

APPLICATION 1

4.1 PZ-2022-05, 501 Talcottville Rd.



RUSO
SURVEYORS-ENGINEERS

1 Shoham Rd. East Windsor, CT 06088

CT: (860) 623-0569

MA: (413) 785-1158

LETTER OF TRANSMITTAL

DATE: 1-24-22	JOB NO. 2021-083
ATTN: George McGregor	
RE:	
The Learning Experience (TLE)	
501 Talcottville Road	
Vernon, CT	

TO Town of Vernon Planning & Zoning Commission
55 West Main Street, 2nd Floor
Vernon, CT 06066

WE ARE SENDING YOU ☒ Attached ☐ Under separate cover Via delivery, the following items:

☒ Cover Letter ☒ Paper Prints ☐ Mylars ☐ Specifications ☐ Report ☒ Other

COPIES	DATE	SHEET NO.	DESCRIPTION
5	1-24-22		Cover Letter
5	1-11-22	-/12	Site Plans
5	1-24-22		Architectural Floor Plans & Elevations
5	12-23-21		Application for Site Plan of Development & Special Permits
5			Abutters List & Mailing Labels
5			Property Deed w/ Description
5			LID Checklist
2	1-05-22		Drainage Report
1	1-24-22		Check No. 13963
5	6-06-21		Traffic Study

THESE ARE TRANSMITTED (as checked below):

☒ For approval ☐ For your use ☐ For review and comment ☐ As requested
☐ For signature ☐ For your records ☐ Returned after loan to us ☐ For bids due _____

REMARKS:

cc: Eric Spungin

SENT BY: Timothy Coon



January 24, 2022

George McGregor
Town Planner
55 West Main Street, 2nd Floor
Vernon, CT 06066

Re: The Learning Experience (TLE)
501 Talcottville Road, Vernon

Dear George,

On behalf of the Vernon Development LLC, I am pleased to submit the attached application for approval of a Site Plan of Development and Special Permits associated with the development of The Learning Experience (TLE) Academy of Early Education at 501 Talcottville Road in Vernon, Connecticut. The subject parcel currently consists of 4.6 acres of undeveloped woodland located at the intersection of Dart Hill Road and Talcottville Road (Rte. 83). In conjunction with the proposed TLE development, the owner is proposing to divide the existing parcel into two lots, including a 2.0 acre lot adjacent to Dart Hill Road to be developed with the TLE facility, and a remaining 2.6 acre lot fronting on Talcottville Road for potential future development. The subdivision application to split the parcel will be submitted to be considered concurrently with the site plan/special permit applications for the TLE development.

The proposed TLE development will involve the construction of a 10,000 square foot building, 5,000 square foot playground and associated parking. Access to the site will be provided via a driveway on Dart Hill Road directly across from the western Walgreens drive. A traffic study prepared by Fuss & O'Neill is included with the application materials. The traffic study concludes that the proposed development will not have a significant impact to the traffic operations in the vicinity of the site.

Runoff from the building and parking lot will be directed to a subsurface infiltration system and surface infiltration basin designed to provide treatment, groundwater recharge and detention prior to discharge at the edge of the existing on-site wetland. A retaining wall is proposed along the southern edge of the development in order prevent any direct wetland disturbance.

If there are any questions, or you require further information, please call me at (860) 623-0569.

Very truly yours,

Timothy A. Coon, P.E.
J.R. Russo & Associates, LLC

Attachments

cc: Vernon Traffic Authority
Eric Spungin

TOWN OF VERNON PLANNING & ZONING COMMISSION (PZC)

APPLICATION

This form is to be used to apply to the Vernon Planning & Zoning Commission (PZC) for a change of zoning district, amendment of the Zoning Regulations, Site Plan of Development (POD), Special Permit(s), amendment of the Subdivision Regulations, and/or approval of a (re) subdivision, or DMV location approval. Provide all the information requested.

The applicant must be the property owner, the property owner's agent, the Town of Vernon, or someone with a direct financial interest in the subject property; said interest shall be explained and written permission for this application must be obtained from the property owner and submitted with this application if the applicant is not the property owner (ZR Section 2.3).

The list of approvals and the references to sections of the Regulations are for informational purposes only to assist with preparation of the PZC application and are not a definitive statement of the sole requirements that may apply to a specific project.

The applicant understands that the application is complete only when all information and documents required by the PZC have been submitted and, further, that any approval by the PZC relies upon complete and accurate information being provided by the applicant. Incorrect information provided by the applicant may make the approval invalid. The PZC may require additional information to be provided by the applicant in the course of reviewing the application and during the monitoring of the project.

Provide all the information requested:

I. APPLICANT:

Name: Eric Spungin
Title: Member
Company: Vernon Development LLC
Address: 56 East Main St., Avon, CT 06001

Telephone: 860-677-5607 Fax: _____
E-mail: espungin@hotmail.com

II. PROPERTY OWNER (S):

Name: James Basile
Title: _____
Company: 501 Talcottville Road LLC
Address: 43 Ridgcrest Lane
Bristol, CT 06010-2910
Telephone: 860-202-9540 Fax _____
E-mail: _____

III. PROPERTY

Address: 501 Talcottville Road

Assessor's ID Code: Map # 09 Block # 007 Lot/Parcel # 0001D

Land Record Reference to Deed Description: Volume: 2026 Page 51

Does this site contain a watercourse and/or wetlands? (See the Inland Wetlands Map and IWR Section 2.14, 2.15, 2.23, 2.24, 3.11; 4)

☐ No

☒ Yes

☐ No work will be done in regulated area

☒ Work will be done in the regulated area

☒ IWC application has been submitted

☐ IWC application has not been submitted

Zoning District Commercial

Is this property located within five hundred (500) feet of a municipal boundary?

☒ No

☐ Yes:

☐ Bolton

☐ Coventry

☐ Ellington

☐ Manchester

☐ South Windsor

☐ Tolland

Check if Historic Status Applies:

☐ Located in historic district:

☐ Rockville

☐ Talcottville

☐ Individual historic property

IV. PROJECT

Project Name: The Learning Experience

Project Contact Person:

Name: Timothy Coon

Title: Project Engineer

Company: J.R. Russo + Associates LLC

Address: P.O. Box 938
East Windsor, CT 06088

Telephone: 860-623-0569 Fax: 860-623-2485

E-mail: tcoon@jrrusso.com

V. PZC APPLICATION PROJECT SUMMARY

Describe the project briefly in regard to the purpose of the project and the activities that will occur. Attach to this application a complete and detailed description with maps and documentation as required by the "Town of Vernon Zoning Regulations" and "Town of Vernon Subdivision Regulations".

Purpose: Construction of a 10,000 SF Daycare Facility

General Activities: Filling, excavation + building
and parking lot construction

VI. APPROVAL (S) REQUESTED

Subdivision or Resubdivision

- Subdivision (Sub. Sec. 4, 5, 6)
- Resubdivision (Sub. Sec. 4, 5, 6)
- Minor modification of subdivision or resubdivision (Sub. Sec. 4.6)
- Town acceptance of a road (Sub. Sec. 6.5-6, 8 & 9)
- Amendment of Subdivision Regulations (Sub. Sec. II)

See Subdivision Regulations Sec. 4 for application fee schedules.

 X Soil Erosion and Sediment Control Plan (ESCP) (ZR Sec. 2.117; 18) (Sub. 6.14)

 X Site Plan of Development (POD) (ZR Sec. 14)

- POD approval (ZR Sec. 14.1.1.1; 14.1.2)
- Modification of an approved POD (ZR Sec. 14.1.1.1)
- Minor modification of a site POD (ZR Sec. 14.1.1.2)

 X Special Permit(s) (ZR Section 17.3)

- Special Permit in an aquifer area (ZR Sec. 2.4; 2.5; 2.119; 20)
- Special Permit for excavation (ZR Sec. 2.52; 2.79; 15)
- Special Permit for use in a district (ZR Sec. 1.2 & 4)
- Special Permit for lot coverage (ZR Sec. 1.2; 2.61; 2.68; 4)
- Special Permit for signs (ZR Sec. 1.2; 2.106-115; 4; 16; 21.7)
- X Special Permit for parking (ZR Sec. 4; 12; 21.4) 4.9.4.15.1
- Special Permit for elderly housing (ZR Sec. 2.60; 17.4)
- Special Permit for Bed & Breakfast (B & B) (ZR Sec. 2.9; 17.3.4)
- Special Permit for serving alcohol (ZR Sec. 2.103, 17.1)
- Special Permit for massage (ZR Sec. 2.76-78; 4)
- Special Permit for telecommunications (ZR Sec. 2.21; 3.23 & 23)
- Special Permit for dumps and/or incinerators (ZR Section 8)

☒ Other Special Permit(s). Cite ZR Section and describe activity:

Special Permit for structure within 200' of resid. structure (4.9.4.15.2)

Special Permit for parking within 100' of resid. structure (4.9.4.15.3)

☐ Special Permit modifications (ZR Sec. 17.3.2.2). Cite ZR Section and describe activity.

☐ **Zoning:**

☐ Site specific change of zoning district and map (ZR Sec. 1.2; 1.3; 4)

☐ Amendment of Zoning Regulations (Sec. 1.2; 1.3; 4)

☐ Site specific change to the Aquifer Protection Overlay Zone Map (ZR Sec. 20.3.2)

See Zoning Regulations Section 22 for application fee schedules.

☐ Dealer or Repairer License (location approval for DMV)

Per Connecticut General Statutes (CGS) Section 8-26: If an application submitted to the Planning & Zoning Commission (PZC) involves any activity or area regulated under the wetlands statutes, an application for this activity must be filed with the Inland Wetlands Commission (IWC) on or before the day the Planning & Zoning Commission (PZC) application is filed by the applicant. (TWR Sec. 3.11)

Per CGS Sec. 8-31: If the proposed activity is to take place within a watershed of a Water company, the applicant is required to file a copy of the application with the Water Company via certified mail within seven (7) days of the date of the application. (TWR Sec. 4.3.6).

The applicant, undersigned, has reviewed the "Town of Vernon Planning and Zoning Regulations and Inland Wetlands and Watercourses Regulations" and has prepared this application with complete and accurate information:

Property Owner, Applicant, or Applicant's Agent:

Eric Spungin Member
Erick Spungin
Vernon Development LLC
Signature
James D. Basore Member
Signature
501 Talcottville Road, LLC

12/23/21
Date
12/23/21
Date

TO BE FILLED IN BY THE PLANNING DEPARTMENT

Date Application Submitted _____

Date Application Received by Commission _____

PZC File: _____

Abutters List/Mailing Labels (200')

KAO LLC
192 Talcott Ridge Rd
South Windsor, CT 06074

Gregory Gozzo
C/O Gozzo Estate Homes
190 Spyglass Lane
Jupiter, FL 33477

Independence Realty Group LLC
c/o Webster Bank: Corp RE 203
145 Bank Street
Waterbury, CT 06702

Kenneth Busenbark
32 Worcester Road
Vernon, CT 06066

Richard & Gloria J. Martocchio
36 Worcester Road
Vernon, CT 06066

Nelson J. & Theresa M. Chiasson
42 Worcester Road
Vernon, CT 06066

Brian F. Oulette
& Stephanie M. Brow
46 Worcester Road
Vernon, CT 06066

John Coro
52 Worcester Road
Vernon, CT 06066

James D. & Bonnie R. King
58 Worcester Road
Vernon, CT 06066

Thomas Shirshac
64 Worcester Road
Vernon, CT 06066

Eugene P. & Judith S. Veillette
786 Dart Hill Road
Vernon, CT 06066-2302

Town of Vernon
14 Park Place
Vernon, CT 06066

Vernon Properties LLC
605 Middel Street, No. 15
Braintree, MA 02184-5817

Realty Income Corp
PM Dept 0704
11995 El Camino Real
San Diego, CA 92130

O'Reilly Automotive Store Inc.
P.O. Box 9167
Springfield, MO 65801

New 500 East LLC
44 Caisson Road
Colchester, CT 06415

Allen C. & Leatrice Trombley
28 Worcester Road
Vernon, CT 06066

Ian B. Carlson
24 Worcester Road
Vernon, CT 06066

Eileen M. Reivik
47 Worcester Road
Vernon, CT 06066

Dorine L. Decarli
51 Worcester Road
Vernon, CT 06066

Monica Cofrancesco
776 Dart Hill Road
Vernon, CT 06066

IF RETURN TO:
STIEBEL
86 FARMINGTON AVE.
HARTFORD, CT. 06105

VOL 2026 PG 55
INST# 51

QUITCLAIM DEED

VICTOR J. BASILE ("Releasor"), of 43 Ridgecrest Lane, Bristol, Connecticut, for no consideration paid, grants to 501 TALCOTTVILLE ROAD, LLC of 43 Ridgecrest Lane, Bristol, Connecticut ("Releasee") all that certain piece or parcel of land together with all improvements thereon and appurtenances thereto in the Town of Vernon, Connecticut, as more particularly described on Exhibit A attached hereto and made a part hereof ("Premises"). Releasee herein assumes and agrees to pay all real property taxes attributable to the Premises on the list of October 1, 2006 and thereafter.

SAID PREMISES IS CONVEYED SUBJECT TO, WITHOUT LIMITATION:

1. Taxes of the Town of Vernon on the List of October 1, 2006 now due and payable and thereafter, which Releasee herein assumes and agrees to pay.
2. Any and all provisions of any ordinance, municipal regulation, or public or private law.
3. Declarations, restrictions, covenants, matters and easements of record and any state of facts an accurate survey or personal inspection of the Premises might reveal.

Signed this 7th day of February, 2007

Witnessed by:


MICHAEL S. STIEBEL, as witness


VICTOR J. BASILE


ROBERT A. FIERCE, as witness

STATE OF CONNECTICUT)

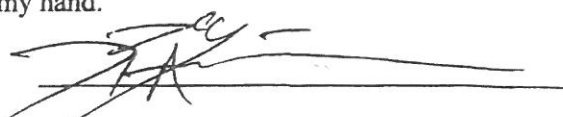
COUNTY OF)

ss:

Hartford
(town)

On this the 7th day of February, 2007, before me, the undersigned officer, personally appeared Victor J. Basile, known to me (or satisfactorily proven) to be the person whose name is subscribed to the within instrument and acknowledged that he executed the same for the purposes therein contained and as his free act and deed.

In witness whereof I hereunto set my hand.


Commissioner of the Superior Court
Notary Public
My Commission Expires:

Grantee's Address: 43 Ridgecrest Lane
Bristol, Connecticut 06010

CONVEYANCE TAX RECEIVED
STATE \$.00
TOWN \$.00
Bernice K. Dixon
TOWN CLERK OF VERNON

Exhibit A

A certain piece or parcel of land situated on the westerly side of Talcottville Road (Route #83) and the southerly side of Dart Hill Road in the Town of Vernon, County of Tolland and State of Connecticut, being shown as "Parcel 3 Parcel contains 200,968 S.F.±, 4.6136 AC±", on a certain map or plan entitled "Sub-division Plan Prepared For John J. Mahr, Sr. Boundary Map Talcottville Rd. & Dart Hill Rd. Vernon, Conn. AR Lombardi Associates, Inc. Consulting Civil and Sanitary Engineers Land Surveyors Vernon Connecticut Comm. No: 84-1091 Date: March 7, 1985 Scale: 1" = 40' Sht. No: 1 of 2", which map or plan is on file or to be filed in the Vernon Town Clerk's Office and to which reference may be had for a more particular description thereof. Said Parcel 3 is more particularly bounded and described as follows:

Commencing at a point in the southerly street line of Dart Hill Road, which point marks the northwesterly corner of the herein-described premises and the northeasterly corner of land now or formerly of Antoine F. Bourcher, as shown on said map; thence proceeding in a southerly direction along said land now or formerly of Antoine F. Bourcher, land now or formerly of Thomas A. & Linda M. Shirshac, and land now or formerly of Eileen M. Strube, partly by each, a distance of 408.91 feet to a point; thence turning an obtuse interior angle of 179° 35' 27" and proceeding along land now or formerly of Alice M. Coro, land now or formerly of Peggy O. Tracy, land now or formerly of Nelson J. and Teresa M. Chiasson, land now or formerly of Paul J. and Linda Jobkowiak and land now or formerly of David M. Glenn Jr. and Sharon L. Glenn, partly by each, as shown on said map, a distance of 470.12 feet to a point; thence turning an obtuse interior angle of 103° 44' 36" and proceeding along Parcel 1, as shown on said map, a distance of 11.73 feet to a point; thence turning an acute interior angle of 76° 53' 42" and proceeding along land now or formerly of Cardinal Industries, Inc., as shown on said map, a distance of 211.66 feet to a point; thence turning an obtuse interior angle of 180° 04' 41" and proceeding along land now or formerly of Gregory Gozzo and David J. Sweeney, as shown on said map, a distance of 268.00 feet to a point; thence turning an obtuse interior angle of 289° 53' 43" and proceeding along said land now or formerly of Gregory Gozzo and David J. Sweeney, a distance of 403.00 feet to a point in the westerly street line of Talcottville Road; thence proceeding along the arc of a curve to the left having a radius of 2,506.67 feet, a distance of 322.92 feet along the westerly street line of Talcottville Road to a Connecticut Highway Department Monument in said westerly street line of Talcottville Road; thence proceeding along said westerly street line of Talcottville Road, a distance of 41.55 feet to a point; thence turning an acute interior angle of 82° 53' 05" and proceeding along Parcel No. 2, as shown on said map, a distance of 200.86 feet to a point; thence turning an interior obtuse angle of 277° 06' 55" and proceeding along Parcel No. 2, as shown on said map, a distance of 159.68 feet to a point; thence turning an interior acute angle of 77° 09' 29" and proceeding along the southerly street line of Dart Hill Road, as shown on said map, a distance of 352.53 feet to the point or place of beginning.

RECORDED IN
VERNON LAND RECORDS
Bernice K. Dixon
VERNON TOWN CLERK
ON Dec 05, 2008 AT 12:30P

LID CHECKLIST

Applicants must complete and submit the following checklist with the application.

Date: 12/22/21

Project: The Learning Experience, 501 Talcothville Road

Conformance with the following criteria shall be initiated in the spaces provided below by a Connecticut Registered Professional Engineer, Land Surveyor, or Certified Soils Scientist as appropriate. If conditions cannot be met comments addressing each item should be provided by the applicant in the space provided below. Comments will be reviewed with Town Staff at the scheduled Development Staff Meeting and documented.

Item	Description	Verified	Comments
1	An Existing Conditions Plan is provided documenting sensitive natural resources including but not limited to existing wetlands (as designated by a Certified Soils Scientist in Connecticut), streams, ponds, vernal pools, flood zones, stream channel encroachment lines, soil types and infiltration rates, wells, tree lines, property boundaries, and other items that may be requested by the Town.	TAC	
2	Utilizing the Existing Conditions Plan as a guide, development has been located to maximize preservation of contiguous natural sensitive areas.	TAC	
3	Proposed site developments for residential or two family dwellings on more than one individual parcel, all commercial, industrial, and retail developments have been guided by the applicable requirements of the Town's Low Impact Development Stormwater Quality Manual and the Connecticut Storm Water Quality Manual.	TAC	
4	Bioretention Basins or Rain Gardens have been incorporated within yards, median strips, cul-de-sacs islands, and parking lot islands.		All runoff diverted to an infiltration basin for treatment + groundwater recharge.

Date: 12/22/21

Project: Five Leaning Experience, 501 Talcoville Rd
Conformance with the following criteria shall be initiated in the spaces provided below by a Connecticut Registered Professional Engineer, Land Surveyor, or Certified Soils Scientist as appropriate. If conditions cannot be met comments addressing each item should be provided below. Comments will be reviewed with Town Staff at the scheduled development staff meeting and documented.

Item	Description	Verified	Comments
5	Dry Wells have been incorporated into the design to control roof and pavement runoff.		Roof runoff diverted directly to infiltration basin.
6	Permeable (Porous) Pavement has been incorporated into areas of low traffic, parking lots, residential and light commercial use driveways, walkways, bike paths, etc.		Infiltration provided at infiltration basin.
7	Natural areas including woodlands, regulated wetland areas, naturally vegetated areas have been preserved/ and or replicated to the maximum extent practical.	TAC	
8	Post Development stormwater runoff is at or less than the predevelopment runoff.	TAC	
9	Stormwater infiltration has been provided by the use of underground storage units, devices, and/or infiltration swales/trenches.	TAC	
10	Level spreaders/vegetation have been provided at storm drainage outfalls to enhance water quality and mitigate erosion.	TAC	

Date: 12/22/21

Project: The Leasing Experience 501 Talcothville Rd

Conformance with the following criteria shall be initiated in the spaces provided below by a Connecticut Registered Professional Engineer, Land Surveyor, or Certified Soils Scientist as appropriate. If conditions cannot be met comments addressing each item should be provided below. Comments will be reviewed with Town Staff at the scheduled development staff meeting and documented.

Item	Description	Verified	Comments
11	On-Site retention/detention facilities have been provided to address water quality and storm water runoff.	TAC	
12	Rain Barrels, cisterns, and/or other rainwater harvesting techniques to reuse rainwater for irrigation and other non-potable uses are incorporated into the design.		Not applicable.
13	An Erosion and Sedimentation Control Plan conforming to the Standards of the Connecticut Guidelines for Soil Erosion and Sediment Control is included in the design.	TAC	
14	A yearly maintenance plan of all components of best management practices associated with storm water management has been provided.	TAC	
15	Impervious area percentages for pre and post development have been provided.	TAC	
16	When conflicts exist between the Town's Low Impact Development Stormwater Quality Manual and the Connecticut Storm Water Quality Manual the State Manual shall govern.	TAC	

From: [George Logan](#)
To: tcoon@jrrusso.com; [McGregor, George](#)
Cc: [Smith, David](#); [Gately, Shaun](#); [Perry, Craig](#); espungin@hotmail.com
Subject: [EXTERNAL] Re: TLE 501 Talcottville Road
Date: Monday, March 28, 2022 11:42:23 AM
Attachments: [image001.png](#)

CAUTION: This Email is from an EXTERNAL source. Ensure you trust this sender before clicking on any links or attachments.

George,

I have left a message with Arthur Allen of Ecotec, to schedule a field visit to review the disputed wetland delineations. Our mutual society, SSSSNE requires, as part of our rules, that a field meeting be had by the two soil scientists to review and make in-field adjustments to the wetland boundaries, or provide opinions in writing, if an agreement cannot be reached. We hope that a resolution can be reached, so that the wetland re-designation procedure is clear and complete.

Best,
George Logan

-----Original Message-----

From: Tim Coon <tcoon@jrrusso.com>
To: McGregor, George <GMcGregor@vernon-ct.gov>
Cc: Smith, David <dsmith@vernon-ct.gov>; Gately, Shaun <sgately@vernon-ct.gov>; Perry, Craig <cperry@vernon-ct.gov>; Eric Spungin <espungin@hotmail.com>; rema8@aol.com <rema8@aol.com>
Sent: Mon, Mar 28, 2022 11:17 am
Subject: TLE 501 Talcottville Road

George,

As you are aware, we will be submitting an application for a Variance today for the proposed TLE development at 501 Talcottville Road. The request is to allow a reduction in the required parking, which will enable us to eliminate the row of parking up along Dart Hill Road and shift the development further away from the wetlands. Because we will not have a decision on the variance until the ZBA meeting on April 20th, I am submitting two alternative plan sets at this time so that both can be reviewed and hopefully approved depending on how the variance plays out. Both plans show the revision to add the limited access driveway to Talcottville Road. Alternative 2 also reflects the approval of the variance to reduce the parking requirement and shift the development further away from the wetland.

I have attached pdfs of both sets to allow you and the Town Staff to begin review. I will also send the associated drainage reports in a separate email due to file size. I will drop off two hard copies of the sets today as well. Please note that the Planting Plans for the two sets are not included because they are still being revised by the Landscape Architect. I will forward them once I receive them. Also, I am awaiting revisions from the lighting supplier to include some pole mounted fixtures along the new driveway. I will forward a revised Utility Plan when I receive the update from the lighting supplier as well. I assume you will forward the pdfs to anyone who needs to look at them, including the third party wetland consultant.

Let me know if you have any questions or comments.

Timothy A. Coon, P.E.

Principal Engineer



J.R. RUSSO & ASSOCIATES, LLC

P.O. Box 938, 1 Shoham Road

East Windsor, CT 06088

CT 860.623.0569 **MA** 413.785.1158

tcoon@jrrusso.com | www.jrrusso.com

From: [Tim Coon](#)
To: [McGregor, George](#); [James Basile](#)
Cc: [Gately, Shaun](#); [Eric Spungin](#); [James Basile](#); [Scrittorale, Ryan](#)
Subject: [EXTERNAL] RE: 501 Talcottville rd
Date: Thursday, March 17, 2022 11:13:13 AM
Attachments: [image001.png](#)

CAUTION: This Email is from an EXTERNAL source. Ensure you trust this sender before clicking on any links or attachments.

George,

The applicant does not have a problem with that. Consider this a formal request to open the Public Hearing on April 21st and grant an extension to the time to May 6, 2022, to allow for it to be continued to the May meeting. Thanks.

Timothy A. Coon, P.E.
Principal Engineer



J.R. RUSSO & ASSOCIATES, LLC

P.O. Box 938, 1 Shoham Road
East Windsor, CT 06088

CT 860.623.0569 **MA** 413.785.1158

tcoon@jrrusso.com | www.jrrusso.com

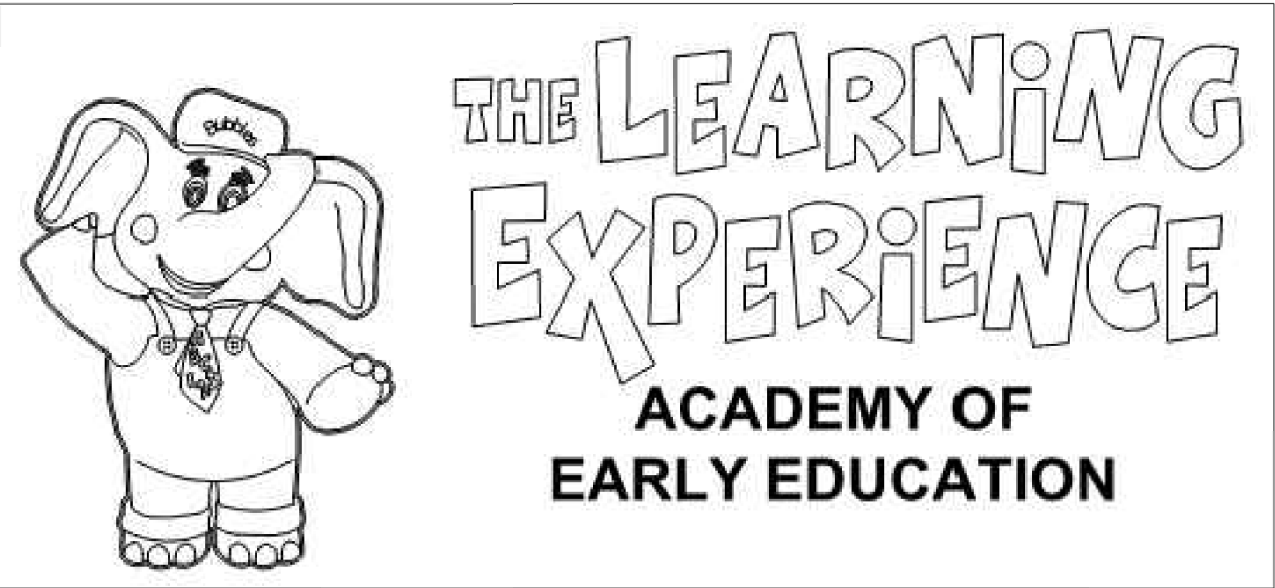
From: McGregor, George <GMcGregor@vernon-ct.gov>
Sent: Thursday, March 17, 2022 10:59 AM
To: Tim Coon <tcoon@jrrusso.com>; James Basile <basile.enterprises@comcast.net>
Cc: Gately, Shaun <sgately@vernon-ct.gov>
Subject: 501 Talcottville rd

Any further thoughts on pushing the opening of the public hearing for both items to April 21 instead of April 7?

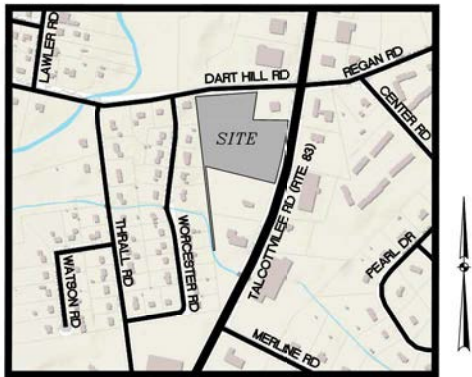
This would at least allow the Traffic Authority to have met and forward a recommendation?

George

George K. McGregor, AICP
Town Planner
Town of Vernon
55 West Main Street
Vernon, CT. 06066-3291
Phone: (860) 870-3640
Mobile: (860) 336-1846



501 Talcottville Road
Vernon, Connecticut



KEY PLAN MAP
1"=500'

Applicant
Vernon Development LLC
56 East Main Street
Avon, CT 06001
(860) 677-5607



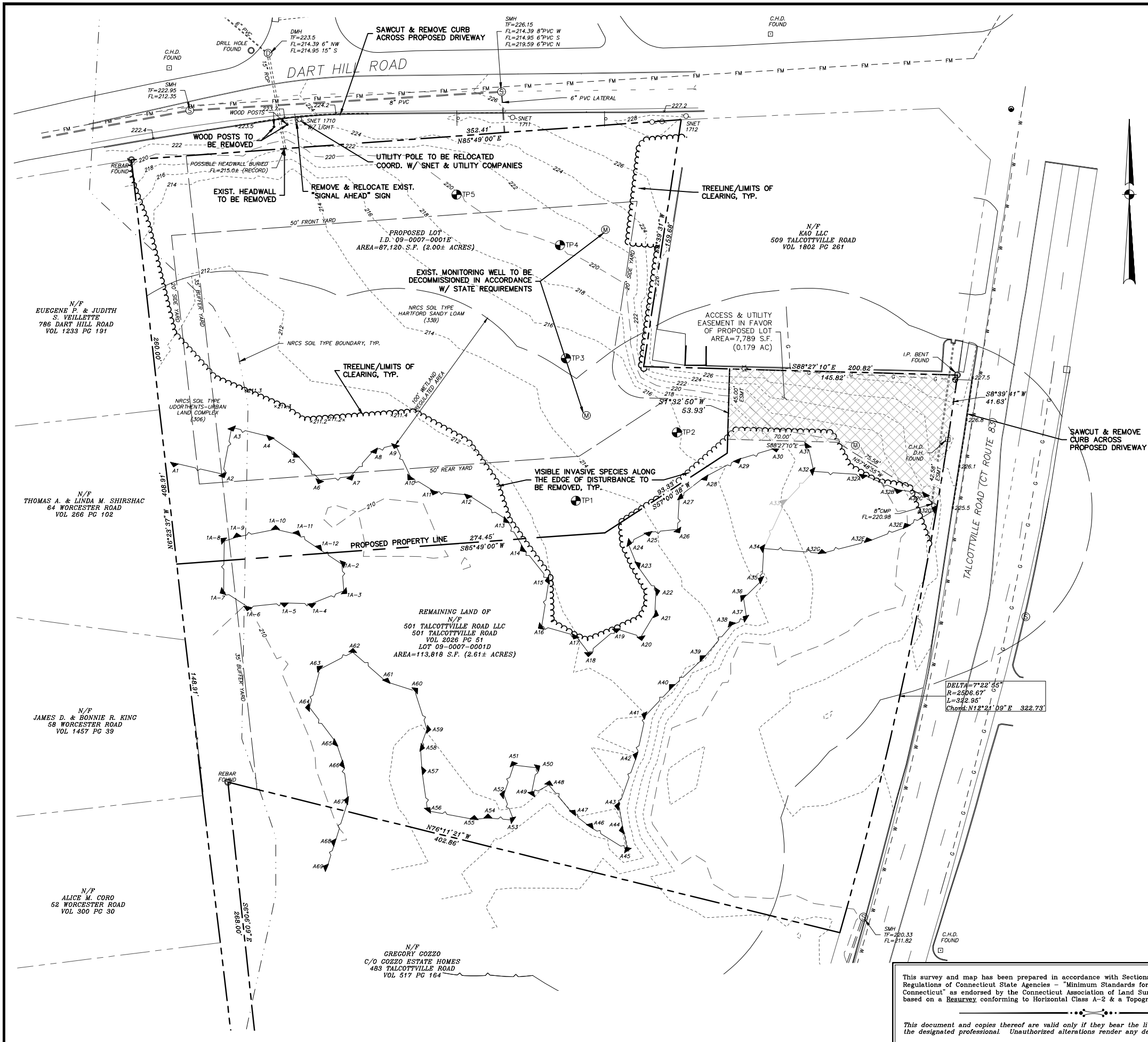
Owner
501 Talcottville Road LLC
43 Ridgecrest Lane
Bristol, CT 06010-2910



ALTERNATE #2

DRAWING INDEX		
SHEET	TITLE	SHEET NO. LATEST REVISION
CIVIL		
COVER SHEET	1 of 12	4-19-2022
EXISTING CONDITIONS & DEMOLITION PLAN	2 of 12	4-19-2022
LAYOUT PLAN	3 of 12	4-19-2022
GRADING & EROSION & SEDIMENT CONTROL PLAN	4 of 12	4-19-2022
STORM SEWER PLAN	5 of 12	4-19-2022
UTILITY PLAN	6 of 12	4-19-2022
PLANTING PLAN	7 of 12	4-06-2022
EROSION & SEDIMENT CONTROL NOTES	8 of 12	3-26-2022
DETAILS	9 of 12	3-26-2022
DETAILS	10 of 12	3-26-2022
DETAILS	11 of 12	3-26-2022
DETAILS	12 of 12	3-26-2022

S:\Acad\2021\Civil\3D\2021-083 Vernon Development - 501 Talcottville Rd\Russio Drawings\2021-083 B - Pushed forward.dwg



Reference Maps:

- "Boundary Survey Prepared for 501 Talcottville Road LLC, 501 Talcottville Road (CT Route 83) & Dart Hill Road, Vernon, Connecticut" Sheet SV.01, Scale: 1"=50' Date: May 2021 by Alfred Benesch & Company.
- "Topographic Survey Prepared for 501 Talcottville Road LLC, 501 Talcottville Road (CT Route 83) & Dart Hill Road, Vernon, Connecticut" Sheet SV.02, Scale: 1"=30' Date: May 2021 by Alfred Benesch & Company.

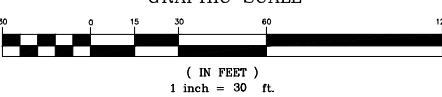
Notes:

- Portion of the parcel is located in inland wetlands as delineated by Rema Ecological Services per reference map #2.
- Proposed lot does not lie within a special flood hazard area per FEMA flood insurance rate map, town of Vernon, Connecticut community panel number 0901310005C, revised August 9, 1999.
- Horizontal and vertical datums are based upon map references prepared by Alfred Benesch & Company.
- All underground utility locations on this plan are approximate and may not be complete. Anyone using this information without verifying the locations does so at their own risk. No construction will be done on this site prior to utility mark out. "Call Before You Dig 1-800-922-4455".
- Boundary and existing conditions shown hereon are based on map references prepared by Alfred Benesch & Company.
- Line striping in Talcottville Road (Rte. 83) approximated based on aerial photography.

LEGEND

- EXISTING UTILITY POLE
- PROPOSED UNDERGROUND ELECTRIC
- EXISTING GAS LINE
- PROPOSED GAS LINE
- EXISTING WATER LINE
- PROPOSED WATER LINE
- PROPOSED FIRE PROTECTION LINE
- EXISTING CATCH BASIN
- PROPOSED CATCH BASIN
- EXISTING DRAINAGE MANHOLE
- PROPOSED DRAINAGE MANHOLE
- EXISTING STORM SEWER
- PROPOSED STORM SEWER
- PROPOSED ROOF LEADER
- EXISTING SANITARY MANHOLE
- EXISTING SANITARY SEWER
- EXISTING SANITARY FORCE MAIN
- PROPOSED SANITARY SEWER
- EXISTING SIGN
- PROPOSED SIGN
- EXISTING IRON PIN (FOUND)
- PROPOSED IRON PIN (TO BE SET)
- EXISTING MONUMENT (FOUND)
- EXISTING SPOT GRADE
- PROPOSED SPOT GRADE
- EXISTING CONTOUR
- PROPOSED CONTOUR
- LIMIT OF WETLANDS
- PROPERTY LINE
- EASEMENT LINE
- BUILDING LINE
- STAKED HAYBALES OR SILT FENCE

GRAPHIC SCALE



This survey and map has been prepared in accordance with Sections 20-300b-1 thru 20-300b-20 of the Regulations of Connecticut State Agencies - "Minimum Standards for Surveys and Maps in the State of Connecticut" as endorsed by the Connecticut Association of Land Surveyors, Inc. It is a **Property Survey** based on a **Resurvey** conforming to Horizontal Class A-2 & a Topographic Survey conforming to Class T-2.

This document and copies thereof are valid only if they bear the live signature and embossed seal of the designated professional. Unauthorized alterations render any declaration hereon null and void.

TO THE BEST OF MY KNOWLEDGE AND BELIEF THIS MAP IS SUBSTANTIALLY CORRECT AS NOTED HEREON.



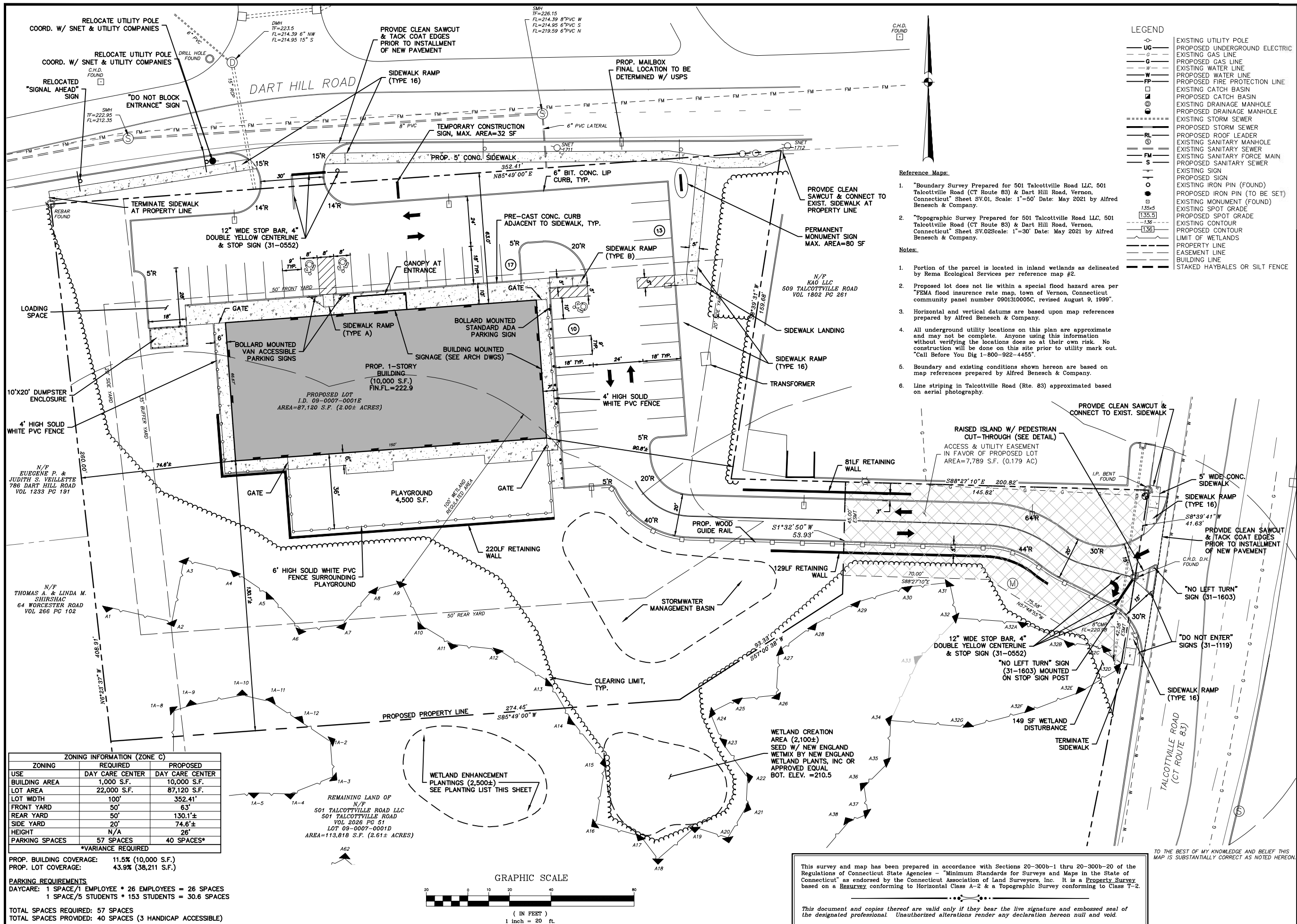
RUSSEO
SURVEYORS-ENGINEERS
SERVING CT & MA
J.R. Russo & Associates, LLC
150 North Rd East Windsor CT 06038 • CT 860.031.0569 • FAX 860.031.0566
www.jrusso.com • jr@jrusso.com

4-19-2022	WETLAND LINE ADJUSTMENT
3-26-2022	TALCOTTVILLE RD ENTRANCE, APPLY VARIANCES
2-14-2022	PAGE NUMBERS

The Learning Experience
Property Of
501 Talcottville Road, LLC
501 Talcottville Road
Vernon, Connecticut 06010
Parcel ID: 09-0007-0001D (Zone: C)

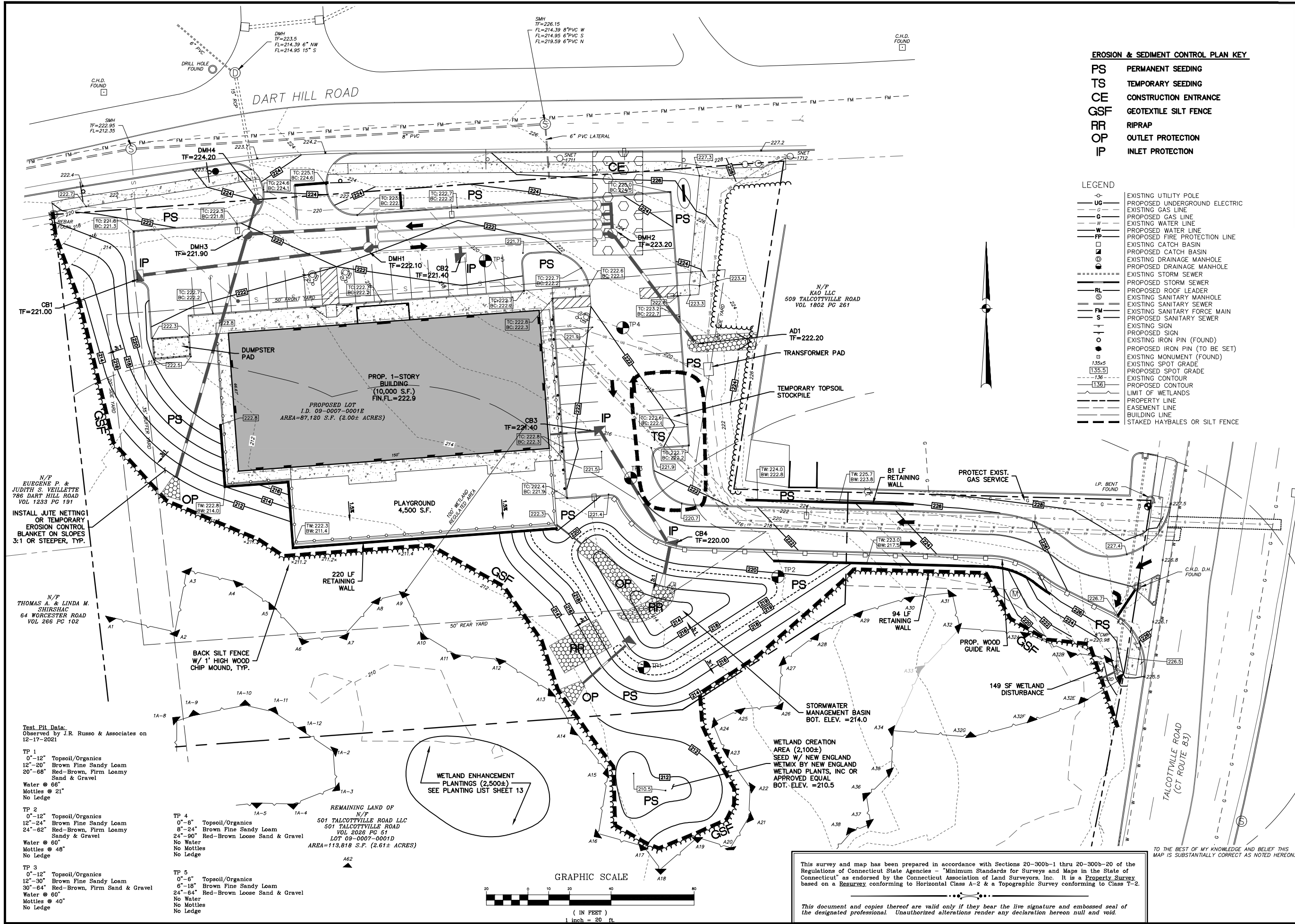
Existing Conditions & Demolition Plan

(ALT. 2)
DATE
1-11-22
SCALE
1"=30'
JOB NUMBER
2021-083
SHEET
2 of 12



<p><i>The Learning Experience</i> <i>Property Of</i> 501 Talcottville Road, LLC 501 Talcottville Road Vernon, Connecticut 06010 Parcel ID: 09-0007-0001D (Zone: C)</p>		<p>Layout Plan (ALT 2)</p>	
<p>DATE 1-11-22</p>		<p>SCALE 1"=20'</p>	
<p>JOB NUMBER 2021-083</p>		<p>SHEET 3 of 12</p>	

S:\Acad\2021\Civil\3D\2021-083\2021-083.dwg Vernon Development - 501 Talcottville Rd\2021-083.dwg - Pushed forward.dwg



- EROSION & SEDIMENT CONTROL PLAN KEY**
- PS PERMANENT SEEDING
 - TS TEMPORARY SEEDING
 - CE CONSTRUCTION ENTRANCE
 - GSF GEOTEXTILE SILT FENCE
 - RR RIPRAP
 - OP OUTLET PROTECTION
 - IP INLET PROTECTION

- LEGEND**
- EXISTING UTILITY POLE
 - PROPOSED UNDERGROUND ELECTRIC
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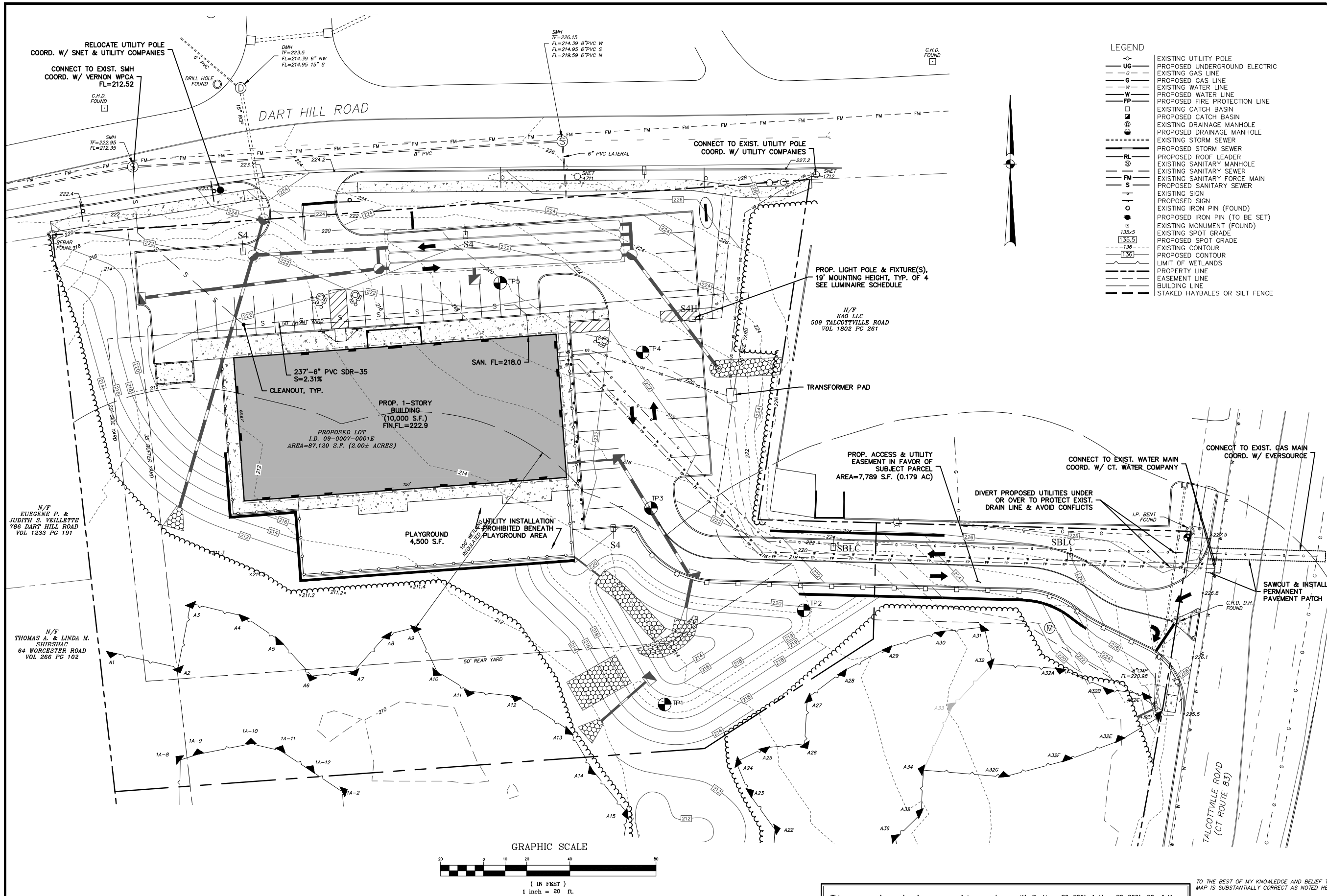
Applicant
Vernon Development LLC
56 East Main Street
Avon, Connecticut 06001

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The Learning Experience
Property Of
501 Talcottville Road, LLC
501 Talcottville Road
Vernon, Connecticut 06010
Parcel ID: 09-0007-0001D (Zone: C)

Grading & Erosion Control Plan (ALT 2)
DATE
1-11-22
SCALE
1"=20'
JOB NUMBER
2021-083
SHEET
4 of 12

S:\Acad\2021 Civil\3D\2021-083 Vernon Development - 501 Talcottville Rd\Russow Drawings\2021-083 B - Pushed forward.dwg



Luminaire Schedule									
Symbol	Qty	Label	Arrangement	Luminaire Lumens	Luminaire Watts	LLF	BUG Rating	Mounting Height	Description
	3	S4	Single	12574	102	0.900	B2-U0-G3	19	Lithonia DSX1 LED P3 40K TFTM MVOLT SPA DBLXD - SSS 18 4C DM19A DBLXD 18FT POLE on 1FT BASE
	1	S4H	Single	11312	125	0.900	B2-U0-G2	19	Lithonia DSX1 LED P4 40K TFTM MVOLT SPA HS DBLXD - SSS 18 4C DM19AS DBLXD 18FT POLE on 1FT BASE
	2	SBLC	Single	7293	70	0.900	B1-U0-G2	19	

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TO THE BEST OF MY KNOWLEDGE AND BELIEF THIS MAP IS SUBSTANTIALLY CORRECT AS NOTED HEREON.

LEGEND	
	EXISTING UTILITY POLE
	PROPOSED UNDERGROUND ELECTRIC
	EXISTING GAS LINE
	PROPOSED GAS LINE
	EXISTING WATER LINE
	PROPOSED WATER LINE
	PROPOSED FIRE PROTECTION LINE
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	PROPOSED DRAINAGE MANHOLE
	EXISTING STORM SEWER
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www.russosurveyors.com • info@russosurveyors.com

Applicant
Vernon Development LLC
56 East Main Street
Avon, Connecticut 06001

REVISIONS	
BY: LF/TAC	CHK: JEU
4-19-2022	WETLAND LINE ADJUSTMENT
3-28-2022	SITE LIGHTING
3-28-2022	TALCOTTVILLE RD ENTRANCE, APPLY VARIANCES
2-14-2022	WETLAND MITIGATION, LEAK-OFF, BASIN OUTLET

The Learning Experience
Property Of
501 Talcottville Road, LLC
501 Talcottville Road
Vernon, Connecticut 06010
Parcel ID: 09-0007-0001D (Zone: C)

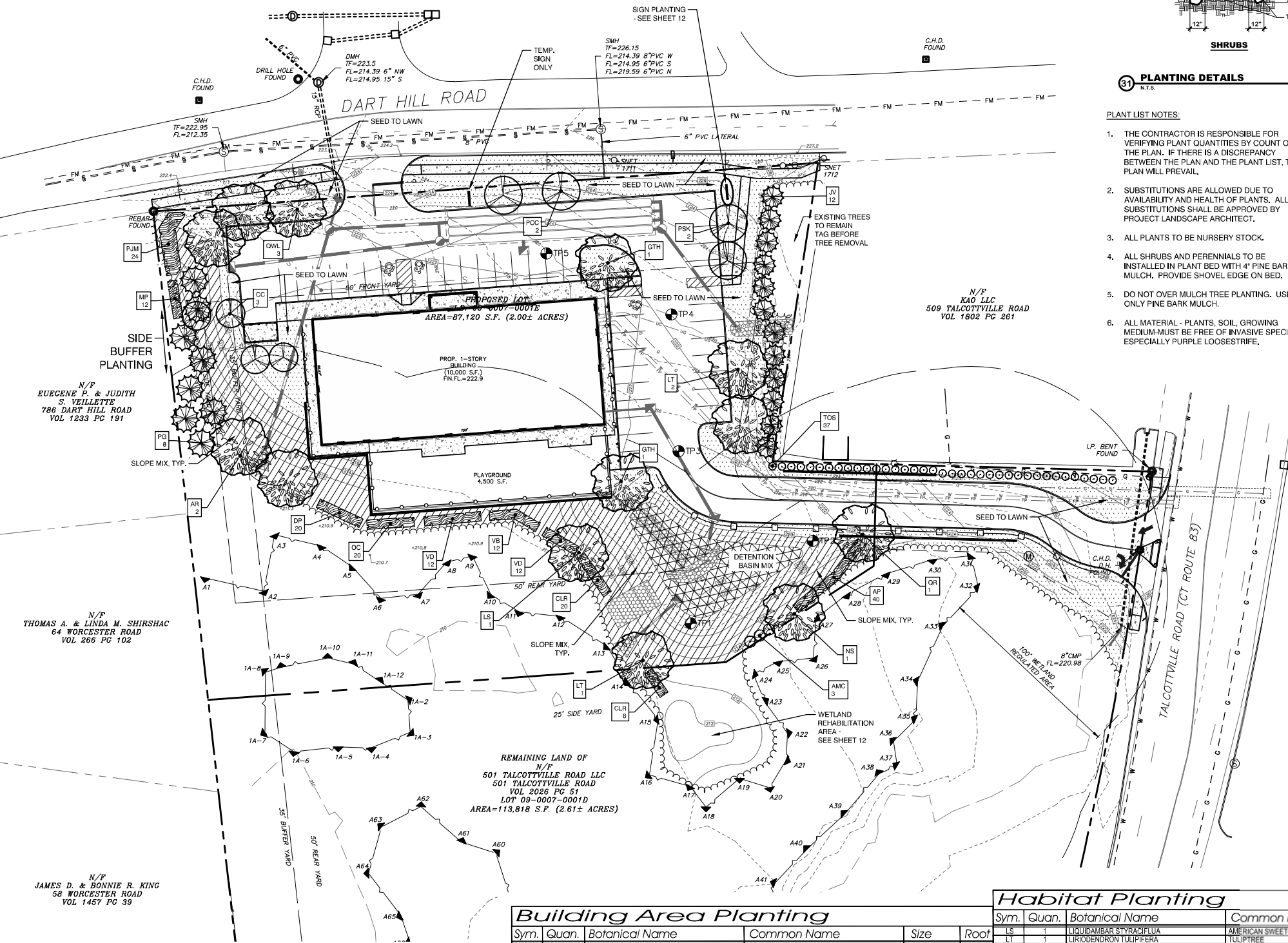
Utility Plan
(ALT 2)

DATE
1-11-22

SCALE
1"=20'

JOB NUMBER
2021-083

SHEET
6 of 12



Side Buffer Planting

Sym.	Quan.	Botanical Name	Common Name	Size	Root
PG	8	PICEA GLAUCA	WHITE SPRUCE	58-61 H.	B&B
MP	12	MYRICA PENSYLVANICA	NORTHERN BAYBERRY	24in-30in HL	CONT.
PJM	24	RHODODENDRON PJM	PJM RHODODENDRON	24in-30in HL	CONT.

Building Area Planting

Sym.	Quan.	Botanical Name	Common Name	Size	Root
AR	2	ACER RUBRUM 'OCTOBER GLORY'	OCTOBER GLORY RED MAPLE	3in-3 1/2in. CAL	B&B
CC	3	CERCIS CANADENSIS	EASTERN REDBUD	2in-2 1/2in. CAL	B&B
GTH	2	GLEDITSIA TRIACANTHOS 'SHADEMASTER'	SHADEMASTER HONEYLOCUST	3in-3 1/2in. CAL	B&B
JN	12	JUNIPERUS VIRGINIANA 'EMERALD SENTINEL'	EMERALD SENTINEL EASTERN RED CEDAR	3in-3 1/2in. CAL	B&B
LT	2	LIQUIDAMBAR STYRACIFLUA	AMERICAN SWEETGUM	3in-3 1/2in. CAL	B&B
PCC	2	PRUNUS SERRULATA 'KAWAZAN'	KAWAZAN CHERRY	3in-3 1/2in. CAL	B&B
PSK	2	PRUNUS SERRULATA 'KAWAZAN'	KAWAZAN CHERRY	3in-3 1/2in. CAL	B&B
TOS	37	TRILIUM OCCIDENTALIS 'NIGRA'	DARK AMERICAN ARBORVITAE	3in-3 1/2in. CAL	B&B
QWL	3	QUERCUS X WAREI 'LONG'	REGAL PRINCE OAK - FASTIGIATE FORM	3in-3 1/2in. CAL	B&B

Habitat Planting

Sym.	Quan.	Botanical Name	Common Name	Size	Root
LS	1	LIQUIDAMBAR STYRACIFLUA	AMERICAN SWEETGUM	3in-3 1/2in. CAL	B&B
LT	1	LIQUIDAMBAR STYRACIFLUA	AMERICAN SWEETGUM	3in-3 1/2in. CAL	B&B
NS	1	NYSSA SYLVATICA	BLACK Tupelo	3in-3 1/2in. CAL	B&B
QR	1	QUERCUS RUBRA	RED OAK	2in-2 1/2in. CAL	B&B
AMC	3	AMELANCHIER CANADENSIS	SHADBLOW SERVICEBERRY - MULTISTEM SHRUB FORM	4in-5in. CONT.	CONT.
CLR	28	C. E. THRA ALNIFOLIA 'RUBY SPICE'	RUBY SPICE SUMMERSWEET	24in-30in HL	CONT.
VB	12	VIBURNUM BURKWOODII	SUPEROOD VIBURNUM	24in-30in HL	CONT.
VD	24	VIBURNUM DENTATUM	ARROWWOOD VIBURNUM	24in-30in HL	CONT.
AP	40	ADANTUM PEDATUM	MAIDENHAIR FERN		CONT.
DP	20	DEINISIA PUNCTILOBULA	HAY SCENTED FERN		CONT.
OC	20	OSMUNDA CINNAMOMEA	CINNAMON FERN		CONT.

PLANTING NOTE

PLACE TOPSOIL MIXTURE IN BOTTOM OF HOLE AND SATURATE. AFTER PLACING PLANT, WORK TOPSOIL MIXTURE CAREFULLY AROUND ROOT BALL AND THEN SATURATE. WHEN HOLE IS ONE HALF FULL OF MIXTURE, SATURATE AGAIN. FILL HOLE COMPLETELY WITH TOPSOIL MIXTURE AND SATURATE AGAIN.

DECIDUOUS TREES WITH LESS THAN 2 1/2" CAL. AND EVERGREEN TREES LESS THAN 4' HT. SHALL BE STAKED WITH TWO EQUALLY SPACED STAKES AROUND THE TREE.

STAKES SHALL BE UNIFORM IN HEIGHT AND FASTENED TO EACH TREE AT ABOUT 5' BY MEANS OF TWO STRANDS OF TWISTED #10 GAUGE IRON WIRE.

REMOVE BURLAP FROM TOP 1/3 OF BALL (SEE PLANTING NOTES)

SELECTIVELY PRUNE ALL TREES AND SHRUBS OF DEAD AND DYING BRANCHES

TOPSOIL AND MANURE

SHRUBS

TREES

SEEDING OPERATIONS

A. FERTILIZER AND LIMESTONE

CONTRACTOR SHALL CONDUCT APPROPRIATE SOIL TESTS TO DETERMINE AMOUNT OF FERTILIZER AND LIMESTONE REQUIRED TO ENSURE AN ACCEPTABLE LAWN. GROUND LIMESTONE: 100LBS/1000S.F. COMMERCIAL FERTILIZER: 20LBS/1000S.F. *PERCENTAGES OF NITROGEN, PHOSPHORIC ACID AND POTASH TO BE DETERMINED BY SOIL TESTS.

B. LAWN SEED

SEED MIXTURE FOR: LATE SUMMER / EARLY FALL / EARLY SPRING PLANTING: SOW AT RATE OF 4 LBS/1000 S.F. CREEPING RED FESCUE 35 PARTS CHEWINGS RED FESCUE 20 PARTS KENTUCKY 31 TALL FESCUE 15 PARTS ROUGH BLUEGRASS 10 PARTS BARON BLUEGRASS 20 PARTS SEED MIXTURE FOR PLANTING MAY 1 TO LATE SUMMER: SOW AT A RATE OF 8 LBS/1000 S.F. CREEPING RED FESCUE 35 PARTS CHEWINGS RED FESCUE 20 PARTS KENTUCKY 31 TALL FESCUE 20 PARTS DOMESTIC RYEGRASS 25 PARTS

C. SLOPE MIX

SOW AT RATE OF 6 LBS/1000 S.F. CREEPING RED FESCUE 42 PARTS FIESTA II PERENNIAL RYEGRASS 34 PARTS RED TOP 8 PARTS ALSKIE CLOVER 8 PARTS BIRDSFOOT TREFOIL 8 PARTS

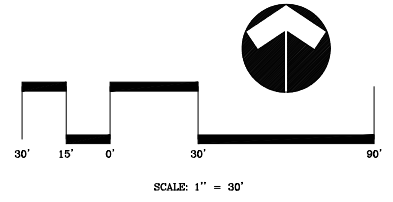
D. DETENTION BASIN MIX

USE "RETENTION BASIN FLOOR SEED" FROM NEW ENGLAND SEED CO. AT A RATE OF 100 LB./ACRE. MIX WITH ANNUAL RYEGRASS AT A RATE OF 1/2LB./1000 S.F.

E. MEADOW MIX

USE NEW ENGLAND SEED CO. "NEW ENGLAND HORSE PASTURE MIXTURE" AT A RATE OF 100 LB./ACRE. MIX WITH VERMONT WILDFLOWER FARM (802) 425-3500 DROUGHT TOLERANT PERENNIAL MIX AT A RATE OF 51 LB./ACRE. MIX WITH 20% ADDITIONAL COMMON TALL FESCUE.

SEEDING OPERATIONS & MIXES



LEGEND

- PROPERTY LINE
- WETLAND LINE
- 100' UPLAND REVIEW AREA
- PROPOSED DECIDUOUS TREE
- PROPOSED FLOWERING TREE
- PROPOSED EVERGREEN TREE
- PROPOSED SHRUB
- PLANT LABEL & COUNT
- EXISTING TREELINE
- DETENTION BASIN MIX
- LAWN SEED MIX
- SLOPE MIX
- MEADOW MIX

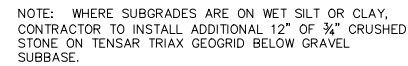


REVISIONS	
BY: DFM	CHK: TPH

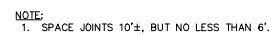
The Learning Experience
Property of
501 Talcottville Road, LLC
501 Talcottville Road
Vernon, Connecticut 06010
Parcel ID: 09-0007-0001D (Zone: C)

Planting Plan

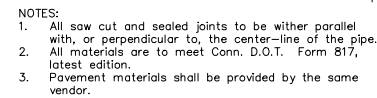
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JOB NUMBER	2021-083
SHEET	8 of 12



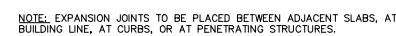
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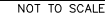
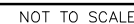
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REVISIONS

Vernon, Connecticut 06010
Parcel ID: 09-0007-0001D (Zone: C)

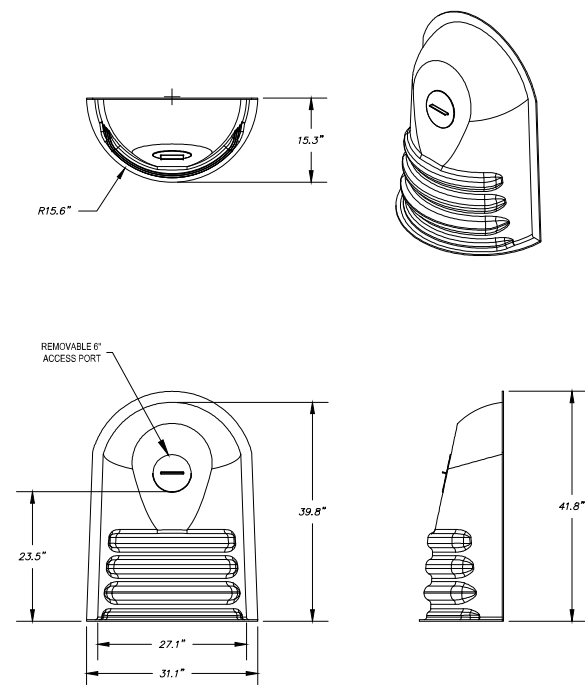
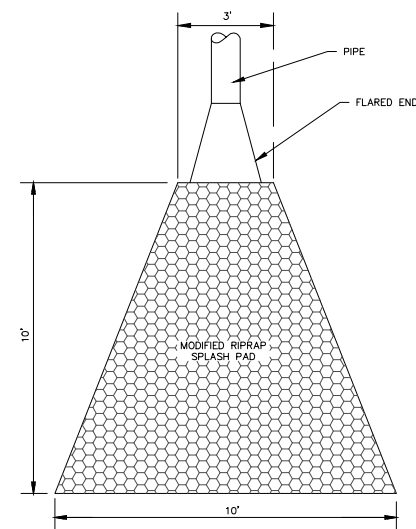
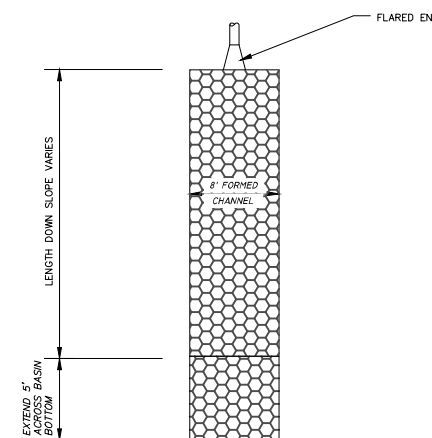
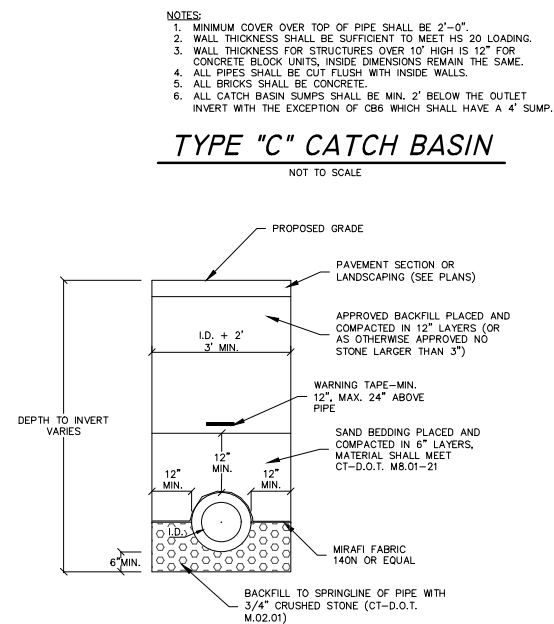
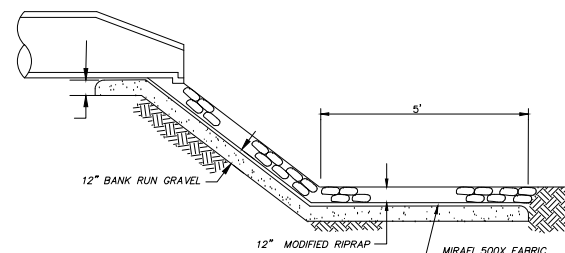
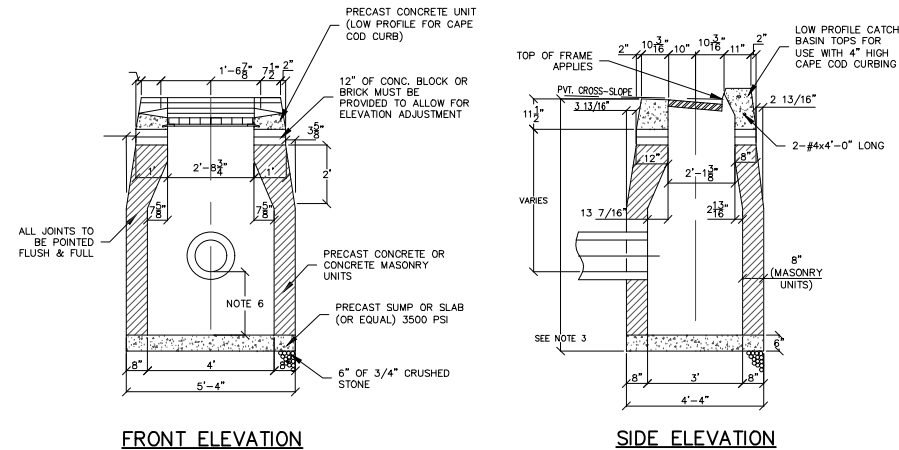
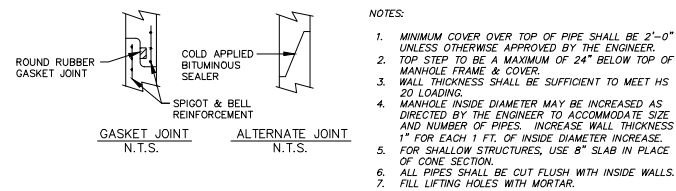
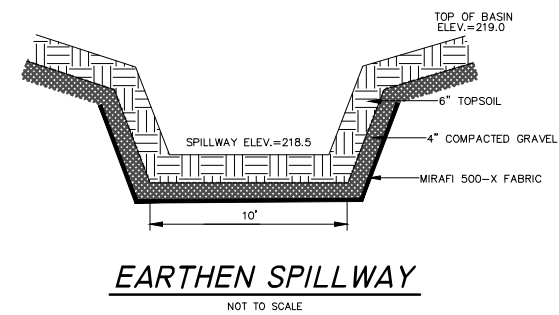
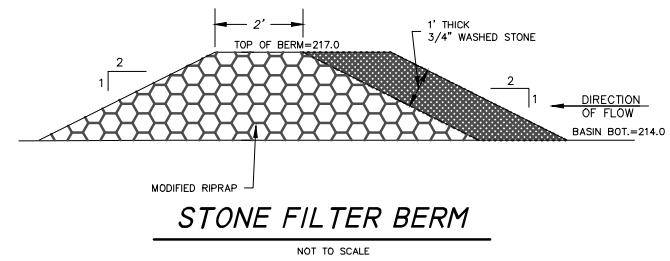
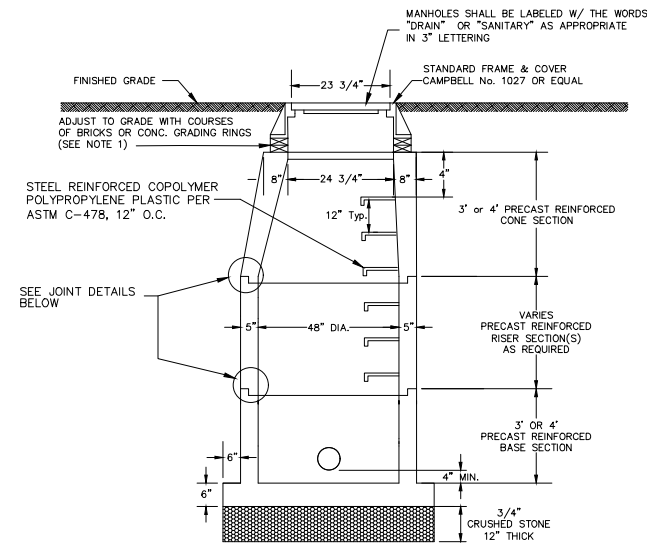
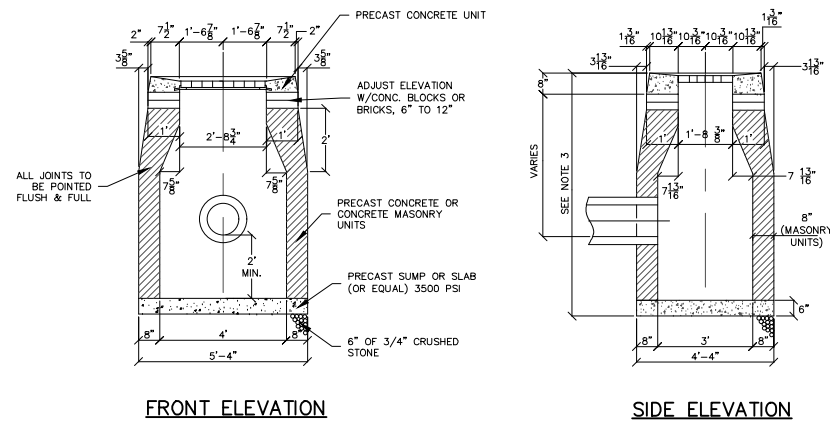
(ALT 2)

1-11-22

1 = 20

2021-083

SHEET



REVISIONS	
BY: LF/TAC	CHK: JEU

The Learning Experience
Property Of
501 Talcottville Road, LLC
501 Talcottville Road
Vernon, Connecticut 06010
Parcel ID: 09-0007-0001D (Zone: C)

Details

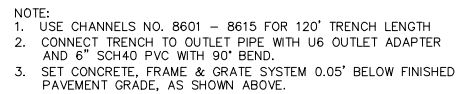
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DATE
1-11-22

SCALE
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JOB NUMBER
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SHEET



NOT TO SCALE



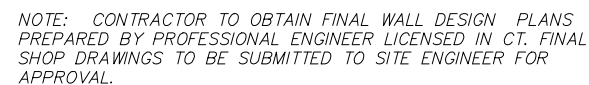
1. PROVIDE WARNING TAPE, 12-24" ABOVE TOP OF PIPE.
2. BACKFILL SHALL BE SUITABLE NATIVE MATERIAL UNLESS DETERMINED TO BE UNSUITABLE BY THE ENGINEER OR GRANULAR FILL M:0.01 PLACED AND COMPACTED IN 12" LOOSE LIFTS.
3. UNPAVED AREAS COMPACT BACKFILL TO 95% OF THE MATERIAL'S MAXIMUM DRY DENSITY AS DETERMINED BY A STANDARD PROCTOR TEST. UNPAVED AREAS 12" TO 208"
4. UNSUITABLE TRENCH MATERIAL TO BE REMOVED AND REPLACED WITH 3/4" CRUSHED STONE AS DIRECTED BY ENGINEER.

NOT TO SCALE

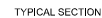


- NOTE:
1. INSTALL CLEANOUTS AT BENDS AND MAX. 100'± INTERVALS ALONG SANITARY LATERALS.

NOT TO SCALE



NOT TO SCALE



NOT TO SCALE



NOT TO SCALE

J.R. Russo & Associates, LLC
Shoham Rd East Windsor CT 06088 • CT 860.623.0569 • MA 413.785.1158
www.jrussoc.com • info@jrusso.com

Applicant

3-26-2022	TALCOTTVILLE RD ENTRANCE, APPLY VARIANCES
3-14-2022	PLACE NUMBERS FOR CULVERT STRUCTURE

BY: LF/TAC	CHK: JEU
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BY: LF/TAC	CHK: JEU
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The Learning Experience

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Details

(1170)

(ALT 2)

DATE _____

1-11-22

SCALE

 $1'' = 20'$

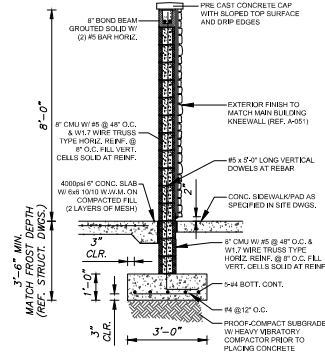
100 NUMBER

JOB NUMBER
0001 0005

2021-083

SHEET

11 of 12





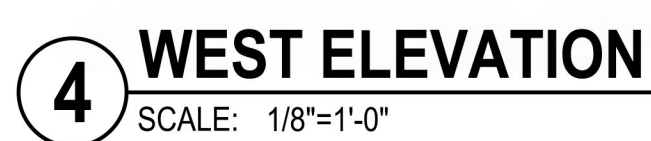
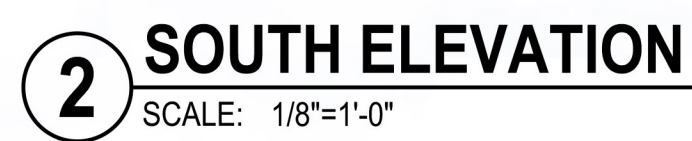
ARCHITECTS AND ENGINEERS INC.
42 OKNER PARKWAY
LIVINGSTON, NEW JERSEY 07039
TEL: 973-994-9669
FAX: 973-994-4069
www.jarmelkizel.com

Architecture
Engineering
Interior Design
Implementation Services

[illegible][illegible]

Project Number: TLECT21-041	Scale: 1/8"=1'-0"
Drawn By: AM	Approved By: MBJ
Drawing Name:	

Drawing Number: SA-1.2	
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EXTERIOR MATERIAL SCHEDULE			
LABEL	MANUFACTURER	SIZE / TYPE	FINISH / COLOR
STOREFRONT	KAWNEER	TRIFAB 451T / 350 PER DOOR SCHEDULE	BONE WHITE
DOOR	-	PER DOOR SCHEDULE	FACTORY PRIMED
FIXED WINDOW	PLY GEM	SILVER LINE V1 SERIES PER WINDOW SCHEDULE	WHITE
STONE-1	BUECHEL STONE CORP.	5" FULL DEPTH STONE	CHILTON TAILORED BLEND
SILL-1	MODERN PRECAST	3 3/4"x6"W W/ 2" FLAT W/ 1/4" DRIP EDGE	REGULAR (LIGHT GREY)
SILL-2	MODERN PRECAST	CUSTOM SILL 3 3/4"x4"W W/ 2" FLAT REF. DTL. 6/A-032	REGULAR (LIGHT GREY)
HARDIE-1	JAMES HARDIE	WIDTH: 8.25"	COLOR: NAVAJO BEIGE
HARDIE-2	JAMES HARDIE	WIDTH: 8.25"	COLOR: KHAKI BROWN
TRIM-1	JAMES HARDIE	3/4" M3 SMOOTH TRIM BOARD WIDTH: 3.5"	COLOR: KHAKI BROWN
TRIM-2	JAMES HARDIE	3/4" M3 SMOOTH TRIM BOARD WIDTH: 11.25"	COLOR: KHAKI BROWN
VINYL-1	ROYAL BUILDING PRODUCTS	COLORSCAPES TRIPLE 4 PERFORATED SOFFIT NOM. THICKNESS 0.042"	WHITE (REF. NOTE 2 BELOW)
VINYL-2	FYPON	WINDOW CROSSHEAD WIDTH 56", HEIGHT 9"	PAINT TO MATCH HARDIE-3 PRODUCT: WCH56X9
VINYL-3	FYPON	DOOR CROSSHEAD WIDTH 48", HEIGHT 9"	PAINT TO MATCH HARDIE-3 PRODUCT: WCH48X9
VINYL-4	FYPON	DOOR CROSSHEAD WIDTH 84", HEIGHT 9"	PAINT TO MATCH HARDIE-3 PRODUCT: WCH84X9
VINYL-5	FYPON	WINDOW CROSSHEAD WIDTH 103", HEIGHT 9"	PAINT TO MATCH HARDIE-3 PRODUCT: WCH103X9
VINYL-6	FYPON	WINDOW CROSSHEAD WIDTH 44", HEIGHT 9"	PAINT TO MATCH HARDIE-3 PRODUCT: WCH44X9
COPING-1	FABRAL (OR EQUAL)	BREAK METAL	DARK BRONZE
KNOX-1	KNOX BOX	3200 SERIES W/ RECESSED MOUNT FLANGE, HINGE DOOR, & TAMPER SWITCH	DARK BRONZE (REF. NOTE 2 BELOW)
FINISH SCHEDULE NOTES: 1. G.C. SHALL VERIFY KNOX BOX MODEL(S) AND LOCATION(S) WITH AUTHORITY HAVING JURISDICTION PRIOR TO ORDERING AND INSTALLATION. 2. G.C. SHALL ENSURE ALL EXTERIOR FINISHES ARE INSTALLED AND FINISHED IN COMPLIANCE WITH MANUFACTURER'S WARRANTY REQUIREMENTS.			

DRAINAGE REPORT
The Learning Experience
501 Talcottville Road
Enfield, CT

ALTERNATIVE 2

Revised March 17, 2022

Prepared for:

*Vernon Development, LLC
56 East Main Street
Avon, Connecticut 06001
(860) 677-5607*

Project No. 2021-083

Prepared by:

*J.R. Russo & Associates, LLC
Land Surveyors & Professional Engineers
P.O. Box 938
East Windsor, CT 06088
(860) 623-0569*

I. INTRODUCTION

A. Project Description

Vernon Development, LLC and The Learning Experience are proposing the development of a 10,000 s.f. daycare center on Dart Hill Road near the intersection of Dart Hill Road and Talcottville Road (Rte. 83) in Vernon. The development will result in an impervious area of approximately 0.97 acres. Runoff from the development will be directed to a subsurface infiltration system and infiltration basin designed to provide treatment, ground water recharge, and peak flow attenuation in accordance with the Connecticut Stormwater Quality Manual.

B. Existing Conditions

The proposed 2.0 acre lot for the development will be split off of the existing 4.6 acre parcel at 501 Talcottville Road. The new lot for the development (the project site) will front Dart Hill Road to the north. Access to the site will be via one driveway off of Dart Hill Road and a second limited access driveway off of Talcottville Road. The site currently consists of undeveloped woodlands, including a wooded wetland on the southern portion of the lot. The wetlands ultimately discharge to Ogden Brook further to the south. Starting at the edge of the road, the site slopes southerly toward the wetland. In addition to runoff from the parcel, stormwater from the building and parking lot at the adjacent Panda Palace to the east, discharges onto the property via a leak-off in the concrete curb at the edge of the parking lot. This runoff flows across the development site into the on-site wetland.

Stormwater overflow from an existing stormwater management basin serving the Walgreens development on the north side of Dart Hill Road is also piped underneath the road to discharge onto the site. The discharge from the stormwater basin consists of a 6" PVC pipe which terminates in an existing manhole in Dart Hill Road. The manhole discharges onto the site via a 15" RCP.

Based on a review of the USDA Soil Survey of Connecticut, the majority of the soil in the area to be developed consists of Hartford sandy loam and the remaining soil on the western portion of the site is Udorthents-Urban land complex. (see Soils Map in Appendix 2). The USDA Soil Survey defines groups of soils into Hydrologic Soil Groups (HSG) according to their runoff-producing characteristics. Soils are assigned to four groups (A, B, C, and D Groups). In group A, are soils having a high infiltration rate when thoroughly wet and having a low runoff potential. They typically are deep, well drained, and sandy or gravelly. In group D, at the other extreme, are soils having a very slow infiltration rate and thus a high runoff potential. They have a hardpan or clay layer at or near the surface, have a permanent high water table, or are shallow over nearly impervious bedrock or other nearly impervious material. The classification of the Hartford sandy loam is HSG A and the Udorthents-Urban land complex is HSG B.

On December 17, 2021, a series of 5 test pits were performed in the area of the proposed development to confirm the existing soil conditions. Test pits were excavated to depths ranging from 62-90 inches. Soils encountered included 6-12 inches of topsoil over brown fine sandy loam subsoils to a depth of 18-30 inches, overlying red-brown sand and gravel. Soil mottling

indicative of the seasonal high water table was encountered in test pits TP1, TP2, and TP3. The depths of the mottling indicate a seasonal high water table approximately at the elevation of 212.0. Test pit logs are provided on the Site Plans.

Soil samples were collected from test pits 1 through 4 at depths ranging from 30-42 inches. These samples were submitted to New England Materials Testing Lab, LLC for permeability testing by ASTM D2434. Calculated permeabilities ranged from 1.10 in/hr for the sample collected at the bottom of the slope in TP1 to 33.95 in/hr for the sample collected mid-slope in TP4. Permeability test results are also provided in Appendix 3.

II. STORMWATER RUNOFF ANALYSIS

A. Methodology

Peak runoff flow rates were determined for pre- and post-development conditions using Applied Microcomputer System's HydroCAD™ Stormwater Modeling System. This computer software employs the SCS Technical Release 55 and 20 (TR-55 & TR-20) methodology. The potential stormwater impacts downstream were evaluated for the 2-yr, 10-yr, 25-yr, and 100-yr; 24-hour storm events. The rainfall for these storm events was taken from NOAA Atlas 14 provided in Appendix 1. Based on the present drainage patterns, all runoff from the proposed development area flows south into the on-site wetlands. As a result, the edge of the wetlands was selected as the design point. The runoff outlet from across the street will be diverted in order to bypass the on-site drainage system and discharge directly at the edge of the wetland. Thus, it acts independent from the site and was not included in these calculations.

B. Pre-Development Hydrology

The pre-development site was modeled as two subcatchments. Subcatchment PRE includes the approximately 2.20 acres of on-site development area that drains to the design point. Subcatchment PP includes the off-site runoff from the adjacent Panda Palace parcel. The pre-development drainage area map is provided in Appendix 4. The pre-development runoff characteristics of the contributing area is provided on the HydroCAD data sheets in Appendix 5. The pre-development discharge rates from the site during the design storms are summarized in Table 1.

C. Post-Development Hydrology

The proposed project will result in approximately 0.97 acres of new impervious area. The development will include a series of catch basins, trench drains, and piping to collect runoff from the development area and divert it to either a new infiltration basin in the southeast corner of the site or subsurface infiltration system under the parking lot. This system will include a riprap swale and area drain installed downgradient of the leak-off from the Panda Palace parking lot. This runoff will be collected and diverted to the subsurface infiltration system. In addition, the roof runoff will be collected and diverted towards the infiltration basin. The subsurface

infiltration system and infiltration basin have been designed in accordance with the CT Storm Water Quality Manual to provide treatment, groundwater recharge, and peak flow attenuation.

As discussed above, several test pits were completed at the site to verify soil conditions. Test pits TP1, TP2 and TP3 were all located within the vicinity of the proposed infiltration basin. Samples collected from these test pits at depths consistent with the proposed bottom of the basin were tested for permeability. The resulting permeabilities were 1.10 in/hr (TP1), 1.97 in/hr (TP2) and 2.34 in/hr (TP3). As a conservative measure, the slowest permeability rate of 1.10 in/hr was used as the basis for the design infiltration rate. This rate was further reduced by 50% to account for potential clogging resulting in a final design infiltration rate for the infiltration basin of 0.55 inches/hour. The subsurface infiltration system will be constructed further up the slope to the north in the vicinity of test pits TP4 and TP5. The soils in these test pits at the elevation of the proposed infiltration system included loose, sand and gravel. The resulting permeability for the soil sample collected from TP4 was 33.95 in/hr. As a conservative measure and to account for potential clogging, the design infiltration rate used for the subsurface infiltration system was selected to be 15.0 in/hr.

The infiltration basin will be equipped with a forebay separated from the main part of the basin by a stone filter berm. The bottom of the basin will be set at elevation 214 which allows 2 feet of separation to the seasonal high water table that was encountered in the test pits. The basin will be equipped with a multi-stage outlet structure constructed from a standard Type CL catch basin. The structures primary outlet will consist of an 8" orifice set at an elevation of 216.6. The secondary outlet will consist of the frame and grate to be set at elevation 218.4. The structure will discharge via a 10" outlet pipe at the edge of the wetland. The basin will also be equipped with an emergency 10-foot wide earthen spillway at elevation 218.5. The subsurface infiltration system under the parking lot will consist of 60 ADS SC740 chambers in stone. The bottom of the chambers will be set at an elevation of 215.0. The subsurface infiltration system will be connected to a proposed manhole for overflow from larger storm events exceeding the capacity of the storage and infiltration capacity of the chambers. In the proposed manhole, a weir set at 217.6 will overflow to a pipe discharging directly to the edge of the wetlands. The proposed storage capacities of the infiltration basin below the primary outlet was sized to exceed the water quality volume. Likewise, the surface basin forebay was sized to contain a minimum of 25% of the WQV per the requirements for infiltration basins. WQV calculations are provided in Appendix 6.

The same design point for the pre-development analysis was used for the post development analysis. The post development site was divided into 9 subcatchments. Subcatchments S1 and S2 include the areas that will be collected by the catch basins in the parking lot and discharged to the subsurface infiltration system. Subcatchments S3 and S4 include the areas that will be collected by the catch basins in the parking lot and discharged to the infiltration basin. Subcatchment S8 includes the roof runoff that will be discharged into a catch basin towards the infiltration basin. Subcatchment S5 includes runoff from the playground area that will be collected in a trench drain and piped directly to the infiltration basin. Subcatchment S6 includes the area that will directly sheet flow into the infiltration basin. Subcatchment S7 includes the area that will continue to drain overland directly to the design point (i.e. wetlands). Subcatchment PP includes the area of the adjacent Panda Palace that discharges to the development area.

The post development drainage area map is provided in Appendix 4. The post development runoff characteristics of the subcatchments are provided on the HydroCAD data sheets in Appendix 5. As shown in the Table 1, the post-development peak rates of runoff from the site to the design point will be maintained or reduced in comparison to the pre-development rates.

**TABLE 1 – COMPARISON OF PRE- & POST-DEVELOPMENT
DISCHARGE RATES (CFS) TO DESIGN POINT**

	2-year	10-year	25-year	100-year
Pre-Development	2.1	3.4	4.2	5.4
Post Development	0.2	1.3	1.8	4.9

D. Pipe Sizing

The piping proposed at the site consists of smooth bore corrugated high density polyethylene pipe with smooth interior walls (CPEP-S). The roughness coefficient used for this pipe type is 0.012. The analysis provided in Appendix 5 indicates headwater elevation in the structure at each pipe inlet for the design storms and compares it to the flood elevation, which corresponds to the top of frame of the structure. The calculations indicate that all proposed pipes will have sufficient capacity to convey the 10-year storm event without surcharging out of the top of the structures.

E. Outlet Protection

Outfall protection for the pipe discharge from the Walgreens Stormwater Management Basin and subsurface infiltration system overflow at the edge of the wetland will consist of a Type A riprap apron. Outfall protection for the pipe discharge from the infiltration basin at the edge of the wetland will consist of a Type A riprap apron. Similarly, riprap channels will be installed at the inlets to the infiltration basin to convey runoff down the slope into the forebay. Outlet protection for the infiltration's spillway will consist of a 12" thick modified riprap slope extended 5 feet beyond the toe of the slope.

F. Summary of Results

The proposed design and analysis indicates that the proposed development will not result in negative impacts downstream. In addition, the proposed stormwater management system will intercept and divert the untreated runoff from the Panda Palace leak-off to the subsurface infiltration system where it will be treated prior to discharge, whereas previously it was discharged to the wetland without treatment.

Appendix 1:
RAINFALL DATA



NOAA Atlas 14, Volume 10, Version 3
Location name: Vernon Rockville, Connecticut,
USA*

Latitude: 41.8502°, Longitude: -72.4855°

Elevation: 224.86 ft**

* source: ESRI Maps

** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps_&_aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.332 (0.254-0.434)	0.403 (0.308-0.528)	0.519 (0.395-0.684)	0.616 (0.466-0.814)	0.749 (0.551-1.03)	0.849 (0.614-1.20)	0.953 (0.672-1.39)	1.07 (0.718-1.60)	1.24 (0.803-1.92)	1.38 (0.875-2.18)
10-min	0.470 (0.359-0.615)	0.571 (0.436-0.748)	0.736 (0.560-0.967)	0.872 (0.660-1.15)	1.06 (0.780-1.46)	1.20 (0.868-1.69)	1.35 (0.951-1.97)	1.52 (1.02-2.27)	1.76 (1.14-2.72)	1.96 (1.24-3.08)
15-min	0.553 (0.423-0.724)	0.672 (0.513-0.880)	0.866 (0.658-1.14)	1.03 (0.778-1.36)	1.25 (0.918-1.72)	1.41 (1.02-1.99)	1.59 (1.12-2.32)	1.79 (1.20-2.67)	2.07 (1.34-3.20)	2.30 (1.46-3.63)
30-min	0.748 (0.571-0.979)	0.909 (0.694-1.19)	1.17 (0.892-1.54)	1.39 (1.05-1.84)	1.69 (1.25-2.34)	1.92 (1.39-2.71)	2.16 (1.52-3.15)	2.43 (1.63-3.62)	2.81 (1.82-4.34)	3.13 (1.98-4.93)
60-min	0.943 (0.720-1.23)	1.15 (0.875-1.50)	1.48 (1.13-1.95)	1.76 (1.33-2.32)	2.14 (1.57-2.95)	2.42 (1.75-3.42)	2.73 (1.92-3.99)	3.07 (2.05-4.58)	3.56 (2.30-5.49)	3.96 (2.51-6.23)
2-hr	1.21 (0.931-1.58)	1.47 (1.12-1.91)	1.88 (1.44-2.46)	2.23 (1.69-2.93)	2.70 (2.00-3.72)	3.05 (2.22-4.30)	3.43 (2.44-5.03)	3.89 (2.61-5.77)	4.57 (2.96-7.02)	5.15 (3.27-8.06)
3-hr	1.40 (1.08-1.82)	1.69 (1.30-2.19)	2.16 (1.66-2.82)	2.56 (1.95-3.36)	3.10 (2.30-4.26)	3.50 (2.56-4.93)	3.94 (2.82-5.78)	4.47 (3.01-6.62)	5.30 (3.44-8.10)	6.01 (3.82-9.36)
6-hr	1.76 (1.36-2.28)	2.13 (1.65-2.76)	2.74 (2.11-3.56)	3.25 (2.49-4.24)	3.95 (2.95-5.41)	4.46 (3.28-6.26)	5.02 (3.62-7.35)	5.72 (3.86-8.42)	6.82 (4.44-10.4)	7.78 (4.96-12.0)
12-hr	2.17 (1.69-2.80)	2.66 (2.06-3.42)	3.45 (2.67-4.46)	4.10 (3.16-5.33)	5.01 (3.76-6.83)	5.67 (4.19-7.92)	6.40 (4.63-9.32)	7.31 (4.95-10.7)	8.74 (5.71-13.2)	9.98 (6.38-15.3)
24-hr	2.56 (1.99-3.27)	3.16 (2.47-4.06)	4.16 (3.23-5.35)	4.98 (3.85-6.45)	6.12 (4.62-8.32)	6.95 (5.16-9.68)	7.87 (5.74-11.4)	9.04 (6.14-13.1)	10.9 (7.13-16.3)	12.5 (8.03-19.1)
2-day	2.88 (2.26-3.67)	3.61 (2.83-4.61)	4.81 (3.75-6.16)	5.80 (4.50-7.47)	7.17 (5.44-9.72)	8.16 (6.10-11.3)	9.28 (6.82-13.5)	10.7 (7.30-15.5)	13.1 (8.59-19.5)	15.2 (9.77-23.0)
3-day	3.13 (2.46-3.98)	3.94 (3.09-5.01)	5.25 (4.10-6.70)	6.33 (4.93-8.12)	7.83 (5.96-10.6)	8.91 (6.69-12.4)	10.1 (7.48-14.7)	11.7 (8.01-16.9)	14.3 (9.44-21.3)	16.7 (10.8-25.2)
4-day	3.37 (2.65-4.27)	4.22 (3.32-5.36)	5.61 (4.40-7.15)	6.77 (5.28-8.67)	8.36 (6.37-11.3)	9.52 (7.15-13.2)	10.8 (7.99-15.7)	12.5 (8.55-18.0)	15.3 (10.1-22.7)	17.8 (11.5-26.8)
7-day	4.00 (3.16-5.06)	4.96 (3.92-6.28)	6.53 (5.14-8.29)	7.83 (6.13-9.99)	9.62 (7.35-12.9)	10.9 (8.22-15.0)	12.4 (9.15-17.8)	14.3 (9.78-20.4)	17.3 (11.4-25.5)	20.0 (12.9-30.0)
10-day	4.64 (3.68-5.85)	5.66 (4.48-7.14)	7.32 (5.78-9.27)	8.70 (6.83-11.1)	10.6 (8.11-14.2)	12.0 (9.02-16.4)	13.5 (9.98-19.3)	15.5 (10.6-22.1)	18.6 (12.3-27.3)	21.3 (13.8-31.8)
20-day	6.67 (5.31-8.37)	7.75 (6.17-9.74)	9.53 (7.55-12.0)	11.0 (8.67-13.9)	13.0 (9.97-17.2)	14.5 (10.9-19.5)	16.1 (11.8-22.5)	18.0 (12.5-25.5)	20.8 (13.9-30.4)	23.2 (15.1-34.4)
30-day	8.40 (6.70-10.5)	9.51 (7.58-11.9)	11.3 (8.99-14.2)	12.8 (10.1-16.2)	14.9 (11.4-19.5)	16.5 (12.3-21.9)	18.1 (13.2-24.9)	19.9 (13.8-27.9)	22.3 (14.9-32.4)	24.3 (15.9-36.0)
45-day	10.6 (8.45-13.2)	11.7 (9.35-14.6)	13.6 (10.8-17.0)	15.1 (12.0-19.0)	17.2 (13.2-22.4)	18.9 (14.1-24.9)	20.5 (14.8-27.8)	22.1 (15.4-31.0)	24.3 (16.3-35.0)	25.8 (16.9-38.1)
60-day	12.4 (9.92-15.4)	13.5 (10.8-16.9)	15.4 (12.3-19.3)	17.0 (13.5-21.4)	19.2 (14.7-24.8)	20.9 (15.7-27.5)	22.6 (16.3-30.4)	24.1 (16.8-33.6)	26.0 (17.4-37.4)	27.3 (17.8-40.1)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PF graphical

Appendix 2:
NRCS SOILS INFORMATION



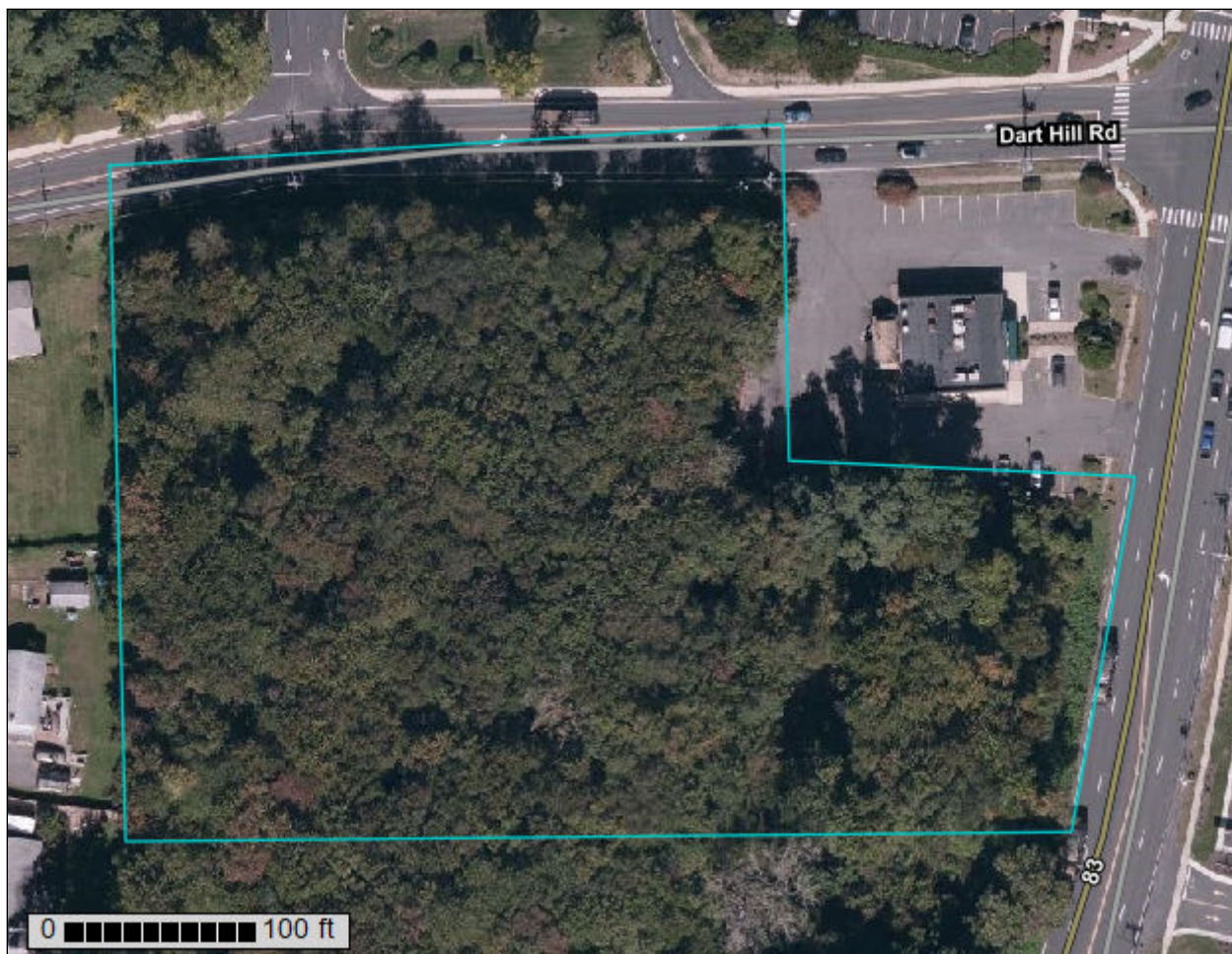
United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for State of Connecticut



Custom Soil Resource Report Soil Map



Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
33B	Hartford sandy loam, 3 to 8 percent slopes	3.3	91.3%
306	Udorthents-Urban land complex	0.3	8.7%
Totals for Area of Interest		3.7	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

State of Connecticut

33B—Hartford sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9lmw

Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 54 inches

Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Hartford and similar soils: 80 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hartford

Setting

Landform: Terraces, outwash plains

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Sandy glaciofluvial deposits derived from sandstone and/or basalt

Typical profile

Ap - 0 to 8 inches: sandy loam

Bw1 - 8 to 20 inches: sandy loam

Bw2 - 20 to 26 inches: loamy sand

2C - 26 to 65 inches: stratified very gravelly coarse sand to loamy fine sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Somewhat excessively drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 4.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: A

Ecological site: F145XY008MA - Dry Outwash

Hydric soil rating: No

Minor Components

Penwood

Percent of map unit: 5 percent

Landform: Terraces, outwash plains

Down-slope shape: Convex

Custom Soil Resource Report

Across-slope shape: Linear

Hydric soil rating: No

Ellington

Percent of map unit: 5 percent

Landform: Terraces, outwash plains

Down-slope shape: Linear

Across-slope shape: Linear

Hydric soil rating: No

Manchester

Percent of map unit: 5 percent

Landform: Terraces, outwash plains, kames, eskers

Down-slope shape: Convex

Across-slope shape: Convex

Hydric soil rating: No

Branford

Percent of map unit: 5 percent

Landform: Terraces, outwash plains

Down-slope shape: Linear

Across-slope shape: Linear

Hydric soil rating: No

306—Udorthents-Urban land complex

Map Unit Setting

National map unit symbol: 9lmg

Elevation: 0 to 2,000 feet

Mean annual precipitation: 43 to 56 inches

Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 120 to 185 days

Farmland classification: Not prime farmland

Map Unit Composition

Udorthents and similar soils: 50 percent

Urban land: 35 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Udorthents

Setting

Down-slope shape: Convex

Across-slope shape: Linear

Parent material: Drift

Typical profile

A - 0 to 5 inches: loam

C1 - 5 to 21 inches: gravelly loam

C2 - 21 to 80 inches: very gravelly sandy loam

Custom Soil Resource Report

Properties and qualities

Slope: 0 to 25 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Very low to high (0.00 to 1.98 in/hr)

Depth to water table: About 54 to 72 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 6.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: B

Hydric soil rating: No

Description of Urban Land

Typical profile

H - 0 to 6 inches: material

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 8

Hydrologic Soil Group: D

Hydric soil rating: Unranked

Minor Components

Unnamed, undisturbed soils

Percent of map unit: 8 percent

Hydric soil rating: No

Udorthents, wet substratum

Percent of map unit: 5 percent

Down-slope shape: Convex

Across-slope shape: Linear

Hydric soil rating: No

Rock outcrop

Percent of map unit: 2 percent

Hydric soil rating: No

Appendix 3:
PERMEABILITY TEST RESULTS



NEW ENGLAND MATERIALS TESTING LAB, LLC.
NEW ENGLAND REGIONAL OFFICE

72 Bissell Street Manchester, CT 06040 • Tel: 860-783-5830 • Fax: 860-783-5832

Client: JR Russo Surveyors Engineers
P. O Box 938
East Windsor, CT. 06088

Report #: 001

Lab ID: 098-21

Project: 501Talcottville Road Vernon, CT.

Client ID: TP-1

Technician: Z. A

Date: 12/27/2021

LAB PERMEABILITY TEST

Sample description: Reddish brown silty clayey sand.

Location: Onsite (501Talcottville Red Vernon, CT).

Sample depth: 30" to 36"

Method: Permeability by ASTM D2434 (Constant Head Method)

$$k = QL/ath$$

Where k = coefficient of permeability,

Q = quantity of water discharged,	Q = 800 cm ³
L = length of sample in centimeters	L = 15.24 cm
A = cross sectional area of specimen,	A = 43.10 cm ²
t = total time for discharge, in seconds	t = 5940 sec
h = difference in head manometers,	h = 61.5 cm

$$k = 0.00077435 \text{ cm/sec.}$$

$$k = 1.0975 \text{ inch/hour}$$

Reported To: JR Russo Surveyors Engineers

Submitted By: New England Materials Testing Lab, LLC.

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NEW ENGLAND REGIONAL OFFICE

72 Bissell Street Manchester, CT 06040 • Tel: 860-783-5830 • Fax: 860-783-5832

Client: JR Russo Surveyors Engineers
P. O Box 938
East Windsor, CT. 06088

Report #: 002

Lab ID: 099-21

Project: 501Talcottville Road Vernon, CT.

Client ID: TP-2

Technician: Z. A

Date: 12/27/2021

LAB PERMEABILITY TEST

Sample description: Reddish brown sand bank run gravel and fines.

Location: Onsite (501Talcottville Red Vernon, CT).

Sample depth: 30" to 36"

Method: Permeability by ASTM D2434 (Constant Head Method)

$$k = QL/ath$$

Where k = coefficient of permeability,

Q = quantity of water discharged,	Q = 1000 cm ³
L = length of sample in centimeters	L = 15.24 cm
A = cross sectional area of specimen,	A = 43.10 cm ²
t = total time for discharge, in seconds	t = 4140 sec
h = difference in head manometers,	h = 61.5 cm

$$k = 0.001388776 \text{ cm/sec.}$$

$$k = 1.968 \text{ inch/hour}$$

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Submitted By: New England Materials Testing Lab, LLC.

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Client: JR Russo Surveyors Engineers
P. O Box 938
East Windsor, CT. 06088

Report #: 003

Lab ID: 100-21

Project: 501Talcottville Road Vernon, CT.

Client ID: TP-3

Technician: Z. A

Date: 12/27/2021

LAB PERMEABILITY TEST

Sample description: Dark brown silty clayey sand and gravel.

Location: Onsite (501Talcottville Red Vernon, CT).

Sample depth: 36" to 42"

Method: Permeability by ASTM D2434 (Constant Head Method)

$$k = QL/ath$$

Where k = coefficient of permeability,

Q = quantity of water discharged,	Q = 1000 cm ³
L = length of sample in centimeters	L = 15.24 cm
A = cross sectional area of specimen,	A = 43.10 cm ²
t = total time for discharge, in seconds	t = 3480 sec
h = difference in head manometers,	h = 61.5 cm

$$k = 0.001652165 \text{ cm/sec.}$$

$$k = 2.341 \text{ inch/hour}$$

Reported To: JR Russo Surveyors Engineers

Submitted By: New England Materials Testing Lab, LLC.

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Client: JR Russo Surveyors Engineers
P. O Box 938
East Windsor, CT. 06088

Report #: 001

Lab ID: 101-21

Project: 501Talcottville Road Vernon, CT.

Client ID: TP-4

Technician: Z. A

Date: 12/27/2021

LAB PERMEABILITY TEST

Sample description: Reddish brown sand and little gravel.

Location: Onsite (501Talcottville Red Vernon, CT).

Sample depth: 30" to 36"

Method: Permeability by ASTM D2434 (Constant Head Method)

$$k = QL/ath$$

Where k = coefficient of permeability,

Q = quantity of water discharged,	Q = 1000 cm ³
L = length of sample in centimeters	L = 15.24 cm
A = cross sectional area of specimen,	A = 43.10 cm ²
t = total time for discharge, in seconds	t = 240 sec
h = difference in head manometers,	h = 61.5 cm

$$k = 0.023956388 \text{ cm/sec.}$$

$$k = 33.953 \text{ inch/hour}$$

Reported To: JR Russo Surveyors Engineers

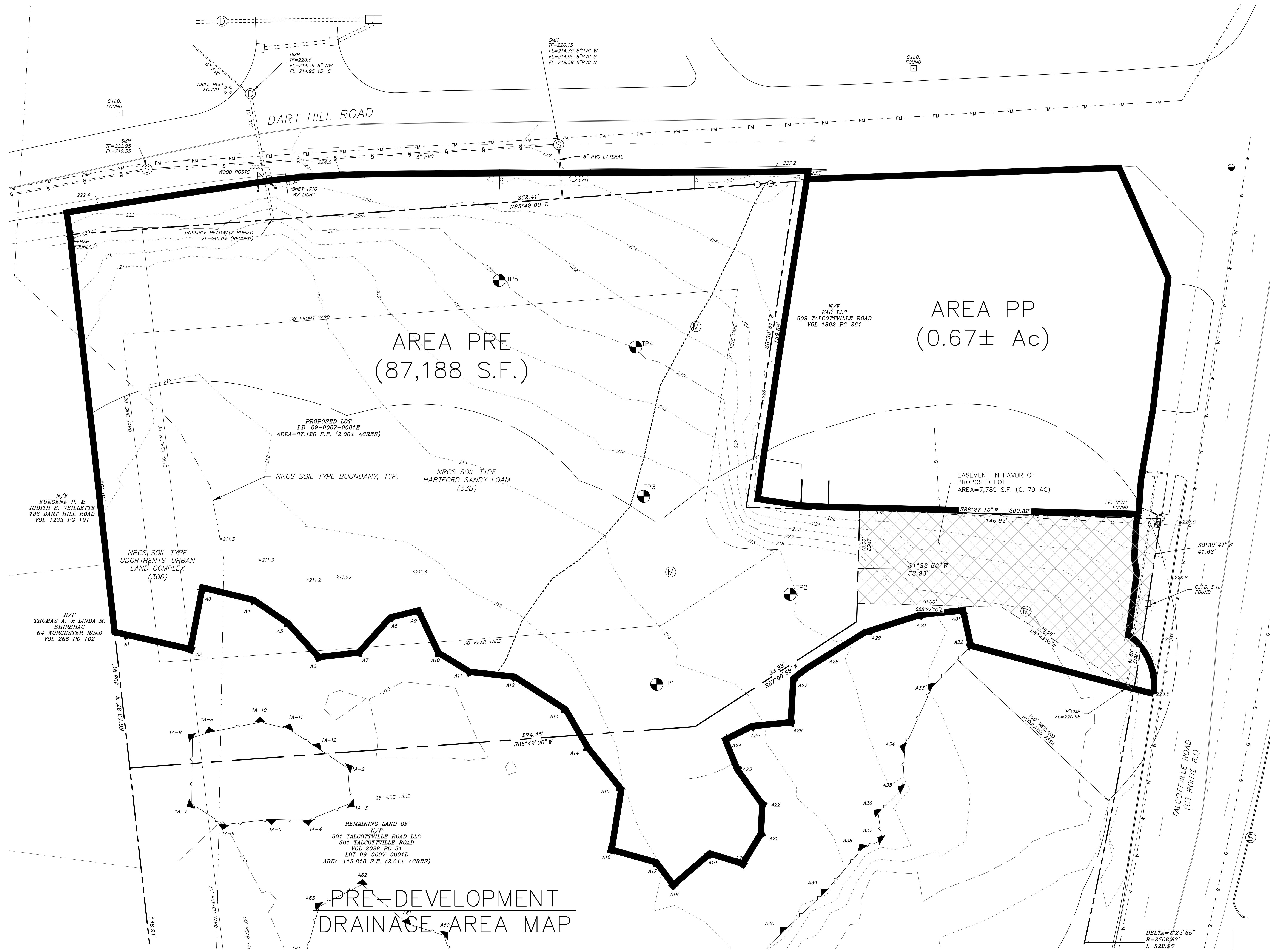
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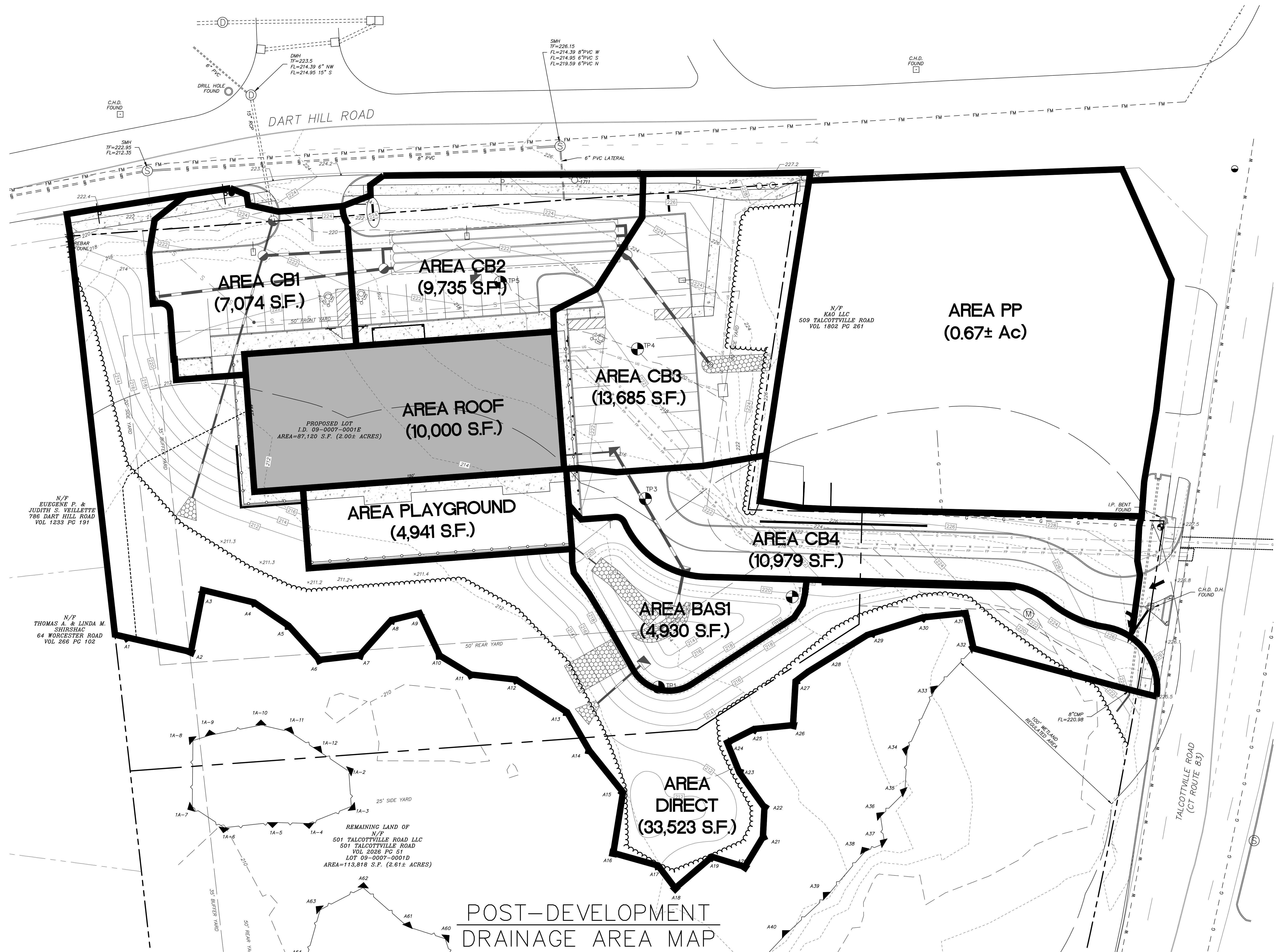
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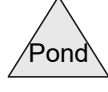
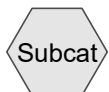
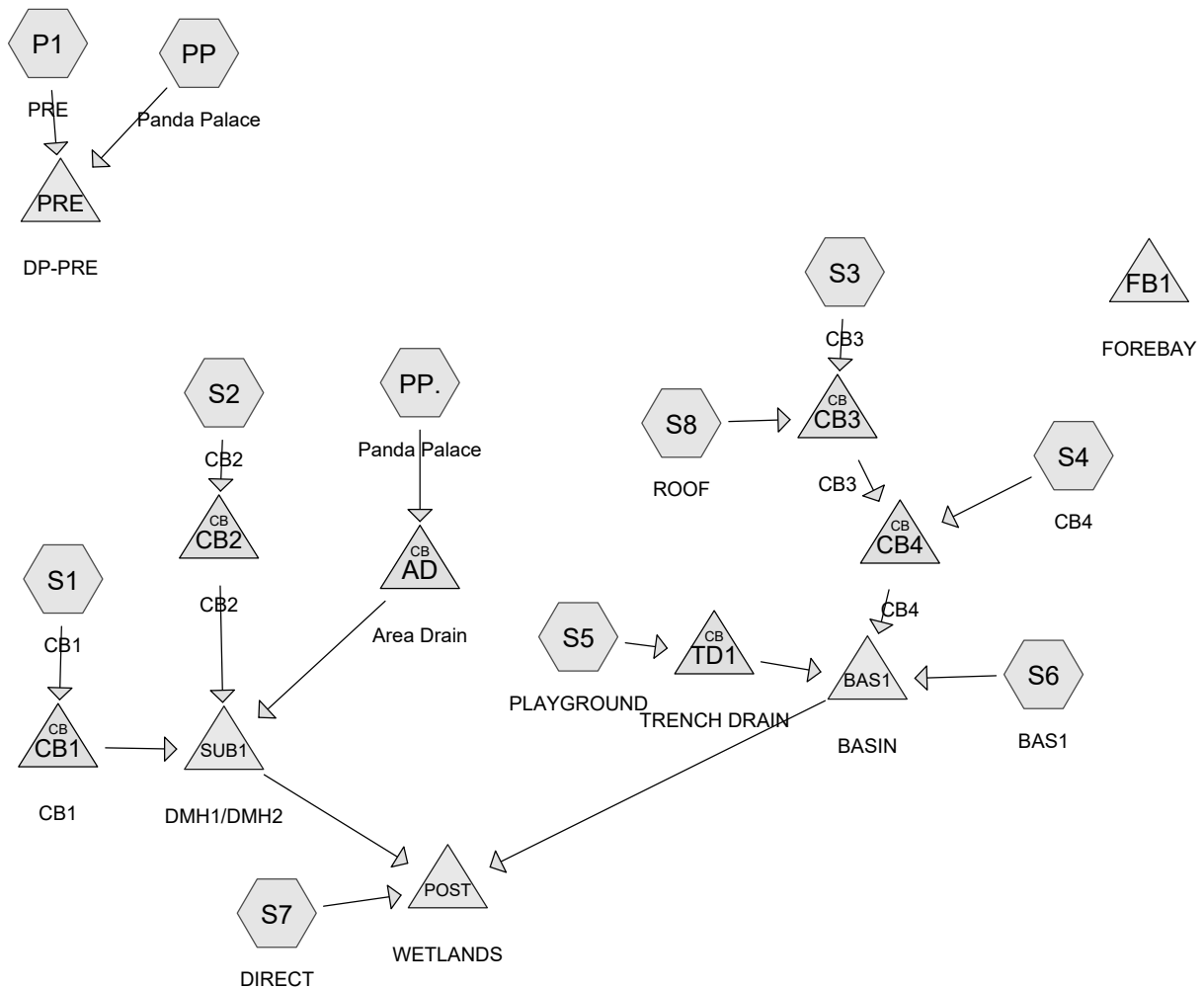
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Appendix 4:
DRAINAGE AREA MAPS





Appendix 5:
HYDROCAD ANALYSES



2021-083 Vernon TLE-B

Type III 24-hr 10-year Rainfall=4.98"

Prepared by J.R. Russo & Associates LLC

Printed 3/23/2022

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Summary for Pond AD: Area Drain

Inflow Area = 0.668 ac, 100.00% Impervious, Inflow Depth > 4.74" for 10-year event
 Inflow = 3.37 cfs @ 12.07 hrs, Volume= 0.264 af
 Outflow = 3.37 cfs @ 12.08 hrs, Volume= 0.264 af, Atten= 0%, Lag= 0.6 min
 Primary = 3.37 cfs @ 12.08 hrs, Volume= 0.264 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 219.86' @ 12.08 hrs

Flood Elev= 221.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	218.90'	15.0" Round Culvert L= 62.0' Ke= 0.500 Inlet / Outlet Invert= 218.90' / 218.28' S= 0.0100 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=3.37 cfs @ 12.08 hrs HW=219.86' TW=215.61' (Dynamic Tailwater)↑**1=Culvert** (Inlet Controls 3.37 cfs @ 3.34 fps)**Summary for Pond BAS1: BASIN**

Inflow Area = 1.022 ac, 66.96% Impervious, Inflow Depth > 2.90" for 10-year event
 Inflow = 3.30 cfs @ 12.09 hrs, Volume= 0.247 af
 Outflow = 1.31 cfs @ 12.32 hrs, Volume= 0.247 af, Atten= 60%, Lag= 13.6 min
 Discarded = 0.05 cfs @ 12.32 hrs, Volume= 0.103 af
 Primary = 1.26 cfs @ 12.32 hrs, Volume= 0.144 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 217.50' @ 12.32 hrs Surf.Area= 2,223 sf Storage= 4,200 cf

Flood Elev= 219.00' Surf.Area= 3,299 sf Storage= 8,327 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 421.2 min (1,210.8 - 789.6)

Volume	Invert	Avail.Storage	Storage Description
#1	214.00'	8,327 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
214.00	353	0	0
216.00	1,249	1,602	1,602
218.00	2,551	3,800	5,402
219.00	3,299	2,925	8,327

Device	Routing	Invert	Outlet Devices
#1	Discarded	214.00'	0.550 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 212.00'
#2	Primary	218.50'	10.0' long x 7.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.40 2.52 2.70 2.68 2.68 2.67 2.66 2.65 2.65

2021-083 Vernon TLE-B

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Type III 24-hr 10-year Rainfall=4.98"

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			2.65	2.66	2.65	2.66	2.68	2.70	2.73	2.78
#3	Primary	212.30'	10.0" Round Culvert L= 28.0' Ke= 0.500							
			Inlet / Outlet Invert= 212.30' / 212.00' S= 0.0107 '/' Cc= 0.900							
			n= 0.012, Flow Area= 0.55 sf							
#4	Device 3	216.60'	8.0" Vert. Orifice/Grate C= 0.600							
#5	Device 3	218.40'	36.0" W x 19.4" H Vert. Orifice/Grate C= 0.600							

Discarded OutFlow Max=0.05 cfs @ 12.32 hrs HW=217.50' (Free Discharge)↑ **1=Exfiltration** (Controls 0.05 cfs)**Primary OutFlow** Max=1.26 cfs @ 12.32 hrs HW=217.50' TW=0.00' (Dynamic Tailwater)↑ **2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)↑ **3=Culvert** (Passes 1.26 cfs of 5.74 cfs potential flow)↑ **4=Orifice/Grate** (Orifice Controls 1.26 cfs @ 3.61 fps)↑ **5=Orifice/Grate** (Controls 0.00 cfs)**Summary for Pond CB1: CB1**

Inflow Area =	0.162 ac, 82.30% Impervious, Inflow Depth = 3.65" for 10-year event
Inflow =	0.70 cfs @ 12.07 hrs, Volume= 0.049 af
Outflow =	0.70 cfs @ 12.08 hrs, Volume= 0.049 af, Atten= 0%, Lag= 0.6 min
Primary =	0.70 cfs @ 12.08 hrs, Volume= 0.049 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 217.89' @ 12.08 hrs

Flood Elev= 221.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	217.50'	15.0" Round Culvert L= 107.0' Ke= 0.500
			Inlet / Outlet Invert= 217.50' / 216.43' S= 0.0100 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=0.70 cfs @ 12.08 hrs HW=217.89' TW=215.63' (Dynamic Tailwater)↑ **1=Culvert** (Inlet Controls 0.70 cfs @ 2.13 fps)**Summary for Pond CB2: CB2**

Inflow Area =	0.223 ac, 67.98% Impervious, Inflow Depth = 2.78" for 10-year event
Inflow =	0.76 cfs @ 12.07 hrs, Volume= 0.052 af
Outflow =	0.76 cfs @ 12.08 hrs, Volume= 0.052 af, Atten= 0%, Lag= 0.6 min
Primary =	0.76 cfs @ 12.08 hrs, Volume= 0.052 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 216.39' @ 12.38 hrs

Flood Elev= 221.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	215.00'	15.0" Round Culvert L= 3.0' Ke= 0.500
			Inlet / Outlet Invert= 215.00' / 215.00' S= 0.0000 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.23 sf

2021-083 Vernon TLE-B

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Primary OutFlow Max=0.09 cfs @ 12.08 hrs HW=215.65' TW=215.65' (Dynamic Tailwater)↑**1=Culvert** (Outlet Controls 0.09 cfs @ 0.20 fps)**Summary for Pond CB3: CB3**

Inflow Area = 0.544 ac, 74.18% Impervious, Inflow Depth > 3.17" for 10-year event
 Inflow = 1.91 cfs @ 12.07 hrs, Volume= 0.144 af
 Outflow = 1.91 cfs @ 12.08 hrs, Volume= 0.144 af, Atten= 0%, Lag= 0.6 min
 Primary = 1.91 cfs @ 12.08 hrs, Volume= 0.144 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 218.58' @ 12.08 hrs

Flood Elev= 221.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	217.90'	15.0" Round Culvert L= 63.0' Ke= 0.500 Inlet / Outlet Invert= 217.90' / 216.50' S= 0.0222 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=1.91 cfs @ 12.08 hrs HW=218.58' TW=217.45' (Dynamic Tailwater)↑**1=Culvert** (Inlet Controls 1.91 cfs @ 2.81 fps)**Summary for Pond CB4: CB4**

Inflow Area = 0.796 ac, 71.73% Impervious, Inflow Depth > 3.02" for 10-year event
 Inflow = 2.74 cfs @ 12.08 hrs, Volume= 0.200 af
 Outflow = 2.74 cfs @ 12.09 hrs, Volume= 0.200 af, Atten= 0%, Lag= 0.6 min
 Primary = 2.74 cfs @ 12.09 hrs, Volume= 0.200 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 217.55' @ 12.28 hrs

Flood Elev= 220.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	216.50'	15.0" Round Culvert L= 16.0' Ke= 0.500 Inlet / Outlet Invert= 216.50' / 216.00' S= 0.0313 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=2.56 cfs @ 12.09 hrs HW=217.47' TW=217.15' (Dynamic Tailwater)↑**1=Culvert** (Outlet Controls 2.56 cfs @ 3.43 fps)**Summary for Pond FB1: FOREBAY**

[43] Hint: Has no inflow (Outflow=Zero)

Volume	Invert	Avail.Storage	Storage Description
#1	214.00'	1,060 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
214.00	50	0	0
216.00	441	491	491
217.00	696	569	1,060

Summary for Subcatchment P1: PRE

Runoff = 0.01 cfs @ 21.37 hrs, Volume= 0.004 af, Depth= 0.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

Area (sf)	CN	Description
89,643	30	Woods, Good, HSG A
6,051	55	Woods, Good, HSG B
95,694	32	Weighted Average
95,694		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.8	100	0.0900	0.14		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
2.5	176	0.0570	1.19		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
14.3	276	Total			

Summary for Pond POST: WETLANDS

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 2.846 ac, 57.57% Impervious, Inflow Depth = 0.67" for 10-year event

Inflow = 1.26 cfs @ 12.32 hrs, Volume= 0.158 af

Primary = 1.26 cfs @ 12.33 hrs, Volume= 0.158 af, Atten= 0%, Lag= 0.6 min

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Summary for Subcatchment PP: Panda Palace

Runoff = 3.37 cfs @ 12.07 hrs, Volume= 0.264 af, Depth> 4.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

Area (sf)	CN	Description
* 29,109	98	Impervious
29,109		100.00% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment PP.: Panda Palace

Runoff = 3.37 cfs @ 12.07 hrs, Volume= 0.264 af, Depth> 4.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

Area (sf)	CN	Description
* 29,109	98	Impervious
29,109		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Pond PRE: DP-PRE

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 2.865 ac, 23.32% Impervious, Inflow Depth > 1.12" for 10-year event

Inflow = 3.37 cfs @ 12.07 hrs, Volume= 0.269 af

Primary = 3.37 cfs @ 12.08 hrs, Volume= 0.269 af, Atten= 0%, Lag= 0.6 min

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Summary for Subcatchment S1: CB1

Runoff = 0.70 cfs @ 12.07 hrs, Volume= 0.049 af, Depth= 3.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

Area (sf)	CN	Description
5,822	98	Paved parking, HSG A
1,252	39	>75% Grass cover, Good, HSG A
7,074	88	Weighted Average
1,252		17.70% Pervious Area
5,822		82.30% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

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Summary for Subcatchment S2: CB2

Runoff = 0.76 cfs @ 12.07 hrs, Volume= 0.052 af, Depth= 2.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

Area (sf)	CN	Description
6,618	98	Paved parking, HSG A
3,117	39	>75% Grass cover, Good, HSG A
9,735	79	Weighted Average
3,117		32.02% Pervious Area
6,618		67.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment S3: CB3

Runoff = 0.76 cfs @ 12.08 hrs, Volume= 0.053 af, Depth= 2.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

Area (sf)	CN	Description
7,570	98	Paved parking, HSG A
3,876	39	>75% Grass cover, Good, HSG A
2,239	30	Woods, Good, HSG A
13,685	70	Weighted Average
6,115		44.68% Pervious Area
7,570		55.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment S4: CB4

Runoff = 0.83 cfs @ 12.08 hrs, Volume= 0.057 af, Depth= 2.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

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Type III 24-hr 10-year Rainfall=4.98"

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Area (sf)	CN	Description
7,293	98	Paved parking, HSG A
3,515	39	>75% Grass cover, Good, HSG A
171	30	Woods, Good, HSG A
10,979	78	Weighted Average
3,686		33.57% Pervious Area
7,293		66.43% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment S5: PLAYGROUND

Runoff = 0.57 cfs @ 12.07 hrs, Volume= 0.045 af, Depth> 4.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

Area (sf)	CN	Description
4,941	98	Paved parking, HSG A
4,941		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment S6: BAS1

Runoff = 0.00 cfs @ 12.46 hrs, Volume= 0.002 af, Depth= 0.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

Area (sf)	CN	Description
4,912	39	>75% Grass cover, Good, HSG A
18	98	Paved parking, HSG A
4,930	39	Weighted Average
4,912		99.63% Pervious Area
18		0.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

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Summary for Subcatchment S7: DIRECT

Runoff = 0.03 cfs @ 12.73 hrs, Volume= 0.015 af, Depth= 0.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

Area (sf)	CN	Description
* 861	39	Disconnected Sidewalk, HSG A
7,246	30	Woods, Good, HSG A
4,092	55	Woods, Good, HSG B
19,366	39	>75% Grass cover, Good, HSG A
1,958	61	>75% Grass cover, Good, HSG B
33,523	40	Weighted Average
33,523		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	6	0.0200	0.78		Sheet Flow, IM
					Smooth surfaces n= 0.011 P2= 3.20"
1.8	22	0.0636	0.20		Sheet Flow, GR
					Grass: Short n= 0.150 P2= 3.20"
1.2	25	0.2400	0.35		Sheet Flow, GR
					Grass: Short n= 0.150 P2= 3.20"
1.4	20	0.1000	0.23		Sheet Flow, GR
					Grass: Short n= 0.150 P2= 3.20"
18.5	74	0.0160	0.07		Sheet Flow, W
					Woods: Light underbrush n= 0.400 P2= 3.20"
23.0	147	Total			

Summary for Subcatchment S8: ROOF

Runoff = 1.16 cfs @ 12.07 hrs, Volume= 0.091 af, Depth> 4.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=4.98"

Area (sf)	CN	Description
10,000	98	Roofs, HSG A
10,000		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Pond SUB1: DMH1/DMH2

[86] Warning: Oscillations may require smaller dt (severity=1057)

[80] Warning: Exceeded Pond CB2 by 0.10' @ 12.13 hrs (1.44 cfs 0.024 af)

Inflow Area = 1.054 ac, 90.49% Impervious, Inflow Depth > 4.16" for 10-year event
 Inflow = 4.83 cfs @ 12.08 hrs, Volume= 0.365 af
 Outflow = 1.53 cfs @ 12.38 hrs, Volume= 0.888 af, Atten= 68%, Lag= 17.6 min
 Discarded = 1.53 cfs @ 12.38 hrs, Volume= 0.888 af
 Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs
 Peak Elev= 216.39' @ 12.38 hrs Surf.Area= 2,264 sf Storage= 2,963 cf
 Flood Elev= 218.00' Surf.Area= 2,264 sf Storage= 5,012 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= (not calculated: outflow precedes inflow)

Volume	Invert	Avail.Storage	Storage Description
#1A	214.50'	2,066 cf	20.50'W x 110.42'L x 3.50'H Field A 7,922 cf Overall - 2,756 cf Embedded = 5,166 cf x 40.0% Voids
#2A	215.00'	2,756 cf	ADS_StormTech SC-740 +Cap x 60 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 60 Chambers in 4 Rows
#3	215.00'	89 cf	4.00'D x 7.12'H DMH1 -Impervious
#4	215.00'	101 cf	4.00'D x 8.05'H DMH2 -Impervious
#5	214.80'	89 cf	4.00'D x 7.10'H DMH3 -Impervious
#6	216.25'	74 cf	15.0" Round OUTLET PIPE -Impervious L= 60.0'
5,176 cf			Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	214.50'	15.000 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 212.50'
#2	Primary	213.20'	10.0" Round Culvert L= 120.0' Ke= 0.500 Inlet / Outlet Invert= 213.20' / 212.00' S= 0.0100 ' / Cc= 0.900 n= 0.012, Flow Area= 0.55 sf
#3	Device 2	217.60'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 2.3' Crest Height

Discarded OutFlow Max=1.53 cfs @ 12.38 hrs HW=216.39' (Free Discharge)

↑ **1=Exfiltration** (Controls 1.53 cfs)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=214.50' TW=0.00' (Dynamic Tailwater)

↑ **2=Culvert** (Passes 0.00 cfs of 2.39 cfs potential flow)

↑ **3=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

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Type III 24-hr 10-year Rainfall=4.98"

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Summary for Pond TD1: TRENCH DRAIN

Inflow Area = 0.113 ac, 100.00% Impervious, Inflow Depth > 4.74" for 10-year event
 Inflow = 0.57 cfs @ 12.07 hrs, Volume= 0.045 af
 Outflow = 0.57 cfs @ 12.08 hrs, Volume= 0.045 af, Atten= 0%, Lag= 0.6 min
 Primary = 0.57 cfs @ 12.08 hrs, Volume= 0.045 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 219.52' @ 12.08 hrs

Flood Elev= 222.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	218.90'	6.0" Round Culvert L= 15.0' Ke= 0.500 Inlet / Outlet Invert= 218.90' / 218.00' S= 0.0600 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=0.57 cfs @ 12.08 hrs HW=219.52' TW=217.10' (Dynamic Tailwater)↑**1=Culvert** (Inlet Controls 0.57 cfs @ 2.92 fps)

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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Sim-Route method - Pond routing by Sim-Route method

Pond AD: Area Drain

Peak Elev=219.62' Inflow=2.12 cfs 0.163 af
15.0" Round Culvert n=0.012 L=62.0' S=0.0100 '/' Outflow=2.12 cfs 0.163 af

Pond BAS1: BASIN

Peak Elev=216.86' Storage=2,918 cf Inflow=1.73 cfs 0.131 af
Discarded=0.04 cfs 0.095 af Primary=0.22 cfs 0.036 af Outflow=0.26 cfs 0.131 af

Pond CB1: CB1

Peak Elev=217.79' Inflow=0.39 cfs 0.027 af
15.0" Round Culvert n=0.012 L=107.0' S=0.0100 '/' Outflow=0.39 cfs 0.027 af

Pond CB2: CB2

Peak Elev=215.39' Inflow=0.35 cfs 0.024 af
15.0" Round Culvert n=0.012 L=3.0' S=0.0000 '/' Outflow=0.35 cfs 0.024 af

Pond CB3: CB3

Peak Elev=218.37' Inflow=1.00 cfs 0.077 af
15.0" Round Culvert n=0.012 L=63.0' S=0.0222 '/' Outflow=1.00 cfs 0.077 af

Pond CB4: CB4

Peak Elev=217.06' Inflow=1.37 cfs 0.103 af
15.0" Round Culvert n=0.012 L=16.0' S=0.0313 '/' Outflow=1.37 cfs 0.103 af

Pond FB1: FOREBAY

Peak Elev=0.00' Storage=0 cf

SubcatchmentP1: PRE

Runoff Area=95,694 sf 0.00% Impervious Runoff Depth=0.00"
Flow Length=276' Tc=14.3 min CN=32 Runoff=0.00 cfs 0.000 af

Pond POST: WETLANDS

Inflow=0.22 cfs 0.036 af
Primary=0.22 cfs 0.036 af

SubcatchmentPP: Panda Palace

Runoff Area=29,109 sf 100.00% Impervious Runoff Depth=2.93"
Tc=5.0 min CN=98 Runoff=2.12 cfs 0.163 af

SubcatchmentPP.: Panda Palace

Runoff Area=29,109 sf 100.00% Impervious Runoff Depth=2.93"
Tc=5.0 min CN=98 Runoff=2.12 cfs 0.163 af

Pond PRE: DP-PRE

Inflow=2.12 cfs 0.163 af
Primary=2.12 cfs 0.163 af

SubcatchmentS1: CB1

Runoff Area=7,074 sf 82.30% Impervious Runoff Depth=1.96"
Tc=5.0 min CN=88 Runoff=0.39 cfs 0.027 af

SubcatchmentS2: CB2

Runoff Area=9,735 sf 67.98% Impervious Runoff Depth=1.31"
Tc=5.0 min CN=79 Runoff=0.35 cfs 0.024 af

SubcatchmentS3: CB3

Runoff Area=13,685 sf 55.32% Impervious Runoff Depth=0.80"
Tc=5.0 min CN=70 Runoff=0.27 cfs 0.021 af

SubcatchmentS4: CB4

Runoff Area=10,979 sf 66.43% Impervious Runoff Depth=1.24"
Tc=5.0 min CN=78 Runoff=0.37 cfs 0.026 af

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SubcatchmentS5: PLAYGROUNDRunoff Area=4,941 sf 100.00% Impervious Runoff Depth=2.93"
Tc=5.0 min CN=98 Runoff=0.36 cfs 0.028 af**SubcatchmentS6: BAS1**Runoff Area=4,930 sf 0.37% Impervious Runoff Depth=0.00"
Tc=5.0 min CN=39 Runoff=0.00 cfs 0.000 af**SubcatchmentS7: DIRECT**Runoff Area=33,523 sf 0.00% Impervious Runoff Depth=0.00"
Flow Length=147' Tc=23.0 min CN=40 Runoff=0.00 cfs 0.000 af**SubcatchmentS8: ROOF**Runoff Area=10,000 sf 100.00% Impervious Runoff Depth=2.93"
Tc=5.0 min CN=98 Runoff=0.73 cfs 0.056 af**Pond SUB1: DMH1/DMH2**Peak Elev=215.38' Storage=1,174 cf Inflow=2.86 cfs 0.214 af
Discarded=1.13 cfs 0.807 af Primary=0.00 cfs 0.000 af Outflow=1.13 cfs 0.807 af**Pond TD1: TRENCH DRAIN**Peak Elev=219.30' Inflow=0.36 cfs 0.028 af
6.0" Round Culvert n=0.012 L=15.0' S=0.0600 '/' Outflow=0.36 cfs 0.028 af**Total Runoff Area = 5.711 ac Runoff Volume = 0.508 af Average Runoff Depth = 1.07"**
59.61% Pervious = 3.404 ac 40.39% Impervious = 2.307 ac

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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Sim-Route method - Pond routing by Sim-Route method

Pond AD: Area Drain

Peak Elev=219.86' Inflow=3.37 cfs 0.264 af
15.0" Round Culvert n=0.012 L=62.0' S=0.0100 '/' Outflow=3.37 cfs 0.264 af

Pond BAS1: BASIN

Peak Elev=217.50' Storage=4,200 cf Inflow=3.30 cfs 0.247 af
Discarded=0.05 cfs 0.103 af Primary=1.26 cfs 0.144 af Outflow=1.31 cfs 0.247 af

Pond CB1: CB1

Peak Elev=217.89' Inflow=0.70 cfs 0.049 af
15.0" Round Culvert n=0.012 L=107.0' S=0.0100 '/' Outflow=0.70 cfs 0.049 af

Pond CB2: CB2

Peak Elev=216.39' Inflow=0.76 cfs 0.052 af
15.0" Round Culvert n=0.012 L=3.0' S=0.0000 '/' Outflow=0.76 cfs 0.052 af

Pond CB3: CB3

Peak Elev=218.58' Inflow=1.91 cfs 0.144 af
15.0" Round Culvert n=0.012 L=63.0' S=0.0222 '/' Outflow=1.91 cfs 0.144 af

Pond CB4: CB4

Peak Elev=217.55' Inflow=2.74 cfs 0.200 af
15.0" Round Culvert n=0.012 L=16.0' S=0.0313 '/' Outflow=2.74 cfs 0.200 af

Pond FB1: FOREBAY

Peak Elev=0.00' Storage=0 cf

SubcatchmentP1: PRE

Runoff Area=95,694 sf 0.00% Impervious Runoff Depth=0.02"
Flow Length=276' Tc=14.3 min CN=32 Runoff=0.01 cfs 0.004 af

Pond POST: WETLANDS

Inflow=1.26 cfs 0.158 af
Primary=1.26 cfs 0.158 af

SubcatchmentPP: Panda Palace

Runoff Area=29,109 sf 100.00% Impervious Runoff Depth>4.74"
Tc=5.0 min CN=98 Runoff=3.37 cfs 0.264 af

SubcatchmentPP.: Panda Palace

Runoff Area=29,109 sf 100.00% Impervious Runoff Depth>4.74"
Tc=5.0 min CN=98 Runoff=3.37 cfs 0.264 af

Pond PRE: DP-PRE

Inflow=3.37 cfs 0.269 af
Primary=3.37 cfs 0.269 af

SubcatchmentS1: CB1

Runoff Area=7,074 sf 82.30% Impervious Runoff Depth=3.65"
Tc=5.0 min CN=88 Runoff=0.70 cfs 0.049 af

SubcatchmentS2: CB2

Runoff Area=9,735 sf 67.98% Impervious Runoff Depth=2.78"
Tc=5.0 min CN=79 Runoff=0.76 cfs 0.052 af

SubcatchmentS3: CB3

Runoff Area=13,685 sf 55.32% Impervious Runoff Depth=2.02"
Tc=5.0 min CN=70 Runoff=0.76 cfs 0.053 af

SubcatchmentS4: CB4

Runoff Area=10,979 sf 66.43% Impervious Runoff Depth=2.69"
Tc=5.0 min CN=78 Runoff=0.83 cfs 0.057 af

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SubcatchmentS5: PLAYGROUNDRunoff Area=4,941 sf 100.00% Impervious Runoff Depth>4.74"
Tc=5.0 min CN=98 Runoff=0.57 cfs 0.045 af**SubcatchmentS6: BAS1**Runoff Area=4,930 sf 0.37% Impervious Runoff Depth=0.20"
Tc=5.0 min CN=39 Runoff=0.00 cfs 0.002 af**SubcatchmentS7: DIRECT**Runoff Area=33,523 sf 0.00% Impervious Runoff Depth=0.23"
Flow Length=147' Tc=23.0 min CN=40 Runoff=0.03 cfs 0.015 af**SubcatchmentS8: ROOF**Runoff Area=10,000 sf 100.00% Impervious Runoff Depth>4.74"
Tc=5.0 min CN=98 Runoff=1.16 cfs 0.091 af**Pond SUB1: DMH1/DMH2**Peak Elev=216.39' Storage=2,963 cf Inflow=4.83 cfs 0.365 af
Discarded=1.53 cfs 0.888 af Primary=0.00 cfs 0.000 af Outflow=1.53 cfs 0.888 af**Pond TD1: TRENCH DRAIN**Peak Elev=219.52' Inflow=0.57 cfs 0.045 af
6.0" Round Culvert n=0.012 L=15.0' S=0.0600 '/' Outflow=0.57 cfs 0.045 af**Total Runoff Area = 5.711 ac Runoff Volume = 0.896 af Average Runoff Depth = 1.88"**
59.61% Pervious = 3.404 ac 40.39% Impervious = 2.307 ac

2021-083 Vernon TLE-B

Prepared by J.R. Russo & Associates LLC

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Type III 24-hr 25-year Rainfall=6.12"

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Page 16

Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Sim-Route method - Pond routing by Sim-Route method

Pond AD: Area Drain

Peak Elev=220.01' Inflow=4.15 cfs 0.327 af
15.0" Round Culvert n=0.012 L=62.0' S=0.0100 '/ Outflow=4.15 cfs 0.327 af

Pond BAS1: BASIN

Peak Elev=217.92' Storage=5,201 cf Inflow=4.36 cfs 0.326 af
Discarded=0.06 cfs 0.107 af Primary=1.67 cfs 0.219 af Outflow=1.73 cfs 0.326 af

Pond CB1: CB1

Peak Elev=217.95' Inflow=0.90 cfs 0.064 af
15.0" Round Culvert n=0.012 L=107.0' S=0.0100 '/ Outflow=0.90 cfs 0.064 af

Pond CB2: CB2

Peak Elev=217.16' Inflow=1.03 cfs 0.071 af
15.0" Round Culvert n=0.012 L=3.0' S=0.0000 '/ Outflow=1.03 cfs 0.071 af

Pond CB3: CB3

Peak Elev=218.70' Inflow=2.53 cfs 0.188 af
15.0" Round Culvert n=0.012 L=63.0' S=0.0222 '/ Outflow=2.53 cfs 0.188 af

Pond CB4: CB4

Peak Elev=217.98' Inflow=3.65 cfs 0.266 af
15.0" Round Culvert n=0.012 L=16.0' S=0.0313 '/ Outflow=3.65 cfs 0.266 af

Pond FB1: FOREBAY

Peak Elev=0.00' Storage=0 cf

SubcatchmentP1: PRE

Runoff Area=95,694 sf 0.00% Impervious Runoff Depth=0.15"
Flow Length=276' Tc=14.3 min CN=32 Runoff=0.04 cfs 0.028 af

Pond POST: WETLANDS

Inflow=1.77 cfs 0.254 af
Primary=1.77 cfs 0.254 af

SubcatchmentPP: Panda Palace

Runoff Area=29,109 sf 100.00% Impervious Runoff Depth>5.88"
Tc=5.0 min CN=98 Runoff=4.15 cfs 0.327 af

SubcatchmentPP.: Panda Palace

Runoff Area=29,109 sf 100.00% Impervious Runoff Depth>5.88"
Tc=5.0 min CN=98 Runoff=4.15 cfs 0.327 af

Pond PRE: DP-PRE

Inflow=4.15 cfs 0.355 af
Primary=4.15 cfs 0.355 af

SubcatchmentS1: CB1

Runoff Area=7,074 sf 82.30% Impervious Runoff Depth=4.74"
Tc=5.0 min CN=88 Runoff=0.90 cfs 0.064 af

SubcatchmentS2: CB2

Runoff Area=9,735 sf 67.98% Impervious Runoff Depth=3.79"
Tc=5.0 min CN=79 Runoff=1.03 cfs 0.071 af

SubcatchmentS3: CB3

Runoff Area=13,685 sf 55.32% Impervious Runoff Depth=2.90"
Tc=5.0 min CN=70 Runoff=1.10 cfs 0.076 af

SubcatchmentS4: CB4

Runoff Area=10,979 sf 66.43% Impervious Runoff Depth=3.69"
Tc=5.0 min CN=78 Runoff=1.13 cfs 0.077 af

2021-083 Vernon TLE-B

Prepared by J.R. Russo & Associates LLC

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Type III 24-hr 25-year Rainfall=6.12"

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SubcatchmentS5: PLAYGROUNDRunoff Area=4,941 sf 100.00% Impervious Runoff Depth>5.88"
Tc=5.0 min CN=98 Runoff=0.70 cfs 0.056 af**SubcatchmentS6: BAS1**Runoff Area=4,930 sf 0.37% Impervious Runoff Depth=0.48"
Tc=5.0 min CN=39 Runoff=0.02 cfs 0.005 af**SubcatchmentS7: DIRECT**Runoff Area=33,523 sf 0.00% Impervious Runoff Depth=0.54"
Flow Length=147' Tc=23.0 min CN=40 Runoff=0.15 cfs 0.034 af**SubcatchmentS8: ROOF**Runoff Area=10,000 sf 100.00% Impervious Runoff Depth>5.88"
Tc=5.0 min CN=98 Runoff=1.43 cfs 0.113 af**Pond SUB1: DMH1/DMH2**Peak Elev=217.16' Storage=4,155 cf Inflow=6.08 cfs 0.462 af
Discarded=1.83 cfs 0.941 af Primary=0.00 cfs 0.000 af Outflow=1.83 cfs 0.941 af**Pond TD1: TRENCH DRAIN**Peak Elev=219.71' Inflow=0.70 cfs 0.056 af
6.0" Round Culvert n=0.012 L=15.0' S=0.0600 ' / ' Outflow=0.70 cfs 0.056 af**Total Runoff Area = 5.711 ac Runoff Volume = 1.178 af Average Runoff Depth = 2.47"**
59.61% Pervious = 3.404 ac 40.39% Impervious = 2.307 ac

2021-083 Vernon TLE-B*Type III 24-hr 100-year Rainfall=7.87"*

Prepared by J.R. Russo & Associates LLC

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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Sim-Route method - Pond routing by Sim-Route method

Pond AD: Area Drain

Peak Elev=220.34' Inflow=5.35 cfs 0.425 af
 15.0" Round Culvert n=0.012 L=62.0' S=0.0100 '/ Outflow=5.35 cfs 0.425 af

Pond BAS1: BASIN

Peak Elev=218.48' Storage=6,719 cf Inflow=6.08 cfs 0.453 af
 Discarded=0.07 cfs 0.112 af Primary=2.32 cfs 0.341 af Outflow=2.38 cfs 0.453 af

Pond CB1: CB1

Peak Elev=218.13' Inflow=1.21 cfs 0.087 af
 15.0" Round Culvert n=0.012 L=107.0' S=0.0100 '/ Outflow=1.21 cfs 0.087 af

Pond CB2: CB2

Peak Elev=217.97' Inflow=1.45 cfs 0.100 af
 15.0" Round Culvert n=0.012 L=3.0' S=0.0000 '/ Outflow=1.45 cfs 0.100 af

Pond CB3: CB3

Peak Elev=219.13' Inflow=3.50 cfs 0.260 af
 15.0" Round Culvert n=0.012 L=63.0' S=0.0222 '/ Outflow=3.50 cfs 0.260 af

Pond CB4: CB4

Peak Elev=218.73' Inflow=5.09 cfs 0.371 af
 15.0" Round Culvert n=0.012 L=16.0' S=0.0313 '/ Outflow=5.09 cfs 0.371 af

Pond FB1: FOREBAY

Peak Elev=0.00' Storage=0 cf

SubcatchmentP1: PRE

Runoff Area=95,694 sf 0.00% Impervious Runoff Depth=0.53"
 Flow Length=276' Tc=14.3 min CN=32 Runoff=0.38 cfs 0.096 af

Pond POST: WETLANDS

Inflow=4.93 cfs 0.451 af
 Primary=4.93 cfs 0.451 af

SubcatchmentPP: Panda Palace

Runoff Area=29,109 sf 100.00% Impervious Runoff Depth>7.63"
 Tc=5.0 min CN=98 Runoff=5.35 cfs 0.425 af

SubcatchmentPP.: Panda Palace

Runoff Area=29,109 sf 100.00% Impervious Runoff Depth>7.63"
 Tc=5.0 min CN=98 Runoff=5.35 cfs 0.425 af

Pond PRE: DP-PRE

Inflow=5.35 cfs 0.521 af
 Primary=5.35 cfs 0.521 af

SubcatchmentS1: CB1

Runoff Area=7,074 sf 82.30% Impervious Runoff Depth=6.44"
 Tc=5.0 min CN=88 Runoff=1.21 cfs 0.087 af

SubcatchmentS2: CB2

Runoff Area=9,735 sf 67.98% Impervious Runoff Depth=5.39"
 Tc=5.0 min CN=79 Runoff=1.45 cfs 0.100 af

SubcatchmentS3: CB3

Runoff Area=13,685 sf 55.32% Impervious Runoff Depth=4.35"
 Tc=5.0 min CN=70 Runoff=1.66 cfs 0.114 af

SubcatchmentS4: CB4

Runoff Area=10,979 sf 66.43% Impervious Runoff Depth=5.27"
 Tc=5.0 min CN=78 Runoff=1.60 cfs 0.111 af

2021-083 Vernon TLE-B*Type III 24-hr 100-year Rainfall=7.87"*

Prepared by J.R. Russo & Associates LLC

Printed 3/23/2022

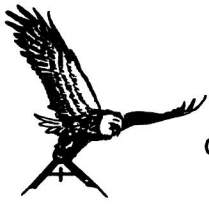
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SubcatchmentS5: PLAYGROUNDRunoff Area=4,941 sf 100.00% Impervious Runoff Depth>7.63"
Tc=5.0 min CN=98 Runoff=0.91 cfs 0.072 af**SubcatchmentS6: BAS1**Runoff Area=4,930 sf 0.37% Impervious Runoff Depth=1.10"
Tc=5.0 min CN=39 Runoff=0.10 cfs 0.010 af**SubcatchmentS7: DIRECT**Runoff Area=33,523 sf 0.00% Impervious Runoff Depth=1.19"
Flow Length=147' Tc=23.0 min CN=40 Runoff=0.48 cfs 0.077 af**SubcatchmentS8: ROOF**Runoff Area=10,000 sf 100.00% Impervious Runoff Depth>7.63"
Tc=5.0 min CN=98 Runoff=1.84 cfs 0.146 af**Pond SUB1: DMH1/DMH2**Peak Elev=217.95' Storage=4,966 cf Inflow=8.00 cfs 0.612 af
Discarded=2.14 cfs 1.002 af Primary=2.73 cfs 0.033 af Outflow=4.87 cfs 1.035 af**Pond TD1: TRENCH DRAIN**Peak Elev=220.07' Inflow=0.91 cfs 0.072 af
6.0" Round Culvert n=0.012 L=15.0' S=0.0600 ' / ' Outflow=0.91 cfs 0.072 af**Total Runoff Area = 5.711 ac Runoff Volume = 1.663 af Average Runoff Depth = 3.49"**
59.61% Pervious = 3.404 ac 40.39% Impervious = 2.307 ac

Appendix 6:

MISCELLANEOUS CALCULATIONS



Water Quality Volume (WQV) Calculations

$$WQV = (1'')RA/12$$
$$R = 0.05 + 0.009I$$

I = percent impervious coverage
R = volumetric runoff coefficient
A = contributing area

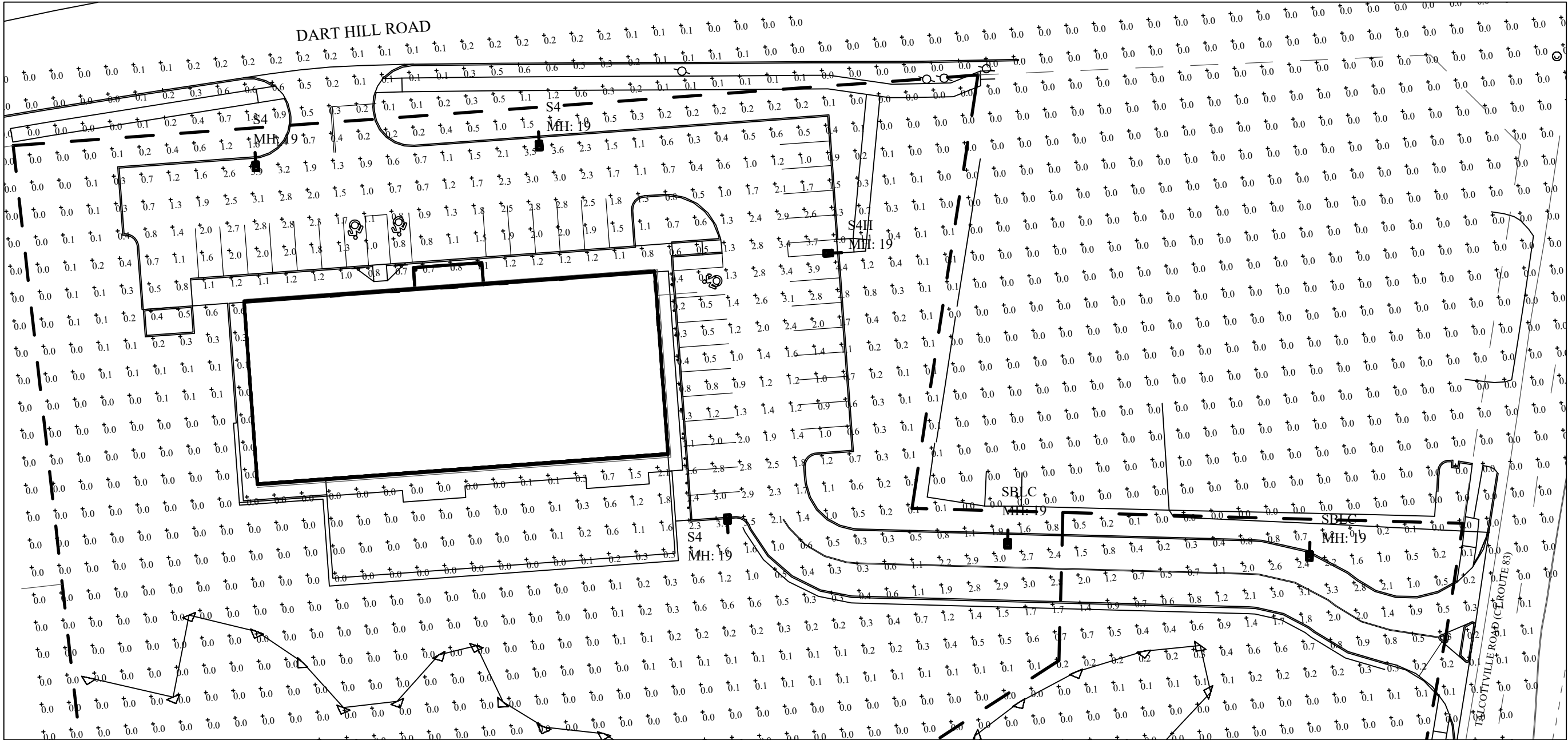
① Basin: $A = 44,535 \text{ s.f.}$
 $I = \frac{29,822 \text{ s.f. impervious}}{44,535 \text{ s.f. total}} = 66.96\%$
 $R = 0.05 + 0.009(66.96) = 0.653$
 $WQV = (1'')(0.653)(44,535)/12 = \boxed{2422 \text{ c.f.}}$

$$\text{Forebay} = 25\%(WQV) = 0.25(2422) = \boxed{606 \text{ c.f.}}$$




② Subsurface Chambers: $A = 45,918 \text{ s.f.}$
 $I = \frac{41,549 \text{ s.f. impervious}}{45,918 \text{ s.f. total}} = 90.49\%$
 $R = 0.05 + 0.009(90.49) = 0.864$
 $WQV = (1'')(0.864)(45,918)/12 = \boxed{3,308 \text{ c.f.}}$

WQV Check:

Basin Volume below outlet = 2,469 c.f.
Forebay Volume = 1,060 c.f.
Subsurface volume below outlet = 4,635 c.f.



Filename: 501 Talcottville Road Site Lighting - Vernon - REV 1 ALT 2.AGI

Luminaire Schedule									
Symbol	Qty	Label	Arrangement	Luminaire Lumens	Luminaire Watts	LLF	BUG Rating	Mounting Height	Description
	2	SBLC	Single	7293	70	0.900	B1-U0-G2	19	Lithonia DSX1 LED P2 40K BLC MVOLT SPA DBLXD - SSS 18 4C DM19A DBLXD 18FT POLE on 1FT BASE
	3	S4	Single	12574	102	0.900	B2-U0-G3	19	Lithonia DSX1 LED P3 40K TFTM MVOLT SPA DBLXD - SSS 18 4C DM19A DBLXD 18FT POLE on 1FT BASE
	1	S4H	Single	11312	125	0.900	B2-U0-G2	19	Lithonia DSX1 LED P4 40K TFTM MVOLT SPA HS DBLXD - SSS 18 4C DM19AS DBLXD 18FT POLE on 1FT BASE

Calculation Summary							
Label	CalcType	Units	Avg	Max	Min	Avg/Min	Max/Min
SITE	Illuminance	Fc	0.34	4.4	0.0	N.A.	N.A.
NEW DRIVEWAY	Illuminance	Fc	1.32	3.3	0.2	6.60	16.50
PARKING	Illuminance	Fc	1.59	4.4	0.2	7.95	22.00

Greg Loda
Lighting Affiliates
1208 Cromwell Ave
Rocky Hill, CT 06067

PHOTOMETRIC PLAN

The Learning Experience

501 Talcottville Road
Vernon, Connecticut



RUSO
SURVEYORS • ENGINEERS
SERVING CT & MA

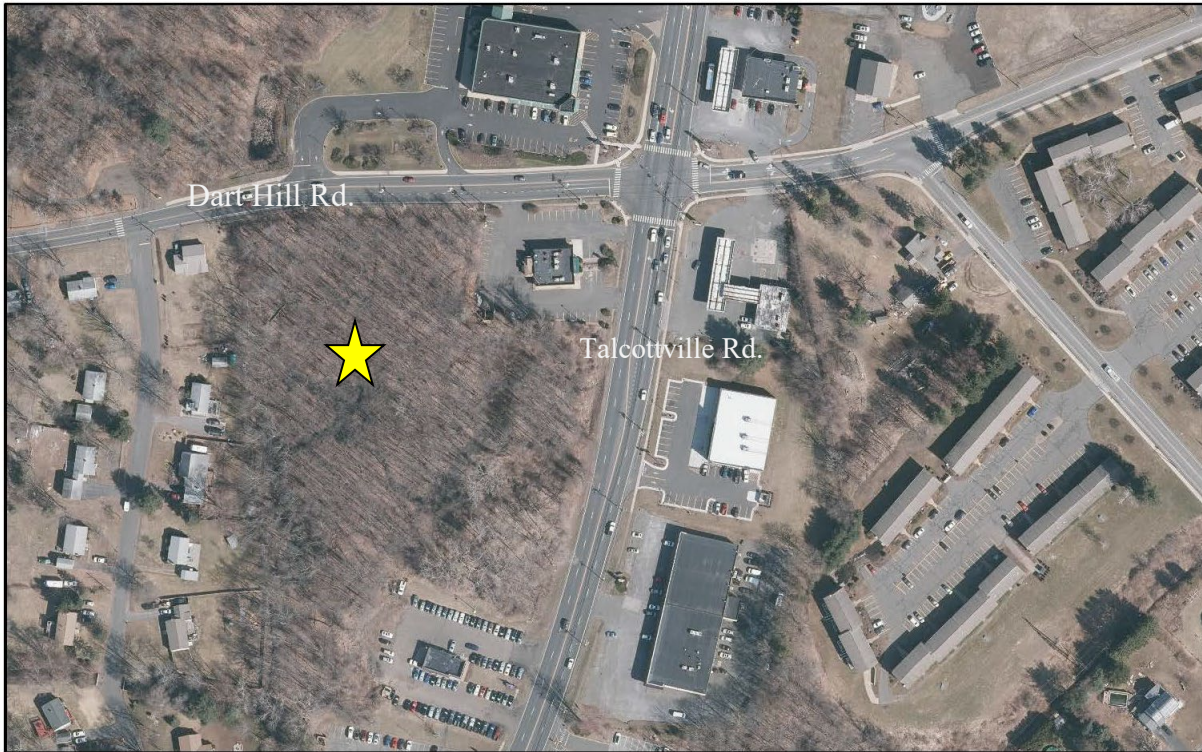
J.R. Russo & Associates, LLC
1 Shoham Rd East Windsor, CT 06088 • CT 860.623.0599 • MA 413.785.1558
www.jrusso.com • info@jrusso.com

DATE
3-28-22

SCALE
1"=40'

JOB NUMBER
2021-083

SHEET
PH-1 (ALTERNATIVE 2)



Site Location

Summary

The Applicant proposes to construct an approximately 10,000 sq. ft. day care facility, with a 5,000 sq. ft. play area, at 501 Talcottville Rd. Although addressed via parcel number on Talcottville Rd., the access to the site is proposed on Dart Hill Rd., with a proposed drive-way entrance across from the westernmost Walgreens entrance. Wetlands is present on the site. The development constitutes approximately .82 acres of disturbance in a regulated area. The Applicant has submitted a narrative, a site plan set, drainage plan, landscaping plan, traffic study, and architectural elevations, all included in the agenda packet.



Concurrent Review

Inland Wetlands Commission. The Applicant is processing a simultaneous Inland Wetlands Application for a re-designation of wetlands and a wetland permit. This application began review at the IWC on February 22; it was continued to the March 22, 2022 meeting. A decision of the IWC application must take place before the Planning & Zoning Commission can act.

Planning & Zoning Commission. In addition, the property owner has filed a re-subdivision application (PZ 2022-06) to create a new, 2.6-acre lot out of the parent 4.6-acre tract, oriented toward Talcottville Rd.

Traffic Authority. The Application has been referred to the Vernon Traffic Authority. Unfortunately, the Authority could not achieve a quorum on February 10; it has been rescheduled for their next regular meeting on March 10, 2022.

Design Review Commission. The advisory Design Review Commission will take up the application's design elements on March 2, 2022.

Issues

Staff has identified two primary issues related to the site plan and special permit application: Traffic and access to the property and site development wetland impacts.

Traffic Impacts. Staff has raised a number of concerns related to access from Dart Hill Rd.: safe turning movements across multi-lanes, internal and external queuing conflicts, among other traffic concerns, in a congested part of the Vernon community¹. The Applicant has responded to the traffic points; the email is attached to this memorandum. The Town Engineer, along with the Traffic Authority, will continue to review the responses and the issue. Staff notes that the Town Engineer had the occasion to observe site conditions on Tuesday, February 22, where he found the vehicle queue eastbound on Dart Hill Rd. extended to the Dart Hill Rd. bridge, well beyond the proposed driveway entrance. This appears to conflict with the Applicant's traffic study modeling. The Town Engineer is preparing formal comments under separate cover.

Site Impacts on the Wetland areas. The Inland Wetlands Commission is reviewing site impacts on the designated wetlands. The site development abuts quite closely to the established wetlands area and includes a retaining wall and significant fill. The Town Engineer and Town Wetlands agent continue to review the matter. The IWC has directed Staff to engage a third-party, wetland professional to review the technical documentation. In addition, the North Central Conservation District, which provides statutory review of erosion and sediment control plans, has identified several outstanding issues and is working directly with the applicant to resolve. This referral will also come under separate cover.

Other. The Zoning Enforcement Official finds that the site plan and special permits appear to meet the site plan and zoning district regulations. The Health Department has provided a referral related to specific administrative health activities for childcare facilities. It is attached to the Staff memorandum.

¹ Town Planner letter to applicant, February 10, 2022

UPDATE FOR April 21, 2022

At the March 17, 2022 meeting, the applicant requested that the Commission NOT open the public hearing and instead move it to April 7, 2022. A subsequent request by the Applicant pushed the hearing start to April 21, 2022.

Adequate extensions have been provided by the Applicant (currently to May 5, 2022, with 26 of the 65 extension days exhausted).

The Applicant has submitted site plan (and drainage) revisions to the PZC and to the IWC for review. The revisions include a new access drive out to Route 83 (as a right in/right out movement) and a reduction in parking spaces in order to move the development away (towards Dart Hill Rd.) from the wetland areas by approximately 25 feet. Staff continues its review.

Two plan sets are included in the packet-alternative 1 (revised to include new access drive) and alternative 2 (revised to include access drive and reduction in parking).

Two additional emails from a neighbor have also been received (attached to staff memo).

Other Commission Activities

The Third-part review requested by the **Inland Wetlands Commission** is expected to be discussed by the IWC at a special meeting on May 3, 2022.

The Traffic Authority, which failed to achieve a quorum in March, will review the Application on April 14. Staff will report those findings on April 21, 2022.

The Applicant has filed a variance request with the **Zoning Board of Appeals** related to the parking reduction mentioned above. This will be heard on April 20, 2022.

Due to the outstanding items and ongoing review by other Commissions, Staff recommends a continuation of the public hearing.

From: [Kelley, John](#)
To: [Ryniewicz, Dwight](#); [Purcaro, Michael](#); [McGregor, George](#); [Gately, Shaun](#); [Smith, David](#); [Perry, Craig](#); [Boucher, Joseph](#); [Gallant, Lucas](#); [Wasilewski, Daniel](#)
Cc: [Wheelock, Diane](#); [Schambach, Jeff](#); [Carlson, Anne-Marie](#)
Subject: RE: Vernon, CT - Information Request
Date: Thursday, December 23, 2021 10:35:37 AM

Tai,

In September 2021 I requested State DOT look at the left turn arrow timing at the intersection of Route 83 and Regan Road. Traffic backs up on Route 83 intending to turn left onto Regan Road due to vehicles leaving Rockville High School. This occurs in the morning after students are dropped off and in the afternoon. I have also received complaints regarding left turn arrows not being long enough for Regan Road traffic intending to turn left onto Route 83. I have not heard back from DOT yet.

John Kelley
Chief of Police
Vernon Police Department
725 Hartford Turnpike
Vernon, CT 06066
jkelly@vernon-ct.gov
(860)872-9126

From: Ryniewicz, Dwight
Sent: Wednesday, December 22, 2021 10:54 AM
To: Purcaro, Michael <mpurcaro@vernon-ct.gov>; McGregor, George <GMcGregor@vernon-ct.gov>; Gately, Shaun <sgately@vernon-ct.gov>; Smith, David <dsmith@vernon-ct.gov>; Perry, Craig <cperry@vernon-ct.gov>; Boucher, Joseph <JBoucher@vernon-ct.gov>; Kelley, John <jkelly@vernon-ct.gov>; Gallant, Lucas <lgallant@vernon-ct.gov>; Wasilewski, Daniel <DWasilewski@vernon-ct.gov>
Cc: Wheelock, Diane <dwheelock@vernon-ct.gov>; Schambach, Jeff <jschambach@vernon-ct.gov>; Carlson, Anne-Marie <acarlson@vernon-ct.gov>
Subject: FW: Vernon, CT - Information Request

Good morning.
Please see email below. I have not responded.

Dwight

From: Tai Le <TLe@fando.com>
Sent: Wednesday, December 22, 2021 8:59 AM
To: Ryniewicz, Dwight <dryniewicz@vernon-ct.gov>
Subject: [EXTERNAL] Vernon, CT - Information Request

CAUTION: This Email is from an EXTERNAL source. Ensure you trust this sender before clicking on any links or attachments.

Good morning Dwight,

I am currently working on a traffic impact study for a proposed daycare center at 501 Talcottville Road (State Route 83) located in Vernon, Connecticut. The existing site is currently vacant, and the proposed development is planned to range from 10,000 to 12,000 square feet with up to 57 supporting parking spaces. Site access to the property will be provided via one full access driveway on Dart Hill Road directly across from the existing Walgreens full access driveway.

I would like to inquire as to whether or not the Town of Vernon has knowledge of any upcoming roadway modifications planned within the study area, signal timing/rephrasing, change in lane arrangement, or anything of that nature?

Please see the following link for a map of the location: <https://maphub.net/tle/501-Talcottville-Road>

Thank you,

Tai



Tai Le, EIT

Transportation Engineer

Fuss & O'Neill, Inc. | 146 Hartford Road | Manchester, CT 06040

860.646.2469 x5291 | tle@fando.com

www.fando.com | [twitter](#) | [facebook](#) | [linkedin](#)



TOWN OF VERNON

Office of the Town Planner
14 Park Place • Vernon, CT 06066
Tel: 860-870-3640 • www.Vernon-ct.gov
gmcgregor@vernon-ct.gov

George McGregor, AICP
Town Planner

February 10, 2022

VIA Email

Tim Coon
Project Engineer
J.R. Russo & Associates, LLC
PO Box 938
East Windsor, CT 06088

Re: PZ 2022-05, 501 Talcottville Rd. Town Planner Comments

Dear Mr. Coon:

With regard to the above referenced application, I offer the following comments and questions.

General Site

1. The Wetlands Re-designation Plan is included as Sheet 2. The final version set does not need to include this sheet.
2. Please provide both a fence detail and a dumpster surround detail in the plan set.
3. Please provide (via email) color elevations; they will be helpful during the Design Review Commission meeting.
4. The commercial entrance is shown at 30' wide, allowing for one lane in and one lane out. This may cause internal queuing issues. Dependent upon the outcome of our traffic discussions (in detail below), please consider potentially separate right out and left out scenarios.

Traffic

5. A most significant challenging facing the development of this site is providing safe and efficient vehicular access to the property. The biggest concern is the clear potential for conflicts when cars drop off or pick up for day care: they must cross two lanes of traffic on Dart Hill Rd. Often during peak hours that traffic may queue (at least anecdotally) in front of the property causing delay and introducing unsafe turning movements. Bottom line: can facility customers enter and exit the site safely during peak a.m. and p.m.? How does the applicant plan to mitigate this potential conflict?
6. The traffic study does attempt to address the intersection of 83 and Dart Hill Rd. which will continue to operate at a level of Service F, unless some unspecified "optimization" takes place in an out year beyond 2025. Please explain how "optimization" will work, the timing therein, who will be responsible and who will pay for the optimization? The traffic study suggests that even with optimization, certain directional queues will get longer.

7. Perhaps, there are mitigation measures or options for alternative access than can be considered.

- a. A second point of access to Route 83
- b. Right in-Right out limitations on Dart Hill and/or Route 83
- c. Commitment to expedite, finance, and complete optimization measures.

8. Staff plans to observe site conditions (especially the dart Hill Rd. queuing scenario) during the a.m. and p.m. peaks and will report these observations to the Planning & Zoning Commission at the time of the public hearing.

With Regards,



George K. McGregor, AICP



CENTRAL CONSERVATION DISTRICT, INC.

AVON • BLOOMFIELD • BOLTON • BRISTOL • BURLINGTON • CANTON • COVENTRY • EAST GRANBY • EAST WINDSOR • EAST HARTFORD • ELLINGTON
ENFIELD • FARMINGTON • GLASTONBURY • GRANBY • HARTFORD • MANCHESTER • PLAINVILLE • SIMSBURY • SOMERS • SOUTH WINDSOR
STAFFORD • SUFFIELD • WEST HARTFORD • WETHERSFIELD • TOLLAND • VERNON • WILLINGTON • WINDSOR • WINDSOR LOCKS

Date: March 1, 2022

To: George K. McGregor, AICP, Town Planner
Town of Vernon Planning & Zoning Commission

From: Barbara Kelly, Program Coordinator, Registered Professional Soil Scientist, SSSSNE
Certified Erosion Control Professional CPESC #2180

Re: E&S Plan Review – The Learning Experience, 501 Talcottville Road, Vernon, Connecticut

This review is conducted pursuant to Section 18 of the Town of Vernon Zoning Regulations. The review is limited to certification of the erosion control plan, based on compliance with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control (Guidelines).

District staff inspected the site on February 22, 2022. And reviewed a 12-sheet plan titled "The Learning Experience, Property of 501 Talcottville Road, LLC" (Plan), prepared by J.R. Russo & Associates, LLC and revised 2-14-22. Following a discussion with the Project Engineer on February 28, 2022, a *Grading & Erosion & Sediment Control* plan sheet and an *Erosion & Sediment Control* plan sheet with revision dates of 3-1-22 were reviewed.

The Plan proposes the construction of a retaining wall, a 10,000 square foot building, a 4,500 square foot playground, associated parking, and a stormwater management basin on the northern two acres of a 4.6 acre lot located at 501 Talcottville Road. The proposed development necessitates up to ten feet of fill. Post development, stormwater will be discharged at grade into wetlands located immediately south of the proposed development.

Background & Observations

Based on the Natural Resources Conservation Service Web Soil Survey, soils throughout the majority of the proposed construction area are mapped as somewhat excessively drained Hartford sandy loam on 3 to 8 percent slopes. The erosion hazard of this soil is considered moderate. The topography of the site will tend to direct any sediment laden stormwater toward the on-site wetlands. The Plan's "Soil Erosion and Sediment Control Notes" address dust control, vehicle tracking of sediments, and the maintenance of E&S measures.

Comment

Based on the observed site conditions, with proper implementation and maintenance, the soil erosion and sediment control measures shown in the Plan updated on 3-1-22 are adequate and appropriate. The District certifies that the Plan complies with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control.

Thank you for the opportunity to comment.



North Central District Health Department

□ Enfield—31 North Main Street, Enfield, CT 06082 * (860) 745-0383 Fax (860) 745-3188

□ Vernon—375 Hartford Turnpike, Room 120, Vernon, CT 06066 * (860) 872-1501 Fax (860) 872 1531

□ Windham—Town Hall, 979 Main Street, Willimantic, CT 06226 * (860) 465-3033 Fax (860) 465-3034

□ Stafford—Town Hall, 1 Main Street, Stafford Springs, CT 06076 * (860) 684-5609 Fax (860) 684-1768

Patrice A. Sulik, MPH, R.S.
Director of Health

February 22, 2022

Mr. George McGregor
Vernon Town Planner
55 West Main Street
Vernon, Connecticut 06066

Re: Special Permit – The Learning Experience
501 Talcottville Road
Vernon, Connecticut

Dear Mr. George McGregor:

I am writing regarding the Special Permit Application review for the Learning Experience at the above referenced address.

North Central District Health Department (NCDHD) has the following comments regarding the proposed Daycare:

- A Daycare Plan Review is required with this department:
 - The Learning Experience shall submit a scalable floor plan of the facility along with an exterior site plan showing the outside playground(s) area(s);
 - The floor plan must show the location of each piece of equipment, floor and counter, clearly labeled with its common name.
 - A menu of snacks and/or meals that will be served to the children shall be submitted;
 - Application for Day Care Center with the application fee
- If the Learning Experience is proposing to serve meals to the children at this daycare facility, a complete plan review for a licensed food service facility based on the NCDHD sanitary code Section 4 and the Public Health Food code CT-PHC19-13-B42 will be required. Should a plan review be required, the following shall be submitted for review:
 - Scalable floor plan of the kitchen/Pantry layout. The plan must show the location of each piece of equipment, floor and counter, clearly labeled with its common name;
 - Equipment specification sheets for proposed equipment. All equipment shall be National Sanitation Foundation (NSF) certified or equivalent;
 - A complete menu of meals that will be offered for review

- The NCDHD has the following comments on the submitted floor plan;
 - For any of the rooms that will have a diaper changing table, a dedicated hand wash sink that supplies running hot water within a range of 60°F to 115°F will be required.
 - An enclosed mop sink will be required for proper cleaning of the pantry floors and general cleaning of the establishment. The mop sink shall have a mop hanger to properly dry mops;
 - A utility wash sink to clean/wash equipment such as paint bowls, brushes etc. will be required;
 - A designated area is required for the storage of toxic chemicals and cleaners. In addition, these items and other potentially hazardous substances are required to be stored in a separate locked area.
 - Dumpsters will be required to reside on a concrete pad or equivalent.
 - The building will be required to connect to public water and public sewer.

Should anyone have any additional questions regarding this matter, I am reachable via email at bbielawiec@ncdhd.org. You can also call me at the NCDHD office at 860-745-0383, extension 114.

Sincerely,

A handwritten signature in blue ink, appearing to read "Brian Bielawiec, R.S.", is written above the typed name.

Brian Bielawiec
Registered Sanitarian



David A. Smith, P.E., L.S.
Town Engineer

Vernon-ct.gov

Memo

To: George McGregor, Vernon Town Planner
From: David Smith, Vernon Town Engineer
Date: March 2, 2022
Re: 501 Talcottville Road

DAS

I would like to take this opportunity to provide the following comments related to the application currently pending before the Planning and Zoning Commission on the subject parcel.

Stormwater Management –

The applicant's Engineer has submitted drainage calculations indicating that the post development condition will not increase stormwater runoff from the project site as compared to the current condition. Roof water will be directed to a recharge system located easterly of the proposed building which will reintroduce clean water into the subsoil gravelly soils. An overflow system has been provided to direct any roof runoff that exceeds the recharge system's capacity to the stormwater basin. That basin is designed to receive runoff from the parking area and some of the landscaped areas where it will be held back during the design storm and allowed to infiltrate through the vegetative and soil layers. This will serve to minimize impacts to the groundwater both in quality and quantity.

The existing pipe in the vicinity of the new driveway location is to be extended toward the limit of disturbance in the southwesterly corner of the project. The origin of the existing pipe under Dart Hill Road is unclear, but it is now part of the stormwater management of the Walgreen's Property. The 6" pipe shown on the upper left corner of the design plans would appear to provide some overflow capacity for their recharge basin (located on what is now Town of Vernon property), but the outlet on the applicant's property is buried, so it is questionable how much or often this pipe carries water. Regardless, the applicant proposes to provide a drainage manhole at the current end of the existing pipe and extend the system beyond their active development. A drainage easement and right to drain should be included as part of the proposal.

Traffic Considerations –

I have reviewed the Traffic Report prepared in support of this application. The driveway location as proposed has adequate sight lines looking both east and west to allow a vehicle to assess whether it is safe enter onto Dart Hill Road. The Traffic Counts for this activity indicate that we would expect approximately 1 vehicle per minute entering and exiting during the peak morning and afternoon hours. These peak periods correlate nicely with the heaviest traffic periods currently experienced on Dart Hill Road.

On Tuesday Feb 22nd, I observed this stretch of road from the westerly drive of the Walgreen's property from approximately 3:30pm to 4:45pm. During the first half hour of this period, the vehicle stack from the light at Route 83 routinely extended to the proposed new driveway. During the second half hour, the stack extended to Thrall Road and the bridge. The intensity abated quite sharply after 4:30pm and the vehicle queue did not extend to the proposed driveway during the last dozen cycles that I observed. This condition is not reflected in the modeling provided in the traffic study. I am concerned that a single lane for entering and a single lane exiting, will negatively impact through traffic as well as vehicles dropping off students during these peak hours. The Traffic Report references optimizing the signal timing, which is presumed to be done by the State of CT sometime in the future. This would be done as part of proposed improvements to the Talcottville Road / Dart Hill / Regan Road independent of any development on this property, indicating that the current situation is already on the State's radar as a concern. The timing and extent of these improvements are mostly beyond our or the applicant's control. I believe it is prudent to review this proposal given the realities of today, acknowledging that some future condition may see changes in the future, but not relying on it as a solution.

The applicant may wish to consider an alternated drive configuration, an additional right turn exit lane at this location or at another. Another option might be to reduce the size of the facility since the trip generation numbers are based on service population related to building square footage. The number of required parking spaces is also driven by the size of the building. A modest reduction might free up areas that could be used to advantage in configuring alternate means of access.

Subdivision

Throughout the review process, the presentation of the subdivision seems to have been downplayed. The site plan has taken centerstage with the question of the lot size being a forgone conclusion. Typically, a lot is not subdivided without some assurance that a viable development can reasonably be expected to had on each parcel. At the February 22nd meeting of the Inland Wetland Commission the subdivision proposal was not discussed, but it will be part of the conversation at their March meeting.

The same wetland system that is part of their site development review is even more prevalent on the lot proposed to have frontage on Route 83. An action by the IWC to recognize a modest scheme could help define what may or may not be acceptable for a future development. It is conceivable that the creation of a new parcel without such a record could oblige the IWC to approve something that would exceed their typical standards to the detriment of the wetlands.

From: [Judith Veillette](#)
To: [McGregor, George](#)
Subject: [EXTERNAL] Talcottville Road project
Date: Saturday, April 2, 2022 11:17:20 PM

CAUTION: This Email is from an EXTERNAL source. Ensure you trust this sender before clicking on any links or attachments.

Hello again George,

I hate to bother you again, but this question comes to mind if this project is approved, despite all of the homeowners' objections.

When the Walgreens project was being built, we watched from a distance as the construction went on-it was quite an interesting sight. However, when this happens so close to our homes, there is going to be construction noise complete with the ground shaking, I'm positive. If the shaking causes pictures to fall off the walls and our belongings to come crashing down off the shelves, who do I call for help or compensation?

Thank you so much for your help-I have a feeling that we are going to be right in the line of fire, so to speak. Please don't ask us to move during construction, because my husband is a partially bed-ridden and wheelchair-bound invalid. This is not going to be easy for any of us living close to this construction site.

Thank you again,
Sincerely, Judith Veillette
786 Dart Hill Road

From: [Judith Veillette](#)
To: [McGregor, George](#)
Subject: [EXTERNAL] Talcottville Road project
Date: Saturday, April 2, 2022 11:17:20 PM

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Thank you again,
Sincerely, Judith Veillette
786 Dart Hill Road

From: [Judith Veillette](#)
To: [McGregor, George](#)
Subject: [EXTERNAL] Talcottville Road project
Date: Friday, March 11, 2022 11:51:57 PM

CAUTION: This Email is from an EXTERNAL source. Ensure you trust this sender before clicking on any links or attachments.

Hello again Mr. McGregor,

Thank you for speaking with me on the phone this week. One thought has stuck in my mind: the developers probably want an access road on Dart Hill Road across from Walgreen's entrance to give clients access to the day care center without going all the way to Talcottville Road, in effect, a "shortcut" into the facility.

I am having nightmares about the problems this would cause-traffic tie-ups, more bumper-to-bumper traffic than we already have on this road, not to mention the increase in fumes and pollution affecting our health. One of the biggest considerations is the potential for this to become a HUGE ACCIDENT-PRONE section of road!

I understand that you are against the site plan as it stands, and I hope that this is one of the proposals that is thrown out once and for all!

Thank you for your time and consideration.

Sincerely,
Judith Veillette

From: [Mark Vertucci](#)
To: [McGregor, George](#)
Cc: [Perry, Craig](#); [Smith, David](#); [rema8@aol.com](#); [Jay Ussery](#); [Eric Spungin](#); [Tim Coon](#); [Tai Le](#); [Mark Vertucci](#)
Subject: [EXTERNAL] RE: 501 Talcottville Road - Traffic Comments Response
Date: Wednesday, February 16, 2022 12:08:59 AM
Attachments: [image001.png](#)

CAUTION: This Email is from an EXTERNAL source. Ensure you trust this sender before clicking on any links or attachments.

Hi George,

Following up on Tim's email, please see our responses below to traffic related comments 4 through 8 in your February 10, 2022 letter regarding the above referenced project:

4. ***The commercial entrance is shown as 30' wide, allowing for one lane in and one lane out. This may cause internal queuing issues. Dependent on the outcome of our traffic discussions (in detail below), please consider potentially separate right out and left out scenarios.***

As noted in the traffic analysis, the site driveway exit is projected to operate efficiently at LOS B during both peak hours with 95th percentile queues averaging one vehicle length or less. The existing single exit lane is adequate to support the volume of exiting traffic expected from this development. Dual exit lanes on an unsignalized intersection approaches should be avoided per CTDOT guidance as vehicles in this scenario jockey for sight line as they attempt to exit.

5. ***A most significant challenging facing the development of this site is providing safe and efficient vehicular access to the property. The biggest concern is the clear potential for conflicts when cars drop off or pick up for day care: they must cross two lanes of traffic on Dart Hill Road. Often during peak hours that traffic may queue (at least anecdotally) in front of the property causing delay and introducing unsafe turning movements. Bottom line: can facility customers enter and exit the site safely during peak a.m. and p.m.? How does the applicant plan to mitigate this potential conflict?***

The traffic study analysis revealed that 95th percentile queues on the Dart Hill Road eastbound approach to Route 83 (Talcottville Road) reach up to 270 feet during the morning peak hour in the combined (build) conditions and up to 245 feet during the afternoon peak hour. The storage length from the proposed site driveway to the Dart Hill Road stop bar at Route 83 is approximately 400 feet, therefore the proposed site driveway is not obstructed by the typical maximum eastbound queues experienced at the intersection during peak hours. Field observations during the peak hours also revealed that these queues were not extending past the proposed site driveway location. It should be noted that the Dart Hill Road eastbound approach is in the process of opening up for a second turn lane in the vicinity of the proposed site driveway therefore westbound vehicles that would be turning left into the site from Dart Hill Road or vehicles exiting the site driveway are essentially yielding to only one lane of oncoming traffic approaching from the west. Results from the traffic analysis indicate that left turns from Dart Hill Road into the proposed driveway will operate very efficiently as LOS A and turns out of the proposed site driveway will also operate efficiently at LOS B with little delay. The proposed site driveway has been located as far west as possible to minimize the chance of queues from the traffic signal extending past the driveway while at the same time aligning the site driveway with the Walgreens driveway across the street. Aligning the two site driveways at a normalized intersection consolidates all vehicle

turning movements at a single point and reduces the number of potential vehicle conflict points. Moving the site driveway any further west would create an offset intersection and reduce safety.

With regard to the safety of vehicles exiting the proposed site, intersection sight distances in both directions were measured to exceed CTDOT criteria for safe egress. In addition, a review of crash data at the existing Walgreens driveway revealed that no crashes have occurred involving a vehicle turning into or out of the site driveway at this location. Based on the results of the traffic study, it is our professional opinion that traffic will be able to safely and efficiently enter and exit the daycare site from Dart Hill Road during both peak hours.

6. ***The traffic study does attempt to address the intersection of 83 and Dart Hill Rd. Which will continue to operate at a level of Service F, unless some unspecified "optimization" takes place in an out year beyond 2025. Please explain how "Optimization" will work, the timing therein, who will be responsible and who will pay for the optimization? The traffic study suggests that even with optimization, certain directional queues will get longer.***

The proposed optimization of signal timings at the intersection of Route 83 at Dart Hill Road and Regan Road will increase the overall efficiency (and reduce overall delay) of the intersection by reallocating the amount of green time provided for each leg of the intersection. The traffic study analysis revealed that this intersection was operating acceptably during the morning peak hour of traffic at LOS C and this LOS will remain unchanged with the additional development traffic. During the afternoon peak hour, timing optimization can improve the intersection operation to LOS E which is a better operation than what is occurring in the existing condition. The optimization will also significantly decrease queueing on the Route 83 through lanes however the tradeoff is some minor increase in queueing in the Route 83 left turn lanes and the Dart Hill Road/Regan Road side street approaches. As this intersection is State owned and maintained, any timing optimizations would need to be coordinated with CTDOT. Revising the timings in the signal controller, if agreed to by CTDOT, would be handled with a service order to DOT maintenance staff. There is no cost involved to modify the signal timing settings in the controller. As noted in the traffic study, the traffic signal at this intersection is slated for complete replacement by CTDOT within the next three years under State project 171-0471. Upon completion of this signal upgrade, the safety and efficiency of traffic operations at this intersection will be improved.

7. ***Perhaps, there are mitigation measures or options for alternative access that can be considered such as a second point of access to Route 83, right in/right out limitation on Dart Hill and/or Route 83, or commitment to expedite, finance and complete optimization measures.***

A second point of access on Route 83 for the daycare center is not recommended as the volume of opposing traffic on Route 83 is substantially higher than the volume of traffic on Dart Hill Road. As a point of comparison, 2,671 vehicles pass the site frontage on Route 83 during the PM peak hour while only 612 vehicles pass the site frontage on Dart Hill Road. In addition, Route 83 is five lanes wide at the site frontage and northbound queues from the traffic signal extend 490 feet past the site frontage during the afternoon peak hour. The combination of heavy traffic volumes, queueing from the signal and the need to cross multiple lanes of traffic would make left turns into or out of a site driveway on Route 83 very difficult. In addition, a right in/right out limitation on Dart Hill Road would offer no option for vehicles exiting the site who intend to head west on Dart Hill Rd. As noted in the response above, left turns into the site driveway on Dart Hill Rd are projected to operate at the most efficient LOS A during both peak hours,

therefore there would be no justification for restricting this movement. Also as noted above, interim timing optimizations at the signal can be reviewed/coordinated with CTDOT and completed at no cost prior to the full signal replacement that is anticipated to be completed by 2025.

8. Staff plans to observe site conditions (especially the Dart Hill Rd queueing scenario) during the AM and PM peaks and will report these observations to the PZC at the time of the public hearing.

Noted.

Please let me know if you have any questions or would like to discuss further.

Thanks,

Mark

Mark Vertucci, PE, PTOE

Vice President

Fuss & O'Neill, Inc. | 146 Hartford Road | Manchester, CT 06040
860.646.2469 x5381 | mvertucci@fando.com | cell: 860.729.0186

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From: Tim Coon <tcoon@jrrusso.com>

Sent: Tuesday, February 15, 2022 3:36 PM

To: McGregor, George <GMcGregor@vernon-ct.gov>

Cc: Perry, Craig <cperry@vernon-ct.gov>; Smith, David <dsmith@vernon-ct.gov>; rema8@aol.com; Jay Ussery <jussery@jrrusso.com>; Eric Spungin <espungin@hotmail.com>; Mark Vertucci <MVertucci@fando.com>

Subject: [External] 501 Talcottville Road

George,

I am providing this email and its attachments (revised plans and drainage report) in response to the plan review comments received from Craig Parry dated 2-7-22. A response to Craig's comments is provided below:

Craigs Re-designation Comments:

The soil scientist, George Logan from REMA Ecological Services, will be preparing and submitting an Impact Assessment/Function & Values Assessment under separate cover. It is anticipated that Mr. Logan will address Craigs comments 1 and 3 regarding the delineation and re-designation in that report. With regard to comment 2, I would like to point out that Note #1 on the Redesignation Plan identifies REMA Ecological Services as the entity which delineated the wetlands shown on the plans.

Craig's comment Regarding the Wetland Permit:

1. Despite the significant amount of fill required at the site, the runoff across the site will be very limited based on the existing sandy soils and limited distance across the site. It is believed that a single silt fence, if maintained properly, is sufficient to protect the downgradient wetland from sedimentation and erosion. However, based on Mr. Perry's concerns, the plans have been revised to include the placement of a wood chip berm to back

up the silt fence installed at the edge of the downgradient wetland.

2. The plans have been revised to extend the sediment barrier (silt fence backed by a wood chip berm) along the utility corridor out to Talcottville Road.
3. As discussed above, a Wetland Impact Assessment/Functions & Values Assessment is being prepared by George Logan of REMA and will be submitted separately. However, in the preparation of his report, George has made a couple of recommendations for wetland enhancement and creation which have been added to these revised plans. In particular, the plans identify a particular area of wetlands to the south of the development area where we are proposing a series of plantings to enhance the existing wetland. In addition, the plans identify an upland area between wetland flags #14 and #24 where we are proposing to create some wetlands by excavating a small pool and seeding with a wetland seed mix. I believe George's report will provide additional discussion regarding these measures.

In addition to the plan revisions noted above, revisions were also made to address the off-site runoff from the adjacent Panda Palace parking lot that is currently directed to the development site through an existing break in the curb line at the western edge of their parking lot. This includes untreated runoff from approximately 0.6 acres of impervious area at the Panda Palace which currently runs across the proposed development area at the site into the on-site wetland. The plan revisions include the installation of a riprap channel, yard drain and piping to intercept this runoff at the property line and convey it to the new on-site stormwater management basin where it will be treated and some of it recharged prior to discharge into the wetland. The attached plans and Drainage Report have been revised to reflect the interception and treatment of this off-site runoff. As a result, the proposed project will eliminate this discharge of untreated runoff to the wetland from the Panda Palace.

Finally, because I have also received your Town Planner comments dated 2-10-22, I have also separated the Wetland Re-designation Plan from the plan set and added both a fence and a dumpster surround detail, which address your first two comments. The architect will be providing color elevations to address your comment #3. Because your remaining comments (#4-#8) are all traffic related, a separate response is being prepared by the traffic engineer, Mark Vertucci at Fuss & O'Neill.

Also, as I have expressed to many of you on this email, I will be out of town until February 28th. In my absence, Jay Ussery from my office (copied above) will be handling this project and the wetland meeting next Tuesday. Please be sure that any correspondence meant for J.R. Russo & Associates is addressed to Jay during this time frame. Thank you.

Timothy A. Coon, P.E.
Principal Engineer



J.R. RUSSO & ASSOCIATES, LLC

P.O. Box 938, 1 Shoham Road
East Windsor, CT 06088
CT 860.623.0569 **MA** 413.785.1158
tcoon@jrrusso.com | www.jrrusso.com



TOWN OF VERNON

Office of the Town Planner
14 Park Place • Vernon, CT 06066
Tel: 860-870-3640 • www.Vernon-ct.gov
gmcgregor@vernon-ct.gov

George McGregor, AICP
Town Planner

February 10, 2022

VIA Email

Tim Coon
Project Engineer
J.R. Russo & Associates, LLC
PO Box 938
East Windsor, CT 06088

Re: PZ 2022-05, 501 Talcottville Rd. Town Planner Comments

Dear Mr. Coon:

With regard to the above referenced application, I offer the following comments and questions.

General Site

1. The Wetlands Re-designation Plan is included as Sheet 2. The final version set does not need to include this sheet.
2. Please provide both a fence detail and a dumpster surround detail in the plan set.
3. Please provide (via email) color elevations; they will be helpful during the Design Review Commission meeting.
4. The commercial entrance is shown at 30' wide, allowing for one lane in and one lane out. This may cause internal queuing issues. Dependent upon the outcome of our traffic discussions (in detail below), please consider potentially separate right out and left out scenarios.

Traffic

5. A most significant challenging facing the development of this site is providing safe and efficient vehicular access to the property. The biggest concern is the clear potential for conflicts when cars drop off or pick up for day care: they must cross two lanes of traffic on Dart Hill Rd. Often during peak hours that traffic may queue (at least anecdotally) in front of the property causing delay and introducing unsafe turning movements. Bottom line: can facility customers enter and exit the site safely during peak a.m. and p.m.? How does the applicant plan to mitigate this potential conflict?
6. The traffic study does attempt to address the intersection of 83 and Dart Hill Rd. which will continue to operate at a level of Service F, unless some unspecified "optimization" takes place in an out year beyond 2025. Please explain how "optimization" will work, the timing therein, who will be responsible and who will pay for the optimization? The traffic study suggests that even with optimization, certain directional queues will get longer.

7. Perhaps, there are mitigation measures or options for alternative access than can be considered.

- a. A second point of access to Route 83
- b. Right in-Right out limitations on Dart Hill and/or Route 83
- c. Commitment to expedite, finance, and complete optimization measures.

8. Staff plans to observe site conditions (especially the dart Hill Rd. queuing scenario) during the a.m. and p.m. peaks and will report these observations to the Planning & Zoning Commission at the time of the public hearing.

With Regards,

A handwritten signature in blue ink that reads "George K. McGregor". The signature is stylized with a large, sweeping "G" and a long horizontal line extending to the right.

George K. McGregor, AICP

From: [Kelley, John](#)
To: [Ryniewicz, Dwight](#); [Purcaro, Michael](#); [McGregor, George](#); [Gately, Shaun](#); [Smith, David](#); [Perry, Craig](#); [Boucher, Joseph](#); [Gallant, Lucas](#); [Wasilewski, Daniel](#)
Cc: [Wheelock, Diane](#); [Schambach, Jeff](#); [Carlson, Anne-Marie](#)
Subject: RE: Vernon, CT - Information Request
Date: Thursday, December 23, 2021 10:35:37 AM

Tai,

In September 2021 I requested State DOT look at the left turn arrow timing at the intersection of Route 83 and Regan Road. Traffic backs up on Route 83 intending to turn left onto Regan Road due to vehicles leaving Rockville High School. This occurs in the morning after students are dropped off and in the afternoon. I have also received complaints regarding left turn arrows not being long enough for Regan Road traffic intending to turn left onto Route 83. I have not heard back from DOT yet.

John Kelley
Chief of Police
Vernon Police Department
725 Hartford Turnpike
Vernon, CT 06066
jkelly@vernon-ct.gov
(860)872-9126

From: Ryniewicz, Dwight
Sent: Wednesday, December 22, 2021 10:54 AM
To: Purcaro, Michael <mpurcaro@vernon-ct.gov>; McGregor, George <GMcGregor@vernon-ct.gov>; Gately, Shaun <sgately@vernon-ct.gov>; Smith, David <dsmith@vernon-ct.gov>; Perry, Craig <cperry@vernon-ct.gov>; Boucher, Joseph <JBoucher@vernon-ct.gov>; Kelley, John <jkelly@vernon-ct.gov>; Gallant, Lucas <lgallant@vernon-ct.gov>; Wasilewski, Daniel <DWasilewski@vernon-ct.gov>
Cc: Wheelock, Diane <dwheelock@vernon-ct.gov>; Schambach, Jeff <jschambach@vernon-ct.gov>; Carlson, Anne-Marie <acarlson@vernon-ct.gov>
Subject: FW: Vernon, CT - Information Request

Good morning.
Please see email below. I have not responded.

Dwight

From: Tai Le <TLe@fando.com>
Sent: Wednesday, December 22, 2021 8:59 AM
To: Ryniewicz, Dwight <dryniewicz@vernon-ct.gov>
Subject: [EXTERNAL] Vernon, CT - Information Request

CAUTION: This Email is from an EXTERNAL source. Ensure you trust this sender before clicking on any links or attachments.

Good morning Dwight,

I am currently working on a traffic impact study for a proposed daycare center at 501 Talcottville Road (State Route 83) located in Vernon, Connecticut. The existing site is currently vacant, and the proposed development is planned to range from 10,000 to 12,000 square feet with up to 57 supporting parking spaces. Site access to the property will be provided via one full access driveway on Dart Hill Road directly across from the existing Walgreens full access driveway.

I would like to inquire as to whether or not the Town of Vernon has knowledge of any upcoming roadway modifications planned within the study area, signal timing/rephrasing, change in lane arrangement, or anything of that nature?

Please see the following link for a map of the location: <https://maphub.net/tle/501-Talcottville-Road>

Thank you,

Tai



Tai Le, EIT

Transportation Engineer

Fuss & O'Neill, Inc. | 146 Hartford Road | Manchester, CT 06040

860.646.2469 x5291 | tle@fando.com

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CENTRAL CONSERVATION DISTRICT, INC.

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STAFFORD • SUFFIELD • WEST HARTFORD • WETHERSFIELD • TOLLAND • VERNON • WILLINGTON • WINDSOR • WINDSOR LOCKS

Date: March 1, 2022

To: George K. McGregor, AICP, Town Planner
Town of Vernon Planning & Zoning Commission

From: Barbara Kelly, Program Coordinator, Registered Professional Soil Scientist, SSSSNE
Certified Erosion Control Professional CPESC #2180

Re: E&S Plan Review – The Learning Experience, 501 Talcottville Road, Vernon, Connecticut

This review is conducted pursuant to Section 18 of the Town of Vernon Zoning Regulations. The review is limited to certification of the erosion control plan, based on compliance with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control (Guidelines).

District staff inspected the site on February 22, 2022. And reviewed a 12-sheet plan titled "The Learning Experience, Property of 501 Talcottville Road, LLC" (Plan), prepared by J.R. Russo & Associates, LLC and revised 2-14-22. Following a discussion with the Project Engineer on February 28, 2022, a *Grading & Erosion & Sediment Control* plan sheet and an *Erosion & Sediment Control* plan sheet with revision dates of 3-1-22 were reviewed.

The Plan proposes the construction of a retaining wall, a 10,000 square foot building, a 4,500 square foot playground, associated parking, and a stormwater management basin on the northern two acres of a 4.6 acre lot located at 501 Talcottville Road. The proposed development necessitates up to ten feet of fill. Post development, stormwater will be discharged at grade into wetlands located immediately south of the proposed development.

Background & Observations

Based on the Natural Resources Conservation Service Web Soil Survey, soils throughout the majority of the proposed construction area are mapped as somewhat excessively drained Hartford sandy loam on 3 to 8 percent slopes. The erosion hazard of this soil is considered moderate. The topography of the site will tend to direct any sediment laden stormwater toward the on-site wetlands. The Plan's "Soil Erosion and Sediment Control Notes" address dust control, vehicle tracking of sediments, and the maintenance of E&S measures.

Comment

Based on the observed site conditions, with proper implementation and maintenance, the soil erosion and sediment control measures shown in the Plan updated on 3-1-22 are adequate and appropriate. The District certifies that the Plan complies with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control.

Thank you for the opportunity to comment.



North Central District Health Department

□ Enfield—31 North Main Street, Enfield, CT 06082 * (860) 745-0383 Fax (860) 745-3188

□ Vernon—375 Hartford Turnpike, Room 120, Vernon, CT 06066 * (860) 872-1501 Fax (860) 872 1531

□ Windham—Town Hall, 979 Main Street, Willimantic, CT 06226 * (860) 465-3033 Fax (860) 465-3034

□ Stafford—Town Hall, 1 Main Street, Stafford Springs, CT 06076 * (860) 684-5609 Fax (860) 684-1768

Patrice A. Sulik, MPH, R.S.
Director of Health

February 22, 2022

Mr. George McGregor
Vernon Town Planner
55 West Main Street
Vernon, Connecticut 06066

Re: Special Permit – The Learning Experience
501 Talcottville Road
Vernon, Connecticut

Dear Mr. George McGregor:

I am writing regarding the Special Permit Application review for the Learning Experience at the above referenced address.

North Central District Health Department (NCDHD) has the following comments regarding the proposed Daycare:

- A Daycare Plan Review is required with this department:
 - The Learning Experience shall submit a scalable floor plan of the facility along with an exterior site plan showing the outside playground(s) area(s);
 - The floor plan must show the location of each piece of equipment, floor and counter, clearly labeled with its common name.
 - A menu of snacks and/or meals that will be served to the children shall be submitted;
 - Application for Day Care Center with the application fee
- If the Learning Experience is proposing to serve meals to the children at this daycare facility, a complete plan review for a licensed food service facility based on the NCDHD sanitary code Section 4 and the Public Health Food code CT-PHC19-13-B42 will be required. Should a plan review be required, the following shall be submitted for review:
 - Scalable floor plan of the kitchen/Pantry layout. The plan must show the location of each piece of equipment, floor and counter, clearly labeled with its common name;
 - Equipment specification sheets for proposed equipment. All equipment shall be National Sanitation Foundation (NSF) certified or equivalent;
 - A complete menu of meals that will be offered for review

- The NCDHD has the following comments on the submitted floor plan;
 - For any of the rooms that will have a diaper changing table, a dedicated hand wash sink that supplies running hot water within a range of 60°F to 115°F will be required.
 - An enclosed mop sink will be required for proper cleaning of the pantry floors and general cleaning of the establishment. The mop sink shall have a mop hanger to properly dry mops;
 - A utility wash sink to clean/wash equipment such as paint bowls, brushes etc. will be required;
 - A designated area is required for the storage of toxic chemicals and cleaners. In addition, these items and other potentially hazardous substances are required to be stored in a separate locked area.
 - Dumpsters will be required to reside on a concrete pad or equivalent.
 - The building will be required to connect to public water and public sewer.

Should anyone have any additional questions regarding this matter, I am reachable via email at bbielawiec@ncdhd.org. You can also call me at the NCDHD office at 860-745-0383, extension 114.

Sincerely,

A handwritten signature in blue ink, appearing to read "Brian Bielawiec, R.S.", is written above the typed name.

Brian Bielawiec
Registered Sanitarian



David A. Smith, P.E., L.S.
Town Engineer

Vernon-ct.gov

Memo

To: George McGregor, Vernon Town Planner
From: David Smith, Vernon Town Engineer
Date: March 2, 2022
Re: 501 Talcottville Road

DAS

I would like to take this opportunity to provide the following comments related to the application currently pending before the Planning and Zoning Commission on the subject parcel.

Stormwater Management –

The applicant's Engineer has submitted drainage calculations indicating that the post development condition will not increase stormwater runoff from the project site as compared to the current condition. Roof water will be directed to a recharge system located easterly of the proposed building which will reintroduce clean water into the subsoil gravelly soils. An overflow system has been provided to direct any roof runoff that exceeds the recharge system's capacity to the stormwater basin. That basin is designed to receive runoff from the parking area and some of the landscaped areas where it will be held back during the design storm and allowed to infiltrate through the vegetative and soil layers. This will serve to minimize impacts to the groundwater both in quality and quantity.

The existing pipe in the vicinity of the new driveway location is to be extended toward the limit of disturbance in the southwesterly corner of the project. The origin of the existing pipe under Dart Hill Road is unclear, but it is now part of the stormwater management of the Walgreen's Property. The 6" pipe shown on the upper left corner of the design plans would appear to provide some overflow capacity for their recharge basin (located on what is now Town of Vernon property), but the outlet on the applicant's property is buried, so it is questionable how much or often this pipe carries water. Regardless, the applicant proposes to provide a drainage manhole at the current end of the existing pipe and extend the system beyond their active development. A drainage easement and right to drain should be included as part of the proposal.

Traffic Considerations –

I have reviewed the Traffic Report prepared in support of this application. The driveway location as proposed has adequate sight lines looking both east and west to allow a vehicle to assess whether it is safe enter onto Dart Hill Road. The Traffic Counts for this activity indicate that we would expect approximately 1 vehicle per minute entering and exiting during the peak morning and afternoon hours. These peak periods correlate nicely with the heaviest traffic periods currently experienced on Dart Hill Road.

On Tuesday Feb 22nd, I observed this stretch of road from the westerly drive of the Walgreen's property from approximately 3:30pm to 4:45pm. During the first half hour of this period, the vehicle stack from the light at Route 83 routinely extended to the proposed new driveway. During the second half hour, the stack extended to Thrall Road and the bridge. The intensity abated quite sharply after 4:30pm and the vehicle queue did not extend to the proposed driveway during the last dozen cycles that I observed. This condition is not reflected in the modeling provided in the traffic study. I am concerned that a single lane for entering and a single lane exiting, will negatively impact through traffic as well as vehicles dropping off students during these peak hours. The Traffic Report references optimizing the signal timing, which is presumed to be done by the State of CT sometime in the future. This would be done as part of proposed improvements to the Talcottville Road / Dart Hill / Regan Road independent of any development on this property, indicating that the current situation is already on the State's radar as a concern. The timing and extent of these improvements are mostly beyond our or the applicant's control. I believe it is prudent to review this proposal given the realities of today, acknowledging that some future condition may see changes in the future, but not relying on it as a solution.

The applicant may wish to consider an alternated drive configuration, an additional right turn exit lane at this location or at another. Another option might be to reduce the size of the facility since the trip generation numbers are based on service population related to building square footage. The number of required parking spaces is also driven by the size of the building. A modest reduction might free up areas that could be used to advantage in configuring alternate means of access.

Subdivision

Throughout the review process, the presentation of the subdivision seems to have been downplayed. The site plan has taken centerstage with the question of the lot size being a forgone conclusion. Typically, a lot is not subdivided without some assurance that a viable development can reasonably be expected to had on each parcel. At the February 22nd meeting of the Inland Wetland Commission the subdivision proposal was not discussed, but it will be part of the conversation at their March meeting.

The same wetland system that is part of their site development review is even more prevalent on the lot proposed to have frontage on Route 83. An action by the IWC to recognize a modest scheme could help define what may or may not be acceptable for a future development. It is conceivable that the creation of a new parcel without such a record could oblige the IWC to approve something that would exceed their typical standards to the detriment of the wetlands.



David A. Smith, P.E., L.S.
Town Engineer

Vernon-ct.gov

Memo

To: Vernon Planning and Zoning Commission

From: David Smith, Town Engineer

Date: April 29, 2022

Re: 501 Talcottville Road

This application has evolved over the last several months with the applicant making various adjustments as input from staff and other regulatory authorities became available. I believe that this final version identified as Alternate 2, adequately addresses my early concerns related to traffic and drainage.

Traffic - There is no doubt that the intersection of Dart Hill and Route 83 is presently underperforming at certain times of the day, but this application is unlikely to add enough new traffic to the area to be significant. With or without this facility, it would be prudent to ask the State DOT to review the timing of these signal lights to insure they operate at peak efficiency. I understand that the actual signals and support arms are due for an upgrade in the next year or two, and that may be a good opportunity for such a review.

The addition of the alternate right in / right out driveway off Route 83 is an enhancement to original plan that will also help reduce the stacking on Dart Hill eastbound. The travel path is convenient for vehicles that will access the site, without being overly attractive as a cut through for commuters.

Drainage – This proposal provides for the direct recharge of roof run-off through a distribution system placed under the pavement. Additionally, run-off from the parking areas and some lawn areas will be directed to a collection and recharge basin to filter and moderate the discharge intensity before directing the outflow to the wetland system. This is in keeping with our Low Impact Development goals. The design addresses the need to mimic current conditions with the post development condition. I do not believe that this construction will reduce or increase run-off from the site.

The applicant is proposing to extend drainage from the northerly property line to a discharge point south of the school. While the presence of the existing pipe is a curiosity, the extension of this pipe does not adversely impact whatever function it originally had. I suggest that a 25' wide drainage easement offset 10' and 15' from the proposed pipe and an unrestricted right to drain in favor of the Town of Vernon be provided with this installation.

Other comments - The driveway at Dart Hill Road should be constructed in conformance with our current Engineering Standards including the increase sidewalk thickness at the drive and a concrete apron to the gutter line.

The division of the current parcel into two separate units needs some clarification. It is referred to in various documents as a Re-subdivision and as a Subdivision. We should be consistent with which ever is appropriate. With regards to the Inland Wetland Application, the focus was primarily on the Site Development Plan, with the Re-designation request. The Subdivision / Resubdivision was given only minor consideration. I believe this was in part due to how the application was presented and the fact that there are two separate firms are preparing the respective documents.

At this time, the IWC has not provided a 'report' to the PZC, but I expect that they will do so, possibly at their meeting of 5/3/22. If they are favorably disposed to this division, I have recommended that a note be clearly displayed on the subdivision map stating "the Vernon IWC has not approved any activities for this parcel at this time".

From: [Mark Vertucci](#)
To: [McGregor, George](#)
Cc: [Perry, Craig](#); [Smith, David](#); [rema8@aol.com](#); [Jay Ussery](#); [Eric Spungin](#); [Tim Coon](#); [Tai Le](#); [Mark Vertucci](#)
Subject: [EXTERNAL] RE: 501 Talcottville Road - Traffic Comments Response
Date: Wednesday, February 16, 2022 12:08:59 AM
Attachments: [image001.png](#)

CAUTION: This Email is from an EXTERNAL source. Ensure you trust this sender before clicking on any links or attachments.

Hi George,

Following up on Tim's email, please see our responses below to traffic related comments 4 through 8 in your February 10, 2022 letter regarding the above referenced project:

4. ***The commercial entrance is shown as 30' wide, allowing for one lane in and one lane out. This may cause internal queuing issues. Dependent on the outcome of our traffic discussions (in detail below), please consider potentially separate right out and left out scenarios.***

As noted in the traffic analysis, the site driveway exit is projected to operate efficiently at LOS B during both peak hours with 95th percentile queues averaging one vehicle length or less. The existing single exit lane is adequate to support the volume of exiting traffic expected from this development. Dual exit lanes on an unsignalized intersection approaches should be avoided per CTDOT guidance as vehicles in this scenario jockey for sight line as they attempt to exit.

5. ***A most significant challenging facing the development of this site is providing safe and efficient vehicular access to the property. The biggest concern is the clear potential for conflicts when cars drop off or pick up for day care: they must cross two lanes of traffic on Dart Hill Road. Often during peak hours that traffic may queue (at least anecdotally) in front of the property causing delay and introducing unsafe turning movements. Bottom line: can facility customers enter and exit the site safely during peak a.m. and p.m.? How does the applicant plan to mitigate this potential conflict?***

The traffic study analysis revealed that 95th percentile queues on the Dart Hill Road eastbound approach to Route 83 (Talcottville Road) reach up to 270 feet during the morning peak hour in the combined (build) conditions and up to 245 feet during the afternoon peak hour. The storage length from the proposed site driveway to the Dart Hill Road stop bar at Route 83 is approximately 400 feet, therefore the proposed site driveway is not obstructed by the typical maximum eastbound queues experienced at the intersection during peak hours. Field observations during the peak hours also revealed that these queues were not extending past the proposed site driveway location. It should be noted that the Dart Hill Road eastbound approach is in the process of opening up for a second turn lane in the vicinity of the proposed site driveway therefore westbound vehicles that would be turning left into the site from Dart Hill Road or vehicles exiting the site driveway are essentially yielding to only one lane of oncoming traffic approaching from the west. Results from the traffic analysis indicate that left turns from Dart Hill Road into the proposed driveway will operate very efficiently as LOS A and turns out of the proposed site driveway will also operate efficiently at LOS B with little delay. The proposed site driveway has been located as far west as possible to minimize the chance of queues from the traffic signal extending past the driveway while at the same time aligning the site driveway with the Walgreens driveway across the street. Aligning the two site driveways at a normalized intersection consolidates all vehicle

turning movements at a single point and reduces the number of potential vehicle conflict points. Moving the site driveway any further west would create an offset intersection and reduce safety.

With regard to the safety of vehicles exiting the proposed site, intersection sight distances in both directions were measured to exceed CTDOT criteria for safe egress. In addition, a review of crash data at the existing Walgreens driveway revealed that no crashes have occurred involving a vehicle turning into or out of the site driveway at this location. Based on the results of the traffic study, it is our professional opinion that traffic will be able to safely and efficiently enter and exit the daycare site from Dart Hill Road during both peak hours.

6. ***The traffic study does attempt to address the intersection of 83 and Dart Hill Rd. Which will continue to operate at a level of Service F, unless some unspecified "optimization" takes place in an out year beyond 2025. Please explain how "Optimization" will work, the timing therein, who will be responsible and who will pay for the optimization? The traffic study suggests that even with optimization, certain directional queues will get longer.***

The proposed optimization of signal timings at the intersection of Route 83 at Dart Hill Road and Regan Road will increase the overall efficiency (and reduce overall delay) of the intersection by reallocating the amount of green time provided for each leg of the intersection. The traffic study analysis revealed that this intersection was operating acceptably during the morning peak hour of traffic at LOS C and this LOS will remain unchanged with the additional development traffic. During the afternoon peak hour, timing optimization can improve the intersection operation to LOS E which is a better operation than what is occurring in the existing condition. The optimization will also significantly decrease queueing on the Route 83 through lanes however the tradeoff is some minor increase in queueing in the Route 83 left turn lanes and the Dart Hill Road/Regan Road side street approaches. As this intersection is State owned and maintained, any timing optimizations would need to be coordinated with CTDOT. Revising the timings in the signal controller, if agreed to by CTDOT, would be handled with a service order to DOT maintenance staff. There is no cost involved to modify the signal timing settings in the controller. As noted in the traffic study, the traffic signal at this intersection is slated for complete replacement by CTDOT within the next three years under State project 171-0471. Upon completion of this signal upgrade, the safety and efficiency of traffic operations at this intersection will be improved.

7. ***Perhaps, there are mitigation measures or options for alternative access that can be considered such as a second point of access to Route 83, right in/right out limitation on Dart Hill and/or Route 83, or commitment to expedite, finance and complete optimization measures.***

A second point of access on Route 83 for the daycare center is not recommended as the volume of opposing traffic on Route 83 is substantially higher than the volume of traffic on Dart Hill Road. As a point of comparison, 2,671 vehicles pass the site frontage on Route 83 during the PM peak hour while only 612 vehicles pass the site frontage on Dart Hill Road. In addition, Route 83 is five lanes wide at the site frontage and northbound queues from the traffic signal extend 490 feet past the site frontage during the afternoon peak hour. The combination of heavy traffic volumes, queueing from the signal and the need to cross multiple lanes of traffic would make left turns into or out of a site driveway on Route 83 very difficult. In addition, a right in/right out limitation on Dart Hill Road would offer no option for vehicles exiting the site who intend to head west on Dart Hill Rd. As noted in the response above, left turns into the site driveway on Dart Hill Rd are projected to operate at the most efficient LOS A during both peak hours,

therefore there would be no justification for restricting this movement. Also as noted above, interim timing optimizations at the signal can be reviewed/coordinated with CTDOT and completed at no cost prior to the full signal replacement that is anticipated to be completed by 2025.

8. Staff plans to observe site conditions (especially the Dart Hill Rd queueing scenario) during the AM and PM peaks and will report these observations to the PZC at the time of the public hearing.

Noted.

Please let me know if you have any questions or would like to discuss further.

Thanks,

Mark

Mark Vertucci, PE, PTOE

Vice President

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From: Tim Coon <tcoon@jrrusso.com>

Sent: Tuesday, February 15, 2022 3:36 PM

To: McGregor, George <GMcGregor@vernon-ct.gov>

Cc: Perry, Craig <cperry@vernon-ct.gov>; Smith, David <dsmith@vernon-ct.gov>; rema8@aol.com; Jay Ussery <jussery@jrrusso.com>; Eric Spungin <espungin@hotmail.com>; Mark Vertucci <MVertucci@fando.com>

Subject: [External] 501 Talcottville Road

George,

I am providing this email and its attachments (revised plans and drainage report) in response to the plan review comments received from Craig Parry dated 2-7-22. A response to Craig's comments is provided below:

Craigs Re-designation Comments:

The soil scientist, George Logan from REMA Ecological Services, will be preparing and submitting an Impact Assessment/Function & Values Assessment under separate cover. It is anticipated that Mr. Logan will address Craigs comments 1 and 3 regarding the delineation and re-designation in that report. With regard to comment 2, I would like to point out that Note #1 on the Redesignation Plan identifies REMA Ecological Services as the entity which delineated the wetlands shown on the plans.

Craig's comment Regarding the Wetland Permit:

1. Despite the significant amount of fill required at the site, the runoff across the site will be very limited based on the existing sandy soils and limited distance across the site. It is believed that a single silt fence, if maintained properly, is sufficient to protect the downgradient wetland from sedimentation and erosion. However, based on Mr. Perry's concerns, the plans have been revised to include the placement of a wood chip berm to back

up the silt fence installed at the edge of the downgradient wetland.

2. The plans have been revised to extend the sediment barrier (silt fence backed by a wood chip berm) along the utility corridor out to Talcottville Road.
3. As discussed above, a Wetland Impact Assessment/Functions & Values Assessment is being prepared by George Logan of REMA and will be submitted separately. However, in the preparation of his report, George has made a couple of recommendations for wetland enhancement and creation which have been added to these revised plans. In particular, the plans identify a particular area of wetlands to the south of the development area where we are proposing a series of plantings to enhance the existing wetland. In addition, the plans identify an upland area between wetland flags #14 and #24 where we are proposing to create some wetlands by excavating a small pool and seeding with a wetland seed mix. I believe George's report will provide additional discussion regarding these measures.

In addition to the plan revisions noted above, revisions were also made to address the off-site runoff from the adjacent Panda Palace parking lot that is currently directed to the development site through an existing break in the curb line at the western edge of their parking lot. This includes untreated runoff from approximately 0.6 acres of impervious area at the Panda Palace which currently runs across the proposed development area at the site into the on-site wetland. The plan revisions include the installation of a riprap channel, yard drain and piping to intercept this runoff at the property line and convey it to the new on-site stormwater management basin where it will be treated and some of it recharged prior to discharge into the wetland. The attached plans and Drainage Report have been revised to reflect the interception and treatment of this off-site runoff. As a result, the proposed project will eliminate this discharge of untreated runoff to the wetland from the Panda Palace.

Finally, because I have also received your Town Planner comments dated 2-10-22, I have also separated the Wetland Re-designation Plan from the plan set and added both a fence and a dumpster surround detail, which address your first two comments. The architect will be providing color elevations to address your comment #3. Because your remaining comments (#4-#8) are all traffic related, a separate response is being prepared by the traffic engineer, Mark Vertucci at Fuss & O'Neill.

Also, as I have expressed to many of you on this email, I will be out of town until February 28th. In my absence, Jay Ussery from my office (copied above) will be handling this project and the wetland meeting next Tuesday. Please be sure that any correspondence meant for J.R. Russo & Associates is addressed to Jay during this time frame. Thank you.

Timothy A. Coon, P.E.
Principal Engineer



J.R. RUSSO & ASSOCIATES, LLC

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