# TOWN OF VERNON Inland Wetlands Commission (IWC) Meeting Notice & Agenda Tuesday, February 22, 2022, 7:00 PM

Join Zoom Meeting

https://us02web.zoom.us/j/6149852850?pwd=NUZwVkEwWkRxS1EweC94cFNHVUxBdz09

Meeting ID: 614 985 2850
Passcode: h72Vs7
Dial by your location:
(646) 876 9923
Meeting ID: 614 985 2850
Passcode: 786221

#### **AGENDA**

- 1. Call to Order & Roll Call
- 2. Administrative Actions
  - 2.1 Call for filing(s) of Intervener Petitions and Determination of Status
- 3. New Applications for Receipt and Determination of Significance, if any
- 4. Public Hearing and Action on New Applications
  - **4.1 IWC 2022-01, 501 Talcottville Rd.** An Application by Vernon Development LLC for a Wetlands Redesignation and a Wetlands Permit for activity in a regulated area, in order to develop a child-care facility at 501 Talcottville Rd. (Tax Map 09, Block 007, Parcel 0001D).
- 5. Status of Cease & Correct Orders, if any
- 6. Wetlands Enforcement Officer Report, if any
- 7. Inland Wetlands Agent Approvals, if any
- 8. Other Business
- 9. Adjournment

Rachel Stansel, Chairperson Inland Wetlands Commission

### **APPLICATION 1**



January 12, 2022

Vernon Inland Wetlands Commission 56 West Main Street, 2<sup>nd</sup> Floor Vernon, CT 06066

Re:

The Learning Experience (TLE) 501 Talcottville Road, Vernon

Dear Commissioners.

On behalf of the Vernon Development LLC, I am pleased to submit the attached application for a wetland redesignation and a wetland permit for regulated activities associated with the development of The Learning Experience (TLE) Academy of Early Education at 501 Talcottville Road in Vernon, Connecticut. The subject parcel currently consists of 4.6 acres of undeveloped woodland located at the intersection of Dart Hill Road and Talcottville Road (Rte. 83). A wooded wetland, delineated by REMA Ecological Services, LLC in May of 2021, occupies the southwestern portion of the parcel. In conjunction with the proposed TLE development, the owner is proposing to divide the existing parcel into two lots, including a 2.0 acre lot adjacent to Dart Hill Road to be developed with the TLE facility, and a remaining 2.6 acre lot fronting on Talcottville Road for potential future development. The subdivision application to split the parcel will be submitted to be considered concurrently with the applications for the Wetland Permit and Special Permit/Site Plan approval for the TLE development. However, the permits for the TLE should not be conditioned on the approval of the subdivision, but should be valid regardless of whether the lot is split or remains a single 4.6 acre parcel.

The proposed TLE development will involve the construction of a 10,000 square foot building, 5,000 square foot playground and associated parking. Runoff from the building and parking lot will be directed to a subsurface infiltration system and surface infiltration basin for treatment, groundwater recharge and detention prior to discharge at the edge of the existing on-site wetland. A retaining wall is proposed along the southern edge of the development in order prevent any direct wetland disturbance. However, the project will involve approximately 0.82 acres of work within wetland regulated area associated with the construction of the building, playground, parking lot, and stormwater management system. The plans call for the removal of existing invasive species along the project perimeter and installation of wetland buffer plantings along the existing wetland.

If there are any questions, or you require further information, please call me at (860) 623-0569.

Very truly yours.

Timothy A. Coon, P.E.

J.R. Russo & Associates, LLC

Attachments

cc:

Eric Spungin



## TOWN OF VERNON INLAND WETLANDS COMMISSION (IWC)

#### **APPLICATION**

This form is to be used to apply to the Vernon Inland Wetlands Commission (IWC) for approval for a redesignation of a wetlands area, a change to the Inland Wetlands and Watercourses Regulations, and/or a permit to conduct a regulated activity in a wetland, watercourse, or upland review area (URA), which are defined as areas within one hundred (100) feet from the boundary of a wetland, watercourse, or intermittent watercourse and areas within two hundred (200) feet from the boundary of Gage's Brook, Hockanum River, Ogden Brook, Railroad Brook, Tankerhoosen River, Valley Falls Pond, Walker Reservoir East, Walker Reservoir West. Any activity that the Commission determines is likely to impact or affect wetlands or watercourses may be considered a regulated activity. **Provide all the information requested**.

The Applicant must be the property owner, the property owner's agent, the Town of Vernon, or someone with a direct financial interest in the subject property. Said interest shall be explained. If the applicant is not the property owner, written permission for this Application must be obtained from the property owner and submitted by letter signed by the property owner authorizing submission of the Application.

The Applicant understands that the Application is complete only when all information and documents required by IWC have been submitted and that any approval by the IWC relies upon complete and accurate information being provided by the Applicant. Incorrect information provided by the Applicant may make the approval invalid. The IWC may require additional information to be provided by the Applicant.

#### I. APPLICANT (S)

Name:
Title: Member
company: Vernon Development LLC
Address: 56 East Main Street, Avon CT 06001
Telephone: 860-677-5607 Fax:
E-mail: espurgin@hotmail.com
II. PROPERTY OWNERS
Name: James Basile
Title:
Company: 501 Talcottville Road LLC
Address: 43 R. Age crest Lane, Bristol, CT 06010-2910
Telephone: 860-202-9540 Fax:
E-mail:

III. PROPERTY 501 Talcottville Road Assessor ID Code: Map # 09 Block # 007 Lot/Parcel # COOLD Land Record Reference to Deed Description: Volume: 2026 USGA Location: Circle the Map Quadrangle Name: Manchester # 38 (Rockville #39 Circle the Sub regional Drainage Basin #: 3108 (4500) 4502 Zoning District: Commercia IV. PROJECT Project Name: The Learning Experience Project Contact Person: Timothy Coon J.R. Russo + Associates LLC Address: P.O. Box 938 , East Windsor. Telephone: 860-623-0569 Fax: 860-623-2485 E-mail: TOON QUETUSSO COM V. PROJECT SUMMARY Describe the project briefly in regard to the purpose of the project and the activities that will occur. Attach to this application a complete and detailed description with maps and documentation as required by the "The Town of Vernon Inland Wetlands and Watercourses Regulations". a 10,000 SF Daycase Facility General Activities: Filling, excavation, and building construction review area. Regulated Activities: Watercourse disturbance (linear feet):

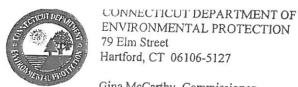
Wetlands disturbance (acres or sq. ft.):

Upland Review Area (URA)disturbance: 0.82 ±

Nonregulated activities & activities outside URA: \_\_\_\_\_\_ O. 89 ±

#### VI. APPLICATION

	41
Redesignation of Wetlands	
Amendment of Inland Wetlands and Watercourses R	egulations
Modification of a Wetlands Redesignation	
Non-significant activity	
Significant activity with less than ½ acre site of	listurbance
Significant activity with site disturbance from 3	secre to and including 2 acres
Significant activity with site disturbance greate	
Commission modification of a welland permit i	n effect
Modification of a wetland permit by ;the Wetlan	ds Agent
Approval of a license by the Wetlands Agent for activiti	es in an upland
Appeal of a decision by the Wetlands Agent	
Subdivision review per CGS Section 8-26	
Jurisdictional ruling regarding permitted and nonregulate	ed uses
Waiver, reduction, or delayed payment of fees (attach si	atement of justification)
Waiver	,
Reduction to \$	
Delay of payment to	
VII. CERTIFICATION AND SIGN	IATURE
, the undersigned Applicant or applicant's Agent, hereby certify the niand Wetlands and Watercourses Regulations* and have preparticulate information.	nat I have reviewed the "Town of Vernor ed this Application with complete and
Applicant of Agent Signature Printed Name	11 12/03/21 Date
Owner's Signature, if different Jol Talastoni 118 Road July Printed Name	
TO BE FILLED IN BY THE PLANNING D	EPARTMENT
DATE APPLICATION SUBMITTED	200 CO
DATE APPLICATION RECEIVED BY COMMISSION	
WC FILE:	



GIS CODE #:				<u>`</u>				
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REV. 5/2007

Gina McCarthy, Commissioner

## Statewide Inland Wetlands & Watercourses Activity Reporting Form

Please complete and mail this form in accordance with the instructions. Please print or type.

PART I: To Be Completed By The Inland Wetlands Agency Only
1. DATE ACTION WAS TAKEN: Year Month
2. ACTION TAKEN (circle one): A B C D E F G H
3. WAS A PUBLIC HEARING HELD? Yes No
4. NAME OF AGENCY OFFICIAL VERIFYING AND COMPLETING THIS FORM:
(print) (signature)
PART II: To Be Completed By The Inland Wetlands Agency Or The Applicant
5. TOWN IN WHICH THE ACTION IS OCCURRING: VETNON
Does this project cross municipal boundaries? Yes No _X
If Yes, list the other town(s) in which the action is occurring:
6. LOCATION: USGS Quad Map Name: Rockville AND Quad Number: 39
Subregional Drainage Basin Number: 4500
7. NAME OF APPLICANT, VIOLATOR OR PETITIONER: Vernon Development LLC
8. NAME & ADDRESS/LOCATION OF PROJECT SITE: 501 Talcotty. 1/2 Road
Briefly describe the action/project/activity: Construction of a 10,000 SF Daylase Facility
9. ACTIVITY PURPOSE CODE: D
10. ACTIVITY TYPE CODE(S): 12 . 14
11. WETLAND / WATERCOURSE AREA ALTERED [must be provided in acres or linear feet as indicated]:
Wetlands: acres
12. UPLAND AREA ALTERED [must be provided in acres as indicated]: acres
13. AREA OF WETLANDS AND / OR WATERCOURSES RESTORED, ENHANCED OR CREATED:
[must be provided in acres as indicated]
DATE RECEIVED: PART III: To Be Completed By The DEP
DATE RECEIVED: PART III: To Be Completed By The DEP DATE RETURNED TO DEP:
FORM COMPLETED: YES NO FORM CORRECTED (COMPLETED: YES NO

#### **APPLICATION FEES**

(Section 19 of the Inland Wetlands and Watercourses Regulations)

19.4 Fee Schedule. Application fees shall be based on the following schedule:

Wetlands redesignation	\$150	
Modification of a wetlands redesignation	75	
Wetland permits		
Non-significant activity	125	
Significant activity with less than ½ acre site disturbance	250	
Significant activity with site disturbance from ½ to and including 2 acres	500	
Significant activity with site disturbance greater than 2 acres	800	
Notification of abutters of public hearings, per abutter	1	
Commission modification of a wetlands permit remaining in effect	150	
Modification of a wetland permit by the Wetland Agent	75	
State mandatory fee (per CGS 22a-27j)	60	
Other fees		335
Approval by Wetland Agent	75	
Appeal of Wetland Agent decision	125	
Jurisdictional rulings regarding permitted and nonregulated uses	25	
Review of proposed subdivision per CGS 8-26 (e) where no regulated activity is proposed	\$125	
Complex Application Fee: The Commission may charge an additional fee sufficient to cover the cost of reviewing and acting on complex applications. Such fee may include, but not be limited to, the cost of retaining experts to analyze, review, and report on issues requiring such experts. The Commission or the Wetland Agent shall estimate the complex application fee which shall be paid pursuant to section 20.1 of these regulations within 10 days of the applicant's receipt or notice of such estimate. Any portion of the complex application fee in a such estimate.		

19.5 Exemption. Boards, commissions, councils and departments of the Town of Vernon are exempt from all fee requirements.

such estimate. Any portion of the complex application fee in excess of the actual cost shall be refunded to the applicant no later than 30 days

after publication of the Commission's decision.

19.6 Waiver. The applicant may petition the Commission to waive, reduce or allow delayed payment of the fee. Such petitions shall be in writing and shall state fully the facts and circumstances the Commission should consider in its determination under this subsection. The Commission may waive all or part of the application fee if the Commission determines that:

#### Abutters List/Mailing Labels

KAO LLC 192 Talcott Ridge Rd South Windsor, CT 06074

Kenneth Busenbark 32 Worcester Road Vernon, CT 06066

Brian F. Oulette & Stephanie M. Brow 46 Worcester Road Vernon, CT 06066

Thomas Shirshac 64 Worcester Road Vernon, CT 06066

Vernon Properties LLC 605 Middel Street, No. 15 Braintree, MA 02184-5817

New 500 East LLC 44 Caisson Road Colchester, CT 06415 Gregory Gozzo C/O Gozzo Estate Homes 190 Spyglass Lane Jupiter, FL 33477

Richard & Gloria J. Martocchio 36 Worcester Road Vernon, CT 06066

John Coro 52 Worcester Road Vernon, CT 06066

Eugene P. & Judith S. Veillette 786 Dart Hill Road Vernon, CT 06066-2302

Realty Income Corp PM Dept 0704 11995 El Camino Real San Diego, CA 92130 Independence Realty Group LLC c/o Webster Bank: Corp RE 203 145 Bank Street
Waterbury, CT 06702

Nelson J. & Theresa M. Chiasson 42 Worcester Road Vernon, CT 06066

James D. & Bonnie R. King 58 Worcester Road Vernon, CT 06066

Town of Vernon 14 Park Place Vernon, CT 06066

O'Reilly Automotive Store Inc. P.O. Box 9167 Springfield, MO 65801 HAMIFURD, CT. 06/05

#### **QUITCLAIM DEED**

VICTOR J. BASILE ("Releasor"), of 43 Ridgecrest Lane, Bristol, Connecticut, for no consideration paid, grants to 501 TALCOTTVILLE ROAD, LLC of 43 Ridgecrest Lane, Bristol. Connecticut ("Releasee") all that certain piece or parcel of land together with all improvements thereon and appurtenances thereto in the Town of Vernon, Connecticut, as more particularly described on Exhibit A attached hereto and made a part hereof ("Premises"). Releasee herein assumes and agrees to pay all real property taxes attributable to the Premises on the list of October 1, 2006 and thereafter.

#### SAID PREMISES IS CONVEYED SUBJECT TO, WITHOUT LIMITATION:

- 1. Taxes of the Town of Vernon on the List of October 1, 2006 now due and payable and thereafter, which Releasee herein assumes and agrees to pay.
  - 2. Any and all provisions of any ordinance, municipal regulation, or public or private law.
- 3. Declarations, restrictions, covenants, matters and easements of record and any state of facts an accurate survey or personal inspection of the Premises might reveal.

,	
Witnessed by:	
Milas S. Still	Vich Basil
MICHAEL S! STIEBEL, as witness	VICTOR I BASILE
The The	V
ROBERT A FIREFERS witness	
STATE OF CONNECTICUT)	16-16-1
COUNTY OF ) ss:	(town)

On this the 7th day of February, 2007, before me, the undersigned officer, personally appeared Victor J. Basile, known to me (or satisfactorily proven) to be the person whose name is subscribed to the within instrument and acknowledged that he executed the same for the purposes therein contained and as his free act and deed.

In witness whereof I hereunto set my hand.

Signed this T day of February, 2007

Commissioner of the Superior Court

Notary Public

My Commission Expires:

Grantee's Address:

43 Ridgecrest Lane

Bristol, Connecticut 06010

CONVEYANCE TAX RECEIVED STATE \$

Bernice K. Dixon TOWN CLERK OF VERNON

#### Exhibit A

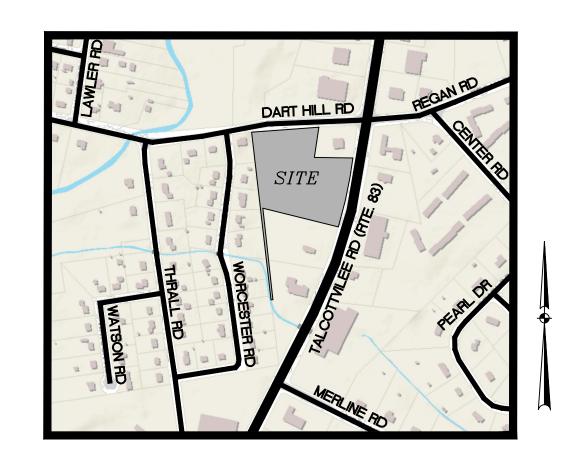
A certain piece or parcel of land situated on the westerly side of Talcottville Road (Route \$83) and the southerly side of Dart Bill Road in the Town of Vernon, County of Tolland and State of Connecticut, being shown as "Parcel 3 Parcel contains 200,968 S.F.i, 4.6136 AC±", on a certain map or plan entitled "Subdivision Plan Prepared For John J. Kahr, Sr. Boundary Kap Talcottville Rd. & Dart Hill Rd. Vernon, Conn. AR Lombardi Associates, Inc. Consulting Civil and Sanitary Engineers Land Surveyors Vernon Connecticut Comm. No: 64-1091 Date: March 7, 1985 Scale: 1" = 40' Sht, No: 1 of 2", which map or plan is on file or to be filed in the Vernon Town Clerk's Office and to which reference may be had for a more particular description thereof. Said Parcel 3 is more particularly bounded and described as follows:

Commencing at a point in the southerly street line of Dart Hill Road, which point marks the northwesterly corner of the hereindescribed premises and the northeasterly corner of land now or formerly of Antoine F. Bourcher, as shown on said map; thence proceeding in a southerly direction along said land now or formerly of Antoine F. Bourcher, land now or formerly of Thomas A. & Linda M. Shirshac, and land now or formerly of Eilean M. Strube, partly by each, a distance of 408.91 feet to a point; thence turning an obtuse interior angle of 179° 35' 27" and proceeding along land now or formerly of Alice M. Coro. land now or formerly of Peggy O. Tracy, land now or formerly of Nelson J. and Teresa M. Chiasson, land now or formerly of Paul J. and Linda Jobkowiak and land now or formerly of David M. Glenn Jr. and Sharon L. Glenn, partly by each, as shown on said map, a distance of 470.12 feet to a point; thence turning an obtuse interior angle of 103° 44' 36" and proceeding along Parcel 1. as shown on and map, a distance of 11.73 feet to a point; thence turning an acute interior angle of 76° 53' 42" and proceeding along land now or formerly of Cardinal Industries, Inc., as shown on said map, a distance of 211.66 feet to a point; thence turning an obtuse interior angle of 180° 04' 41" and proceeding along land now or formerly of Gregory Gozzo and David J. Sweeney, as shown on said map, a distance of 268.00 feet to a point; thence turning an obtuse interior angle of 289° 53' 43" and proceeding along said land now or formerly of Gregory Gozzo and David J. Sweeney, a distance of 403.00 feet to a point in the westerly street line of Talcottville Road; thence proceeding along the arc of a curve to the left having a radius of 2.506.67 feet, a distance of 322.92 feet along the westerly street line of Talcottville Road to a Connecticut Highway Department Monument in said westerly street line of Talcottville Road: thence proceeding along said westerly street line of Talcottville Road, a distance of 41.55 feet to a point; thence turning an acute interior angle of 82° 53' 05" and proceeding along Parcel No. 2. as shown on said map, a distance of 200.86 feet to a point; thence turning an interior obtuse angle of 277° 06' 55" and proceeding along Parcel No. 2, as shown on said map, a distance of 159.68 feet to a point; thence turning an interior acute angle of 77° 09' 29" and proceeding along the southerly street line of Dart Hill Road, as shown on said map, a distance of 352.53 feet to the point or place of beginning.

RECORDED IN
VERNON LAND RECORDS
Bernice K. Dixon
VERNON TOWN CLERK
ON Dec 05,2008 AT 12:30P



# 501 Talcottville Road Vernon, Connecticut



KEY PLAN MAP

1"=500"

Applicant

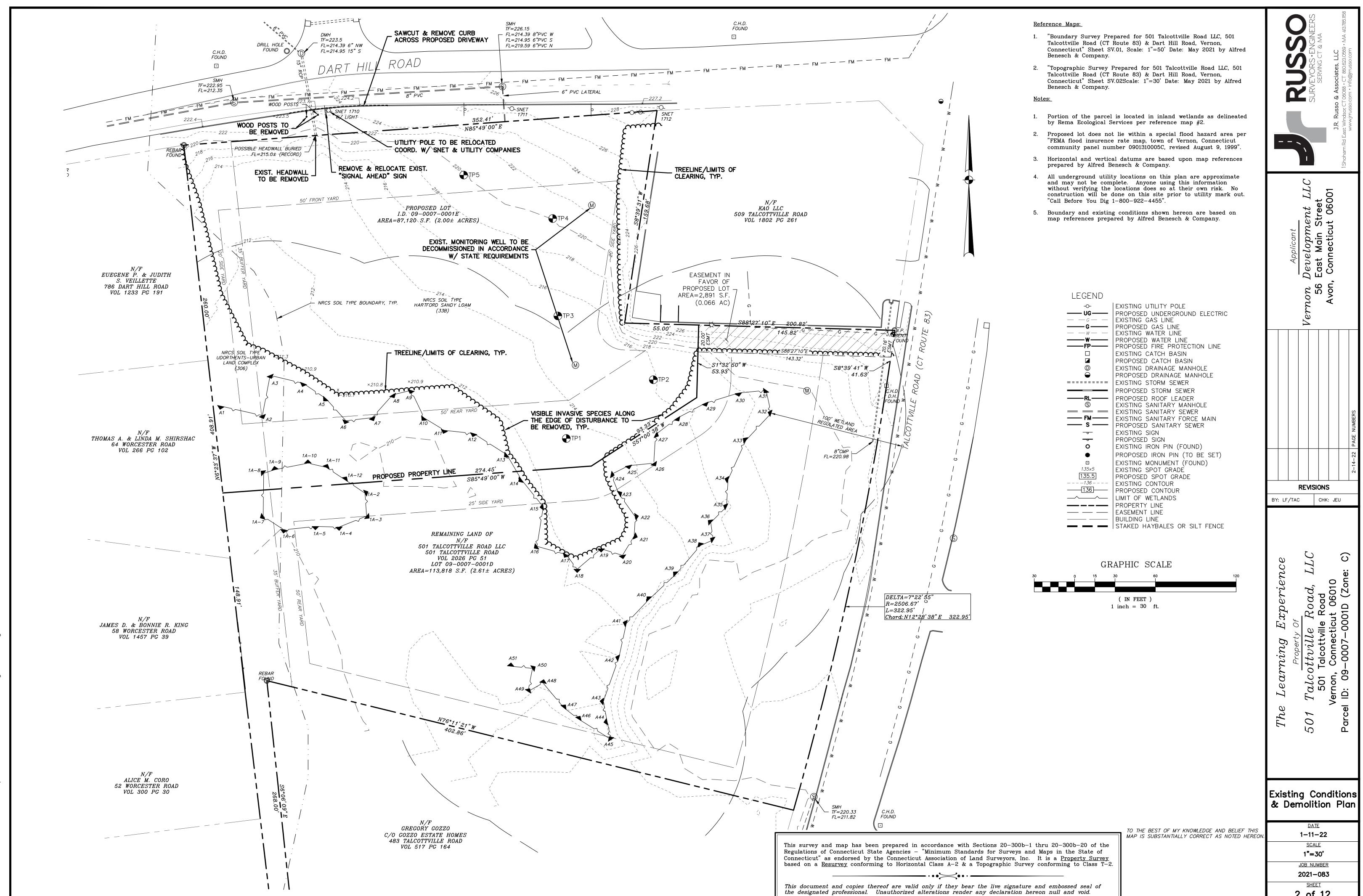
Vernon Development LLC
56 East Main Street
Avon, CT 06001
(860) 677-5607

501 Talcottville Road LLC
43 Ridgecrest Lane
Bristol, CT 06010-2910

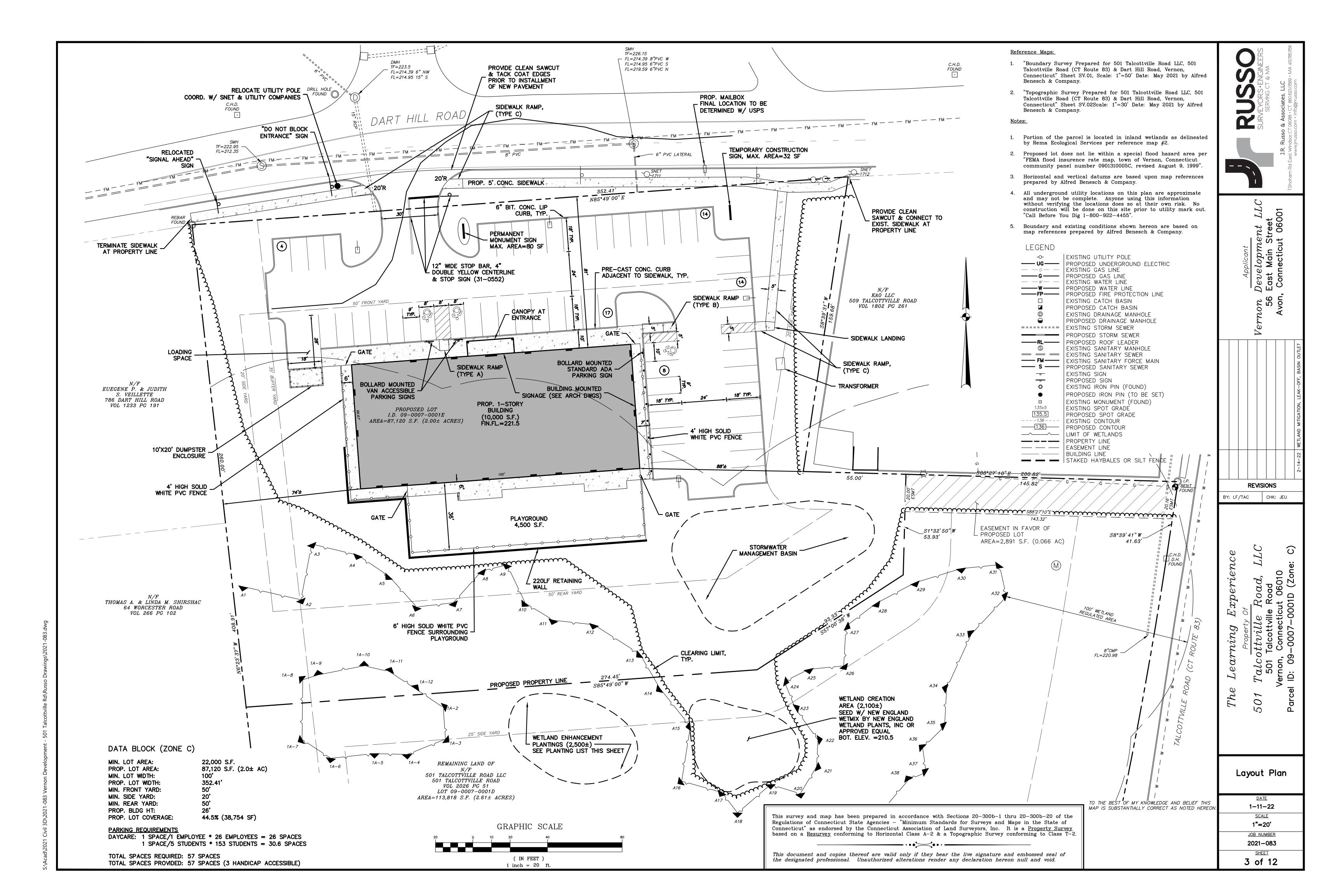


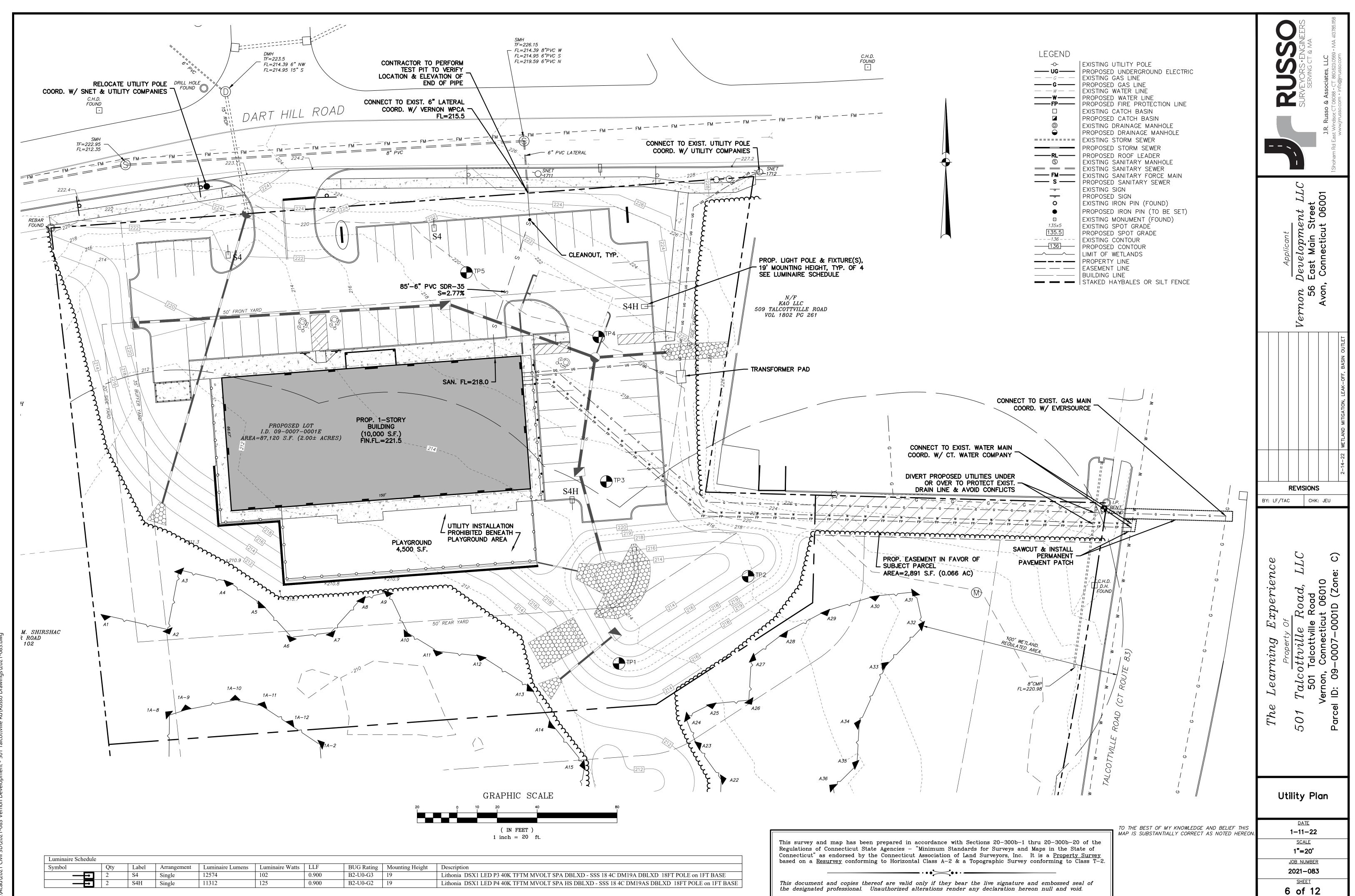


DRAWING INDEX		
SHEET TITLE	SHEET NO.	LATEST REVISION
CIVIL COVER SHEET EXISTING CONDITIONS & DEMOLITION PLAN LAYOUT PLAN GRADING & EROSION & SEDIMENT CONTROL PLAN STORM SEWER PLAN UTILITY PLAN PLANTING PLAN EROSION & SEDIMENT CONTROL NOTES DETAILS DETAILS DETAILS DETAILS	2 of 12 3 of 12 4 of 12 5 of 12 6 of 12 7 of 12 8 of 12 9 of 12 10 of 12	2-14-22 2-14-22 2-14-22 2-14-22 2-14-22 2-14-22 2-14-22 2-14-22 2-14-22 2-14-22 2-14-22 2-14-22

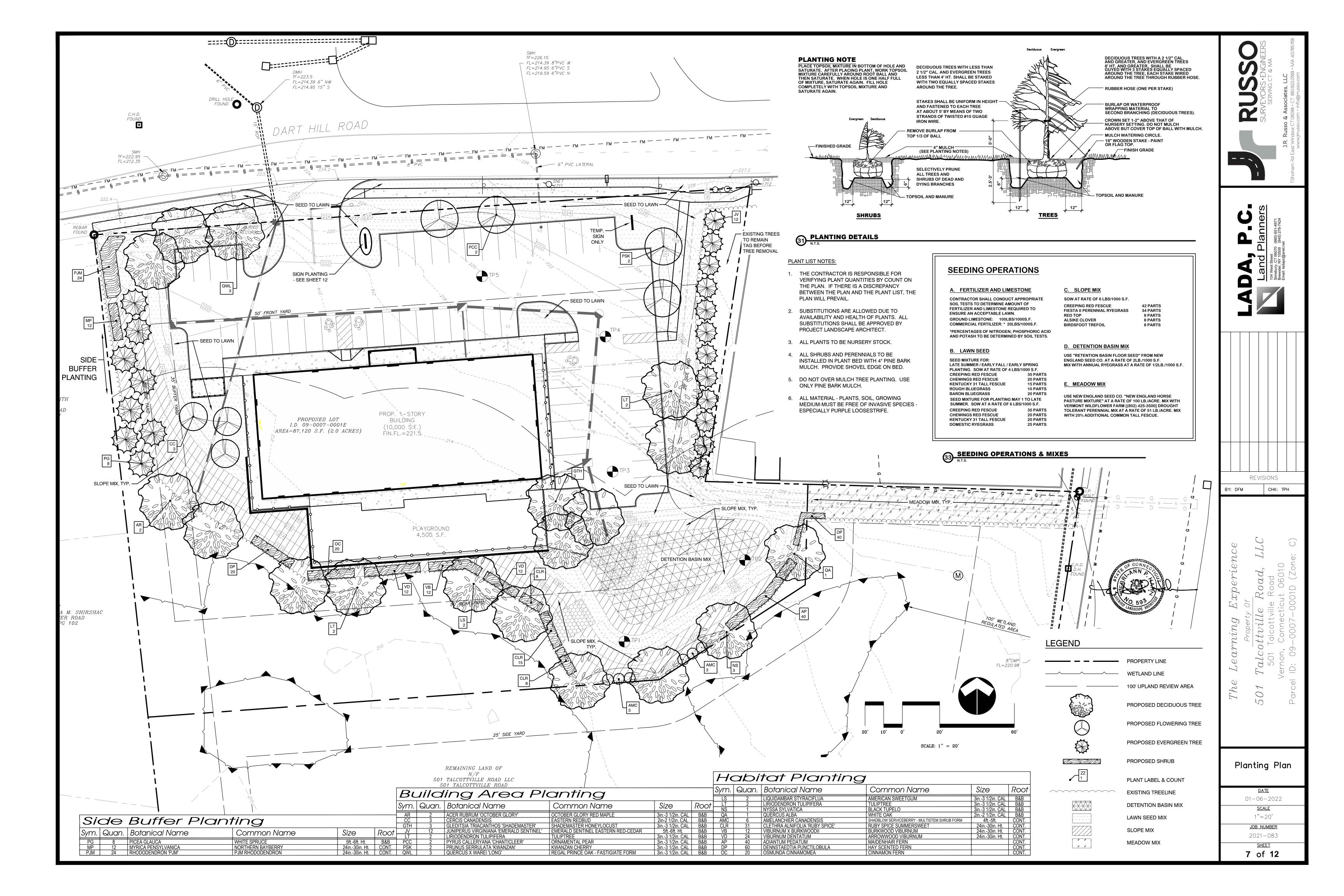


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ad\2021 Civil 3D\2021-083 Vernon Development - 501 Talcottville Rd\Russo D





REPORT DATE: <u>June 6, 2021</u> PAGE <u>1</u> OF <u>3</u>

#### REMA ECOLOGICAL SERVICES, LLC

164 East Center Street, Suite 8 Manchester, CT 06040

860.649.REMA (7362)

UN-SITE SOIL INVESTIGA	TION & WEILAND DELINEATION REPORT
PROJECT NAME & SITE LOCATION:	REMA Job No.: 21-2390-VER55
(+/- 4.87 acres)	Field Investigation Date(s): 5/6 § 5/11/2021
501 Talcotville Road	Field Investigation Method(s):
Vernon, CT	Spade and Auger
	Backhoe Test Pits
	Other:
REPORT PREPARED FOR:	Field Conditions:
Alfred Benesch & Company	<b>G</b>
120 Hebron Avenue, Floor 2	Soil Moisture: <u>Moderate to hígh</u>
Glastonbury, CT 06033	Snow Depth: N/A
· 	Frost Depth: N/A
Other:	n or Topographic Plan Soil Scientist from The Soil Survey of Connecticut Maps (USDA-NRCS)  USDA-NRCS) (attached); Figure A (attached) -A1 to RES-A-51 (open line), and RES-1A-1 to RES-1A-12
	"study area" encompasses roughly 4.87-acres of undeveloped,
,	de of Dart Híll Road, and to the west of Talcotville Road (Route are both disturbed and undisturbed, with the disturbed soils
•	its southeastern section. The study area's soils are derived from
	l), but with a silty mantle. The undisturbed upland soil types
	oderately well-drained Tisbury (702) soil series. The wetland-
	soil series. The disturbed upland soils are mapped as udorthents
(308). The delineated regulated areas ass	ociated with the study area, include seasonally saturated to
	bedded emergent marsh inclusions. Dominant and common
· · · · · · · · · · · · · · · · · · ·	d, and American elm. The shrub stratum includes elderberry,
	willows, winterberry, arrowwood, sweet pepperbush, Morrow's
•	d gooseberry. Herbs include sensitive and cinnamon ferns,
	ds, skunk cabbage, jack-in-the pulpit, jewelweed, clearweed, fowl
meadow grass, poison ivy, cattails, and virg	ınıa Jumpseed.

PAGE  $\underline{2}$  OF  $\underline{3}$ 

#### ON-SITE SOIL INVESTIGATION & WETLAND DELINEATION REPORT (CONTINUED)

PROJECT NAME & SITE LOCATION:	<u>(+/- 4.87 acres)</u>
	501 Talcotville Road, Vernon, CT

#### SOIL MAP UNITS

#### **Upland Soils**

Enfield silt loam (704). This series consists of deep, well drained soils formed in a coarse-silty mantle underlain by sandy water deposited glacial outwash materials. They are level to very steep soils on outwash plains and high stream terraces. The soils formed in loamy over stratified sandy and gravelly glacial outwash derived mainly from a variety of acid rocks. Typically, these soils have a dark grayish brown silt loam surface layer 8 inches thick. The subsoil from 8 to 26 inches is strong brown and light olive brown silt loam. The substratum from 26 to 60 inches is brown to reddish brown is stratified sand and gravel.

**Tísbury sílt loam (702).** This series consists of deep, moderately well drained soils formed in a coarse-silty mantle underlain by sandy water deposited glacial outwash materials. They are level to gently sloping soils in broad drainage swales and low lying positions on outwash plains and terraces. The soils formed in loamy over stratified sandy and gravelly glacial outwash derived mainly from a acid crystalline rocks (granite, gneiss and schist). Typically, these soils have a very dark grayish brown silt loam surface layer 8 inches thick. The subsoil from 8 to 26 inches is yellowish brown and brownish yellow silt loam, with mottles common below 16 inches. The substratum from 26 to 60 inches is grayish brown, mottled stratified sand and gravel.

**Udorthents (308).** This soil mapping unit consists of well drained to moderately well drained soils that have been altered by cutting, filling, or grading. The areas either have had two feet or more of the upper part of the original soil removed or have more than two feet of fill material on top of the original soil. *Udorthents* or Made Land soils can be found on any soil parent material but are typically fluvial on glacial till plains and outwash plains and stream terraces.

#### Wetland Soils

Raypol silt loam (12). This series consists of deep, poorly drained soils formed in a coarse-loamy mantle underlain by sandy water deposited glacial outwash materials. They are nearly level and gently sloping soils on outwash plains and high stream terraces. The soils formed in loamy over stratified sandy and gravelly glacial outwash derived mainly from acid rocks. Typically, these soils have very dark brown, silt loam Ap horizons, grayish brown and dark yellowish brown, mottled, silt loam and very fine sandy loam B2 horizons over light olive brown, mottled gravelly sand IIC horizons at a depth of 29 inches.

PAGE 3 OF 3

ON-SITE SOIL INVESTIGATION & WETLAND DELINEATION REPORT (CONTINUED)

PROJECT NAME & SITE LOCATION: (+/- 4.87 acres)

501 Talcotville Road, Vernon, CT

#### **SOIL MAP UNITS**

See previous page

Any accompanying soil logs and soil maps, and the on-site soil investigation narrative are in accordance with the taxonomic classification of the National Cooperative Soil Survey of the USDA Natural Resource Conservation Service, and with the Connecticut Soil Legend (DEP Bulletin No.5, 1983), as amended by USDA-NRCS. Jurisdictional wetland boundaries were delineated pursuant to the Connecticut General Statutes (CGS Sections 22a-36 to 22a-45), as amended. The site investigation was conducted and/or reviewed by the undersigned Registered Soil Scientist(s) [registered with the Society of Soil Scientists of Southern New England (SSSSNE) in accordance with the standards of the Federal Office of Personnel Management].

Respectfully submitted,

REMA ECOLOGICAL SERVICES, LLC

George T. Logan, MS, PWS, CSE

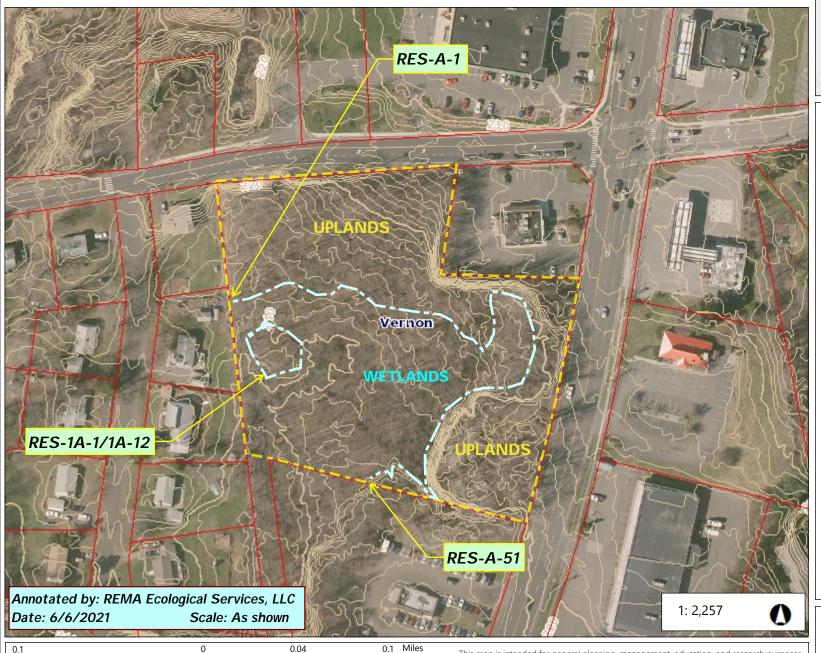
Registered Soil Scientist

Field Investigator/Senior Reviewer



## CT Environmental FIGURE A: Wetland Delineations Sketch Map 501 Talcotville Road Vernon CT

501 Talcotville Road, Vernon, CT





#### Legend

- Parcels for Protected Open Sp **Town Boundary** 
  - State Boundary
  - Town Boundary
  - Coastline

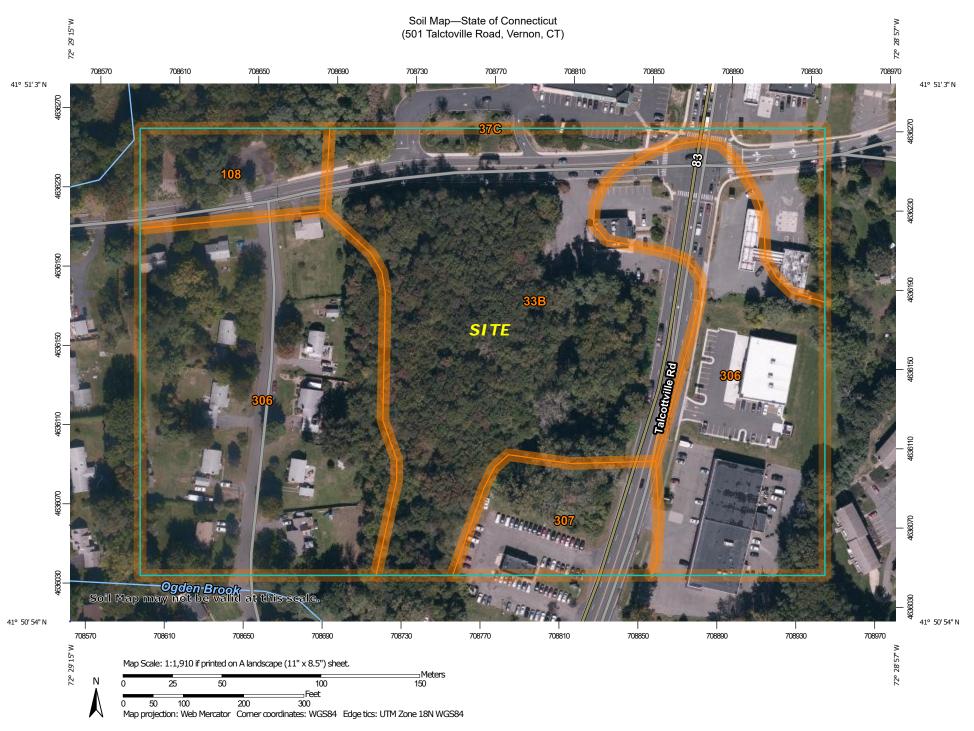
Light Gray Canvas Base

Notes

© Connecticut Environmental Conditions Online

THIS MAP IS NOT TO BE USED FOR NAVIGATION

This map is intended for general planning, management, education, and research purposes only. Data shown on this map may not be complete or current. The data shown may have been compiled at different times and at different map scales, which may not match the scale at which the data is shown on this map.



#### MAP LEGEND

#### Area of Interest (AOI)

Area of Interest (AOI)

#### Soils

Soil Map Unit Polygons



Soil Map Unit Points

#### Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

... Gravelly Spot

Landfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

+ Saline Spot

Sandy Spot

Severely Eroded Spot

- -----

Sinkhole

Slide or Slip

Sodic Spot

Stony Spot

Very Stony Spot

Spoil Area

Wet Spot

Other

Special Line Features

#### **Water Features**

Δ

Streams and Canals

#### Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

#### Background

Aerial Photography

#### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: State of Connecticut Survey Area Data: Version 20, Jun 9, 2020

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Sep 3, 2019—Oct 22, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Soil Map—State of Connecticut 501 Talctoville Road, Vernon, CT

### **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
33B	Hartford sandy loam, 3 to 8 percent slopes	7.6	39.2%
37C	Manchester gravelly sandy loam, 3 to 15 percent slopes	0.0	0.0%
108	Saco silt loam	1.1	5.6%
306	Udorthents-Urban land complex	9.4	48.4%
307	Urban land	1.3	6.9%
Totals for Area of Interest	•	19.4	100.0%

# DRAINAGE REPORT The Learning Experience 501 Talcottville Road Enfield, CT

Revised February 14, 2022

Prepared for:

Vernon Development, LLC 56 East Main Street Avon, Connecticut 06001 (860) 677-5607

Project No. 2021-083

Prepared by:

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#### I. INTRODUCTION

#### A. Project Description

Vernon Development, LLC and The Learning Experience are proposing the development of a 10,000 s.f. daycare center on Dart Hill Road near the intersection of Dart Hill Road and Talcottville Road (Rte. 83) in Vernon. The development will result in an impervious area of approximately 0.91 acres. Runoff from the development will be directed to a subsurface infiltration system and infiltration basin designed to provide treatment, ground water recharge, and peak flow attenuation in accordance with the Connecticut Stormwater Quality Manual.

#### B. Existing Conditions

The proposed 2.0 acre lot for the development will be split off of the existing 4.6 acre parcel at 501 Talcottville Road. The new lot for the development (the project site) will front Dart Hill Road to the north. The site currently consists of undeveloped woodlands, including a wooded wetland on the southern portion of the lot. The wetlands ultimately discharge to Ogden Brook further to the south. Starting at the edge of the road, the site slopes southerly toward the wetland. In addition to runoff from the parcel, stormwater from the building and parking lot at the adjacent Panda Palace to the east, discharges onto the property via a leak-off in the concrete curb at the edge of the parking lot. This runoff flows across the development site into the on-site wetland.

Stormwater overflow from an existing stormwater management basin serving the Walgreens development on the north side of Dart Hill Road is also piped underneath the road to discharge onto the site. The discharge from the stormwater basin consists of a 6" PVC pipe which terminates in an existing manhole in Dart Hill Road. The manhole discharges onto the site via a 15" RCP.

Based on a review of the USDA Soil Survey of Connecticut, the majority of the soil in the area to be developed consists of Hartford sandy loam and the remaining soil on the western portion of the site is Udorthents-Urban land complex. (see Soils Map in Appendix 2). The USDA Soil Survey defines groups of soils into Hydrologic Soil Groups (HSG) according to their runoff-producing characteristics. Soils are assigned to four groups (A, B, C, and D Groups). In group A, are soils having a high infiltration rate when thoroughly wet and having a low runoff potential. They typically are deep, well drained, and sandy or gravelly. In group D, at the other extreme, are soils having a very slow infiltration rate and thus a high runoff potential. They have a hardpan or clay layer at or near the surface, have a permanent high water table, or are shallow over nearly impervious bedrock or other nearly impervious material. The classification of the Hartford sandy loam is HSG A and the Udorthents-Urban land complex is HSG B.

On December 17, 2021, a series of 5 test pits were performed in the area of the proposed development to confirm the existing soil conditions. Test pits were excavated to depths ranging from 62-90 inches. Soils encountered included 6-12 inches of topsoil over brown fine sandy loam subsoils to a depth of 18-30 inches, overlying red-brown sand and gravel. Soil mottling indicative of the seasonal high water table was encountered in test pits TP1, TP2, and TP3. The

depths of the mottling indicate a seasonal high water table approximately at the elevation of 212.0. Test pit logs are provided the Site Plans.

Soil samples were collected from test pits 1 through 4 at depths ranging from 30-42 inches. These samples were submitted to New England Materials Testing Lab, LLC for permeability testing by ASTM D2434. Calculated permeabilities ranged from 1.10 in/hr for the sample collected at the bottom of the slope in TP1 to 33.95 in/hr for the sample collected mid-slope in TP4. Permeability test results are also provided in Appendix 3.

#### II. STORMWATER RUNOFF ANALYSIS

#### A. Methodology

Peak runoff flow rates were determined for pre- and post-development conditions using Applied Microcomputer System's HydroCAD<sup>TM</sup> Stormwater Modeling System. This computer software employs the SCS Technical Release 55 and 20 (TR-55 & TR-20) methodology. The potential stormwater impacts downstream were evaluated for the 2-yr, 10-yr, 25-yr, and 100-yr; 24-hour storm events. The rainfall for these storm events was taken from NOAA Atlas 14 provided in Appendix 1. Based on the present drainage patterns, all runoff from the proposed development area flows south into the on-site wetlands. As a result, the edge of the wetlands was selected as the design point. The runoff outlet from across the street will be diverted in order to bypass the on-site drainage system and discharge directly at the edge of the wetland. Thus, it acts independent from the site and was not included in these calculations.

#### B. Pre-Development Hydrology

The pre-development site was modeled as two subcatchments. Subcatchment PRE includes the approximately 1.86 acres of on-site development area that drains to the design point. Subcatchment PP includes the off-site runoff from the adjacent Panda Palace parcel. The pre-development drainage area map is provided in Appendix 4. The pre-development runoff characteristics of the contributing area is provided on the HydroCAD data sheets in Appendix 5. The pre-development discharge rates from the site during the design storms are summarized in Table 1.

#### C. Post-Development Hydrology

The proposed project will result in approximately 0.91 acres of new impervious area. The development will include a series of catch basins, trench drains, and piping to collect runoff from the development area and divert it to a new infiltration basin in the southeast corner of the site. This system will include a riprap swale and area drain installed downgradient of the leak-off from the Panda Palace parking lot. This runoff will be collected and diverted to the new infiltration basin as well. In addition, the roof runoff will be collected separately and diverted to a subsurface infiltration system under the parking lot which will overflow to the drainage system and ultimately the surface infiltration basin. The subsurface infiltration system and infiltration

basin have been designed in accordance with the CT Storm Water Quality Manual to provide treatment, groundwater recharge, and peak flow attenuation.

As discussed above, several test pits were completed at the site to verify soil conditions. Test pits TP1, TP2 and TP3 were all located within the vicinity of the proposed surface infiltration basin. Samples collected from these test pits at depths consistent with the proposed bottom of the basin were tested for permeability. The resulting permeabilities were 1.10 in/hr (TP1), 1.97 in/hr (TP2) and 2.34 in/hr (TP3). As a conservative measure, the slowest permeability rate of 1.10 in/hr was used as the basis for the design infiltration rate. This rate was further reduced by 50% to account for potential clogging resulting in a final design infiltration rate for the surface basin of 0.55 inches/hour. The subsurface infiltration basin will be constructed further up the slope to the north in the vicinity of test pits TP4 and TP5. The soils in these test pits at the elevation of the proposed infiltration system included loose, sand and gravel. The resulting permeability for the soil sample collected from TP4 was 33.95 in/hr. As a conservative measure and to account for potential clogging, the design infiltration rate used for the subsurface infiltration system was selected to be 15.0 in/hr.

The infiltration basin will be equipped with a forebay separated from the main part of the basin by a stone filter berm. The bottom of the basin will be set at elevation 214 which allows 2 feet of separation to the seasonal high water table that was encountered in the test pits. The basin will be equipped with a multi-stage outlet structure constructed from a standard Type CL catch basin. The structures primary outlet will consist of a 10" orifice set at an elevation of 216.1. The secondary outlet will consist of the frame and grate to be set at elevation 218.3. The structure will discharge via a 10" outlet pipe at the edge of the wetland. The basin will also be equipped with an emergency 10-foot wide earthen spillway at elevation 218.5. The subsurface infiltration system for the roof runoff will consist of 15 ADS SC740 chambers in stone. Flows from larger storm events exceeding the capacity of the storage and infiltration capacity of the chambers will discharge via an overflow pipe to the on-site drainage system and the surface infiltration basin. The proposed storage capacities of the surface infiltration basin below the primary outlet was sized to exceed the water quality volume. Likewise, the forebay was sized to contain a minimum of 25% of the WQV per the requirements for infiltration basins. WQV calculations are provided in Appendix 6.

The same design point for the pre-development analysis was used for the post development analysis. The post development site was divided into 9 subcatchments. Subcatchments S1, S2, and S3 include the areas that will be collected by the catch basins in the parking lot and discharge to the surface infiltration basin. Subcatchment S4 includes the roof runoff that will be discharged into the subsurface infiltration chambers. Subcatchment S5 includes runoff from the playground area that will be collected in a trench drain and piped directly to the infiltration basin. Subcatchment S6 includes the area that will directly sheet flow into the infiltration basin. Subcatchment S7 includes the area that will continue to drain overland directly to the design point (i.e. wetlands). Subcatchment PP includes the area of the adjacent Panda Palace that discharges to the development area.

The post development drainage area map is provided in Appendix 4. The post development runoff characteristics of the subcatchments are provided on the HydroCAD data sheets in

Appendix 5. As shown in the Table 1, the post-development peak rates of runoff from the site to the design point will be maintained or reduced in comparison to the pre-development rates.

## TABLE 1 – COMPARISON OF PRE- & POST-DEVELOPMENT DISCHARGE RATES (CFS) TO DESIGN POINT

	2-year	10-year	25-year	100-year
Pre-Development	0.0	0.0	0.1	0.4
Post Development	0.0	0.0	0.1	0.4

#### D. Pipe Sizing

The piping proposed at the site consists of smooth bore corrugated high density polyethylene pipe with smooth interior walls (CPEP-S). The roughness coefficient used for this pipe type is 0.012. The analysis provided in Appendix 5 indicates headwater elevation in the structure at each pipe inlet for the design storms and compares it to the flood elevation, which corresponds to the top of frame of the structure. The calculations indicate that all proposed pipes will have sufficient capacity to convey the 10-year storm event without surcharging out of the top of the structures.

#### E. Outlet Protection

Outfall protection for the pipe discharge from the Walgreens Stormwater Management Basin overflow at the edge of the wetland will consist of a Type A riprap apron. Similarly, a riprap channel will be installed at the inlet to the infiltration basin to convey runoff down the slope into the forebay. Outlet protection for the infiltration's spillway will consist of a 12" thick modified riprap slope extended 5 feet beyond the toe of the slope.

#### F. Summary of Results

The proposed design and analysis indicates that the proposed development will not result in negative impacts downstream. In addition, the proposed stormwater management system will intercept and divert the untreated runoff from the Panda Palace leak-off to the surface infiltration basin where it will be treated prior to discharge, whereas previously is was discharged to the wetland without treatment.

## Appendix 1:

RAINFALL DATA



#### NOAA Atlas 14, Volume 10, Version 3 Location name: Vernon Rockville, Connecticut, USA\*

Latitude: 41.8502°, Longitude: -72.4855° Elevation: 224.86 ft\*\* \* source: ESRI Mapa \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Paviovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

#### PF tabular

Duration				Average	recurrence	Interval (y	ears)			
Duradon	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.332</b> (0.254-0.434)	<b>0,403</b> (0.308-0.528)	<b>0.519</b> (0.395-0.684)	<b>0.616</b> (0.488-0.814)	<b>0.749</b> (0.551-1.03)	<b>0.849</b> (0.814-1.20)	<b>0,953</b> (0.672-1.39)	1.07 (0.718-1.60)	<b>1.24</b> (0.803-1.82)	1.38 (0.875-2.
10-mln	<b>0,470</b> (0.359-0,615)	<b>0.571</b> (0.436-0.748)	0.738 (0.560-0.987)	<b>0.872</b> (0.660-1.15)	1.06 (0.780-1.46)	<b>1.20</b> (0.868-1.69)	<b>1.35</b> (0.951-1.97)	1.52 (1.02-2.27)	1.76 (1.14-2.72)	<b>1.96</b> (1.24-3.0
15-min	<b>0.553</b> (0.423-0.724)	<b>0.672</b> (0.513-0.880)	<b>0.866</b> (0.858-1.14)	<b>1.03</b> (0.778-1.36)	<b>1.25</b> (0.918-1.72)	1.41 (1.02-1.99)	1.59 (1.12-2.32)	1.79 (1.20-2.67)	2.07 (1.34-3.20)	2.30 (1.48-3.6
30-min	<b>0.748</b> (0.571-0.979)	<b>0.909</b> (0.694-1.19)	1.17 (0.892-1.54)	1,39 (1.05-1.84)	1. <b>69</b> (1.25-2.34)	<b>1.92</b> (1.39-2.71)	2.16 (1.62-3.15)	2.43 (1.63-3.62)	2.81 (1.82-4.34)	3.13 (1.98-4.6
60-min	0.943 (0.720-1.23)	1.15 (0.875-1.50)	<b>1.48</b> (1.13-1.95)	1.78 (1.33-2.32)	2.14 (1.57-2.95)	2.42 (1.75-3.42)	2,73 (1.92-3.99)	3.07 (2.05-4.58)	3.56 (2.30-5.49)	3.96 (2.51-8.2
2-hr	<b>1.21</b> (0.931-1.58)	1.47 (1.12-1.91)	1.88 (1.44-2.46)	2.23 (1.69-2.93)	2.70 (2.00-3.72)	<b>3.05</b> (2.22-4.30)	3.43 (2.44-5.03)	3.89 (2.61-5.77)	<b>4.57</b> (2.98-7.02)	5.15 (3.27-8.0
3-hr	1.40 (1.08-1.82)	<b>1.69</b> (1.30-2.19)	2.16 (1.66-2.82)	2.58 (1.95-3.36)	3.10 (2.30-4.26)	3.50 (2.58-4.93)	3.94 (2.82-5.78)	4.47 (3.01-6.62)	5.30 (3.44-8.10)	6.01 (3.82-9.3
6-hr	1.76 (1.36-2.28)	2.13 (1.85-2.76)	2.74 (2.11-3.56)	3.25 (2.49-4.24)	<b>3.95</b> (2.95-5.41)	4.46 (3.28-6.26)	5,02 (3,62-7,35)	5.72 (3.88-8,42)	8.82 (4.44-10,4)	7.78 (4.96-12
12-hr	2.17 (1.69-2.80)	<b>2.66</b> (2.06-3.42)	3.45 (2.67-4.46)	4.10 (3.16-5.33)	5.01 (3.76-6.83)	<b>5.67</b> (4.19-7.92)	6.40 (4.63-9.32)	7.31 (4.95-10.7)	<b>8.74</b> (6.71-13.2)	<b>9.98</b> (8.38-15
24-hr	2.56 (1.99-3.27)	3.16 (2.47-4.06)	4.16 (3.23-5.35)	4.98 (3.85-8.45)	<b>6.12</b> (4.62-8.32)	<b>6.95</b> (5.16-9.68)	7.87 (5.74-11.4)	9.04 (6.14-13.1)	10.9 (7.13-16.3)	12.5 (8.03-19
2-day	<b>2.88</b> (2.26-3.67)	3.61 (2.83-4.61)	4.81 (3.76-6.16)	5,80 (4.50-7.47)	7.17 (5.44-9.72)	<b>8,16</b> (6.10-11.3)	9.28 (6.82-13.5)	10.7 (7.30-15.5)	13.1 (8.59-19.5)	15,2 (9.77-23
3-day	3.13 (2.46-3.98)	3.94 (3.09-5.01)	<b>5.25</b> (4.10-6.70)	<b>6.33</b> (4.93-8.12)	7.83 (5.96-10.6)	8.91 (6.69-12.4)	10.1 (7.48-14.7)	11.7 (8.01-16.9)	14.3 (9.44-21,3)	16.7 (10.8-25
4-day	3.37 (2.65-4.27)	<b>4.22</b> (3.32-5.36)	<b>5.61</b> (4.40-7.15)	<b>6.77</b> (5.28-8.67)	8.36 (6.37-11.3)	<b>9.52</b> (7.15-13.2)	10.8 (7.99-15.7)	12.5 (8.55-18.0)	15.3 (10.1-22.7)	17.8 (11.5-26.
7-day	<b>4.00</b> (3.16-5.08)	<b>4.96</b> (3.92-8.28)	<b>6.53</b> (5.14-8.29)	7.83 (6.13-9.99)	9.62 (7.35-12.9)	<b>10.9</b> (8.22-15.0)	12.4 (9.15-17.8)	14.3 (9.78-20.4)	17.3 (11.4-25.5)	<b>20.0</b> (12.9-30.
10-day	<b>4.64</b> (3.68-5.85)	<b>5.68</b> (4.48-7.14)	7.32 (5.78-9.27)	<b>8.70</b> (6.83-11.1)	10.6 (8.11-14.2)	12.0 (9.02-16.4)	<b>13.5</b> (9.98-19.3)	15.5 (10.8-22.1)	18.6 (12.3-27.3)	<b>21.3</b> (13.8-31.
20-day	<b>6.67</b> (5.31-8.37)	7.75 (6.17-9.74)	<b>9.53</b> (7.55-12.0)	11.0 (8.67-13.9)	13.0 (9.97-17.2)	<b>14.5</b> (10.9-19.5)	16.1 (11.8-22.5)	18.0 (12.5-25.5)	<b>20.8</b> (13.9-30.4)	23.2 (15.1-34.
30-day	<b>8.40</b> (8.70-10.5)	9.51 (7.58-11.9)	11.3 (8.99-14.2)	<b>12.8</b> (10.1-16.2)	<b>14.9</b> (11.4-19.5)	16.5 (12.3-21.9)	18,1 (13.2-24.9)	<b>19.9</b> (13.8-27.9)	<b>22.3</b> (14.9-32.4)	24.3 (15.9-36.
45-day	<b>10.6</b> (8.45-13.2)	11.7 (9.35-14.6)	<b>13.6</b> (10.8-17.0)	<b>15.1</b> (12.0-19.0)	17.2 (13.2-22.4)	18.9 (14.1-24.9)	<b>20.5</b> (14.8-27.8)	<b>22.1</b> (15.4-31.0)	24.3 (16.3-35.0)	<b>25.8</b> (16.9-38.
60-day	12.4 (9.92-15.4)	13.5 (10.8-16.9)	<b>15.4</b> (12.3-19.3)	17.0 (13.5-21.4)	19.2 (14.7-24.8)	<b>20.9</b> (15.7-27.5)	22.5 (16.3-30.4)	24.1 (16.8-33.6)	26.0 (17.4-37.4)	<b>27.3</b> (17.8-40.

Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atles 14 document for more information.

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#### PF graphical

## Appendix 2: NRCS SOILS INFORMATION

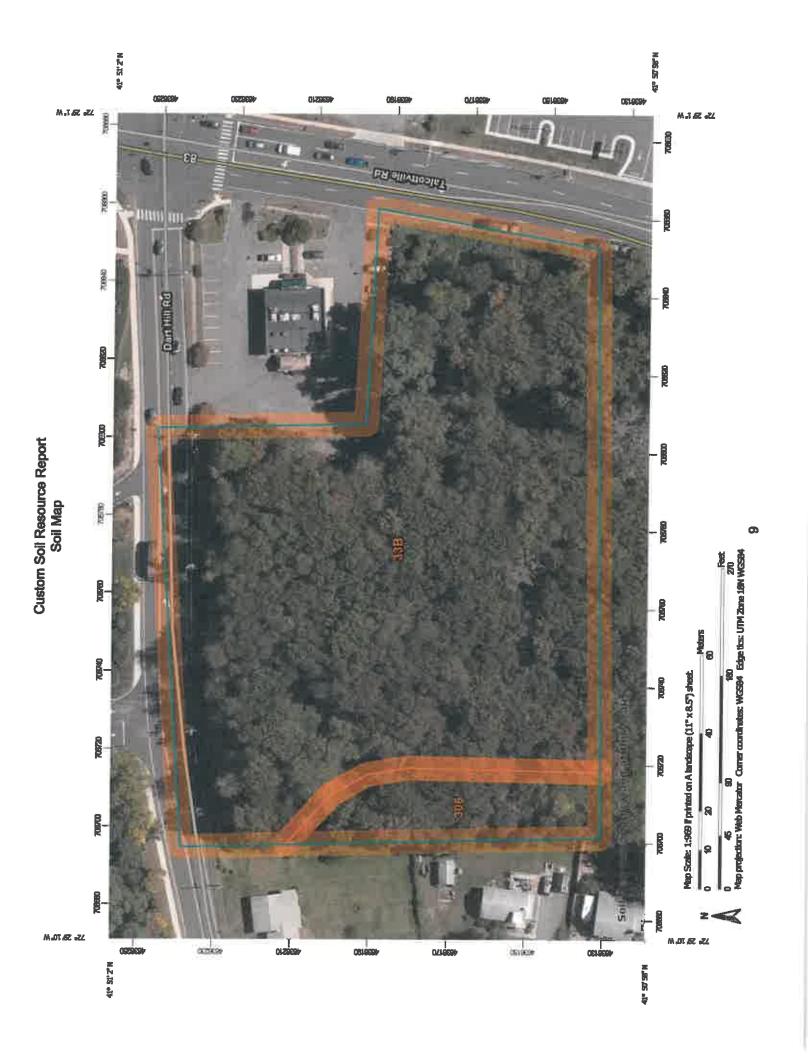


**NRCS** 

Natural Resources Conservation Service A product of the National Cooperative Soll Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

## Custom Soil Resource Report for State of Connecticut





#### **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
33B	Hartford sandy loam, 3 to 8 percent slopes	3.3	91.3%
306	Udorthents-Urban land complex	0.3	8.79
Totals for Area of Interset		3.7	100,09

#### **Map Unit Descriptions**

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soll map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

# State of Connecticut

# 33B—Hartford sandy loam, 3 to 8 percent slopes

# **Map Unit Setting**

National map unit symbol: 9lmw

Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 54 inches
Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: All areas are prime farmland

# **Map Unit Composition**

Hartford and similar soils: 80 percent Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

# **Description of Hartford**

#### Setting

Landform: Terraces, outwash plains

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Sandy glaciofluvial deposits derived from sandstone and/or basalt

# Typical profile

Ap - 0 to 8 inches: sandy loam Bw1 - 8 to 20 inches: sandy loam Bw2 - 20 to 26 inches: loamy sand

2C - 26 to 65 inches: stratified very gravelly coarse sand to loamy fine sand

# Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches Drainage class: Somewhat excessively drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95

in/hr)

Depth to water table: More than 80 Inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 4.5 inches)

# interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonimigated): 2e

Hydrologic Soil Group: A

Ecological site: F145XY008MA - Dry Outwash

Hydric soil rating: No

# **Minor Components**

#### Penwood

Percent of map unit: 5 percent Landform: Terraces, outwash plains

Down-slope shape: Convex

#### **Custom Soil Resource Report**

Across-slope shape: Linear Hydric soil rating: No

# Ellington

Percent of map unit: 5 percent Landform: Terraces, outwash plains

Down-slope shape: Linear Across-slope shape: Linear Hydric soll rating: No

#### Manchester

Percent of map unit: 5 percent

Landform: Terraces, outwash plains, kames, eskers

Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

#### Branford

Percent of map unit: 5 percent Landform: Terraces, outwash plains

Down-slope shape: Linear Across-slope shape: Linear Hydric soll rating: No

# 306—Udorthents-Urban land complex

# **Map Unit Setting**

National map unit symbol: 9Img Elevation: 0 to 2,000 feet

Mean annual precipitation: 43 to 56 inches
Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 120 to 185 days

Farmland classification: Not prime farmland

# **Map Unit Composition**

Udorthents and similar solls: 50 percent

Urban land: 35 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

# **Description of Udorthents**

#### Settina

Down-slope shape: Convex Across-slope shape: Linear Parent meterial: Drift

# Typical profile

A - 0 to 5 inches: loam

C1 - 5 to 21 inches: gravelly loam

C2 - 21 to 80 inches: very gravelly sandy loam

#### **Custom Soll Resource Report**

# Properties and qualities

Slope: 0 to 25 percent

Depth to restrictive feature: More than 80 Inches

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Very low to high (0.00

to 1.98 ln/hr)

Depth to water table: About 54 to 72 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 6.8 inches)

# Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soll Group: B Hydric soll rating: No

# **Description of Urban Land**

# Typical profile

H - 0 to 6 inches: material

# **Interpretive groups**

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 8

Hydrologic Soil Group: D Hydric soil rating: Unranked

# **Minor Components**

# Unnamed, undisturbed soils

Percent of map unit: 8 percent

Hydric soil rating: No

# Udorthents, wet substratum

Percent of map unit: 5 percent Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

# Rock outcrop

Percent of map unit: 2 percent

Hydric soil rating: No

# Appendix 3: PERMEABILITY TEST RESULTS



# NEW ENGLAND MATERIALS TESTING LAB, LLC. NEW ENGLAND REGIONAL OFFICE

72 Bissell Street Manchester, CT 06040 • Tel: 860-783-5830 • Fax: 860-783-5832

Client: JR Russo Surveyors Engineers

P. O Box 938

East Windsor, CT. 06088

**Report #: 001** 

Lab ID: 098-21

Project: 501Talcottville Road Vernon, CT.

Client ID: TP-1

Technician: Z. A

Date: 12/27/2021

# LAB PERMEABILITY TEST

Sample description: Reddish brown silty clayey sand.

Location: Onsite (501Talcottville Red Vernon, CT).

Sample depth: 30" to 36"

Method: Permeability by ASTM D2434 (Constant Head Method)

k = QL/ath

Where k = coefficient of permeability,

Q = quantity of water discharged, Q =  $800 \text{ cm}^3$ L = length of sample in centimeters L = 15.24 cmA = cross sectional area of specimen, A =  $43.10 \text{ cm}^2$ t = total time for discharge, in seconds t = 5940 sech = difference in head manometers, h = 61.5 cm

k = 0.00077435 cm/sec.

k = 1.0975 inch/hour

Reported To: JR Russo Surveyors Engineers

Submitted By: New England Materials Testing Lab, LLC.

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Client: JR Russo Surveyors Engineers

P. O Box 938

East Windsor, CT. 06088

**Report #: 002** 

Lab ID: 099-21

Project: 501Talcottville Road Vernon, CT.

Client ID: TP-2

Technician: Z. A

Date: 12/27/2021

# LAB PERMEABILITY TEST

Sample description: Reddish brown sand bank run gravel and fines.

Location: Onsite (501Talcottville Red Vernon, CT).

Sample depth: 30" to 36"

Method: Permeability by ASTM D2434 (Constant Head Method)

k = QL/ath

Where k = coefficient of permeability,

Q = quantity of water discharged, Q =  $1000 \text{ cm}^3$ L = length of sample in centimeters L = 15.24 cmA = cross sectional area of specimen, A =  $43.10 \text{ cm}^2$ t = total time for discharge, in seconds t = 4140 sech = difference in head manometers, h = 61.5 cm

k = 0.001388776 cm/sec.

k = 1.968 inch/hour

Reported To: JR Russo Surveyors Engineers

Submitted By: New England Materials Testing Lab, LLC.

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Client: JR Russo Surveyors Engineers

P. O Box 938

East Windsor, CT. 06088

**Report #: 003** 

Lab ID: 100-21

Project: 501Talcottville Road Vernon, CT.

Client ID: TP-3

Date: 12/27/2021

Technician: Z. A

# LAB PERMEABILITY TEST

Sample description: Dark brown silty clayey sand and gravel.

Location: Onsite (501Talcottville Red Vernon, CT).

Sample depth: 36" to 42"

Method: Permeability by ASTM D2434 (Constant Head Method)

k = QL/ath

Where k = coefficient of permeability,

Q = quantity of water discharged, Q =  $1000 \text{ cm}^3$ L = length of sample in centimeters L = 15.24 cmA = cross sectional area of specimen, A =  $43.10 \text{ cm}^2$ t = total time for discharge, in seconds t = 3480 sech = difference in head manometers, h = 61.5 cm

k = 0.001652165 cm/sec.

k = 2.341 inch/hour

Reported To: JR Russo Surveyors Engineers

Submitted By: New England Materials Testing Lab, LLC.

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Client: JR Russo Surveyors Engineers

P. O Box 938

East Windsor, CT. 06088

**Report #: 001** 

Lab ID: 101-21

Project: 501Talcottville Road Vernon, CT.

Client ID: TP-4

Technician: Z. A

Date: 12/27/2021

# LAB PERMEABILITY TEST

Sample description: Reddish brown sand and little gravel.

Location: Onsite (501Talcottville Red Vernon, CT).

Sample depth: 30" to 36"

Method: Permeability by ASTM D2434 (Constant Head Method)

k = QL/ath

Where k = coefficient of permeability,

Q = quantity of water discharged, Q =  $1000 \text{ cm}^3$ L = length of sample in centimeters L = 15.24 cmA = cross sectional area of specimen, A =  $43.10 \text{ cm}^2$ t = total time for discharge, in seconds t = 240 sech = difference in head manometers, h = 61.5 cm

k = 0.023956388 cm/sec.

k = 33.953 inch/hour

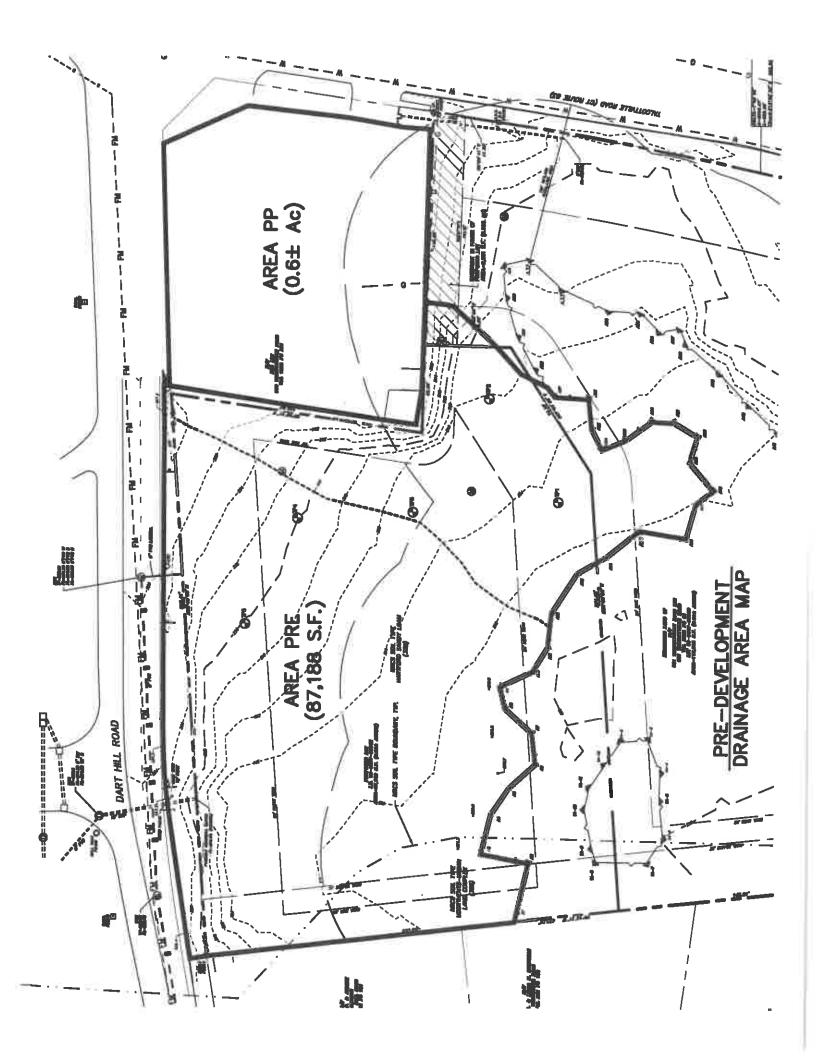
Reported To: JR Russo Surveyors Engineers

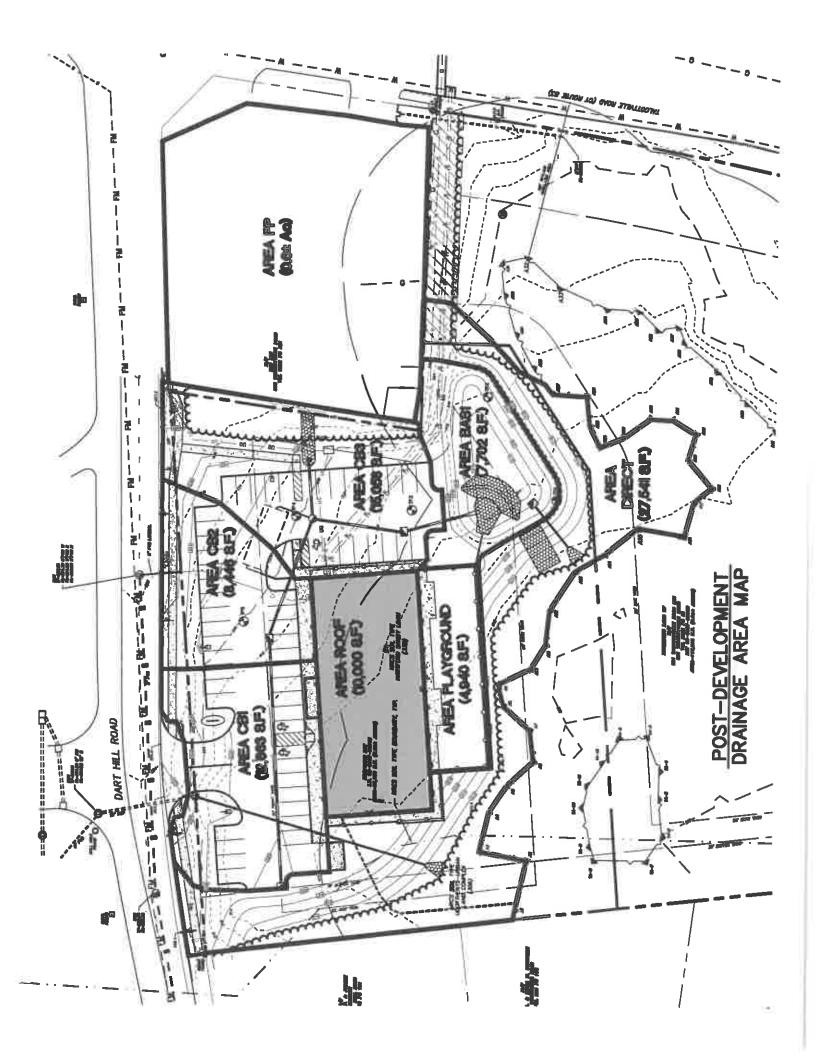
Submitted By: New England Materials Testing Lab, LLC.

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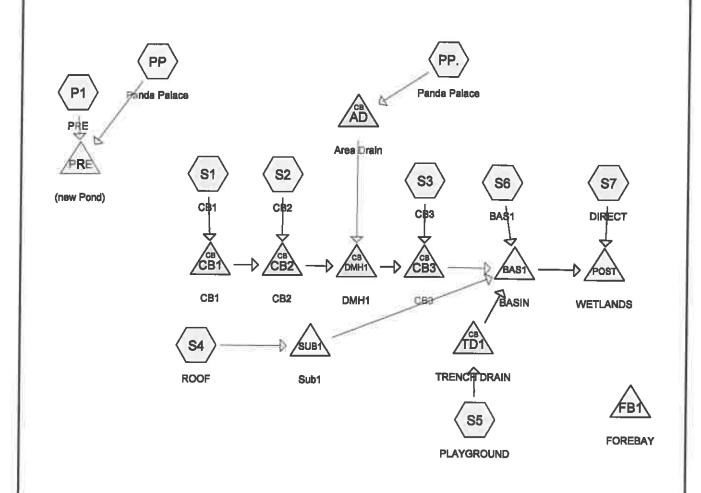
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# Appendix 4: DRAINAGE AREA MAPS





# Appendix 5: HYDROCAD ANALYSES











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# Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Sim-Route method - Pond routing by Sim-Route method

				-, -, -, -, -, -, -, -, -, -, -, -, -, -	- mounou	
Subcatchment P1: PRE		Runoff Area=8 Flow Length=276'				
Subcatchment PP: Pand	a Palace	Runoff Area≖0.6	600 ac 100 Tc=5.0 r	0.00% Imper min CN=98	vious Runoff Runoff=3.03	Depth>4.74" cfs 0.237 af
Subcatchment PP.: Pane	ia Palace	Runoff Area=0.6	00 ac 100 Tc=5.0 r	0.00% Impen min CN=98	vious Runoff i Runoff=3.03	Depth>4.74" cfs 0.237 af
Subcatchment S1: CB1		Runoff Area=12	,663 sf <i>77</i> Tc=5.0 r	7.95% Imper min CN=85	vious Runoff I Runoff=1.17	Depth=3.35" cfs 0.081 af
Subcatchment S2: CB2		Runoff Area=8	,446 sf  70 Tc=5.0 r	0.89% (mperv min CN=81	ious Runoff I/	Depth=2.97" cfs 0.048 af
Subcatchment S3: CB3		Runoff Area=15			vious Runoff I Runoff=0.83	
Subcatchment S4: ROOF	:	Runoff Area=10,0	000 sf 100 Tc=5.0 n	0.00% !mperv min CN=98	vious Runoff I Runoff=1.16 d	Depth>4.74" cfs 0.091 af
Subcatchment S5: PLAY	GROUND	Runoff Area=4,9	040 sf 100 Tc=5.0 n	0.00% Imperv min CN=98	rious Runoff [ Runoff=0.57 d	Depth>4.74" cfs 0.045 af
Subcatchment S6: BAS1		Runoff Area=	7,702 sf 0 Tc=5.0 n	0.16% Imperv min CN=38	rious Runoff [ Runoff=0.00 c	Depth=0.16" cfs 0.002 af
Subcatchment S7: DIREC		Runoff Area=27 Flow Length=147'				
Pond AD: Area Drain	15.0" Round	Culvert n=0.012	Peak E L=50.0' S	Elev=218.68' S=0.0050 '/' (	Inflow=3.03 c Outflow=3.03 c	ifs 0.237 af ifs 0.237 af
Pond BAS1: BASIN	Discarded=0.09 c	Peak Elev=217 s 0.197 af Prima	7.27' Stora ary=2.27 cf	age=8,274 cf fs 0.275 af (	Inflow=6.26 c Outflow=2.37 c	ifs 0.472 af ifs 0.472 af
Pond CB1: CB1	15.0" Round	Cuivert n=0.012 L	Peak E =154.0' S	Elev=218.52' 6=0.0050 '/' (	Inflow=1.17 c Outflow=1.17 c	fs 0.081 af fs 0.081 af
Pond CB2: CB2	15.0" Round	Culvert n=0.012	Peak E L=76.0' S	Eiev=218.48' 6=0.0050 '/' (	Inflow=1.87 c Outflow=1.87 c	fs 0.129 af fs 0.129 af
Pond CB3: CB3	15.0" Round	Culvert n=0.012			Inflow=5.71 c Outflow=5.71 c	
Pond DMH1: DMH1	15.0" Round	Culvert n=0.012			inflow=4.89 c Outflow=4.89 c	

2021-083 Vernon TLE2

Type III 24-hr 10-year Rainfall=4.98"

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Pond FB1: FOREBAY

Peak Elev=0.00' Storage=0 cf

**Pond POST: WETLANDS** 

Inflow=2.28 cfs 0.287 af

Primary=2.28 cfs 0.287 af

Pond PRE: (new Pond)

Inflow=3.03 cfs 0.241 af

Primary=3.03 cfs 0.241 af

Pond SUB1: Sub1

Peak Elev=216.23' Storage=697 cf inflow=1.16 cfs 0.091 af

Discarded=0.36 cfs 0.236 af Primary=0.00 cfs 0.000 af Outflow=0.36 cfs 0.236 af

**Pond TD1: TRENCH DRAIN** 

Peak Elev=218.12' Inflow=0.57 cfs 0.045 af

6.0" Round Culvert n=0.012 L=20.0' S=0.0250 '/' Outflow=0.57 cfs 0.045 af

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# Summary for Subcatchment P1: PRE

Runoff

0.01 cfs @ 21.37 hrs, Volume=

0.004 af, Depth= 0.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=4.98"

Α	rea (sf)	CN	Description					
	81,137		Woods, Good, HSG R					
	6,051		Weighted Average					
87,188 32 Weighted Average 87,188 100.00% Pervious Area					a			
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description			
11.8	100	0.0900	0.14		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"			
2.5	176	0.0570	1.19		Shallow Concentrated Flow, Woodland Kv= 5.0 fps			
14.3	276	Total						

# Summary for Subcatchment PP: Panda Palace

Runoff

3.03 cfs @ 12.07 hrs, Volume=

0.237 af, Depth> 4.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=4.98"

_	Area	(ac)	CN D	escription			
*	0.	.600	98 In	npervious			
	0.	.600	10	00.00% Impe	ervious Area		
	Tc (min)	Lengti (feet				Description	
	5.0					Direct Entry,	

# Summary for Subcatchment PP.: Panda Palace

Runoff

3.03 cfs @ 12.07 hrs, Volume=

0.237 af, Depth> 4.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=4.98"

	Area (ac)	CN	Description
*	0.600	98	Impervious
	0.600		100.00% Impervious Area

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Type III 24-hr 10-year Rainfall=4.98"

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Tc (min)	Length (feet)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0				Direct Entry,

# **Summary for Subcatchment S1: CB1**

Runoff

1.17 cfs @ 12.07 hrs, Volume=

0.081 af, Depth= 3.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=4.98"

A	rea (sf)	CN	Description	l		
	9,871	98	Paved park	ing, HSG A		
	2,792	39	>75% Ġras	s cover Go	ood, HSG A	
	12,663 2,792 9,871		_	verage rvious Area pervious Ar		
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
5.0					Direct Entry,	

# Summary for Subcatchment \$2: CB2

Runoff

0.70 cfs @ 12.07 hrs, Volume=

0.048 af. Depth= 2.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=4.98"

,A	rea (sf)	CN	Description			
	5,987	98	Paved park	ing, HSG A	<u> </u>	
	2 459	39	>75% Gras	s cover. Go	ood, HSG A	
	8,446 2,459 5,987		Weighted A 29.11% Pei 70.89% Imp	rvious Area		
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description	
5.0					Direct Entry,	

# **Summary for Subcatchment S3: CB3**

Runoff

0.83 cfs @ 12.08 hrs, Volume=

0.058 af, Depth= 2.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=4.98"

# 2021-083 Vernon TLE2

Type III 24-hr 10-year Rainfall=4.98"

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A	vea (sf)	CN	Description						
	8,302	98	Paved parking, HSG A						
	3,743		75% Grass cover, Good, HSG A						
	3.013			od, HSG A	•				
	15,058	70 \	Weighted Average						
	6,756			rvious Area					
	8,302			pervious Ar					
Tc	Length	Slope		Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.0					Direct Entry.				

# **Summary for Subcatchment S4: ROOF**

Runoff = 1.16 cfs @ 12.07 hrs, Volume=

0.091 af, Depth> 4.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=4.98"

A	rea (sf)	CN [	Description			
	10,000	98 F	Roofs, HSC	S A		
	10,000	1	00.00% In	npervious A	Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
5.0					Direct Entry,	

# Summary for Subcatchment S5: PLAYGROUND

Runoff = 0.57 cfs @ 12.07 hrs, Volume=

0.045 af, Depth> 4.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=4.98"

A	rea (sf)	CN	Description			
	4,940	98	Paved park	ing HSG A	4	
	4,940		100.00% In	npervious A	Area	
Tc (min)_	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
5.0					Direct Entry,	

# **Summary for Subcatchment S6: BAS1**

Runoff = 0.00 cfs @ 13.67 hrs, Volume=

0.002 af, Depth= 0.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=4.98"

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Α	rea (sf)	CN I	Description						
	7,112	39 >	75% Grass cover, Good, HSG A						
	578			/oods, Good, HSG A					
	12	_ 98 F	Paved parking, HSG A						
	7,702	38 V	Weighted Average						
	7,690			vious Area					
	12	C	).16% Impe	ervious Are	a				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

# **Summary for Subcatchment S7: DIRECT**

Runoff = 0.03 cfs @ 12.73 hrs, Volume=

0.012 af, Depth= 0.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfali=4.98"

	rea (sf)	CN I	Description		
	995	39 [	Disconnect	ed Sidewal	k. HSG A
	8,548			od, HSG A	
	3,631			od, HSG B	
	11,947		,	•	ood, HSG A
	2,420				ood HSG B
	27,541		Neighted A		
	27,541			ervious Are	a
		,			vee
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
0.1	6	0.0200	0.78		Sheet Flow, IM
				Smooth surfaces n= 0.011 P2= 3.20"	
1.8	22	0.0636	0.20		Sheet Flow, GR
					Grass: Short n= 0.150 P2= 3.20"
1.2	25	0.2400	0.35		Sheet Flow, GR
					Grass: Short n= 0.150 P2= 3,20"
1.4	20	0.1000	0.23		Sheet Flow, GR
					Grass: Short n= 0.150 P2= 3.20"
18.5	74	0.0160	0.07		Sheet Flow, W
					Woods: Light underbrush n= 0.400 P2= 3.20"
23.0	147	Total			

# Summary for Pond AD: Area Drain

Inflow Area =	0.600 ac,100.00% Impervious, Inflow I	Depth > 4.74" for 10-year event	
Inflow =	3.03 cfs @ 12.07 hrs, Volume=	0.237 af	
Outflow =	3.03 cfs @ 12.08 hrs, Volume=	0.237 af, Atten= 0%, Lag= 0.6 min	
Primary =	3.03 cfs @ 12.08 hrs, Volume=	0.237 af	

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

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Peak Elev= 218.68' @ 12.12 hrs

Flood Elev= 221.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	217.50'	15.0" Round Culvert L= 50.0' Ke= 0.500
			Inlet / Outlet Invert= 217.50' / 217.25' S= 0.0050 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf
			n= 0.012. Flow Area= 1.23 st

Primary OutFlow Max=2.88 cfs @ 12.08 hrs HW=218.54' TW=218.08' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.88 cfs @ 3.57 fps)

# **Summary for Pond BAS1: BASIN**

Inflow Area =	1.950 ac, 76.81% Impervious, Inflow Do	epth = 2.90" for 10-year event
Inflow =	6.26 cfs @ 12.10 hrs, Volume=	0.472 af
Outflow =	2.37 cfs @ 12.34 hrs, Volume=	0.472 af, Atten= 62%, Lag= 14.3 min
Discarded =	0.09 cfs @ 12.34 hrs, Volume=	0.197 af
Primary =	2.27 cfs @ 12.34 hrs, Volume=	0.275 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 217.27' @ 12.34 hrs Surf.Area= 3,671 sf Storage= 8,274 cf Flood Elev= 219.00' Surf.Area= 5,027 sf Storage= 15,782 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 388.3 min ( 1,168.3 - 780.0 )

#1	214.00'	15,782 cf <b>Custom</b>	Stage Data (Prismatic) Listed below (	Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
214.00	1,466	0	0	
216.00	2,747	4,213	4,213	
216.10	2,814	278	4,491	
218.00	4,210	6,673	11,164	
219.00	5,027	4,619	15,782	

Device	Routing	Invert	Outlet Devices
#1	Discarded	214.00'	0.550 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 212.00'
#2	Primary	218.50'	10.0' long x 7.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.40 2.52 2.70 2.68 2.68 2.67 2.66 2.65 2.65
			2.65 2.66 2.65 2.66 2.68 2.70 2.73 2.78
#3	Primary	212.30'	10.0" Round Culvert L= 34.0' Ke= 0.500
			Inlet / Outlet Invert= 212.30' / 212.00' S= 0.0088 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.55 sf
#4	Device 3	216.10'	10.0" Vert. Orifice/Grate C= 0.600
#5	Device 3	218.30'	18.0" x 36.0" Horiz. Orifice/Grate
			Limited to weir flow at low heads

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Discarded OutFlow Max=0.09 cfs @ 12.34 hrs HW=217.27' (Free Discharge) 1=Exfiltration (Controls 0.09 cfs)

Primary OutFlow Max=2.27 cfs @ 12.34 hrs HW=217.27' TW=0.00' (Dynamic Tailwater)

—2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
—3=Culvert (Passes 2.27 cfs of 5.60 cfs potential flow)

-4=Orifice/Grate (Orifice Controls 2.27 cfs @ 4.17 fps)

-5=Orifice/Grate (Controls 0.00 cfs)

# **Summary for Pond CB1: CB1**

Inflow Area = 0.291 ac, 77.95% Impervious, Inflow Depth = 3.35" for 10-year event

Inflow = 1.17 cfs @ 12.07 hrs, Volume= 0.081 af

Outflow = 1.17 cfs @ 12.08 hrs, Volume= 0.081 af, Atten= 0%, Lag= 0.6 min

Primary = 1.17 cfs @ 12.08 hrs, Volume= 0.081 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 218.52' @ 12.13 hrs

Flood Elev= 219.60'

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=217.92' TW=218.06' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

# **Summary for Pond CB2: CB2**

Inflow Area = 0.485 ac, 75.12% Impervious, Inflow Depth = 3.20" for 10-year event

Inflow = 1.87 cfs @ 12.08 hrs, Volume= 0.129 af

Outflow = 1.87 cfs @ 12.09 hrs, Volume= 0.129 af, Atten= 0%, Lag= 0.6 min

Primary = 1.87 cfs @ 12.09 hrs, Volume= 0.129 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 218.48' @ 12.12 hrs

Flood Elev= 220.00'

Device	Routing	invert	Outlet Devices
#1	Primary	215.33'	15.0" Round Culvert L= 76.0' Ke= 0.500 Inlet / Outlet Invert= 215.33' / 214.95' S= 0.0050 '/' Cc= 0.900 n= 0.012. Flow Area= 1.23 sf

Primary OutFlow Max=0.00 cfs @ 12.09 hrs HW=218.19' TW=218.23' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

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# Summary for Pond CB3: CB3

1.430 ac. 80.73% Impervious. Inflow Depth > 3.56" for 10-year event Inflow Area =

Inflow 5.71 cfs @ 12.09 hrs, Volume= 0.424 af

Outflow 5.71 cfs @ 12.10 hrs, Volume= = 0.424 af. Atten= 0%, Lag= 0.6 min

5.71 cfs @ 12.10 hrs, Volume= Primary = 0.424 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 217.78' @ 12.12 hrs

Flood Elev= 220.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	214.68'	15.0" Round Culvert L= 44.0' Ke= 0.500 Inlet / Outlet Invert= 214.68' / 214.46' S= 0.0050 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=5.54 cfs @ 12.10 hrs HW=217.74' TW=216.86' (Dynamic Tailwater) 1=Culvert (Inlet Controls 5.54 cfs @ 4.52 fps)

# Summary for Pond DMH1: DMH1

Inflow Area = 1.085 ac, 88.89% Impervious, Inflow Depth > 4.05" for 10-year event

Inflow 4.89 cfs @ 12.08 hrs, Volume= 0.366 af

4.89 cfs @ 12.09 hrs, Volume= Outflow 0.366 af, Atten= 0%, Lag= 0.6 min

4.89 cfs @ 12.09 hrs. Volume= Primary 0.366 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 218.39' @ 12.11 hrs

Flood Elev= 220.81'

Device	Routing	Invert	Outlet Devices
#1	Primary	214.95'	15.0" Round Culvert L= 54.0' Ke= 0.500 Inlet / Outlet Invert= 214.95' / 214.68' S= 0.0050 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=4.53 cfs @ 12.09 hrs HW=218.27' TW=217.68' (Dynamic Tailwater) -1=Culvert (Inlet Controls 4.53 cfs @ 3.69 fps)

# **Summary for Pond FB1: FOREBAY**

Volume	Invert Av	/ail.Storage	Storage	e Description	
#1	214.00'	1,207 cf	Custom	m Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-fl		c.Store c-feet)	Cum.Store (cubic-feet)	
214.00	14	4	0	0	
216.00	47:	3	617	617	
217.00	70	7	590	1.207	

1.207

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# **Summary for Pond POST: WETLANDS**

Inflow Area = 2.582 ac, 58.01% Impervious, Inflow Depth = 1.34" for 10-year event

Inflow = 2.28 cfs @ 12.34 hrs, Volume= 0.287 af

Primary = 2.28 cfs @ 12.35 hrs, Volume= 0.287 af, Atten= 0%, Lag= 0.6 min

Routing by Sim-Route method, Time Span= 1,00-72.00 hrs. dt= 0.01 hrs.

# **Summary for Pond PRE: (new Pond)**

Inflow Area = 2.602 ac, 23.06% Impervious, Inflow Depth > 1.11" for 10-year event

Inflow = 3.03 cfs @ 12.07 hrs, Volume= 0.241 af

Primary = 3.03 cfs @ 12.08 hrs, Volume= 0.241 af, Atten= 0%, Lag= 0.6 min

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs. dt= 0.01 hrs

# **Summary for Pond SUB1: Sub1**

Inflow Area = 0.230 ac,100.00% Impervious, Inflow Depth > 4.74" for 10-year event Inflow = 0.091 af

Outflow = 0.36 cfs @ 12.36 hrs, Volume= 0.236 af, Atten= 69%, Lag= 17.3 min

Discarded = 0.36 cfs @ 12.36 hrs, Volume= 0.236 af Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 216.23' @ 12.36 hrs Surf.Area= 618 sf Storage= 697 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 10.6 min ( 757.8 - 747.2 )

Volume	Invert	Avail.Storage	Storage Description
#1A	214.50'	589 cf	15.75'W x 39.22'L x 3.50'H Fleid A
			2,162 cf Overall - 689 cf Embedded = 1,473 cf x 40.0% Voids
#2A	215.00'	689 cf	ADS_StormTech SC-740 +Cap x 15 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			15 Chambers in 3 Rows
#3	217.00'	39 cf	Manhole (Prismatic) Listed below (Recalc)
		1.317 cf	Total Available Storage

# Storage Group A created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
217.00	13	0	0
220.00	13	39	39

Device	Routing	Invert	Outlet Devices	
#1	Discarded	214.50'	15.000 in/hr Exfiltration over Surface area	
			Conductivity to Groundwater Elevation = 212,00'	

# 2021-083 Vernon TLE2

Type III 24-hr 10-year Rainfall=4.98"

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#2 Primary

217.00' **6.0" Round Culvert** L= 22.0' Ke= 0.500

Inlet / Outlet Invert= 217.00' / 216.80' S= 0.0091 '/' Cc= 0.900

n= 0.012, Flow Area= 0.20 sf

Discarded OutFlow Max=0.36 cfs @ 12.36 hrs HW=216.23' (Free Discharge) 1=Exfiltration (Controls 0.36 cfs)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=214.50' TW=214.00' (Dynamic Tailwater)

—2=Culvert (Controls 0.00 cfs)

# **Summary for Pond TD1: TRENCH DRAIN**

Inflow Area = 0.113 ac,100.00% Impervious, Inflow Depth > 4.74" for 10-year event

Inflow = 0.57 cfs @ 12.07 hrs, Volume= 0.045 af

Outflow = 0.57 cfs @ 12.08 hrs, Volume= 0.045 af, Atten= 0%, Lag= 0.6 min

Primary = 0.57 cfs @ 12.08 hrs, Volume= 0.045 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 218.12' @ 12.08 hrs

Flood Elev= 220.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	217.50'	6.0" Round Culvert L= 20.0' Ke= 0.500 Inlet / Outlet Invert= 217.50' / 217.00' S= 0.0250 '/' Cc= 0.900 n= 0.012. Flow Area= 0.20 sf

Primary OutFlow Max=0.57 cfs @ 12.08 hrs HW=218.12' TW=216.74' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.57 cfs @ 2.91 fps)

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# Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Sim-Route method - Pond routing by Sim-Route method

Subcatchment P1: PRE	Runoff Area=87,188 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=276' Tc=14.3 min CN=32 Runoff=0.00 cfs 0.000 af
Subcatchment PP: Pand	a Palace Runoff Area=0.600 ac 100.00% Impervious Runoff Depth=2.93"  Tc=5.0 min CN=98 Runoff=1.91 cfs 0.146 af
Subcatchment PP.: Pand	la Palace Runoff Area=0.600 ac 100.00% Impervious Runoff Depth=2.93"  Tc=5.0 min CN=98 Runoff=1.91 cfs 0.146 af
Subcatchment S1: CB1	Runoff Area=12,663 sf 77.95% Impervious Runoff Depth=1.72" Tc=5.0 min CN=85 Runoff=0.61 cfs 0.042 af
Subcatchment S2: CB2	Runoff Area=8,446 sf 70.89% impervious Runoff Depth=1.44" Tc=5.0 min CN=81 Runoff=0.34 cfs 0.023 af
Subcatchment S3: CB3	Runoff Area=15,058 sf 55.13% Impervious Runoff Depth=0.80" Tc=5.0 min CN=70 Runoff=0.30 cfs 0.023 af
Subcatchment S4: ROOF	Runoff Area=10,000 sf 100.00% Impervious Runoff Depth=2.93" Tc=5.0 min CN=98 Runoff=0.73 cfs 0.056 af
Subcatchment S5: PLAY	GROUND Runoff Area=4,940 sf 100.00% Impervious Runoff Depth=2.93" Tc=5.0 min CN=98 Runoff=0.36 cfs 0.028 af
Subcatchment S6: BAS1	Runoff Area=7,702 sf 0.16% Impervious Runoff Depth=0.00" Tc=5.0 min CN=38 Runoff=0.00 cfs 0.000 af
Subcatchment S7: DIREC	Runoff Area=27,541 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=147' Tc=23.0 min CN=40 Runoff=0.00 cfs 0.000 af
Pond AD: Area Drain	Peak Elev=218.28' Inflow=1.91 cfs 0.146 af 15.0" Round Culvert n=0.012 L=50.0' S=0.0050 '/' Outflow=1.91 cfs 0.146 af
Pond BAS1: BASIN	Peak Elev=216.51' Storage=5,692 cf Inflow=3.49 cfs 0.262 af Discarded=0.07 cfs 0.181 af Primary=0.57 cfs 0.082 af Outflow=0.64 cfs 0.263 af
Pond CB1: CB1	Peak Elev=216.62' Inflow=0.61 cfs 0.042 af 15.0" Round Culvert n=0.012 L=154.0' S=0.0050 '/' Outflow=0.61 cfs 0.042 af
Pond CB2: CB2	Peak Elev=216.52' Inflow=0.94 cfs 0.065 af 15.0" Round Culvert n=0.012 L=76.0' S=0.0050 '/' Outflow=0.94 cfs 0.065 af
Pond CB3: CB3	Peak Elev=216.52' Inflow=3.14 cfs 0.235 af 15.0" Round Culvert n=0.012 L=44.0' S=0.0050 '/' Outflow=3.14 cfs 0.235 af
Pond DMH1: DMH1	Peak Elev=216.52' Inflow=2.84 cfs 0.211 af 15.0" Round Culvert n=0.012 L=54.0' S=0.0050 '/' Outflow=2.84 cfs 0.211 af

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**Pond FB1: FOREBAY** 

Peak Elev=0.00' Storage=0 cf

**Pond POST: WETLANDS** 

Inflow=0.57 cfs 0.082 af

Primary=0.57 cfs 0.082 af

Pond PRE: (new Pond)

Inflow=1.91 cfs 0.146 af

Primary=1.91 cfs 0.146 af

Pond SUB1: Sub1

Peak Elev=215.35' Storage=294 cf Inflow=0.73 cfs 0.056 af

Discarded=0.29 cfs 0.219 af Primary=0.00 cfs 0.000 af Outflow=0.29 cfs 0.219 af

**Pond TD1: TRENCH DRAIN** 

Peak Elev=217.90' Inflow=0.36 cfs 0.028 af

6.0" Round Culvert n=0.012 L=20.0' S=0.0250 '/' Outflow=0.36 cfs 0.028 af

# Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Sim-Route method - Pond routing by Sim-Route method

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Subcatchment P1: PRE		Runoff Area=8 Flow Length=276'	7,188 sf 0.0 Tc=14.3 min	0% Imperviou CN=32 Ru	s Runoff De inoff=0.04 cf:	epth=0.15" s 0.025 af
Subcatchment PP: Panda Palace		Runoff Area=0.6	00 ac 100.0 Tc=5.0 min	0% Imperviou CN=98 Ru	s Runoff De inoff=3.73 cfs	epth>5.88" s 0.294 af
Subcatchment PP.: Pane	da Palace	Runoff Area=0.6	00 ac 100.00 Tc=5.0 min	0% Imperviou CN=98 Ru	s Runoff De noff=3.73 cfs	epth>5.88" s 0.294 af
Subcatchment S1: CB1		Runoff Area=12	,663 sf 77.98 Tc=5.0 min	5% Imperviou CN=85 Ru	s Runoff De noff=1.53 cfs	epth=4.42" 3 0.107 af
Subcatchment S2: CB2		Runoff Area=8,	446 sf 70.89 Tc=5.0 min	9% Imperviou CN=81 Ru	Runoff De	epth=3.99" 5 0.065 af
Subcatchment S3: CB3		Runoff Area=15,	058 sf 55.13 Tc=5.0 min	3% Impervious CN=70 Ru	Runoff De	pth=2.90" 0.084 af
Subcatchment S4: ROOF	•	Runoff Area=10,0	00 sf 100.00 Tc=5.0 min	% Impervious CN=98 Ru	Runoff De	pth>5.88" 0.113 af
Subcatchment S5: PLAY	GROUND	Runoff Area=4,9	40 sf 100.00 Tc=5.0 min	)% Impervious CN=98 Ru	Runoff Denoff=0.70 cfs	pth>5.88" 0.056 af
Subcatchment S6: BAS1		Runoff Area=7	7,702 sf 0.16 Tc=5.0 min	3% impervious CN=38 Rui	Runoff Der	pth=0.43" 0.006 af
Subcatchment S7: DIREC		Runoff Area=27 Flow Length=147'	7,541 sf 0.00 Tc=23.0 min	% Impervious CN=40 Rui	Runoff Dep	oth=0.54" 0.028 af
Pond AD: Area Drain	15.0" Roun	d Culvert n=0.012	Peak Elev L=50.0' S=0.	/=220.12' Inf .0050 '/' Outf	low=3.73 cfs low=3.73 cfs	0.294 af 0.294 af
Pond BAS1: BASIN	Discarded=0.11	Peak Elev=217. cfs 0.203 af Prima	75' Storage= ry=2.92 cfs 0	10,130 cf Inf 0.408 af Outf	low=8.05 cfs low=3.02 cfs	0.611 af 0.611 af
Pond CB1: CB1	15.0" Round	Culvert n=0.012 L	Peak Elev =154.0' S=0.	/=220.05' Inf 0050 '/' Outf	low=1.53 cfs low=1.53 cfs	0.107 af 0.107 af
Pond CB2: CB2	15.0" Roun	d Culvert n=0.012	Peak Elev L=76.0' S=0.	/=219.97' infl 0050'/' Outfl	ow=2.46 cfs ow=2.46 cfs	0.171 af 0.171 af
Pond CB3: CB3	15.0" Roun	d Culvert n=0.012	Peak Elev L=44.0' S=0.	r=218.80' Infl 0050 '/' Outfl	ow=7.37 cfs ow=7.37 cfs	0.549 af 0.549 af
Pond DMH1: DMH1	15.0" Roun	d Culvert n=0.012	Peak Elev L=54.0' S=0.	/=219.81' infl 0050 '/' Outfl	ow=6.18 cfs ow=6.18 cfs	0.466 af 0.466 af

2021-083 Vernon TLE2

Type III 24-hr 25-year Rainfall=6.12"

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**Pond FB1: FOREBAY** 

Peak Elev=0.00' Storage=0 cf

**Pond POST: WETLANDS** 

Inflow=3.00 cfs 0.436 af

Primary=3.00 cfs 0.436 af

Pond PRE: (new Pond)

Inflow=3.73 cfs 0.319 af

Primary=3.73 cfs 0.319 af

Pond SUB1: Sub1

Peak Elev=216.88' Storage=961 cf Inflow=1.43 cfs 0.113 af

Discarded=0.42 cfs 0.247 af Primary=0.00 cfs 0.000 af Outflow=0.42 cfs 0.247 af

**Pond TD1: TRENCH DRAIN** 

Peak Elev=218.31' Inflow=0.70 cfs 0.056 af

6.0" Round Culvert n=0.012 L=20.0' S=0.0250 '/' Outflow=0.70 cfs 0.056 af

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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Sim-Route method - Pond routing by Sim-Route method

Reach ro	uting by Sim-Route method - Pond routing by Sim-Route method
Subcatchment P1: PRE	Runoff Area=87,188 sf 0.00% Impervious Runoff Depth=0.53" Flow Length=276' Tc=14.3 min CN=32 Runoff=0.35 cfs 0.088 af
Subcatchment PP: Pand	la Palace Runoff Area=0.600 ac 100.00% Impervious Runoff Depth>7.63" Tc=5.0 min CN=98 Runoff=4.80 cfs 0.381 af
Subcatchment PP.: Pane	da Palace Runoff Area=0.600 ac 100.00% Impervious Runoff Depth>7.63"  Tc=5.0 min CN=98 Runoff=4.80 cfs 0.381 af
Subcatchment S1: CB1	Runoff Area=12,663 sf 77.95% Impervious Runoff Depth=6.09" Tc=5.0 min CN=85 Runoff=2.08 cfs 0.147 af
Subcatchment S2: CB2	Runoff Area=8,446 sf 70.89% Impervious Runoff Depth=5.62" Tc=5.0 min CN=81 Runoff=1.30 cfs 0.091 af
Subcatchment S3: CB3	Runoff Area=15,058 sf 55.13% Impervious Runoff Depth=4.35" Tc=5.0 min CN=70 Runoff=1.83 cfs 0.125 af
Subcatchment S4: ROOF	Runoff Area=10,000 sf 100.00% Impervious Runoff Depth>7.63" Tc=5.0 min CN=98 Runoff=1.84 cfs 0.146 af
Subcatchment S5: PLAY	GROUND Runoff Area=4,940 sf 100.00% Impervious Runoff Depth>7.63"  Tc=5.0 min CN=98 Runoff=0.91 cfs 0.072 af
Subcatchment S6: BAS1	Runoff Area=7,702 sf 0.16% Impervious Runoff Depth=1.01" Tc=5.0 min CN=38 Runoff=0.13 cfs 0.015 af
Subcatchment S7: DIREC	Runoff Area=27,541 sf 0.00% Impervious Runoff Depth=1.19" Flow Length=147' Tc=23.0 min CN=40 Runoff=0.40 cfs 0.063 af
Pond AD: Area Drain	Peak Elev=222.87' Inflow=4.80 cfs 0.381 af 15.0" Round Culvert n=0.012 L=50.0' S=0.0050 '/' Outflow=4.80 cfs 0.381 af
Pond BAS1: BASIN	Peak Elev=218.38' Storage=12,818 cf Inflow=10.95 cfs 0.833 af Discarded=0.12 cfs 0.212 af Primary=4.24 cfs 0.621 af Outflow=4.36 cfs 0.833 af
Pond CB1: CB1	Peak Elev=222.79' Inflow=2.08 cfs 0.147 af 15.0" Round Culvert n=0.012 L=154.0' S=0.0050 '/' Outflow=2.08 cfs 0.147 af
Pond CB2; CB2	Peak Elev=222.64' Inflow=3.37 cfs 0.238 af 15.0" Round Culvert n=0.012 L=76.0' S=0.0050 '/' Outflow=3.37 cfs 0.238 af
Pond CB3: CB3	Peak Elev=220.54' Inflow=9.95 cfs 0.745 af 15.0" Round Culvert n=0.012 L=44.0' S=0.0050 '/' Outflow=9.95 cfs 0.745 af
Pond DMH1: DMH1	Peak Elev=222.34' Inflow=8.16 cfs 0.620 af 15.0" Round Culvert n=0.012 L=54.0' S=0.0050 '/' Outflow=8.16 cfs 0.620 af

2021-083 Vernon TLE2

Type III 24-hr 100-year Rainfall=7.87"

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**Pond FB1: FOREBAY** 

Peak Elev=0.00' Storage=0 cf

**Pond POST: WETLANDS** 

Inflow=4.60 cfs 0.684 af

Primary=4.60 cfs 0.684 af

Pond PRE: (new Pond)

Inflow=4.80 cfs 0.469 af

Primary=4.80 cfs 0.469 af

Pond SUB1: Sub1

Peak Elev=218.67' Storage=1,300 cf Inflow=1.84 cfs 0.146 af

Discarded=0.58 cfs 0.265 af Primary=0.52 cfs 0.001 af Outflow=1.10 cfs 0.266 af

Pond TD1: TRENCH DRAIN

Peak Elev=218.67' Inflow=0.91 cfs 0.072 af

6.0" Round Culvert n=0.012 L=20.0' S=0.0250 '/' Outflow=0.91 cfs 0.072 af

# Appendix 6: MISCELLANEOUS CALCULATIONS

J.R. RUSSO & ASSOCIATES, LLC

Professional Engineers & Surveyors
SERVING CONNECTICUT & MASSACHUSETTS
1 Shoham Rd. • East Windsor, CT 06088
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JOB 2021-085	
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SCALE	

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Water (	Suality Volume	Calculations	
	(1") RA/12 0.05+0.009 I	where I = percent um  R = voluntiae The  A = contributi	not coeff.
F	n! Cexcludes roof = 74,967 SF = 73,7270	Tunoff which is inflicate	R vier sobsectace system)
P	= 0.05 + 0.009 (73.7)	20 = 0.713	
wo	N= (1")(0.713)(74,0	76758)/12 = 4,457	CF
Foreloa Reguntame	y = 0.25 WQV :	= (0.25)(4,057 CF) =	1114 CF
Bas	in Volume below i	outlet = 4,491 CF	oray
Forelo	nay Volume =	1,207 CF OKay	

# **STAFF COMMENTS**



# TOWN OF VERNON

55 West Main St., VERNON, CT 06066-3291 (860) 870-3640 gmcgregor@vernon-ct.gov

# **MEMORANDUM**

TO: Inland Wetlands Commission

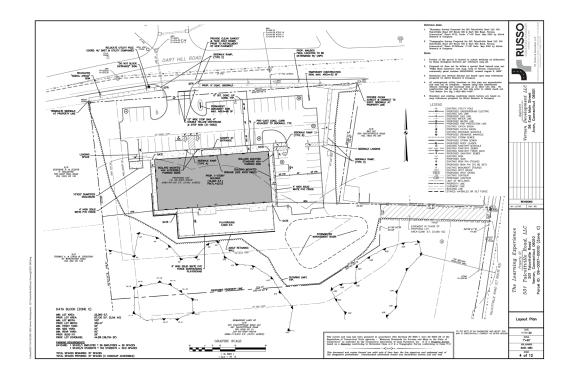
FROM: George K. McGregor, AICP, Town Planner

SUBJECT: IWC 2022-01, 501 Talcottville Rd.

DATE: February 22, 2022

# Request

**IWC 2022-01, 501 Talcottville Rd.** An Application by Vernon Development LLC for a Wetlands Redesignation and a Wetlands Permit for activity in a regulated area, in order to develop a child-care facility at 501 Talcottville Rd. (Tax Map 09, Block 007, Parcel 0001D).





**Site Location** 

# **Summary and Analysis**

The Applicant proposes to construct an approximately 10,000 sq. ft. day care facility at 501 Talcottville Rd. Although addressed via parcel number on Talcottville Rd., the access to the site is proposed on Dart Hill Rd., with a proposed drive-way entrance across from the westernmost Walgreens entrance. The development constitutes approximately .82 acres of disturbance in a regulated area.

A Re-Designation is requested, along with a Wetland Permit by Commission. No quorum was available at the regular January meeting for acceptance of the application; under State Law, the application is deemed accepted as of January 25. Staff has also placed the item on the IWC special meeting agenda on February 15, 2022 for a determination of significance. The application has been advertised for a public hearing to include BOTH the re-designation and the wetlands permit.

The Application submission includes a site plan set, a drainage report, a re-designation map, and a wetlands delineation report. As of Tuesday, February 15, no function and values report nor an analysis of habitat impacts has been submitted. The Applicant is working to complete these documents; expect them under a separate, supplemental transmission.

Note: A site plan, special permit, and re-subdivision application is also tracking to the Planning & Zoning Commission with a public hearing scheduled for March 3, 2022.

#### **Staff Review**

The Town's Wetland Agent, Mr. Perry has identified several areas where additional information or clarification is needed related to both the re-designation and the permit. His review is attached to this memorandum. He recommends that the Commission direct a third-party expert review of the application due to "the intense disturbance associated with this proposed development<sup>1</sup>." This would be an independent Professional Wetlands Scientist who would provide expert review on the Commission's behalf, with the Applicant responsible for the expense.

Assuming the details regarding the re-designation can be resolved, Staff has no other objections to the new wetland boundary and re-designation submitted by the Applicant.

As to the request for a wetland permit for the significant activity, Staff is unable to make a recommendation at this time, due to the delay in obtaining the wetlands assessment report.

To restate, the Wetland Agent recommends outside professional experts review the scope and scale of the request. This can only come at the Commission's direction; Staff will be prepared to explain the steps and process at the time of the hearing.

Draft Motions shall be prepared under separate cover.

GKM

<sup>&</sup>lt;sup>1</sup> Wetland Agent Letter 2-7-22, p. 2.



# TOWN OF VERNON

55 WEST MAIN ST., VERNON, CT 06066 Tel: (860) 870-3638 Fax: (860) 870-3683 E-Mail: cperry@vernon-ct.gov

TO: Town Of Vernon Inland Wetlands Commission

George McGregor - Town Planner

FROM: Craig Perry, Town of Vernon Wetlands Agent

DATE: 02-07-2022

RE: Staff Review of The Learning Experience (501 Talcottville Road) IWC-2022-01

I have reviewed the site plans titled The Learning Experience, dated January 11, 2022, prepared by Russo Surveyors / Engineers which consisted of 12 sheets in total.

# **Regarding the Re-Designation:**

- This property was the subject to a prior re-designation. The current request for a second re-designation shows the wetland limits in generally the same locations but it has reduced the size of the wetland in the area between flags A13 and A24. I recommend that the Soil Scientist explains any changes to the previous delineation.
- The newly proposed delineation as shown on the Wetland Redesignation Plan does not reference a Soil Scientist.
- The proposed delineation does not cover the entire property and is open ended. Almost the entire property was flagged with the exception of the southwest corner. A complete survey would help in up-dating the Wetlands Map and understanding the wetlands area.

# **Regarding the Wetlands Permit:**

• In referencing the Grading / Erosion and Sediment Control Plan, there is a significant amount of proposed site disturbance, depositing of fill material, and other construction activities associated with this project that are shown within a few feet of the wetland areas. I do not believe it is realistic to expect the erosion and sedimentation controls to prevent any impacts to the wetlands during heavy rain events when the installation is proposed in such close proximity to the wetland boundary itself. It has been my experience that a single row of silt fence is very limited in preventing impacts to the

- wetlands when there is a substantial risk of the fence being over-loaded due to the site grade and/or the amount of fill within the disturbed area.
- There is no proposed erosion control along the south side of the utility corridor that is coming off of Route 83 on the east side of the property.
- A Wetlands Assessment and environmental impacts report has not been submitted.

In summary, there is a significant amount of site disturbance and construction activity proposed in close proximity to the wetland delineation. There have been several studies done regarding the importance of the riparian (or buffer) areas in preserving significant wetland functions. This development, as presented, removes nearly all the riparian area to the north of the existing wetlands.

Due to the intense disturbance associated with this proposed development, my recommendation to the commission is to use your option to request an independent Professional Wetland Scientist that can perform a Complex Review of this application, on your behalf.

C.C.

David Smith, PE – Town Engineer

From: McGregor, George
To: McGregor, George

**Subject:** RE: 501 Talcottville rd- inland Wetlands **Date:** Friday, February 11, 2022 10:26:00 AM

-----Original Message-----From: McGregor, George

Sent: Thursday, February 10, 2022 2:59 PM

To: Tim Coon <tcoon@jrrusso.com>

Cc: Perry, Craig <cperry@vernon-ct.gov>; Smith, David <dsmith@vernon-ct.gov>

Subject: 501 Talcottville rd- inland Wetlands

Tim: please accept this as my comments on iwc 2022-01.

The submission does not appear to include the traditional wetland impact narrative, habitat discussion, or a functions and values analysis.

This makes it quite difficult to review and provide clear insight to the commission.

Please forward the requested analysis as soon as possible.

George

George K. McGregor, aicp Office: 860-870-3640

 From:
 Tim Coon

 To:
 McGregor, George

Cc: Perry, Craig; Smith, David; rema8@aol.com; Jay Ussery; Eric Spungin; Mark Vertucci

Subject: [EXTERNAL] 501 Talcottville Road

Date: Tuesday, February 15, 2022 3:38:35 PM

Attachments: <u>image001.png</u>

TLE Vernon Site Plans 2-14-22.pdf

Sscan22021418220.pdf

**CAUTION:** This Email is from an EXTERNAL source. Ensure you trust this sender before clicking on any links or attachments.

George,

I am providing this email and its attachments (revised plans and drainage report) in response to the plan review comments received from Craig Parry dated 2-7-22. A response to Craig's comments is provided below:

# Craigs Re-designation Comments:

The soil scientist, George Logan from REMA Ecological Services, will be preparing and submitting an Impact Assessment/Function & Values Assessment under separate cover. It is anticipated that Mr. Logan will address Craigs comments 1 and 3 regarding the delineation and re-designation in that report. With regard to comment 2, I would like to point out that Note #1 on the Redesignation Plan identifies REMA Ecological Services as the entity which delineated the wetlands shown on the plans.

# Craig's comment Regarding the Wetland Permit:

- 1. Despite the significant amount of fill required at the site, the runoff across the site will be very limited based on the existing sandy soils and limited distance across the site. It is believed that a single silt fence, if maintained properly, is sufficient to protect the downgradient wetland from sedimentation and erosion. However, based on Mr. Perry's concerns, the plans have been revised to include the placement of a wood chip berm to back up the silt fence installed at the edge of the downgradient wetland.
- 2. The plans have been revised to extend the sediment barrier (silt fence backed by a wood chip berm) along the utility corridor out to Talcottville Road.
- 3. As discussed above, a Wetland Impact Assessment/Functions & Values Assessment is being prepared by George Logan of REMA and will be submitted separately. However, in the preparation of his report, George has made a couple of recommendations for wetland enhancement and creation which have been added to these revised plans. In particular, the plans identify a particular area of wetlands to the south of the development area where we are proposing a series of plantings to enhance the existing wetland. In addition, the plans identify an upland area between wetland flags #14 and #24 where we are proposing to create some wetlands by excavating a small pool and seeding with a wetland seed mix. I believe George's report will provide additional discussion regarding these measures.

In addition to the plan revisions noted above, revisions were also made to address the off-site runoff from the adjacent Panda Palace parking lot that is currently directed to the development site through an existing break in the curb line at the western edge of their parking lot. This includes

untreated runoff from approximately 0.6 acres of impervious area at the Panda Palace which currently runs across the proposed development area at the site into the on-site wetland. The plan revisions include the installation of a riprap channel, yard drain and piping to intercept this runoff at the property line and convey it to the new on-site stormwater management basin where it will be treated and some of it recharged prior to discharge into the wetland. The attached plans and Drainage Report have been revised to reflect the interception and treatment of this off-site runoff. As a result, the proposed project will eliminate this discharge of untreated runoff to the wetland from the Panda Palace.

Finally, because I have also received your Town Planner comments dated 2-10-22, I have also separated the Wetland Re-designation Plan from the plan set and added both a fence and a dumpster surround detail, which address your first two comments. The architect will be providing color elevations to address your comment #3. Because your remaining comments (#4-#8) are all traffic related, a separate response is being prepared by the traffic engineer, Mark Vertucci at Fuss & O'Neill.

Also, as I have expressed to many of you on this email, I will be out of town until February 28<sup>th</sup>. In my absence, Jay Ussery from my office (copied above) will be handling this project and the wetland meeting next Tuesday. Please be sure that any correspondence meant for J.R. Russo & Associates is addressed to Jay during this time frame. Thank you.

Timothy A. Coon, P.E. Principal Engineer



# J.R. RUSSO & ASSOCIATES, LLC

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