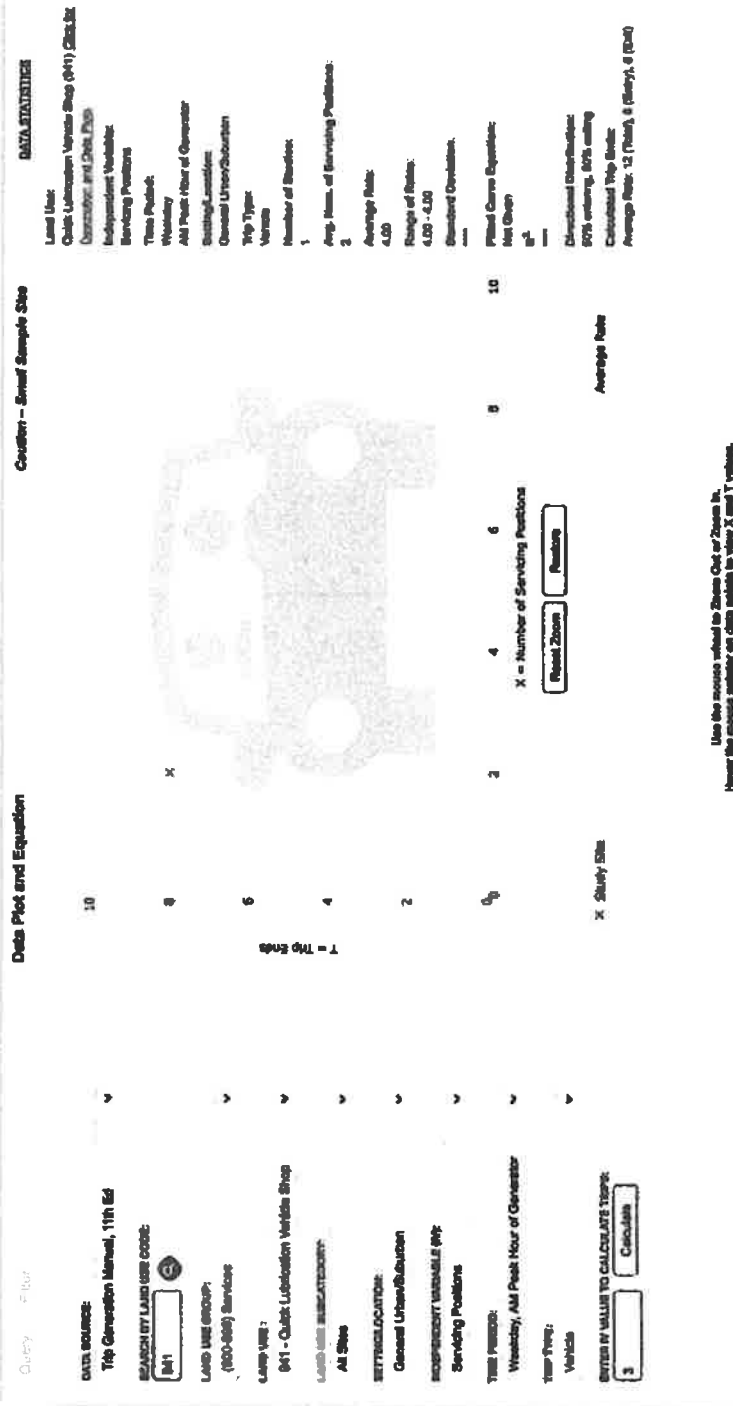
Average Rate: 30 (Trips), 18 (Entry), 20 (Exit)

Graph Look Up

ITripGen Web-based App

Query	File	Data Plot and Equation	Caution - Small Sample Size	Data Statistics
DATA SOURCE: Trip Generation Manual, 11th Ed SEARCH BY LAND USE CODE: 941				Land Use: Quick Lubrication Vehicle Shop (941) C85.102 DISPERSED AND DENSE DATA Independent Variables: 1000 Sq. Ft. CPA Time Period: Weekday Setting/Location: General Urban/Suburban Trip Type: Vehicle Number of Routes: 1 Avg. 1000 Sq. Ft. CPA: 1 Average Rate: 89.57 Range of Rates: 89.57 - 89.57 Standard Deviation: --- Plotted Curve Equation: Not Given R^2 : --- Directional Distribution: 80% entering, 80% exiting Calculated Trip Rate: Average Rate: 287 (Total), 123 (Entry), 124 (Exit)
LAND USE GROUP: (900-999) Services LAND USE: 941 - Quick Lubrication Vehicle Shop LAND USE SUBCATEGORY: All Sites SETTING/LOCATION: General Urban/Suburban INDEPENDENT VARIABLE (X): 1000 Sq. Ft. CPA TIME PERIOD: Weekday TRIP TYPE: Vehicle ENTER IN VALUE TO CALCULATE TRIP RATE: 3.84 Calculate		T = Trip Ends 300 150 100 50 0	X = Study Site 2 4 6 8 10 X = 1000 Sq. Ft. CPA Reset Zoom Pan/Move	

Use the mouse wheel to Zoom Out or Zoom In.
 Hover the mouse pointer on data points to view X and Y values.



Graph Look Up

LIFE SIZE WEATHERED

DATE: 4/18/22

DATA SOURCE:

Tip Generation Manual, 11th Ed

SEARCH BY LAND USE CODE:

941

LAND USE GROUP:

(900-999) Services

LAND USE:

941 - Quick Lubrication Vehicle Shop

LAND USE SUBCATEGORY:

All Shops

SETTING/LOCATION:

General Urban/Suburban

INTERDEPENDENT VARIABLES (X):

Servicing Positions

TIME PERIOD:

Weekday, PM Peak Hour of Generator

TRIP TYPE:

Vehicle

ENTER IN VALUES TO CALCULATE TIME:

3

Calculate

Data Plot and Equation

20

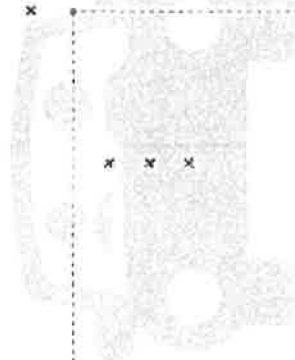
15

10

5

0

T = Trip Ends



X = Number of Servicing Positions

Reset Zoom

Restore

X Study Site

Average Rate

DATA STATISTICS

Land Use:

Quick Lubrication Vehicle Shop (941) Click for

Description and Data Plot

Independent Variables:

Servicing Positions

Time Period:

Weekday, PM Peak Hour of Generator

Setting/Location:

General Urban/Suburban

Trip Type:

Vehicle

Number of Studies:

6

Avg. No. of Servicing Positions:

3

Average Rate:

4.80

Range of Rate:

3.30 - 6.00

Standard Deviation:

1.07

Planned Curve Segment:

Not Given

R²:

Directional Characteristics:

55% entering, 45% exiting

Calculated Trip Ends:

Average Rate: 14 (Peak), 8 (Entry), 6 (Exit)

Use the mouse wheel to Zoom Out or Zoom In.
Hover the mouse pointer on data points to view X and Y values.

Graph Look Up

ITETripGen Web-based App

Display Filter

DATA SOURCE:

Trip Generation Manual, 11th Ed

SEARCH BY LAND USE CODE:

S41

LAND USE GROUP:

(000-699) Services

LAND USE 1:

S41 - Quiet Livestock Vehicle Shop

LAND USE SUBCATEGORY:

All Sites

SETTLEMENT:

General Urban/Suburban

REPRESENTATIVE VARIABLE (V):

Servicing Positions

TRIP RATE:

Saturday

TRIP TYPE:

Vehicle

ENTER A VALUE TO CALCULATE TRIP:

3

Calculate

Data Plot and Equation

Condition - Street Sample Size

DATA STATISTICS

Land Use

Quiet Livestock Vehicle Shop (S41) Click for

Description and Data Files

Independent Variables

Servicing Positions

Trip Rate

Saturday

Settlement

General Urban/Suburban

Representative Variable (V)

Servicing Positions

Trip Type

Vehicle

Number of Studies

1

Avg. Max. of Servicing Positions

2

Average Rate

42.00

Range of Rate

42.00 - 42.00

Standard Deviation

0.00

Plot Curve Equation

Not Given

R²

0.00

Discretized Distribution

80% entering, 80% exiting

Calculated Trip Rate

Average Rate: 158 (Total, 42 (Sat), 42 (Sat))

100

X



T = Trip Ends

2

4

6

10

X = Number of Servicing Positions

Reset Zoom

Reset

X: Study Size

Average Rate

Use the mouse wheel to Zoom Out or Zoom In.
 Hover the mouse pointer on data points to view X and Y values.

Graph Look Up

ITE TripGen Web-based App

Go to the main page

How to Use the TripGen

TripGen Resources

Get TripGen

Get TripGen

Get TripGen

Get TripGen

Query

Data sources

Trip Generation Manual, 11th Ed

SEARCH BY LAND USE CODE

BMT

LAND USE GROUP

(R00-800) Services

LAND USE

B41 - Quick Lubrication Vehicle Shop

LAND USE SUBCATEGORY

All Sites

SET TRIP LOCATION

General Urban/Suburban

INDEPENDENT VARIABLE (X)

Servicing Positions

TRIP PURPOSE

Saturday, Peak Hour of Generator

TRIP TYPE

Vehicle

ENTER Y VALUES TO CALCULATE TRIP

Y

Calculate

Data Plot and Equation

20

Cautions - Small Sample Size

DATA PRESENTATION

Land Use

Quick Lubrication Vehicle Shop (B41) Data Set

Generation and Data Entry

Independent Variable

Servicing Positions

Trip Purpose

Saturday, Peak Hour of Generator

Set Trip Location

General Urban/Suburban

Trip Type

Vehicle

Number of Studies

3

Avg. Num. of Servicing Positions

3

Average Rate

4.00

Range of Rate

2.67 - 7.00

Standard Deviation

2.68

Peak Curve Equation

Not Given

g₂

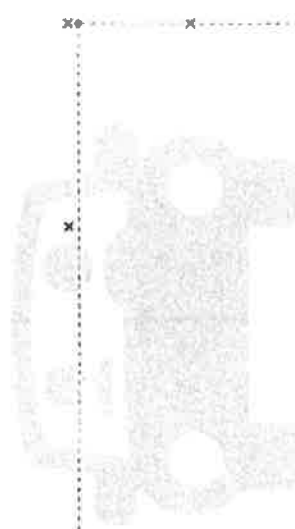
m

Directional Distribution

47% urban, 53% sub

Calculated Trip Rate

Average Rate: 14 (Trip), 6 (Shop), 8 (Sat)



X = Number of Servicing Positions

Reset Zoom

Full Screen

X Study Size

Average Rate

Use the mouse wheel to Zoom Out or Zoom In.

Hover the mouse pointer on data points to view X and Y values.

Graph Look Up

iETripGen Web-based App

Query Filter

DATA SOURCE

T10 Generation Manual, 11th Ed

SEARCH BY LAND USE CODE

841

LAND USE GROUP

(800-899) Services

LAND USE /

841 - Quick Lubrication Vehicle Shop

LAND USE SUBCATEGORY

All Sites

SETTLEMENT LOCATION

General Urban/Rural/Urban

INDEPENDENT VARIABLE (X)

Servicing Positions

TIME PERIOD

Weekday

TRIP TYPE

Vehicle

ENTER X VALUES TO CALCULATE TRIP:

3

Calculate

Data Plot and Equation

Caution - Small Sample Size

DATA STATISTICS

Land Use:
Quick Lubrication Vehicle Shop (841) 200.00Directional Distribution:
Independent Variable:
Servicing PositionsTime Period:
WeekdaySettlement Location:
General Urban/Rural/UrbanTrip Type:
VehicleNumber of Studies:
1Avg. Num. of Servicing Positions:
2Average Rate:
40.00Range of Rates:
40.00 - 40.00Standard Deviation:
---Planned Curve Signature:
Not Chosenp²:
---Directional Distribution:
85% entering, 85% exitingCalculated Trip Rate:
Average Rate: 125 (Week, 60 Study), 60 (Week, 60 Study), 60 (Week)

X = Number of Servicing Positions

Fixed Zoom

Reset

X Study Size

Average Rate

Use this mouse wheel to Zoom Out or Zoom In.
Move the mouse pointer on data points to view X and Y values.

Access to my data

Log out

Graph Look Up

iETripGen Web-based App

Query

Filter

DATA SOURCE:

Trip Generation Manual, 11th Ed

SEARCH BY LAID USE CODE:

947

LAID USE CATEGORY:

(900-999) Services

LAID USE:

947 - Self-Service Car Wash

LAID USE SPECIFICATION:

All Sites

SETTING LOCATION:

General Urban/Suburban

INDEPENDENT VARIABLE (X):

Wash Stalls

TRIP PERIOD:

Weekday, AM Peak Hour of Generator

TRIP TYPE:

Vehicle

ENTER IN VALUE TO CALCULATE Y-VALUE:

0

Calculate

DATA PLOT AND EQUATION

30

40

50

60

70

80

90

100

Y = Trip Ends

X = Wash Stalls

Y = Study Site

Caution - Small Sample Size

Plot Zoom

Reset

Y = Number of Wash Stalls

Plot Zoom

Reset

Average Rate

DATA STATISTICS

Laid Use:

Self-Service Car Wash (947) [Click for Description](#)

Independent Variable:

Wash Stalls

Time Period:

Weekday

AM Peak Hour of Generator

Setting Location:

General Urban/Suburban

Trips Type:

Vehicle

Number of Stalls:

1

Avg. Num. of Wash Stalls:

1

Average Rate:

0.00

Range of Points:

0.00 - 0.00

Standard Deviation:

0.00

Planned Curve Equation:

Not Given

Chi-Square:

0.00

Directional Distribution:

80% entering, 20% exiting

Calculated Trip Rate:

Average Rate: 04 (Peak), 32 (Entry), 32 (Exit)

Use the mouse wheel to Zoom Out or Zoom In.
 Hover the mouse pointer on data points to view X and Y values.

Graph Look Up

DATA SOURCE

The Generation Manual, 11th Ed

SEARCH BY LAND USE CODE

847

LAND USE GROUP

(800-850) Services

LAND USE

847 - Self-Service Car Wash

LAND USE SUBCATEGORY

All

SETTING LOCATION

General Urban/Suburban

INDEPENDENT VARIABLE

Wash Stalls

TIME PERIOD

Weekday

TRIP TYPE

Vehicle

ENTER IN VALUES TO CALCULATE TRIP

8

Calculate

DATA PLOT AND EQUATION

847

800

400

200

$T = \text{Trip Ends}$

D_0

$X = \text{Study Site}$

$X = \text{Number of Wash Stalls}$

Reset Zoom

Fullscreen

CONDITION - Small Sample Size

DATA STATISTICS

Land Use: Self-Service Car Wash (847) Click to Download

Est Data Plot

Independent Variable: Wash Stalls

Time Period: Weekday

Setting Location: General Urban/Suburban

Trip Type: Vehicle

Number of Stalls: 1

Avg. No. of Wash Stalls: 0

Average Rate: 100.00

Range of Rate: 100.00 - 100.00

Standard Deviation: ---

Planned Curve Equivalent: Not Given

Dev: ---

Distributional Distribution: 80% entering, 20% exiting

Calculated Trip Ends: Average Rate: 804 (Total), 422 (Study), 422 (Study)

Average Rate

Use the mouse wheel to Zoom Out or Zoom In.
Hover the mouse pointer on data points to view X and Y values.

VERSION: 4.0 (UPDATES) | DATA: 11TH ED T.O.M. TERMS AND CONDITIONS | PRIVACY | ITETripGen.org/Query

https://itetripgen.org/Query

1/1

SYNCHRO Capacity Analysis Worksheets

HCM Unsignalized Intersection Capacity Analysis

1: ROUTE 83 & SITE DRIVE 2

BACKGROUND TRAFFIC VOLUMES

A.M.

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	11	10	0	527	1328	0
Future Volume (Veh/h)	11	10	0	527	1328	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	11	0	573	1443	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1730	722	1443			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1730	722	1443			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	85	97	100			
cM capacity (veh/h)	79	369	466			
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2
Volume Total	12	11	286	286	722	722
Volume Left	12	0	0	0	0	0
Volume Right	0	11	0	0	0	0
cSH	79	369	1700	1700	1700	1700
Volume to Capacity	0.15	0.03	0.17	0.17	0.42	0.42
Queue Length 95th (ft)	13	2	0	0	0	0
Control Delay (s)	58.3	15.0	0.0	0.0	0.0	0.0
Lane LOS	F	C				
Approach Delay (s)	37.6		0.0		0.0	
Approach LOS	E					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			46.7%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2: ROUTE 83 & SITE DRIVE 1

BACKGROUND TRAFFIC VOLUMES
A.M.

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	6	26	522	1312	26
Future Volume (Veh/h)	5	6	26	522	1312	26
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	7	28	567	1426	28
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1780	727	1454			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1780	727	1454			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	93	98	94			
cM capacity (veh/h)	69	366	461			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	12	28	284	284	951	503
Volume Left	5	28	0	0	0	0
Volume Right	7	0	0	0	0	28
cSH	131	461	1700	1700	1700	1700
Volume to Capacity	0.09	0.06	0.17	0.17	0.56	0.30
Queue Length 95th (ft)	7	5	0	0	0	0
Control Delay (s)	35.2	13.3	0.0	0.0	0.0	0.0
Lane LOS	E	B				
Approach Delay (s)	35.2	0.6			0.0	
Approach LOS	E					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			47.1%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 1: ROUTE 83 & SITE DRIVE 2

BACKGROUND TRAFFIC VOLUMES
P.M.

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (veh/h)	38	33	0	1179	880	0
Future Volume (Veh/h)	38	33	0	1179	880	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	41	36	0	1282	957	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1598	478	957			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1598	478	957			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	58	93	100			
cM capacity (veh/h)	97	533	714			
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2
Volume Total	41	36	641	641	478	478
Volume Left	41	0	0	0	0	0
Volume Right	0	36	0	0	0	0
cSH	97	533	1700	1700	1700	1700
Volume to Capacity	0.42	0.07	0.38	0.38	0.28	0.28
Queue Length 95th (ft)	44	5	0	0	0	0
Control Delay (s)	66.8	12.2	0.0	0.0	0.0	0.0
Lane LOS	F	B				
Approach Delay (s)	41.3		0.0		0.0	
Approach LOS	E					
Intersection Summary						
Average Delay			1.4			
Intersection Capacity Utilization			42.6%		ICU Level of Service	A
Analysis Period (min)			15			

HQM Unsignalized Intersection Capacity Analysis

2: ROUTE 83 & SITE DRIVE 1

BACKGROUND TRAFFIC VOLUMES

P.M.

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	17	22	55	1162	858	55
Future Volume (Veh/h)	17	22	55	1162	858	55
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	18	24	60	1263	933	60
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1714	496	993			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1714	496	993			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
pD queue free %	76	95	91			
cM capacity (veh/h)	74	519	692			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	42	60	632	632	622	371
Volume Left	18	60	0	0	0	0
Volume Right	24	0	0	0	0	60
cSH	145	692	1700	1700	1700	1700
Volume to Capacity	0.29	0.09	0.37	0.37	0.37	0.22
Queue Length 95th (ft)	28	7	0	0	0	0
Control Delay (s)	39.6	10.7	0.0	0.0	0.0	0.0
Lane LOS	E	B				
Approach Delay (s)	39.6	0.5			0.0	
Approach LOS	E					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization			42.1%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

2: ROUTE 83 & SITE DRIVE 1

BACKGROUND TRAFFIC VOLUMES
SATURDAY

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	16	21	50	1002	1036	50
Future Volume (Veh/h)	16	21	50	1002	1036	50
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	17	23	54	1089	1126	54
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1806	590	1180			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1806	590	1180			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
IF (s)	3.5	3.3	2.2			
p0 queue free %	73	95	91			
cM capacity (veh/h)	64	451	588			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	40	54	544	544	751	429
Volume Left	17	54	0	0	0	0
Volume Right	23	0	0	0	0	54
cSH	126	588	1700	1700	1700	1700
Volume to Capacity	0.32	0.09	0.32	0.32	0.44	0.25
Queue Length 95th (ft)	31	8	0	0	0	0
Control Delay (s)	46.2	11.7	0.0	0.0	0.0	0.0
Lane LOS	E	B				
Approach Delay (s)	46.2	0.6			0.0	
Approach LOS	E					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization			46.9%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 1: ROUTE 83 & SITE DRIVE 2

BACKGROUND TRAFFIC VOLUMES
SATURDAY



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↰	↱		↕	↕	↱
Traffic Volume (veh/h)	36	32	0	1018	1054	0
Future Volume (Veh/h)	36	32	0	1018	1054	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	39	35	0	1107	1146	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1700	573	1146			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1700	573	1146			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	53	92	100			
cM capacity (veh/h)	83	463	605			
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2
Volume Total	39	35	554	554	573	573
Volume Left	39	0	0	0	0	0
Volume Right	0	35	0	0	0	0
cSH	83	463	1700	1700	1700	1700
Volume to Capacity	0.47	0.08	0.33	0.33	0.34	0.34
Queue Length 95th (ft)	49	6	0	0	0	0
Control Delay (s)	82.0	13.4	0.0	0.0	0.0	0.0
Lane LOS	F	B				
Approach Delay (s)	49.5		0.0		0.0	
Approach LOS	E					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utilization			39.1%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 1. ROUTE 83 & SITE DRIVE 2

COMBINED TRAFFIC VOLUMES
A.M.

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	←	→	←	↑↑	↑↑	
Traffic Volume (veh/h)	11	10	0	537	1328	0
Future Volume (Veh/h)	11	10	0	537	1328	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	11	0	584	1443	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1735	722	1443			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
yCu, unblocked vol	1735	722	1443			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	85	97	100			
cM capacity (veh/h)	79	369	466			
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2
Volume Total	12	11	292	292	722	722
Volume Left	12	0	0	0	0	0
Volume Right	0	11	0	0	0	0
cSH	79	369	1700	1700	1700	1700
Volume to Capacity	0.15	0.03	0.17	0.17	0.42	0.42
Queue Length 95th (ft)	13	2	0	0	0	0
Control Delay (s)	58.9	15.0	0.0	0.0	0.0	0.0
Lane LOS	F	C				
Approach Delay (s)	37.9		0.0		0.0	
Approach LOS	E					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			46.7%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

2. ROUTE 83 & SITE DRIVE 1

COMBINED TRAFFIC VOLUMES
A.M.



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↰		↰	↑↑	↑↑	↱
Traffic Volume (veh/h)	15	16	36	522	1312	36
Future Volume (Veh/h)	15	16	36	522	1312	36
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	17	39	567	1426	39
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
XX, platoon unblocked						
vC, conflicting volume	1807	732	1465			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1807	732	1465			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	75	95	91			
cM capacity (veh/h)	64	363	457			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	33	39	284	284	951	514
Volume Left	16	39	0	0	0	0
Volume Right	17	0	0	0	0	39
pSH	112	457	1700	1700	1700	1700
Volume to Capacity	0.30	0.09	0.17	0.17	0.56	0.30
Queue Length 95th (ft)	28	7	0	0	0	0
Control Delay (s)	50.2	13.6	0.0	0.0	0.0	0.0
Lane LOS	F	B				
Approach Delay (s)	50.2	0.9			0.0	
Approach LOS	F					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization			47.4%		ICU Level of Service	A
Analysis Period (min)			15			

HCM-Unsignalized Intersection Capacity Analysis

1: ROUTE 83 & SITE DRIVE 2

COMBINED TRAFFIC VOLUMES
P.M.

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (veh/h)	38	33	0	1189	888	0
Future Volume (Veh/h)	38	33	0	1189	888	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	41	36	0	1292	965	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1611	482	965			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1611	482	965			
IC, single (s)	6.8	6.9	4.1			
IC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	57	93	100			
cM capacity (veh/h)	95	530	709			
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2
Volume Total	41	36	646	646	482	482
Volume Left	41	0	0	0	0	0
Volume Right	0	36	0	0	0	0
cSH	95	530	1700	1700	1700	1700
Volume to Capacity	0.43	0.07	0.38	0.38	0.28	0.28
Queue Length 95th (ft)	45	5	0	0	0	0
Control Delay (s)	68.8	12.3	0.0	0.0	0.0	0.0
Lane LOS	F	B				
Approach Delay (s)	42.4		0.0		0.0	
Approach LOS	E					
Intersection Summary						
Average Delay			1.4			
Intersection Capacity Utilization			42.9%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

2: ROUTE 83 & SITE DRIVE 1

COMBINED TRAFFIC VOLUMES
P.M.

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (veh/h)	27	32	63	1162	858	63
Future Volume (Veh/h)	27	32	63	1162	858	63
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	29	35	68	1263	933	68
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1734	500	1001			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1734	500	1001			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
IF (s)	3.5	3.3	2.2			
pD queue free %	59	93	90			
cM capacity (veh/h)	71	516	687			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	64	68	632	632	622	379
Volume Left	29	68	0	0	0	0
Volume Right	35	0	0	0	0	68
cSH	134	687	1700	1700	1700	1700
Volume to Capacity	0.48	0.10	0.37	0.37	0.37	0.22
Queue Length 95th (ft)	55	8	0	0	0	0
Control Delay (s)	54.1	10.8	0.0	0.0	0.0	0.0
Lane LOS	F	B				
Approach Delay (s)	54.1	0.6			0.0	
Approach LOS	F					
Intersection Summary						
Average Delay			1.8			
Intersection Capacity Utilization			42.7%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

1: ROUTE 83 & SITE DRIVE 2

COMBINED TRAFFIC VOLUMES
SATURDAY

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	36	32	0	1028	1064	0
Future Volume (Veh/h)	36	32	0	1028	1064	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	39	35	0	1117	1157	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1716	578	1157			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1716	578	1157			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
pD queue free %	52	92	100			
cM capacity (veh/h)	81	459	600			
Direction Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2
Volume Total	39	35	558	558	578	578
Volume Left	39	0	0	0	0	0
Volume Right	0	35	0	0	0	0
cSH	81	459	1700	1700	1700	1700
Volume to Capacity	0.48	0.08	0.33	0.33	0.34	0.34
Queue Length 95th (ft)	50	6	0	0	0	0
Control Delay (s)	85.2	13.5	0.0	0.0	0.0	0.0
Lane LOS	F	B				
Approach Delay (s)	51.3		0.0		0.0	
Approach LOS	F					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utilization			39.4%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2: ROUTE 83 & SITE DRIVE 1

COMBINED TRAFFIC VOLUMES
SATURDAY

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (veh/h)	28	31	60	1002	1036	60
Future Volume (Veh/h)	26	31	60	1002	1036	60
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	28	34	65	1089	1126	65
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1833	596	1191			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1833	596	1191			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	53	92	89			
cM capacity (veh/h)	60	447	582			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	62	65	544	544	751	440
Volume Left	28	65	0	0	0	0
Volume Right	34	0	0	0	0	65
cSH	114	582	1700	1700	1700	1700
Volume to Capacity	0.54	0.11	0.32	0.32	0.44	0.26
Queue Length 95th (ft)	64	9	0	0	0	0
Control Delay (s)	68.8	12.0	0.0	0.0	0.0	0.0
Lane LOS	F	B				
Approach Delay (s)	68.8	0.7			0.0	
Approach LOS	F					
Intersection Summary						
Average Delay			2.1			
Intersection Capacity Utilization			47.2%		ICU Level of Service	A
Analysis Period (min)			15			

May 9, 2022

Mr. Alan Borghesi
Borghesi Building & Engineering
2155 East Main Street
Torrington, CT 06790

**Re: Proposed Valvoline Oil and Lube Center
371-373 Talcottville Road (Route 83)
Vernon, CT
Our File: 22055**

Dear Mr. Borghesi:

Pursuant to your request and authorization our office has prepared this letter to outline the trip generation potential of a proposed 3,844 s.f. Valvoline Instant Oil Change facility as part of the existing car wash facility at that location. The site location is presented in Figure 1.

The current site consists of a car wash with two automated tunnels and eight self service bays. Access to the site is provided by two unsignalized driveways to Route 83. The southerly driveway is a full service driveway and the northerly driveway is a one way exit only driveway. Both driveway approaches operate under stop sign control. The existing car wash facility will remain in its current condition. The Oil Change facility access is proposed off of the southerly car wash entrance drive.

The Connecticut DOT maintains a traffic volume count program on all state highways and some local roadways. Included within the DOT database is a count on Route 83, south of Dart Hill Road. These counts were conducted in 2020 and in 2017. The 2020 count was conducted during the pandemic, and the observed traffic volumes were lower than the 2017 counts, therefore, to be conservative, we used the 2017 count data for

analysis, as shown in Table 1. Since traffic volumes have declined due to the pandemic, a growth rate has not been applied.

In order to determine the trip generation potential of the proposed development, the Institute of Transportation Engineers (ITE) *Trip Generation* Report was consulted. *Trip Generation* presents trip generation estimates for many land uses based on counts conducted at existing facilities throughout the country. Included within the ITE database are the following land uses; Land Use Code (LUC): 947 – Self Serve Car Wash; LUC 948 – Automated Car Wash; and LUC 941: Quick Lubrication Vehicle Shop. The *Trip Generation* Report presents data based on building size, tunnels, and service bays.

According to the ITE report the existing car wash facility has a trip generation potential of 103 trips during the morning peak hour, a total of 219 trips, during the afternoon peak hour and a total of 204 trips during the Saturday peak hour. The proposed development with the quick lube center has a trip generation potential of 142 trips during the morning peak hour, 255 trips during the afternoon peak hour and 243 trips during the Saturday peak hour. Therefore, the proposed site will result in an increase of 39 trips during the morning and Saturday peak hour, and 36 trips during the afternoon peak hour. The site generated traffic is summarized in Table 2.

The site generated traffic was distributed to the local roadway network with a 50/50 directional distribution along Route 83. This distribution was used for the existing car wash as well as the proposed oil change facility. Capacity analyses were conducted for the background and the combined traffic volume conditions. The analysis was completed using a computer program known as SYNCHRO. The level of service results are presented in Table 3.

North Site Drive - This is an existing unsignalized driveway with Route 83 oriented in the north/south direction. The site driveway approaches from the west. Route 83

provides two southbound lanes and three northbound lanes, with one lane reserved for left turns into the residential driveway located immediately north of the site. The site driveway provides a two lane approach and operates under stop sign control. For purposes of this analysis we have analyzed the intersection with two lanes on each of the Route 83 approaches. The results indicate that the Route 83 approaches operate at a LOS A during peak hours under the background and combined traffic volume conditions. The site driveway approach operates with a LOS F for left turns and a LOS C for right turns during the morning peak hour and a LOS B during the afternoon and Saturday peak hours. These levels of service are the same under the background and combined traffic volume conditions.

As indicated above, there is a traffic signal located within 100 feet (north) of this intersection. The signal will result in queues in the northbound Route 83 lanes that will restrict exiting left turns. The queues are frequent, but of short duration, and should not significantly impact operations at the driveway.

North Site Drive - This is an existing unsignalized driveway with Route 83 oriented in the north/south direction. The site driveway approaches from the west. Route 83 provides two southbound lanes and three northbound lanes, with one lane reserved for left turns into the residential driveway located immediately north of the site. The left turn lane is utilized by vehicles entering the site driveway. The site driveway provides a single approach and operates under stop sign control. The results indicate that the Route 83 northbound approach operates at a LOS B for left turns and a LOS A for through vehicles. The southbound Route 83 approach operates at a LOS A during peak hours. These LOS are experienced under the background and combined traffic volume conditions. The site driveway approach operates with a LOS E during peak hours under the background traffic volumes. With the introduction of the site generated traffic the approach will operate at a LOS F during peak hours.

Mr. Alan Borghesi
May 9, 2022
Page 4

The LOS F calculated for the site driveways is not unusual for unsignalized driveways on Route 83 within the Town of Vernon. Although the LOS is F, average delays are not excessive and the volume to capacity ratios are a maximum of 56%.

Observations at the existing site driveways indicate that available intersection sight distances are in excess of 700 feet in each direction. The available sight distances meet the current ConnDOT requirement for an approach speed of 50 miles per hour. Route 83 in this area is posted at 40 miles per hour.

Based on the current traffic volumes on Route 83, the existing and proposed trip generation potential of the development, and the calculated levels of service as outlined above, it is my professional opinion that the traffic associated with the proposed Oil Change facility will not have a significant impact on the local roadway network. The site driveways to the site are existing to remain. They are properly designed to accommodate the anticipated driveway volumes and they are properly located with respect to available intersection sight distances.

We appreciate the opportunity to provide this analysis to you. We will be available to offer testimony in support of your application before local planning agencies upon your request. If you require additional information regarding this application, please do not hesitate to contact our office.

Very truly yours,

F. A. Hesketh & Associates, Inc.

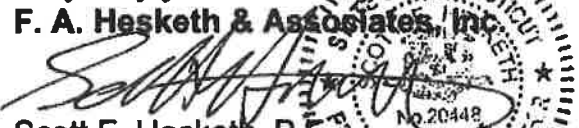

Scott F. Hesketh, P.E.
Manager of Transportation Engineering

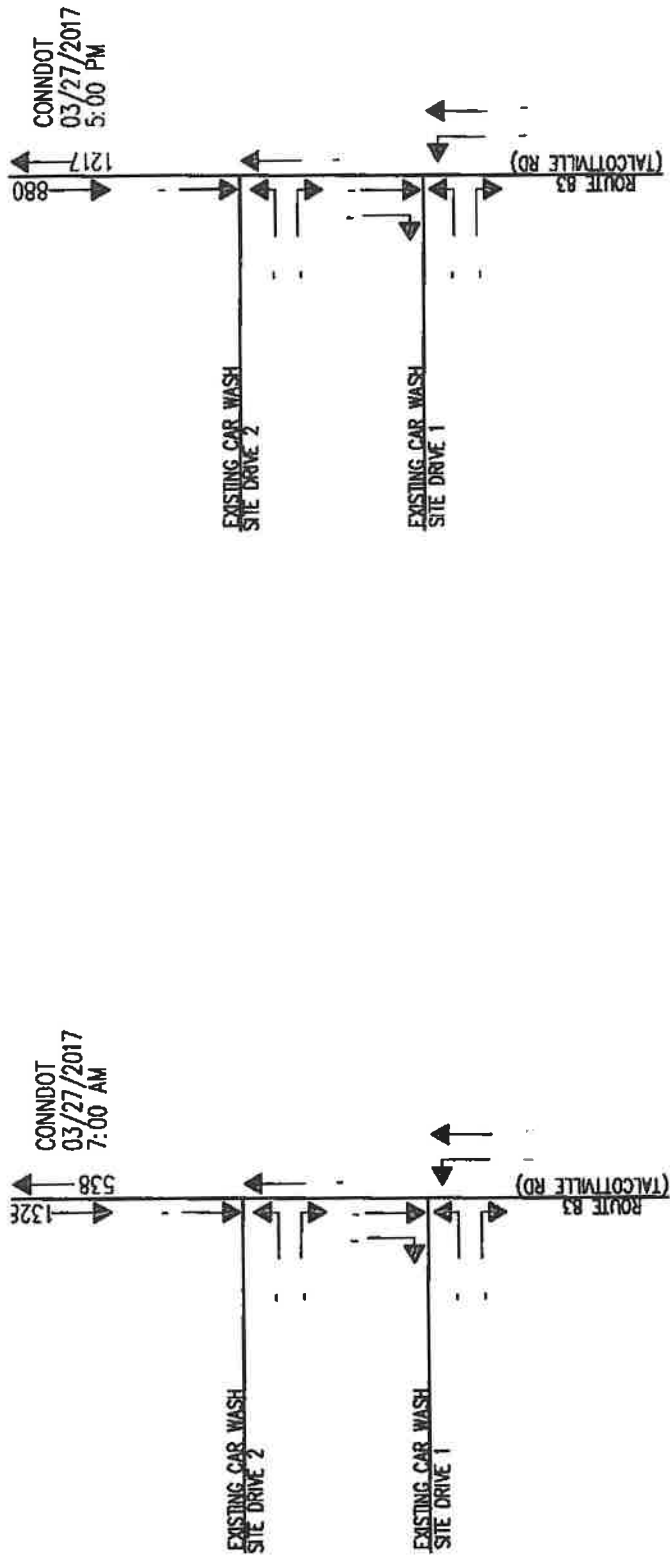




TABLE 1
ConnDOT TRAFFIC VOLUMES
Route 83 south of Dart Hill Road
STATION NO. 45

	27-Mar-17 Monday			28-Mar-17 Tuesday		
	<u>NB</u>	<u>SB</u>	<u>Total</u>	<u>NB</u>	<u>SB</u>	<u>Total</u>
12:00				91	43	134
1:00				40	35	75
2:00				20	28	48
3:00				27	45	72
4:00				72	102	174
5:00	81	310	391			
6:00	317	815	1132			
7:00	538	1328	1866			
8:00	454	1166	1620			
9:00	569	840	1409			
10:00	601	699	1300			
11:00	648	737	1385			
12:00	681	800	1481			
1:00	701	805	1506			
2:00	769	895	1664			
3:00	904	901	1805			
4:00	1103	935	2038			
5:00	1217	880	2097			
6:00	930	744	1674			
7:00	581	528	1109			
8:00	447	344	791			
9:00	335	189	524			
10:00	206	133	339			
11:00	109	93	202			
	11191	13142	24333	250	253	503

2017 ADT = 24,300 for station 45 in Vernon



P.M. PEAK HOUR

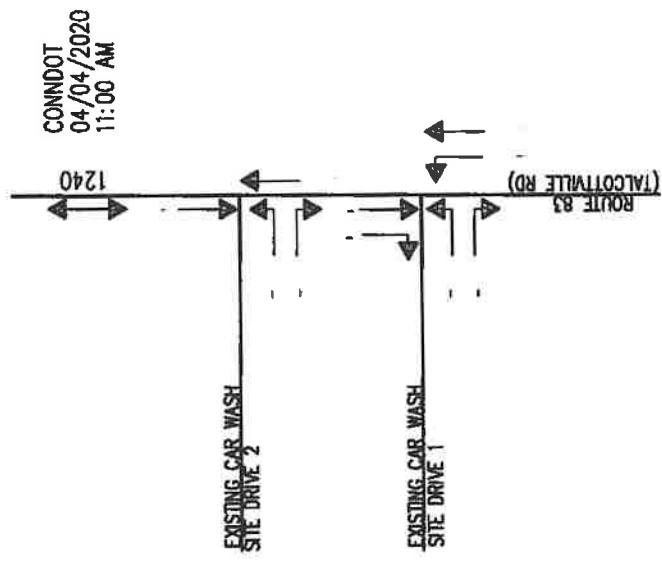


FIGURE 2 SATURDAY PEAK HOUR

OBSERVED TRAFFIC VOLUMES
A.M., P.M. & SATURDAY
PEAK HOURS

PROPOSED VALVOLINE

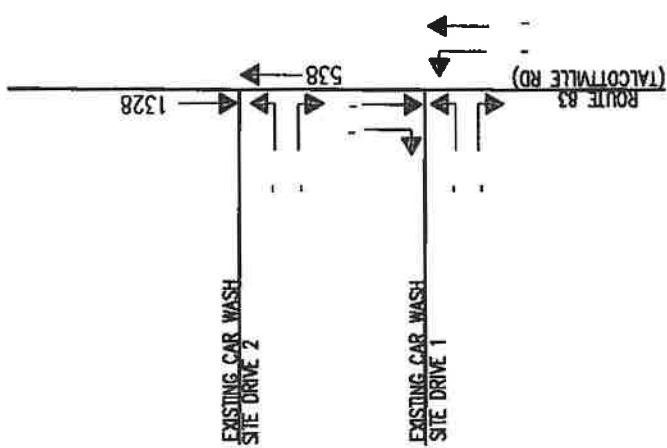
371 & 373 TALCOTTVILLE ROAD
VERNON, CONNECTICUT

F.A.H.
F. A. Heaketh & Associates, Inc.
3 CREAMERY BROOK, EAST GRAFTON, CT 06033

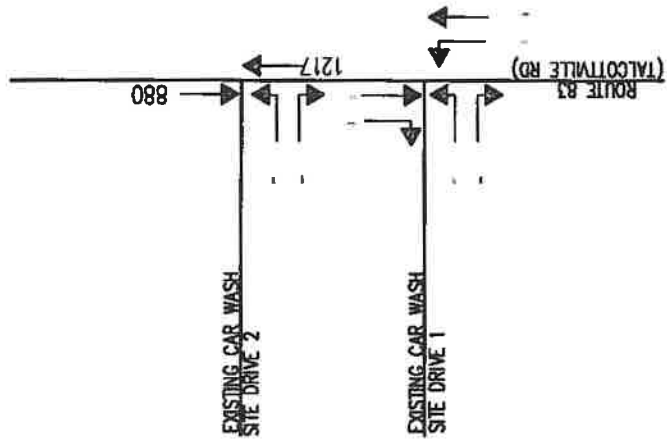
TRAFFIC
PLANNING
ENGINEERING
DESIGN

04-06-2022

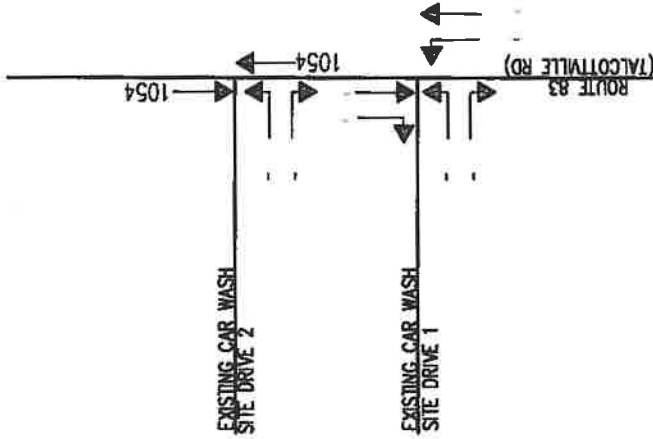
NOT TO SCALE



A.M. PEAK HOUR



P.M. PEAK HOUR



SATURDAY PEAK HOUR

CONDOT CONDUCTED A SATURDAY COUNT IN APRIL 2020. IT WAS CONDUCTED DURING COVID, THEREFORE THE DIFFERENCE IN THE 2017 ADT AND THE 2020 ADT WAS FOUND (APPROX. 70%) AND THE 2020 SATURDAY VOLUMES WERE INCREASED BY THAT PERCENTAGE.

FIGURE 3

EXISTING TRAFFIC VOLUMES
A.M., P.M. & SATURDAY
PEAK HOURS

PROPOSED VALVOLINE
371 & 373 TALCOTTVILLE ROAD
VERNON, CONNECTICUT

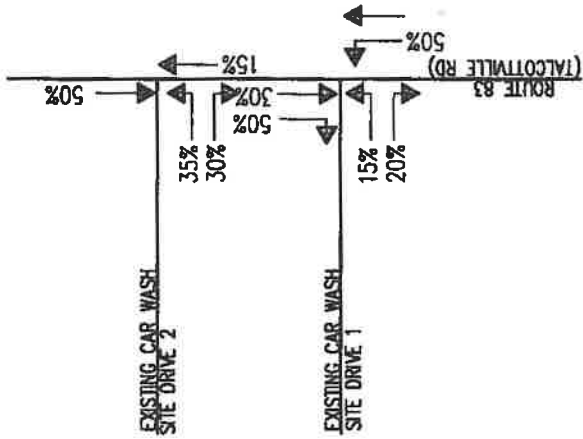
F. A. Heaketh & Associates, Inc.
3 CRENSHAW BROOK, EAST GRAFTON, CT 06033

FAH

TRAFFIC
PLANNING
ENGINEERING
DESIGN

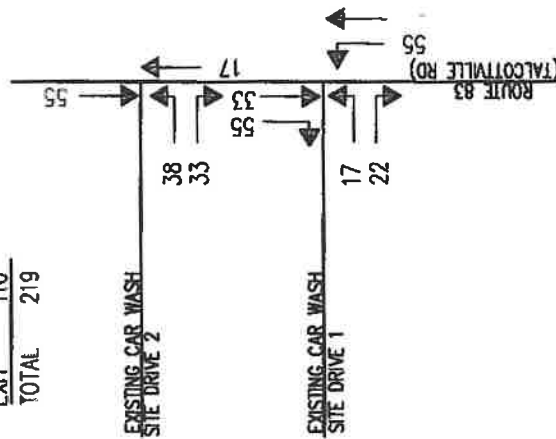
04-06-2022

NOT TO SCALE

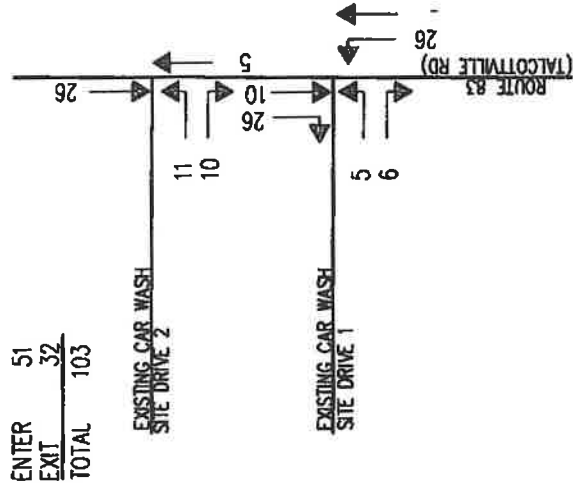


DIRECTIONAL DISTRIBUTION
ALL PEAK HOURS

ENTER	109
EXIT	110
TOTAL	219



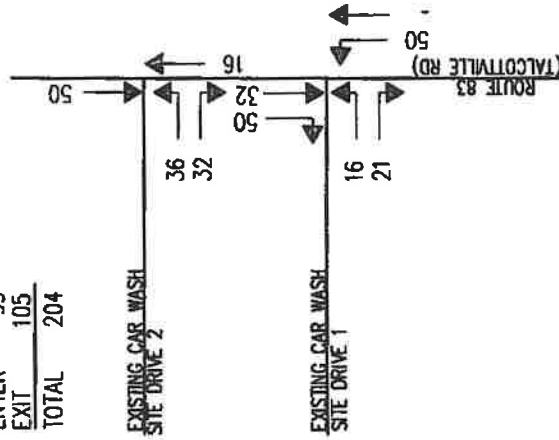
P.M. PEAK HOUR



A.M. PEAK HOUR

ENTER	51
EXIT	32
TOTAL	103

ENTER	99
EXIT	105
TOTAL	204



SATURDAY PEAK HOUR

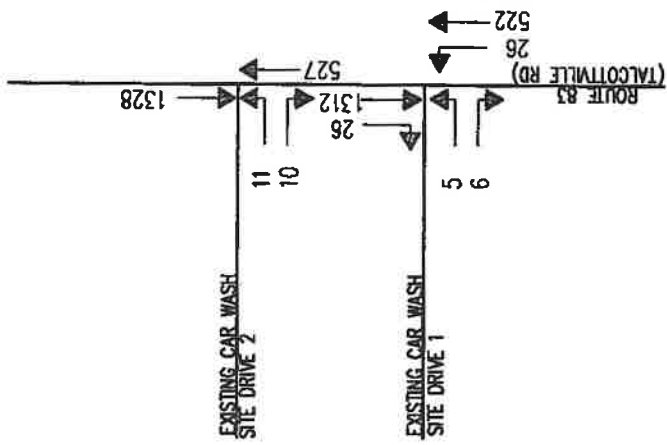
FIGURE 4

DIRECTIONAL DISTRIBUTION OF
EXISTING SITE TRAFFIC &
SITE GENERATED TRAFFIC VOLUMES
PROPOSED VALVOLINE
371 & 373 TALCOTTVILLE ROAD
VERNON, CONNECTICUT

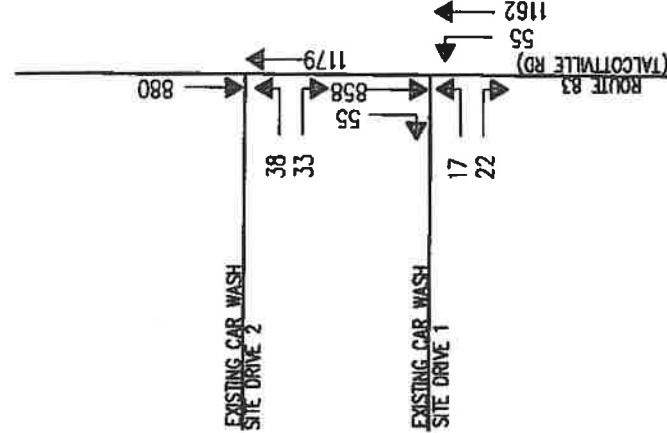
F. A. Hesketh & Associates, Inc.
3 CREAMERY BROOK, EAST GRANBY, CT 06026

FAH
TRAFFIC
PLANNING
ENGINEERING
DESIGN

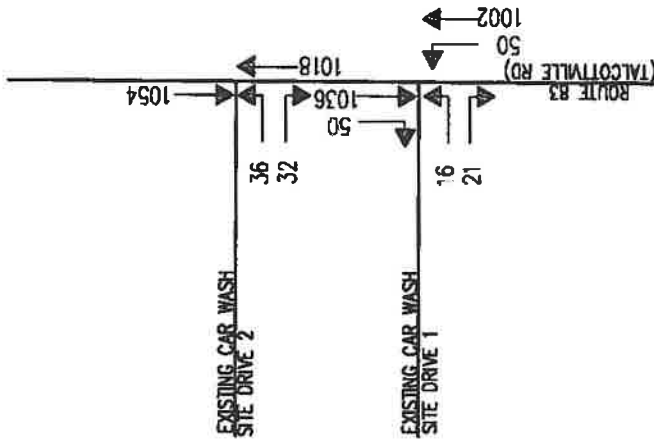
NOT TO SCALE



A.M. PEAK HOUR



P.M. PEAK HOUR



SATURDAY PEAK HOUR

THE BACKGROUND TRAFFIC VOLUMES ARE PROVIDED BY COMBINING THE EXISTING TRAFFIC VOLUMES FROM FIGURE 3 WITH THE EXISTING SITE GENERATED TRAFFIC VOLUMES FROM FIGURE 4. A REVIEW OF THE MOST RECENT CONDOT COUNTS SHOWS THAT TRAFFIC VOLUMES HAVE DECREASED FROM 2008 TO 2017. IN ORDER TO BE CONSERVATIVE, WE DID NOT APPLY A GROWTH RATE TO THE EXISTING TRAFFIC VOLUMES.

FIGURE 5

BACKGROUND TRAFFIC VOLUMES
A.M., P.M. & SATURDAY
PEAK HOURS

PROPOSED VALVOLUME

371 & 373 TALCOTTVILLE ROAD
VERNON,
CONNECTICUT

04-06-2022

F. A. Heaketh & Associates, Inc.
3 CREAMERY BROOK, EAST GRANBY, CT 06028

FAH

TRAFFIC
PLANNING
ENGINEERING
DESIGN

NOT TO SCALE

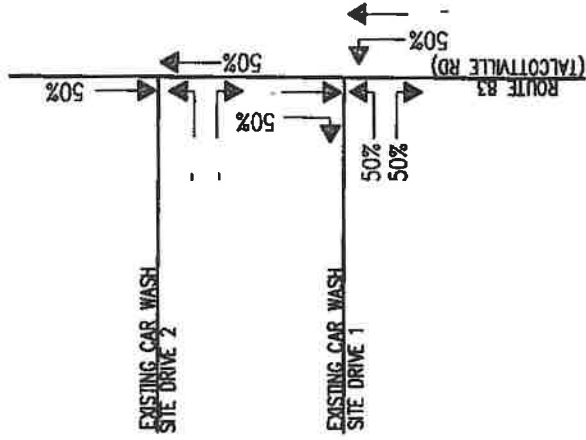
Table 2
Trip Generation Summary
Proposed Car Wash & Quick Lubrication Vehicle Shop
371-373 Talcottville Road Vernon, CT

Land Use	Size	Weekday ADT	A.M. Peak Hour			P.M. Peak Hour			Saturday ADT	SAT. Peak Hour	
			Enter	Exit	Total	Enter	Exit	Total		Enter	Total
Existing	Automated Car Wash*	-	19	20	39	77	78	155	-	38	82
	Self-Service Car Wash	864	32	12	64	32	32	64	1062	61	122
	Combined	-	51	32	103	109	110	219	-	99	204
Proposed	Quick Lubrication Vehicle Shop	267	19	20	39	16	20	36	-	19	39
	3 Servicing Positions	120	6	6	12	8	6	14	126	6	14
	*Total	-	70	52	142	125	130	255	-	118	243

*Morning Trips for the Automated Car Wash not available therefor 25% of the Afternoon trips were used for the Morning.

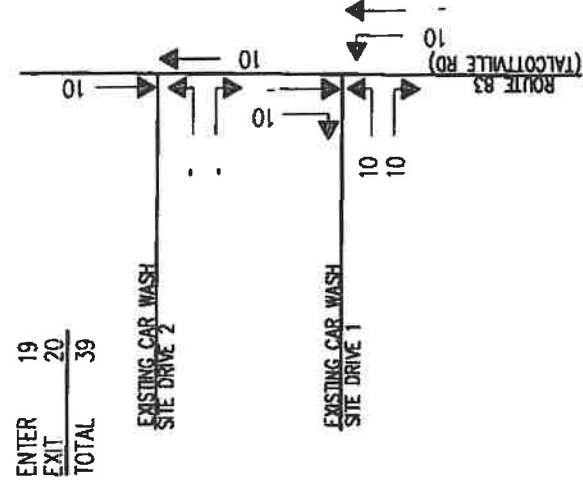
**Saturday trips for the Quick Lubrication Vehicle Shop not available therefor the morning trips were used.

*Existing car wash plus oil change center.



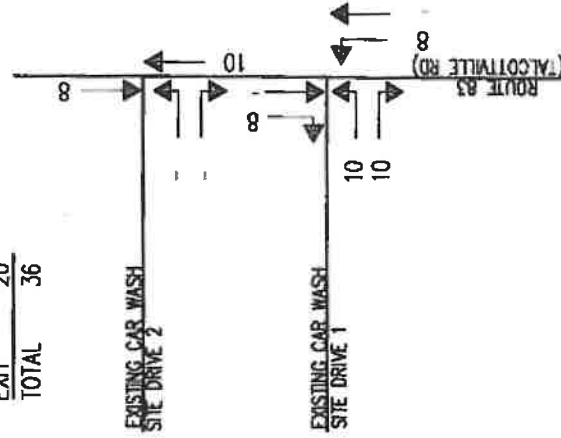
DIRECTIONAL DISTRIBUTION
ALL PEAK HOURS

ENTER	16
EXIT	20
TOTAL	36



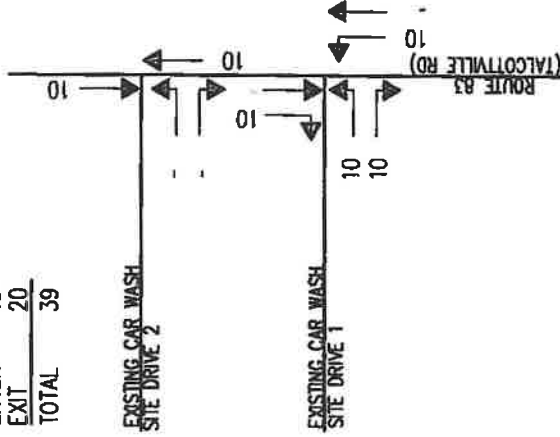
A.M. PEAK HOUR

ENTER	19
EXIT	20
TOTAL	39



P.M. PEAK HOUR

ENTER	19
EXIT	20
TOTAL	39



SATURDAY PEAK HOUR

FIGURE 6

**DIRECTIONAL DISTRIBUTION OF
PROPOSED SITE TRAFFIC &
SITE GENERATED TRAFFIC VOLUMES
PROPOSED VALVOLINE
371 & 373 TALCOTTVILLE ROAD
VERNON, CONNECTICUT**

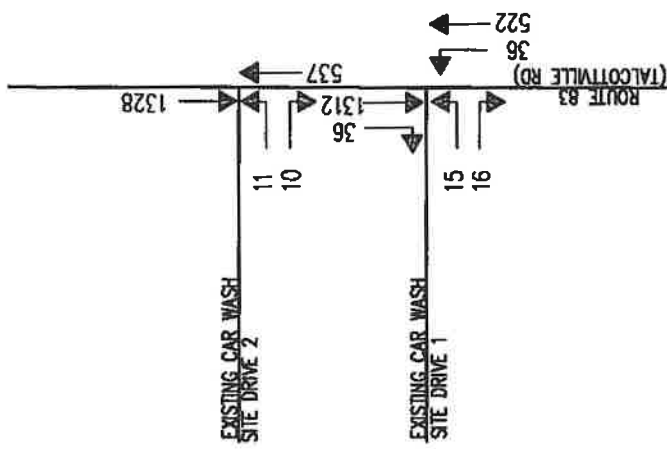
F. A. Hesketh & Associates, Inc.
3 CREAMY BROOK EAST GRANET, CT 06038

FAH

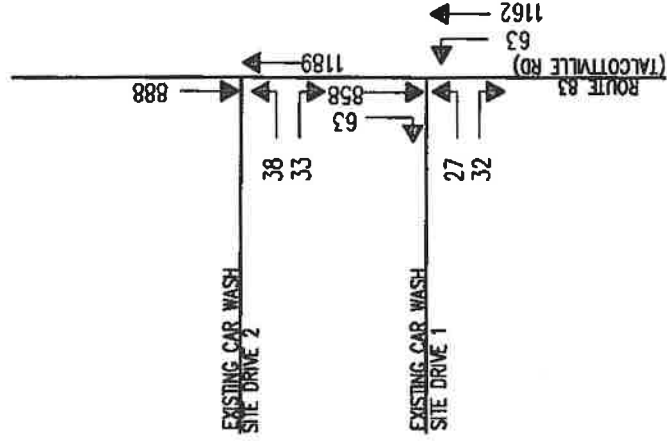
TRAFFIC
PLANNING
ENGINEERING
DESIGN

04-08-2022

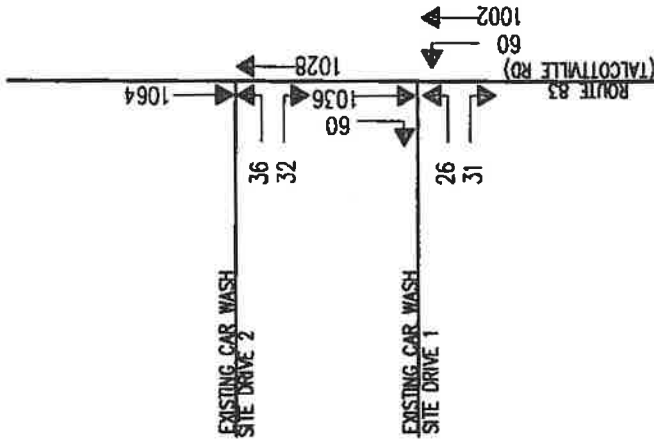
NOT TO SCALE



A.M. PEAK HOUR



P.M. PEAK HOUR



SATURDAY PEAK HOUR

THE COMBINED TRAFFIC VOLUMES ARE PROVIDED BY COMBINING THE BACKGROUND TRAFFIC VOLUMES FROM FIGURE 5 WITH THE SITE GENERATED TRAFFIC VOLUMES FROM FIGURE 6.

FIGURE 7

COMBINED TRAFFIC VOLUMES
A.M., P.M. & SATURDAY
PEAK HOURS
PROPOSED VALVOLUME
371 & 373 TALCOTTVILLE ROAD
VERNON, CONNECTICUT

F.A.H.
F. A. Hecketh & Associates, Inc.
3 CREAM HILL, EAST GRANBY, CT 06026
TRAFFIC
PLANNING
ENGINEERING
DESIGN

04-08-2022

NOT TO SCALE

Table 3
Level of Service Summary
Valvoline
371 & 373 Talcoltville Road Vernon, CT

Time Period	A.M. Peak Hour						P.M. Peak Hour						Saturday Peak Hour											
	Background Traffic			Combined Traffic			Background Traffic			Combined Traffic			Background Traffic			Combined Traffic								
	LOS	delay	v/c	Queue	LOS	delay	v/c	Queue	LOS	delay	v/c	Queue	LOS	delay	v/c	Queue	LOS	delay	v/c	Queue				
Route 83 & Site Drive 1																								
EB	E	35.2	0.09	7	F	50.2	0.30	28	E	39.6	0.29	28	F	54.1	0.48	55	E	46.2	0.32	31	F	68.8	0.54	64
NB	B	13.3	0.06	5	B	13.6	0.09	7	B	10.7	0.09	7	B	10.8	0.10	8	B	11.7	0.09	8	B	12.0	0.11	9
	A	0.0	0.17	0	A	0.0	0.17	0	A	0.0	0.37	0	A	0.0	0.37	0	A	0.0	0.32	0	A	0.0	0.32	0
SB	A	0.0	0.56	0	A	0.0	0.56	0	A	0.0	0.37	0	A	0.0	0.37	0	A	0.0	0.44	0	A	0.0	0.44	0
Route 83 & Site Drive 2																								
EB	F	58.3	0.15	13	F	58.9	0.15	13	F	66.8	0.42	44	F	68.8	0.43	45	F	82.0	0.47	49	F	85.2	0.48	50
	C	15.0	0.03	2	C	15.0	0.03	2	B	12.2	0.07	5	B	12.3	0.07	5	B	13.4	0.08	6	B	13.5	0.08	6
NB	A	0.0	0.17	0	A	0.0	0.17	0	A	0.0	0.38	0	A	0.0	0.38	0	A	0.0	0.33	0	A	0.0	0.33	0
SB	A	0.0	0.42	0	A	0.0	0.42	0	A	0.0	0.28	0	A	0.0	0.28	0	A	0.0	0.34	0	A	0.0	0.34	0

APPENDIX

ConnDOT Count Data

Status: OK

North

Combined

South

VERN-045 - North

Route 83 - 13.48 mi South of Dart Hill Road

Town.....	Vernon	27-Mar	28-Mar
Station.....	45	Mon	Tue
Location.....	41.85013, -72.483926	12:00am	91
2015-Principal Arterial - Other 3...2015-Urban		01:00am	40
Start Report.....	27-Mar-2017 05:00AM	02:00am	20
End Report.....	28-Mar-2017 05:00AM	03:00am	27
Axle Correction Factor.....	None	04:00am	72
		05:00am	81
24-Hour Count.....	11441 * G4(0.98) = 11212.2	06:00am	317
UnRounded AADT.....	11212.2 / 1 = 11212.2	07:00am	538
OK 2020 Fri 03-Apr	14300	08:00am	454
OK 2017 Mon 27-Mar -this report-.....	24300	09:00am	569
OK 2014 Wed 23-Jul	24400	10:00am	601
OK 2008 Wed 21-May	29400	11:00am	648
		12:00pm	681
		01:00pm	701
		02:00pm	769
		03:00pm	904
		04:00pm	1103
		05:00pm	1217
		06:00pm	930
		07:00pm	581
		08:00pm	447
		09:00pm	335
		10:00pm	206
		11:00pm	109
		Totals	11191 250

Status: OK

North

Combined

South

VERN-045 - South

Route 83 - 13.48 mi South of Dart Hill Road

Town.....	Vernon	27-Mar	28-Mar
Station.....	45	Mon	Tue
Location.....	41.85013,-72.483926	12:00am	43
2015-Principal Arterial - Other 3...2015-Urban		01:00am	35
Start Report.....	27-Mar-2017 05:00AM	02:00am	28
End Report.....	28-Mar-2017 05:00AM	03:00am	45
Axle Correction Factor.....	None	04:00am	102
		05:00am	310
24-Hour Count.....	13395 * G4(0.98) = 13127.1	06:00am	815
UnRounded AADT.....	13127.1 / 1 = 13127.1	07:00am	1328
OK 2020 Fri 03-Apr	14300	08:00am	1166
OK 2017 Mon 27-Mar -this report-.....	24300	09:00am	840
OK 2014 Wed 23-Jul	24400	10:00am	699
OK 2008 Wed 21-May	29400	11:00am	737
		12:00pm	800
		01:00pm	805
		02:00pm	895
		03:00pm	901
		04:00pm	935
		05:00pm	880
		06:00pm	744
		07:00pm	528
		08:00pm	344
		09:00pm	189
		10:00pm	133
		11:00pm	93
		Totals	13142 253

Status: OK

North

Combined

South

VERN-045 - Combined - n/s

Route 83 - 13.48 mi South of Dart Hill Road

Town.....	Vernon	27-Mar Mon	28-Mar Tue
Station.....	45		
Location.....	41.85013,-72.483926	12:00am	134
2015-Principal Arterial - Other 3...2015-Urban		01:00am	75
Start Report.....	27-Mar-2017 05:00AM	02:00am	48
End Report.....	28-Mar-2017 05:00AM	03:00am	72
Axle Correction Factor.....	None	04:00am	174
		05:00am	391
24-Hour Count.....	24836 * G4(0.98) = 24339.3	06:00am	1132
UnRounded AADT.....	24339.3 / 1 = 24339.3	07:00am	1866
OK 2020 Fri 03-Apr	14300	08:00am	1620
OK 2017 Mon 27-Mar -this report-.....	24300	09:00am	1409
OK 2014 Wed 23-Jul	24400	10:00am	1300
OK 2008 Wed 21-May	29400	11:00am	1385
		12:00pm	1481
		01:00pm	1506
		02:00pm	1664
		03:00pm	1805
		04:00pm	2038
		05:00pm	2097
		06:00pm	1674
		07:00pm	1109
		08:00pm	791
		09:00pm	524
		10:00pm	339
		11:00pm	202
		Totals	24333 503

Status: OK

Class

Speed

VERN-045 - Combined - n/s

Route 83 - 13.48 mi South of Dart Hill Road

Collected during COVID-19 epoch

	03-Apr Fri	04-Apr Sat	05-Apr Sun	06-Apr Mon	07-Apr Tue
Town.....Vernon	12:00am	100	93	59	87
Station.....45	01:00am	44	67	38	34
Location..... 41.85013, -72.483926	02:00am	34	29	46	32
Posted Speed Limit.....40 MPH	03:00am	55	31	32	40
2015-Principal Arterial - Other 3...2015-Urban	04:00am	62	39	106	114
Start Report.....03-Apr-2020 12:00PM	05:00am	117	86	262	272
End Report.....07-Apr-2020 01:00PM	06:00am	293	187	598	640
	07:00am	425	295	759	765
24-Hour Count.....14854 * G4 (0.90) = 13368.6	08:00am	596	452	776	824
Day 1.....+13994 * G4 (1.03) = 27782.4	09:00am	824	626	841	850
Day 2.....+11553 * G4 (1.26) = 42339.2	10:00am	982	788	925	930
Day 3.....+15164 * G4 (0.98) = 57199.9	11:00am	x	1240	894	1083
UnRounded AADT.....57199.9 / 4 = 14300.0	12:00pm	1192	1217	1118	1186
OK 2020 Fri 03-Apr -this report-.....14300	01:00pm	1150	1218	1086	1163
OK 2017 Mon 27-Mar24300	02:00pm	1281	1250	1080	1202
OK 2014 Wed 23-Jul24400	03:00pm	1350	1205	1103	1355
OK 2008 Wed 21-May29400	04:00pm	1291	1012	896	1239
	05:00pm	1192	942	841	1084
	06:00pm	937	776	589	843
	07:00pm	625	596	429	573
	08:00pm	428	410	317	407
	09:00pm	248	241	213	245
	10:00pm	231	213	185	202
	11:00pm	157	142	109	140
Totals	10082	13994	11553	15164	6748

ITE Trip Generation Data

Graph Look Up

ITripGen Web-based App

4/20/22, 2:24 PM

4/20/22, 2:24 PM

4/20/22, 2:24 PM

4/20/22, 2:24 PM

4/20/22, 2:24 PM

4/20/22, 2:24 PM

4/20/22, 2:24 PM

4/20/22, 2:24 PM

4/20/22, 2:24 PM

